

City of Sandy

Agenda

Planning Commission Meeting
Meeting Location: City Hall- Council
Chambers, 39250 Pioneer Blvd.,
Sandy, Oregon 97055

Meeting Date: Tuesday, June 30,

2020

Meeting Time: 6:30 PM

Page

1. MEETING FORMAT NOTICE

Note: The Planning Commission will conduct this meeting electronically using the Zoom video conference platform. Members of the public may listen, view, and/or participate in this meeting using Zoom. Using Zoom is free of charge. See the instructions below:

Please click the link below to join the webinar: https://us02web.zoom.us/j/81660200390

Or Telephone: +1 669 900 6833

Webinar ID: 816 6020 0390

International numbers available: https://us02web.zoom.us/u/kZXUQz8av

2. ROLL CALL

3. APPROVAL OF MINUTES

3.1. Draft Planning Commission Minutes for May 27, 2020
Planning Commission - 27 May 2020 - Minutes - Pdf

3 - 11

4. REQUESTS FROM THE FLOOR - CITIZEN COMMUNICATION ON NON- AGENDA ITEMS

5. PUBLIC COMMENT

This meeting will include two public hearings. <u>If you would like to offer testimony during the hearings, see the instructions below:</u>

Testimony for each public hearing will be called for in three groups: testimony in favor of the proposal, testimony opposed to the proposal, and neutral testimony.

If you are participating online, click the "raise hand" button at the appropriate time

and wait to be recognized.

If you are participating via telephone, <u>dial *9 to "raise your hand"</u> at the appropriate time and wait to be recognized.

If you choose to submit testimony in written form, please send to planning@ci.sandy.or.us as soon as possible.

Thank you for your flexibility during the COVID-19 public health emergency. Please call City Hall with any questions: (503) 668-5533.

6. NEW BUSINESS

6.1.	20-021 EXT Jewelberry Ridge Subdivision Extension	12 - 18
	20-021 EXT Jewelberry Ridge Subdivision Extension - Pdf	
6.2.	20-006 DR/VAR/DEV/ADJ Clackamas County Health Clinic 20-006 DR/VAR/DEV/ADJ Clackamas County Health Clinic - Pdf	19 - 329
6.3.	20-012 DCA 5G Small Cell Code Change 20-012 DCA 5G Small Cell Code Change - Pdf	330 - 339

7. ITEMS FROM COMMISSION AND STAFF

8. ADJOURN



MINUTES Planning Commission Meeting Wednesday, May 27, 2020 Virtual via Zoom 7:00 PM

COMMISSIONERS PRESENT: Don Carlton, Commissioner, Ron Lesowski, Commissioner, Hollis MacLean-Wenzel,

Commissioner, Jerry Crosby, Commissioner, John Logan, Commissioner, Chris Mayton,

Commissioner, and Todd Mobley, Commissioner

COMMISSIONERS ABSENT: None

STAFF PRESENT: Kelly O'Neill, Development Services Director, Emily Meharg, Senior Planner, David

Doughman, City Attorney, and Shelley Denison, Associate Planner

MEDIA PRESENT: None

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Webinar ID: 839 4780 9040

International numbers available: https://us02web.zoom.us/u/kb7YbrTKLH

2. Roll Call

Chairman Crosby called the meeting to order at 7:02 p.m.

3. Approval of Minutes

3.1. Draft Planning Commission Minutes for April 27, 2020

Motion: To approve minutes for April 27, 2020.

Moved By: Commissioner Logan

Seconded By: Commissioner Mayton

Yes votes: All Ayes No votes: None Abstentions: None The motion passed.

4. Requests From the Floor - Citizen Communication on Non- Agenda Items

ZOOM WEBINAR CHAT:

From Kathleen Walker to All panelists:

"Not sure if this is applicable, but Hollis's email on the website is incorrect and bounding back emails. So if you used that to send her zoom invite, it may be what is causing problems....KW"

From Kelly O'Neill Jr. to All panelists:

"Okay thanks for the heads up."

Kathleen Walker

15920 Bluff Road

Sandy, OR 97055

Commented that Hollis Maclean-Wenzel's email address on the City of Sandy website is not working. Kelly O'Neill Jr. said that IT can likely look into the email address concerns. Greg Brewster stated that IT can resolve any potential issues with Commissioner Maclean-Wenzel's email account.

5. Public Comment

This meeting will include two public hearings. If you would like to offer testimony during the hearings, see the instructions below:

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6. NEW BUSINESS

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6.1. Gunderson Road & Parkland Annexation (20-001 ANN/CPA/ZC):

Chairman Crosby opened the public hearing on File No. 20-001 ANN/CPA/ZC at 7:14 p.m. Crosby called for any abstentions, conflicts of interest, ex-parte contact, challenges to the jurisdiction of the Planning Commission, or any challenges to any individual member of the Planning Commission. Commissioner Mobley abstained. Commissioner Mayton declared he received an email from Kathleen Walker concerning the application. Commissioner Carlton also received an email from Kathleen Walker. The email is included in the record so is not ex parte. Chairman Crosby requested that members of the public send comments to staff only, not to the individual Commissioners. No challenges were made, and no declarations were made by the Planning Commissioners.

Staff Report:

Development Services Director Kelly O'Neill Jr. summarized the staff report. O'Neill updated the Commission on the UGB expansion hearing dates, which were rescheduled due to COVID-19 concerns and the Bailey Meadows Subdivision decision. O'Neill stated the annexation proposal is for parkland, a road, and associated facilities; there will be no additional housing even though some of the land will be zoned residential. Commissioner Carlton mentioned Kathleen Walker's comment regarding houses facing the park and asked Director O'Neill to pull up a picture of the plat. O'Neill stated the proposal does not include a review of Chapter 17.86. Doughman reiterated that the decision about the parkland was part of the subdivision application and that the criteria in Chapter 17.86 aren't included in tonight's decision.

Applicant Testimony:

Mike Robinson 1211 SW 5th Ave. Suite 1900 Portland, OR, 97204

Robinson alerted staff and the Commissioners that they only received Kathleen Walker's letter. O'Neill clarified that the Fair Housing Council did not submit testimony on this application.

Chris Goodell
12965 SW Herman Rd. Suite 100
Tualatin, OR 97062
Goodell stated the applicant reviewed the staff report and agrees with it.

Proponent Testimony:

None

Opponent Testimony:

Kathleen Walker 15920 SE Bluff Rd.

Sandy, OR 97055

Walker stated the parkland doesn't meet the existing code and that the final staff report didn't address it. Walker explained to the residents of Sandy what kind of park they'll end up with. She stated the park will have no vehicle access, no sidewalks for pedestrian access, won't be graded or seeded, and will be vacant land. Walker stated none of the park requirements have been met.

ZOOM WEBINAR CHAT:

From Kathleen Walker to All panelists:

"It's code not my opinion! They have to pay for half od the oroad and the sidewalk is only along Gunderson."

From Kathleen Walker to All panelists:

"Who is paying for the road? You as a planning commission, staff, and council failed to document (until long after public comment was closed) to address how Bailey Meadows did or did not meet all of 17.86 code requirements. You and the staff ignored us. You implied that the code could be waived without a variance. So don't tell us that we are not accurately reporting this! You ignored it before and now you decided it is out the scope. Great!:)"

Neutral Testimony:

None

Staff Recap:

O'Neill stated he's reluctant to go into discussion on Chapter 17.86 as that was part of the Bailey Meadows subdivision land use decision. O'Neill stated sidewalks along parks are typically paid for by the City through SDCs. Doughman reiterated that parkland was a matter germane to the subdivision decision.

Applicant Rebuttal:

Mike Robinson 1211 SW 5th Ave. Suite 1900

Portland, OR, 97204

Robinson reiterated that the applicant agrees with the staff report and recommendation of approval. Robinson also reiterated that Chapter 17.86 is not relevant.

Chris Goodell 12965 SW Herman Rd. Suite 100 Tualatin, OR 97062 Goodell stated that he had nothing to add.

Discussion:

Chairman Crosby noted the Commission has more background on this annexation

proposal than is typical of an annexation application. Commissioner Carlton stated he submitted testimony on Bailey Meadows. Carlton stated he was the one who brought up the houses facing the park during the Bailey Meadows subdivision hearing. Carlton stated it appears there's a street adjacent to the park to the west. Lesowski hopes the public realizes the City is trying to make the best out of the situation given the legal confines. Carlton is in favor of the annexation. Logan agreed with what's been said. Mayton stated he listens to the public and reads all the letters but is also bound to follow the code. Mayton and Maclean-Wenzel stated they support the annexation.

Motion: Motion to close the public hearing at 7:57 p.m.

Moved By: Commissioner Carlton Seconded By: Commissioner Logan

Yes votes: Commissioners Carlton, Lesowski, Maclean-Wenzel, Logan, Mayton, and

Crosby.

No votes: None Abstentions: None

The motion passed at 7:57 p.m.

Motion: Motion to forward a recommendation of approval for 20-001 ANN/CPA/ZC

Gunderson Road and Parkland Annexation.

Moved By: Commissioner Mayton

Seconded By: Commissioner Maclean-Wenzel

Yes votes: Commissioners Carlton, Lesowski, Maclean-Wenzel, Logan, Mayton, and

Crosby.

No votes: None Abstentions: None

The motion passed at 8:01 p.m.

5-minute recess

6.2. Chapter 17.78 Annexation Code Amendments (20-010 DCA):

Chairman Crosby opened the public hearing on File No. 20-010 DCA at 8:08 p.m. Crosby called for any abstentions, conflicts of interest, ex-parte contact, challenges to the jurisdiction of the Planning Commission, or any challenges to any individual member of the Planning Commission. No challenges were made, and no declarations were made by the Planning Commissioners.

Staff Report:

Senior Planner Emily Meharg summarized the staff report using a PowerPoint presentation. Commissioner Mayton asked if the commission has the opportunity to ask questions on the code changes. Chairman Crosby stated that the Planning Commission will have an opportunity to ask detailed questions regarding the code changes.

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Planning Commission May 27, 2020

Proponent Testimony:

Kathleen Walker 15920 Bluff Road Sandy, OR 97055

Mrs. Walker read her written testimony that she submitted on May 27, 2020.

Erin Findley 37616 Rachel Drive Sandy, OR 97055

Not too familiar with the planning process. Found it interesting that two former mayors spoke during Bailey Meadows that development should pay for itself. Thanked the Planning Commission and asked for the commission to consider more code changes.

Marie DeBatty 37176 Rachel Drive Sandy, OR 97055

Would like the Planning Commission to take notice of future annexations. Thinks the City of Sandy should be more careful with future annexations.

Other Testimony:

None

Staff Recap:

Meharg stated the intention of the code changes is to clarify annexations and to require more robust analysis. Changes are to address potential weakness. O'Neill stated that staff is trying to make the regulations more robust. Staff has heard from the public and we want to be more selective about future annexations. Doughman said that annexations are both land use regulatory and political in nature. The question really comes down to do we want to expand our territory or not. Annexation proposals are when local decision makers have more discretion. Voter approved annexations are no longer allowed in Oregon. Since voter approved annexations are no longer allowed a lot of cities in Oregon have revised their municipal code to make standards more robust. Before property is annexed the city can require applicants to prove they can meet master plans and other city codes.

Discussion:

Commissioner Crosby asked is it possible for an applicant to force an annexation into the UGB area. Doughman stated that as long as you have criteria you have the opportunity to deny an annexation request. There have been discussions from both sides of the aisle that the legislature might make changes in the future to the annexation process.

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Motion: Motion to close the public hearing at 8:42 p.m.

Moved By: Commissioner Carlton

Seconded By: Commissioner Maclean-Wenzel

Commissioners: All ayes No votes: None Abstentions: None

The motion passed at 8:42 p.m.

Commissioner Carlton stated he appreciates public comments. He stated that the UGB expansion analysis was very in-depth and analyzed items, such as city services. He stated that we have a lot of development code regulations to develop your land. Carlton also stated that the proposed language has a lot of subjectivity. Stated that he just heard about the annexation code changes recently. Commissioner Mobley stated there was not a lot of public input and not a lot of public participation. Commissioner Mayton said he was hoping for more analysis. He was surprised there was not more public feedback. Commissioner Lesowski stated he agrees with the other commissioners. He would like to see the code changes to be less subjective. Commissioner Logan said that almost all of the new language that staff has proposed to add are being adopted in other local cities. He said that he would like to see more information and a work group. Commissioner Maclean-Wenzel said she agrees with many of the comments and wants to see responsible growth. She stated it seems that properties that have been added to the UGB have come to expect annexation. She would like to see development pay for itself. Commissioner Logan stated that other cities have 300-foot notification requirements.

Commissioner Lesowski asked if property that is annexed gets any benefits if it's not developed? O'Neill stated that newly annexed property can connect to city utilities (i.e. water and sanitary sewer) and get police service. O'Neill stated that staff is trying to be responsive to requests from the public and the City Council. Staff is trying to be responsive to requests and adopt code quickly. Meharg stated that criterion A in Section 17.78.50 of the proposed annexation code is already in the existing code. Commissioner Maclean-Wenzel asked Doughman to make some clarification. Doughman stated this is a great discussion where policy hits law. Most development code regulations must be clear and objective. However, annexation code can be written very subjective. Cities can choose to exercise a lot of discretion. The proposed code changes are supposed to assist with responsible growth and are supposed to be subjective and allow for discretion for the decision makers. Commissioner Lesowski asked how some of the criteria are very vague and some of the code is very specific.

Commissioner Mayton asked where does 10 years and 1,000 feet come from? Commissioner Carlton stated he would like to see the tree standards related to 5 years remain the same. He elaborated on the meaning of affordable housing and that it creates more traffic. Does the City have to analyze property being brought into the City via island annexations? Commissioner Maclean-Wenzel provided reasons for why the Tree Code Committee has been delayed in making progress. Developers state that tree retention is not financially viable for them. More than a 5 year waiting period for

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significant tree removal seems fine. Commissioner Lesowski stated that the code changes are not decreasing regulations. Commissioner Carlton stated that the proposed code changes transposes work from one place to another place. Commissioner Mayton asked staff to clarify how property development works. O'Neill explained that a lot of property development occurs with contracts and contingencies prior to sale. He explained that individual property owners will not necessarily be on the hook for additional analysis. Meharg stated that 17.78.50 (A) is already in the Development Code. Planning Commission stated that (A) should be removed. Doughman stated it was already in the code. He elaborated there will be more analysis required at annexation but does not believe it will make a big difference on future growth. O'Neill stated that we could look at alternative annexation analysis for smaller properties simply looking to connect to city services. Doughman stated that an annexation agreement could take care of some of the concerns. Lesowski stated he was looking for less code criteria for properties with utility issues. Doughman said that the trade for annexation is higher taxes. Meharg stated the notification area is proposed to be increased to allow more public input and participation. Mayton stated he appreciates staff's time on the code changes, is good with an 8 to 10 year waiting period for significant tree removal, is good with the 1,000 feet notification, and moving subsection A. in 17.78.50 into the header. Crosby said he suggests passing it along to City Council for approval with some suggested modifications. He summarized the discussion and asked how they can move forward with a motion. The Commissioners talked about tree retention. Commissioner Maclean-Wenzel said she suggests keeping the tree standard at 5 years and possibly modify the code once the Tree Code Committee forwards decisions. Mobley stated that 17.78.50 (C) should be modified to add the words 'modified if necessary'. Commissioner Carlton encourages staff to notify property owners inside the UGB, but outside city limits. O'Neill stated that staff will consider noticing more property owners prior to the legislative hearing before City Council.

Motion: Motion to move the proposed code changes to Council with leaving A. in 17.78.50 as an introductory sentence, add additional language for property owners looking to annex for utility reasons or smaller lots, modify C. in 17.78.50 to include the words 'modified if necessary', and leave 5 years for trees instead of 10 years.

Moved By: Commissioner Mobley

Seconded By: Commissioner Mayton

Commissioners Carlton, Maclean-Wenzel, Logan, Mobley, Mayton, and Crosby.

No votes: None

Abstentions: Lesowski (no longer part of the meeting because the battery life on his computer expired)

The motion passed at 10:11 p.m.

7. Items from Commission and Staff

Chairman Carlton thanked staff for forwarding the Council report. O'Neill reminded the commission of the meetings in June and July. Commissioner Mayton stated he might miss the June 30, 2020 meeting.

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Planning Commission May 27, 2020

8. Adjourn

Motion: To adjourn

Moved By: Commissioner Carlton Seconded By: Commissioner Carlton

Yes votes: All Ayes No votes: None Abstentions: None The motion passed.

Chairman Crosby adjourned the meeting at 10:14 p.m.

Chair, Jerry Crosby

Planning Director, Kelly O'Neill Jr



Staff Report

Meeting Date: June 30, 2020

From Emily Meharg, Senior Planner

SUBJECT: 20-021 EXT Jewelberry Ridge Subdivision Extension

Background:

On July 12, 2018 the Planning Commission approved the Jewelberry Ridge Subdivision (File No. 18-014 SUB/VAR/TREE) which is a 9-lot subdivision located northwest of Jewelberry Avenue, east of the Sandy Bluff Annex subdivision, and south of American Street. The tentative subdivision plat approval was valid for one (1) year. On July 25, 2019 the Development Services Director granted an extension to the approval that extended the tentative plat approval for one (1) additional year to July 12, 2020.

The Development Code does not grant the Director authority to grant additional extensions. The applicant was advised by the Director to ask the Planning Commission to grant a tentative plat extension for one additional year to July 12, 2021 or another date as approved by the Planning Commission.

The typical reason that development codes contain expiration dates is because development codes are periodically modified. Modifications to development codes oftentimes lead to alternative findings or conditions of approval than were originally conditioned. In the case of Jewelberry Ridge no applicable development code modifications have occurred that would lead to a different outcome or list of conditions than what was applied to Jewelberry Ridge. Since no applicable development code modifications have occurred the exercise of making the applicant for Jewelberry Ridge seek re-approval for the subdivision would be immaterial.

The legal notice regarding this request was published in the Sandy Post on June 10, 2020.

Recommendation:

Staff recommends the Planning Commission hear the extension request from the applicant and then make a decision on granting an extension. If Planning Commission grants an extension it shall be to July 12, 2021 or another date as approved by the Planning Commission.

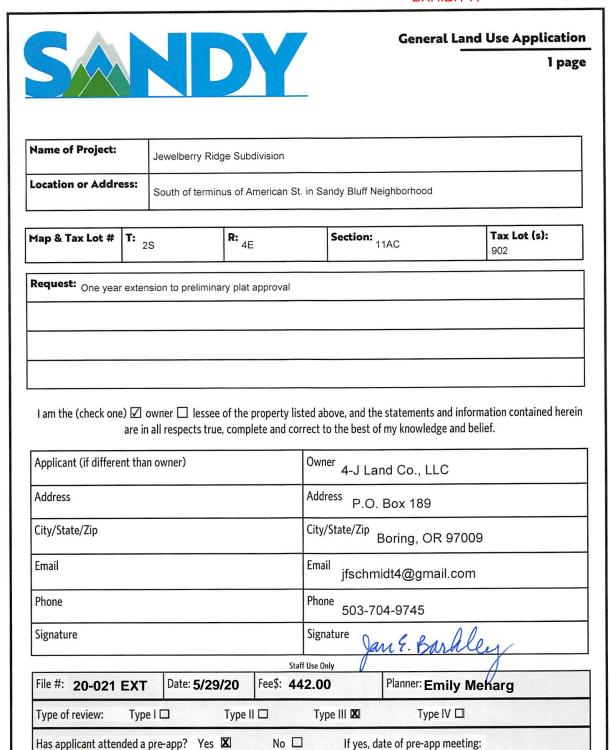
Code Analysis: ATTACHMENTS:

A. Land Use Application

B. Narrative

C. Preliminary Plat
D. Director Extension Letter
D. Director Extension Letter
E. Public Comment – Sally and Jerry Jacobson
Budgetary Impact:
None
e.ie

EXHIBIT A



Development Services Department, 39250 Pioneer Blvd, Sandy, OR 97055, 503.489.2160

EXHIBIT B

Narrative in Support of One Year Extension Jewelberry Ridge Subdivision Preliminary Plat Approval

Dear City of Sandy Planning Commission & Planning Staff,

The Jewelberry Ridge Subdivision (File # 18-014 SUB/VAR/TREE) was approved on July 12th 2018 and per normal approval timelines was good for one year. We had hoped to position the subdivision for construction during summer of 2019 but by the time we had approved engineering plans and other necessary entitlements we were well into the summer of 2019 and missed that dry weather construction season. The preliminary plat approval was then extended for one year through July 12th, 2020. We had a buyer lined up to close on the property this spring and move forward with construction this spring/summer but they ended up backing out at the last minute in mid-April, citing concerns with the economic fallout from COVID-19.

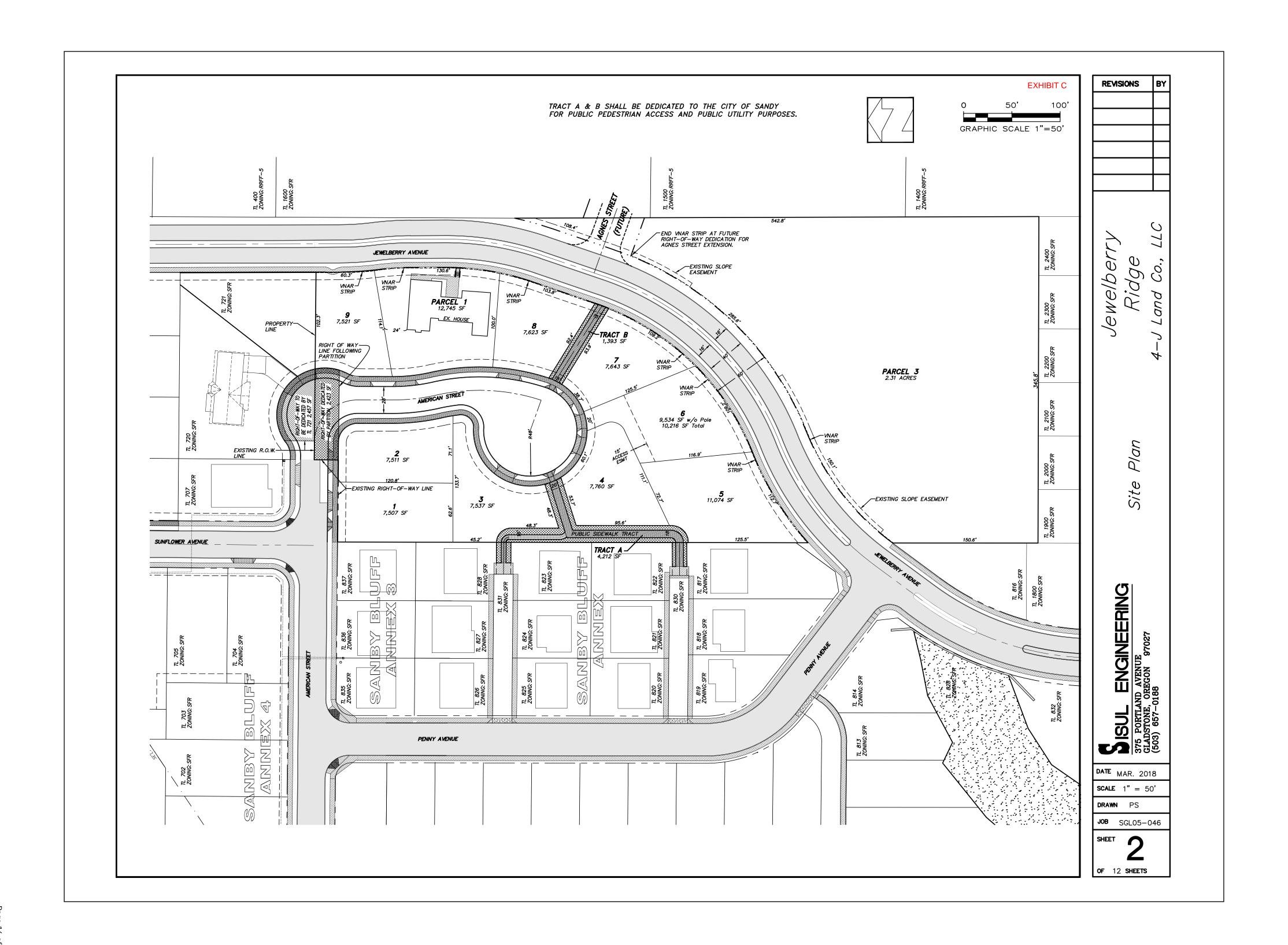
Following that setback we re-listed the property and are now under contract to sell the property in early July (2020) to a builder/developer that intends to move forward with construction immediately. Assuming an approximately three-month construction project, they should wrap up in the fall and get a final plat later this year or early next year depending on how everything falls into place.

4-J Land Co., LLC appreciates your consideration of this one year extension to the preliminary plat approval, which will allow enough time for the subdivision to be completed per the approved plans.

Sincerely,

John Schmidt & Jan Barkley

4-J Land Co., LLC







July 25, 2019

4-J Land Co., LLC ATTN: Jan Barkley PO Box 189 Boring, OR 97009

Dear Ms. Barkley and Mr. Schmidt,

This letter regards my email to you on July 12, 2019 and the email request you submitted on July 13, 2019 to extend the tentative subdivision approval for the Jewelberry Ridge subdivision approved with File No. 18-014 SUB. The original expiration date as set forth in the findings of fact and final order was July 12, 2019. The extension I am permitting with this letter allows for a modified expiration date of **July 12, 2020**.

If you have any questions about this letter, you are welcome to call the City of Sandy Development Services Department at (503) 668-0880 for additional information.

Thank you,

Kelly O'Neill Jr. Planning & Building Director koneill@cityofsandy.com direct line: (503) 489-2163

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EXHIBIT E

COMMENT SHEET for File No. 20-021 EXT:	June 9, 2020
Os stated before a Equipment may not this is a service now by the homes I rese 377th. The clearing using 377th for squir Stop!	ad maintained
	JUN 15 2020 City of Sandy
Sally Jerry Jacobson Your Name J 15035 SE 377th ave Address	971 - 404-8773 Phone Number Sandy, DR. 97055
APPLICABLE CRITERIA: Sandy Municipal Code: Chapte Processing Applications; 17.20 Public Hearings; 17.22 Notice	er 17.12 Procedures for Decision Making; 17.18 s.

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20-021 EXT Jewelberry Ridge Subdivision Extension Notice



Staff Report

Meeting Date: June 30, 2020

From Emily Meharg, Senior Planner

SUBJECT: 20-006 DR/VAR/DEV/ADJ Clackamas County Health Clinic

Background:

Clackamas County submitted this land use application for a new health clinic facility located at 39831 Highway 26. The proposal includes demolishing the existing building and constructing a new 9,381 square foot building, along with associated site improvements. In addition, the applicant is requesting the following:

- Type II Adjustment to Section 17.90.110(B.3.b) to not include a base material on 18 percent of the building's façade.
- Type III Design Deviation to Section 17.90.110(B.3.d.1) to use vertical grooved sheet siding.
- Type III Design Deviation to Section 17.90.120(D.4) to not provide a primary entrance at the corner or within 40 feet of the corner.
- Type III Design Deviation to Section 17.90.110(D.7) to not provide a primary entrance that faces a public street or civic space.
- Type III Special Variance to Section 17.90.110(E.2) to not meet the 40 percent window requirement along street frontages and where the building abuts a civic space.

Recommendation:

Staff recommends the Planning Commission **approve** the requested Design Review, adjustment and variance associated with the Clackamas County Health Clinic as modified by the conditions of approval listed below.

Staff recommends the Planning Commission **approve** a Type II Adjustment to allow the proposed building to not include a base material on 18 percent of the building's facades.

Staff recommends the Planning Commission **approve** a Special Variance to Section 17.90.110(E.2) to not provide 40 percent window coverage along street frontages.

Staff recommends the Planning Commission **approve** the applicant's request for a design deviation to Section 17.90.110(B.3.d.1) to allow the use of Nichiha siding.

Staff recommends the Planning Commission **approve** the requested Design Deviation to Section 17.90.110(D.4) to not provide a corner entrance.

Staff recommends the Planning Co	ommission approve the requested Design Deviation
to Section 17.90.110(D.7) to not pr	rovide a public entrance facing a street or civic space.
Code Analysis: See attached staff report and exhil	bits
Budgetary Impact: None	



PLANNING COMMISSION STAFF REPORT TYPE III LAND USE PROPOSAL

DATE: June 23, 2020

FILE NO.: 20-006 DR/VAR/DEV/ADJ

PROJECT NAME: Clackamas County Health Clinic

APPLICANT: Steve Kelly (Clackamas County)

OWNER: Clackamas County

LEGAL DESCRIPTION: T2S R4E Section 13 AD, Tax Lot 1001

SITUS ADDRESS: 39831 Highway 26

The above-referenced proposal was reviewed as a Type III Design Review with variances, deviations, and adjustments and the following Findings of Fact are adopted supporting approval of the tentative plan in accordance with Chapter 17 of the Sandy Municipal Code.

EXHIBITS:

Applicant's Submittals

- A. Land Use Application and Supplemental Application
- B. Narrative
- C. Plan Set
 - Sheet CS: Cover Sheet
 - Sheet G0.11: Existing Site Survey
 - Sheet C0.3: Site Demolition Plan
 - Sheet C1.0: Site Hardscape Plan
 - Sheet A1.01: Site Plan
 - Sheet C2.0: Grading Plan
 - Sheet C3.0: Utility Plan
 - Sheet A3.11: Building Elevations
 - Sheet A4.01: Building Sections
 - Sheet G0.34: Floor Plan
 - Sheet A2.03: Roof Plan
 - Sheet A1.21: Trash Enclosure / Site Details
 - Sheet G1.30: Window Area and Base Calculations
 - Sheet E0.01: Electrical Symbol Legend
 - Sheet E0.02: Lighting Fixture Schedule
 - Sheet E1.01: Electrical Site Plan
 - Sheet E1.02: Photometric Site Plan

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- Sheet L1.01: Landscape Plan
- Sheet L1.02: Landscape Details
- Sheet L4.00: Landscape Notes and Plant Schedule
- Sheet L6.01: Planting Details
- D. Exterior Light Fixture Cut Sheets: S1 through S7
- E. Renderings
- F. Exterior Building Materials
- G. Nichiha Details
- H. Traffic Impact Analysis Letter
- I. Shared Parking Letter to Police
- J. Parking Space Lease Agreement with Immanuel Lutheran Church
- K. Stormwater Utility Narrative
- L. 1:60 Vicinity map
- M. 500 foot radius map

Agency Comments

- N. City Engineer (May 29, 2020)
- O. Fire Marshal (June 3, 2020)
- P. Transportation Engineer (June 8, 2020)
- Q. Oregon Department of Transportation ODOT (June 17, 2020)
- R. Public Works Director (June 18, 2020)

Supplemental Documents Provided by Staff

- S. Email from applicant regarding Nichiha
- T. Ten Eyck Road & Pleasant Street Curb & Sidewalk Improvements Final Plans

FINDINGS OF FACT

<u>General</u>

- 1. These findings are based on the applicant's submittal materials received on February 12, 2020 with additional items received on 3/13/20, 3/24/20, 3/27/20, 3/31/20, and 4/1/20. The application was deemed complete on April 3, 2020. The original 120-day deadline was August 1, 2020; however, in response to COVID-19, the applicant submitted a letter on April 4, 2020 extending the 120-day deadline 30 days to August 31, 2020.
- 2. The subject site is approximately 0.47 acres and is located on the north side of Highway 26, east of Ten Eyck Road and south of Pleasant Street.
- 3. The parcel has a Plan Map designation of Commercial and a Zoning Map designation of C-1, Central Business District.
- 4. Clackamas County submitted this land use application for a new health clinic facility located at 39831 Highway 26. The proposal includes demolishing the existing building and constructing a new 9,381 square foot building, along with associated site improvements. In addition, the applicant is requesting the following:
 - Type II Adjustment to Section 17.90.110(B.3.b) to not include a base material on 18 percent of the building's façade.

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- Type III Design Deviation to Section 17.90.110(B.3.d.1) to use vertical grooved sheet siding.
- Type III Design Deviation to Section 17.90.120(D.4) to not provide a primary entrance at the corner or within 40 feet of the corner.
- Type III Design Deviation to Section 17.90.110(D.7) to not provide a primary entrance that faces a public street or civic space.
- Type III Special Variance to Section 17.90.110(E.2) to not meet the 40 percent window requirement along street frontages and where the building abuts a civic space.
- 5. Notification of the proposal was mailed to property owners within 500 feet of the subject property on May 27, 2020 and to affected agencies on May 26, 2020. A legal notice was published in the Sandy Post on June 3, 2020.
- 6. Per the City Engineer (Exhibit N), a demolition permit is required from the City prior to demolishing the existing building. The applicant applied for a demolition permit on June 15, 20202, which was issued on June 17, 2020.
- 7. The Planning Commission reviewed the proposal at the June 30, 2020 hearing.

17.42 – Central Business District (C-1)

- 8. The applicant proposes a health clinic, which is a primary use permitted outright in Section 17.42.20(B.2.g).
- 9. The C-1 zoning district does not require a minimum front yard setback, but does have a maximum front yard setback of 10 feet. The zoning district does not contain any side yard or rear yard setbacks. There is an existing building on the subject property that will be demolished. Chapter 17.10 contains the following definition for "lot line, front": "In the case of an interior lot, a property line that abuts the street. In the case of a corner lot, the front line shall be determined by orientation of the structure based on at least two of the following factors: location of the front door, location of the driveway, or legal street address." The subject property is a corner lot with frontage on three public streets. The proposed front door faces the parking lot to the east, the driveway is located on Pleasant Street, and the address is on Highway 26. Thus, no two factors currently align to determine the front lot line. While the front door faces the parking lot, it is located towards the north end of the east elevation, closest to Pleasant Street. Moreover, the driveway is located off Pleasant Street so that is how patients who drive to the clinic will access the building. Thus, staff believes it makes the most sense to readdress the health clinic with a Pleasant Street address. This will result in Pleasant Street being the front lot line. The proposed new building is set back 10 feet from Pleasant Street (the front lot line), in compliance with setback standards. The applicant shall update the address of the site to be addressed from Pleasant Street and shall pay the re-addressing fee (\$206) to the City of Sandy.
- 10. Per the submitted Landscape Plan (Exhibit C, Sheet L1.01), the site will include a total of 15 percent landscaping in compliance with the standards of Section 17.42.30(A).

17.66 – Adjustments and Variances

11. Chapter 17.66 contains regulations associated with adjustments and variances. Adjustments are a means to vary the development standards normally applied in a particular district and variances

are a means of requesting a complete waiver or major adjustment to certain development standards. The applicant requested the following adjustment and variance:

- Type II Adjustment to Section 17.90.110(B.3.b) to not include a base material on 18 percent of the building's façade.
- Type III Special Variance to Section 17.90.110(E.2) to not meet the 40 percent window requirement along street frontages and where the building abuts a civic space.

12. BASE MATERIAL ADJUSTMENT

Section 17.90.110(B.3.b) states: "Strong base materials such as natural stone (e.g., basalt, granite, river stone), split-faced rusticated concrete block, or brick are required. Cultured stone may be allowed if it has a stone texture and is similar in appearance and durability to natural stone. A building's base must extend at least 36 inches but not more than 60 inches above the adjacent finished grade and be included on those sides of the building visible from the abutting public street. If the site contains a grade differential making construction of a minimum 36-inch base impracticable, the reviewing body may allow portions of the base to be less than 36-inches."

Request: The applicant requests a Type II Adjustment to Section 17.90.110(B.3.b) to not include a base material on 18 percent of the building's facade. The proposed gaps in base material are as follows:

- East elevation: one (1) 10'-4" gap and one (1) 3'-6" gap
- South elevation: one (1) 22'-6" gap and one (1) 26'-3" gap
- West elevation: two (2) 14'-8" gaps

Section 17.66.40 contains Type I and Type II Adjustment criteria. Criteria A of Section 17.66.40 states "The proposed development will not be contrary to the purposes of this chapter, policies of the Comprehensive Plan, and any other applicable policies and standards adopted by the City." The Comprehensive Plan states that the Central Business (C-1) District is intended to provide the community with a mix of civic, retail, personal services, offices and residential needs of the community and its trade area in the city's traditional commercial core. The intent of Section 17.90.110(B) is to provide building façades, materials and colors consistent with the Sandy Style. The proposed development is consistent with the intent of the Central Business zone by providing a community service. The proposal includes Sandy Style elements and contains base material on 82 percent of the building's facades. Criterion A is met.

Criteria B states "The proposed development will not substantially reduce the amount of privacy enjoyed by users of nearby structures when compared to the same development located as specified by this Code." The proposal to reduce the percent of base material from 100 to 82 percent will not affect the amount of privacy enjoyed by users of nearby structures. Criterion B is met.

Criteria C states "The proposed development will not adversely affect existing physical systems and natural systems, such as traffic, drainage, dramatic land forms, or parks." Based on the Transportation Impact Analysis (TIA), the proposed development will result in 34 net new AM peak hour trips, 30 net new PM peak hour trips, and 335 net new daily trips. The TIA analyzed the intersection of Highway 26 and Ten Eyck Road and the intersection of Pleasant Street and Ten Eyck Road; both intersections were found to meet ODOT and City operational standards

either with or without development. The City's Transportation Engineer (Exhibit P) reviewed the TIA and did not find a need for mitigation measures to address traffic impacts of the development or to address safety issues. Criterion C is met.

Criteria D states "Architectural features of the proposed development will be compatible to the design character of existing structures on adjoining properties and on the proposed development site." The proposed building will be designed to be compatible with Sandy Style and includes gabled roofs, secondary roof forms, a mix of siding material, and strong base material. The subject property is directly west of the Sandy Police Station, which also includes many Sandy Style elements. Criterion D is met.

Staff believes that an 18 percent reduction in base material on the building's frontages is not contrary to the Comprehensive Plan or other City policies, does not reduce the amount of privacy enjoyed by users of nearby structures, will not adversely affect existing systems, and will be compatible to the design character of existing structures on adjoining properties. Staff recommends the Planning Commission approve a Type II Adjustment to allow the proposed building to not include a base material on 18 percent of the building's facades.

13. WINDOW PERCENT SPECIAL VARIANCE

Section 17.90.110(E.2) states: "The ground floor elevation of all new buildings shall contain display areas, windows, and doorways along street frontages and where the building abuts a civic space as follows: Lots with multiple street frontages are required to meet this standard on only two frontages." Buildings up to 10,000 square feet in size are required to provide windows on 40 percent of the ground floor elevation.

Request: The applicant requests a Type III Special Variance to Section 17.90.110(E.2) to not meet the 40 percent window requirement along street frontages and where the building abuts a civic space.

Section 17.66.80 contains criteria for granting a Special Variance. The Planning Commission may grant a special variance waiving a specified provision under the Type III procedure if it finds that the provision is unreasonable and unwarranted due to the specific nature of the proposed development. In submitting an application for a Type III Special Variance, the proposed development explanation shall provide facts and evidence sufficient to enable the Planning Commission to make findings in compliance with the criteria set forth in this section while avoiding conflict with the Comprehensive Plan.

One of the following sets of criteria shall be applied as appropriate.

- A. The unique nature of the proposed development is such that:
 - 1. The intent and purpose of the regulations and of the provisions to be waived will not be violated; and
 - Authorization of the special variance will not be materially detrimental to the public welfare and will not be injurious to other property in the area when compared with the effects of development otherwise permitted.
- B. The variance approved is the minimum variance needed to permit practical compliance with a requirement of another law or regulation.

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C. When restoration or replacement of a nonconforming development is necessary due to damage by fire, flood, or other casual or natural disaster, the restoration or replacement will decrease the degree of the previous noncompliance to the greatest extent possible.

The applicant is proposing to provide windows as follows:

- 43 percent windows on the east façade (parking lot)
- 27 percent windows on the south façade (Highway 26)
- 28 percent windows on the west façade (Ten Eyck Road)
- 23 percent windows on the north façade (Pleasant Street)

The proposed building is approximately 9,381 square feet, which is just under 10,000 square feet. The percent of ground floor windows required for buildings greater than 10,000 square feet is 25 percent of the ground floor elevation. Per the submitted narrative (Exhibit B), it is not practical to have 40 percent windows on two of the elevations due to the use of the building as a health clinic. The applicant is proposing 43 percent windows on the east elevation; however, the east elevation faces the parking lot and not a street. The applicant is not proposing to meet the 40 percent window requirement on any of the three street-facing facades. Staff believes a reduction below the 40 percent minimum window coverage on the street-facing facades will not violate the intent of the code nor be detrimental to public welfare, in compliance with Criterion A. Because the proposed use is a health clinic, staff understands the need for a certain degree of privacy. Alternatively, the applicant could install additional windows with permanent window coverings to provide privacy, but staff finds this method as overly burdensome and an unnecessary expense. Staff recommends the Planning Commission approve a Special Variance to Section 17.90.110(E.2) to not provide 40 percent window coverage along street frontages.

17.74 – Accessory Development

14. Section 17.74.40(B) contains height requirements for fences and retaining walls in commercial and industrial zones. The subject property is zoned Central Business District (C-1). The applicant is proposing two screening fences along the south edge of the site adjacent to Highway 26. The proposed fences are 8 feet in height, which is the maximum height allowed for walls/fences in the rear yards of commercial properties. As discussed in Chapter 17.42 of this document, the applicant will be required to re-address the site to a Pleasant Street address such that Pleasant Street is the front lot line. Once this occurs, Highway 26 will be the rear lot line and the proposed 8 foot tall screening fences will be allowed per the code. If the proposal includes a retaining wall, the applicant shall submit additional details on the proposed retaining wall for staff review and approval. All retaining walls shall have architectural finish.

17.84 – Improvements Required with Development

- 15. Chapter 17.84 provides general information regarding improvements required in association with development. All required improvements shall be installed or financially guaranteed prior to final occupancy.
- 16. Section 17.84.20 provides information on timing of improvements. Submission of preliminary street and utility plans during the land use review process is solely for compliance with the data requirements of Section 17.100.60 (D). Preliminary plat approval does not connote utility or public improvements plan approval which will be reviewed and approved separately upon submittal of public improvement construction plans.

- 17. Section 17.84.30 requires sidewalks and planting areas along all public streets. Per the Public Works Director (Exhibit R), the applicant shall remove the existing west driveway approach **onto Pleasant Street.** The existing driveway approach doesn't meet the minimum spacing standard in Section 17.98.80(A) or the maneuvering standard in Section 17.98.70(B). The applicant shall improve the Pleasant Street frontage of the site including but not limited to: curbs, sidewalks, storm drainage, streetlighting, and street trees per the requirements in Chapter 15.20 and Section 17.84.30 of the Sandy Municipal Code. The sidewalk shall be five feet wide separated from the curb with a five-foot-wide planter strip including street trees specified by the City on 30 foot centers. Alternatively, the sidewalk may be curb-tight, minimum 8 feet in width with street trees specified by the City in tree wells on 30 foot centers. Per the City Engineer (Exhibit N), where the existing driveway is removed on Pleasant Street, it shall be replaced with sidewalks and an ADA ramp shall be constructed at the intersection with Ten Eyck Road to current PROWAG requirements as required by Clackamas County. Ten Eyck Road is a County road so the City Engineer recommends the County require sidewalks to be constructed along the entire site frontage to match the existing sidewalks on Highway 26. Per the Public Works Director (Exhibit R), the existing driveway approach onto Ten Eyck Road from the site does not meet the minimum spacing standards in Section 17.98.80(A) of the Sandy Municipal Code (SMC). The applicant shall abandon the existing driveway approach and improve the Ten Eyck Road frontage of the site including but not limited to: curbs, sidewalks, storm drainage, streetlighting, and street trees per the requirements in Chapter 15.20 and Section 17.84.30 of the Sandy Municipal Code. This section of Ten Eyck Road is under the jurisdiction of Clackamas County. The applicant shall coordinate with the Clackamas County Department of Transportation and Development (DTD) to determine the required section for Ten Eyck Road. This may include relocating the existing fire hydrant at the intersection of Ten Eyck Road and Pleasant Street to install **ADA compliant access ramps.** Ten Evck Road is a minor arterial street. Both Clackamas County and the City of Sandy require minimum 6-foot-wide sidewalks on arterial streets. The County standard and the proposed ultimate section for Ten Eyck Road will only permit a curb tight sidewalk in the available right-of-way. The City recently designed a pedestrian improvement project on Ten Eyck Road at the subject site. The Clackamas County DTD plan review fee has been paid and the plans have been approved by DTD. The applicant shall be responsible for these improvements and is welcome to use the approved plan set (Exhibit T) for this work. The Urban Renewal Agency has agreed to reimburse the applicant \$14,000 for completing the Ten Eyck Road improvements.
- 18. 17.84.50 contains standards for street improvements and traffic evaluations. The applicant submitted a Transportation Impact Analysis (TIA; Exhibit H) dated March 12, 2020 and prepared by John Manix of PBS. The City's Transportation Engineer (Exhibit P) reviewed the TIA and concurs with the TIA's conclusions. The Transportation Engineer does not find a need for mitigation measures to address traffic impacts of the development or to address safety issues. The Transportation Engineer recommends that ODOT requirements and standards associated with frontage improvements where the development abuts Highway 26 are made conditions of approval for the development. ODOT (Exhibit Q) reviewed the proposal and Traffic Impact Analysis and determined that a westbound right turn lane at the Highway 26 and Ten Eyck Road intersection is not warranted. ODOT further determined that there will be no significant impacts to state highway facilities and no additional state review is required. Per the Public Works Director (Exhibit R), the applicant shall improve the Highway 26 frontage of the site in

compliance with ODOT requirements including modifying the existing slip/right-turn lane to reduce the curb radius at the corner to accommodate east-west and north-south ADA compliant ramps if required by ODOT. This may require right-of-way dedication at the NE corner of Highway 26 and Ten Eyck Road. The applicant shall dedicate sufficient right-of-way at the SW corner of the site to accommodate the required street section and pedestrian improvements including ADA-compliant ramp(s) at the intersection with Highway 26. The exact dedication area shall be determined during construction plan review. The applicant shall be responsible for providing legal descriptions and sketch maps of the dedication area, dedicating the right-of-way using the City's standard documents, and paying all recording costs.

- 19. Sections 17.84.60 and 17.84.70 relate to public utilities. Utility and right-of-way improvement plans are submitted with the land use application solely for conformance with the submittal requirements in Section 17.100.60(D). Land use approval does note connote approval of public improvement plans. Per the Public Works Director (Exhibit R), the site is served by the existing 16-inch water main in Highway 26 and the existing 8-inch sanitary sewer main in Pleasant Street. The applicant is proposing a drywell for stormwater disposal. Typically, the soils in Sandy do not permit treatment and discharge of stormwater in this manner. Stormwater management shall conform to the requirements in the City of Portland Stormwater Management Manual and the requirements in Sections 13.18 and 13.20 of the Sandy Municipal Code. The City Engineer reviewed the preliminary stormwater calculations and found that the calculations do not meet the water quality/quantity criteria as stated in the City of Sandy Municipal Code 13.18 Standards and the 2016 City of Portland Stormwater Management Manual (SWMM) Standards that were adopted by reference into the Sandy Development Code. The water quality shall be designed based on 0.19 in/hr rate for 5 minutes time of concentration. While the water quality shall be designed for 2, 5, 10, and 25-year storm events and not 25 and 100 year storm events only. A detailed final report stamped by a licensed professional shall be resubmitted for review with the final construction plans.
- 20. Section 17.84.80 contains standards for franchise utility installation. Private utility services will be submitted for review and approval by service providers and City staff in association with construction plans, and all utility lines will be extended to the perimeter of the site. All franchise utilities shall be installed underground and in conformance with City standards. The applicant shall call the PGE Service Coordinators at 503-323-6700 when the developer is ready to start the project.
- 21. The Fire Marshal (Exhibit O) reviewed the proposal and states the new fire department connection (FDC) shall be relocated to the Highway 26 side of the proposed fire vault to be as close as possible to the existing fire hydrant located in that area. Per the City Engineer (Exhibit N), the final construction plans shall be submitted to the Sandy Fire Marshal for review and approval to ensure that the proposed vault has adequate fire protection.
- 22. Section 17.84.100 contains requirements for mail delivery facilities. The applicant will need to coordinate with the United States Postal Service (USPS) to relocate mail facilities and these will be approved by the City and USPS. Mail delivery facilities shall be provided by the applicant in conformance with 17.84.100 and the standards of the USPS. The applicant shall submit a mail delivery plan to the City and USPS for review and approval prior to installation of mailboxes.

17.90 – Design Standards

- 23. The proposal is subject to all the requirements for Design Review as stated in Section 17.90.00.
- 24. Section 17.90.70 specifies that design review approval shall be void after two (2) years from the date of the Final Order unless the applicant has submitted plans for building permit approval.
- 25. Section 17.90.110(A) contains site layout and vehicle access standards intended to provide for compact, walkable development, and to design and manage vehicle access and circulation in a manner that supports pedestrian safety, comfort and convenience. All lots shall abut or have cross access to a dedicated public street. The subject property abuts Highway 26, Ten Eyck Road, and Pleasant Street.
- 26. Section 17.90.110(A)(3) requires that off-street parking shall be located to the rear or side of buildings with no portion located within 10 feet of the public right-of-way. All parking spaces are proposed to the east side of the proposed building.
- 27. Section 17.90.110(A)(5) states that raised walkways or painted crossings shall be provided from the sidewalk to entrances of buildings. The primary building entrance is located on the east elevation facing the parking area. The proposal includes a pedestrian walkway along the east elevation that's connected to the Pleasant Street sidewalk through a civic area. In addition, the proposal includes walkway connections to the Highway 26 sidewalk from two exit doors and an exterior staff break area on the south elevation.
- 28. Section 17.90.110(A)(9) states that joint use of access points and cross-over easements between parcels shall be required where the City determines it is practicable and necessary. Prior to occupancy the applicant and the City of Sandy shall record cross-over easements for vehicular and pedestrian traffic and a shared parking agreement. The terms of that agreement shall be negotiated by the two parties.
- 29. Section 17.90.110(B) contains standards regarding building facades, materials, and colors intended to be consistent with the Sandy Style. Section 17.90.110(B.1) requires that buildings be articulated, varied and provide visual interest. The proposed elevations of the structure include varying wall planes that do not exceed 30 feet in length. Variations include secondary roofs, wall projections, and changes in siding material.
- 30. Section 17.90.110(B.2) requires that buildings incorporate pedestrian shelters over primary building entrances. Pedestrian shelters shall extend at least five feet over the pedestrian area. The proposal includes a gabled primary pedestrian entry on the east façade of the building with a five (5) foot roof overhang. In addition, there are two pedestrian doors for staff use on the south façade and a third on the north façade. Based on the submitted Floor Plan (Exhibit C, Sheet G0.34), the door from the break room on the south facade has a metal canopy overhang that extends at least 5 feet. The canopy overhangs on the door from the hallway on the south side and the door on the north façade are only 4 feet; however, neither door is a primary entrance or a public entrance.

- 31. Section 17.90.110(B.3) specifies approved building materials. Section 17.90.110(B.3.b) requires buildings to include strong base materials such as natural stone, split-faced rusticated concrete block, or brick on all sides of a building visible from an abutting public street. All four elevations are visible from the public right-of-way and, therefore, require a strong base material. The applicant is requesting a Type II Adjustment to not include base material on 18 percent of the building's façade. The Adjustment request is reviewed in Chapter 17.66 of this document.
- 32. Section 17.90.110(B.3.d) contains approved siding. The applicant is proposing to use a mix of vertical and horizontal siding, both of which are Nichiha siding, which is a panelized fiber cement product. Per Section 17.90.110(B.3.d.1) "Vertical grooved (i.e., T1-11) sheet siding and similar materials are prohibited." Therefore, the applicant is requesting a design deviation to Section 17.90.110(B.3.d.1) to allow the use of Nichiha vertical grooved sheet siding. Per email correspondence with the applicant (Exhibit S), "The vertical ribbed Nichiha product is a panelized fiber cement product with 1 5/8" vertical slats and 3/8" reveals between each slat which add depth and rustic texture to the façade. The product is available in custom colors that can match any selected Miller or Sherwin Williams paint. The ribbed fiber cement is not the same as board-and-batten siding or T1-11 sheet siding. The product can be installed either vertically or horizontally. In terms of design, the vertical siding is a darker earth tone and is the bulk of the siding. The vertical orientation was selected to distinguish it from the horizontal redwood Nichiha material pops of warm color at the bumpouts and recesses in the facade. Both siding products are Nichiha so all the siding can come from a single source manufacturer. This will make the construction process smoother and simplify the detailing. The vertical siding has been selected as the main siding, because the vertical orientation is better for cleaning and maintenance over time." The applicant submitted photo samples of the Nichiha siding (Exhibit G) along with the proposed colors: redwood for the horizontal siding and weathered copper for the vertical siding. Staff believes the Nichiha siding meets the intent of the code. Staff recommends the Planning Commission approve the applicant's request for a design deviation to Section 17.90.110(B.3.d.1) to allow the use of Nichiha siding.
- 33. Section 17.90.110(B.4) specifies approved colors. The applicant is proposing a redwood color for the horizontal siding and a weathered copper color for the vertical siding. Staff is unsure if these proposed colors are stains or actual paint colors. If they are paint colors, they are not on the City's approved color palette. The applicant shall provide clarification on the proposed redwood and weathered copper colors. If they are paint colors, the applicant shall choose alternate colors from the City's approved color palette, or shall apply for a Design Deviation. The trim and fascia are Sturgis Gray, which is an approved color on the City's color palette.
- 34. Section 17.90.110(C) requires gable roofs on new buildings. The proposed building includes a gabled roof on the south elevation as well as a shed roof on the east elevation of the primary entrance. All primary roof pitches are 6:12 or greater in compliance with Section 17.90.110(C.1).
- 35. Section 17.90.110(C.4) requires secondary roof forms based on roof length. The applicant is proposing secondary shed roofs at a different pitch (4 1/2:12) than the primary roof to break up the roofline in compliance with the code.

- 36. Section 17.90.110(C.5) requires visible roof materials to be wood shingle or architectural grade composition shingle, slate, or concrete tile. Metal with standing or batten seam may also be used conforming to the Color Palette in Appendix D of the development code. The applicant is proposing to use standing seam metal roof panels with the proposed color of Old Town Gray. The applicant shall choose a metal roof color from the City's approved metal roof color palette, or shall apply for a Design Deviation.
- 37. Section 17.90.110(D) contains standards regarding building orientation and entrances intended to maintain and enhance downtown and village commercial streetscapes as public spaces, emphasizing a pedestrian-scale and character in new development, consistent with the Sandy Style; and to provide for a continuous pedestrian network that promotes pedestrian safety, comfort and convenience, and provides materials and detailing consistent with the Sandy Style. The proposed building has frontage on Highway 26, Ten Eyck Road, and Pleasant Street. Section 17.90.100(D)(1) requires at least 50 percent of the subject site's street frontage to be comprised of building(s) placed within 10 feet of the sidewalk. The building occupies more than 50 percent of the street frontage on all three abutting streets.
- 38. Section 17.90.110(D.4) specifies buildings located at the intersection of two streets shall use a corner building entrance; where a corner entrance is not practical due to the internal functioning of the building space or due to physical constraints of the site (e.g., topography, accessibility, or similar circumstances), a building entrance must be provided within 40 feet of the corner. The applicant is proposing to locate the public entrance on the east elevation of the proposed building and is therefore requesting a Design Deviation to Section 17.90.110(D.4) to not provide a primary entrance at the corner or within 40 feet of the corner. Per the applicant's narrative (Exhibit B), "a direct corner entrance is not practical due to the nature of the building's use as a health clinic and its relation to the parking lot." Staff recognizes that the parking lot is existing and that it makes sense to have a primary entrance near the parking. While it would be feasible to have a second pedestrian entrance at the corner, staff realizes that this is less practical due to the nature of a health clinic, which typically has one central public entrance with a reception desk and waiting area. Staff recommends the Planning Commission approve the requested Design Deviation to Section 17.90.110(D.4) to not provide a corner entrance.
- 39. Section 17.90.110(D.6) specifies that buildings shall provide at least two elevations where the pedestrian environment is "activated." An elevation is "activated" when it meets the window transparency requirements in Subsection 17.90.110(E) and contains a customer entrance with a pedestrian shelter extending at least five (5) feet over an adjacent sidewalk, walkway or civic space. Where providing a customer entrance on two (2) elevations is not practical, the reviewing body may allow a single entrance. The proposed building does not meet the window percent requirements on any elevations and the applicant has requested a Special Variance, which is reviewed in Section 17.90.110(E) of this document. The proposed building includes a single primary public entrance that faces the parking lot. The applicant's narrative (Exhibit B) states: "There is an entrance with a pedestrian shelter facing Hwy 26 and facing Pleasant Street, however, due to the nature of the clinic, these are staff entrances, not customer entrances. The patient entrance faces the parking lot. Patients may leave the other (2) doors, but these are primarily intended for clinic staff." Staff concurs that it is not practical to provide a second public entrance due to the nature of the health clinic.

- 40. Section 17.90.110(D.7) specifies primary entries shall face a public street or a civic space and shall be spaced not more than 30 feet apart on average. The proposed primary entrance faces the parking lot. The applicant is requesting a Design Deviation to Section 17.90.110(D.7) to not provide a primary entrance that faces a public street or civic space. Staff recognizes that the parking lot is existing and that it makes sense to have a primary entrance near the parking area. While it would be feasible to have a second pedestrian entrance at the corner, staff realizes that this is less practical due to the nature of a health clinic, which typically has one central public entrance with a reception desk and waiting area. Staff recommends the Planning Commission approve the requested Design Deviation to Section 17.90.110(D.7) to not provide a public entrance facing a street or civic space.
- 41. Section 17.90.110(E) contains standards for construction and placement of windows. The intent of windows is to promote business vitality, public safety, and aesthetics through effective window placement and design. Section 17.90.110(E)(2) specifies that the activated frontage shall contain a minimum of 40 percent of the building elevation in windows and that lots with multiple street frontages are required to meet this standard on two frontages. The proposed percent windows on each elevation are as follows:
 - 43 percent windows on the east façade (parking lot)
 - 27 percent windows on the south façade (Highway 26)
 - 28 percent windows on the west façade (Ten Eyck Road)
 - 23 percent windows on the north façade (Pleasant Street)

Only the east elevation, which faces the parking area, meets the 40 percent window requirement. The applicant requested a Special Variance to Section 17.90.110(E.2) to not meet the 40 percent window requirement along two (2) street frontages. The Special Variance request is reviewed in Chapter 17.66 of this document.

- 42. Section 17.90.110(F) contains additional landscaping and streetscape design standards for parcels along Highway 26 and all other adjacent rights-of-way. This provision requires properties in the C-1 Central Business District to comply with the Downtown Sandy Streetscape Design as contained in Appendix F. The proposal includes street trees planted in tree wells located within the right-of-way as opposed to the traditional landscape strip as these two frontages are not intended to have landscape strips. The streetscape on Highway 26 is required to conform to the standards of Appendix F Streetscape Design Detail, therefore the applicant shall update the plan set to indicate conformance with the sidewalk tree grate and lighting details identified in Appendix F of the development code. Additionally, the streetscapes on Pleasant Street and Ten Eyck Road shall conform to the Curb & Sidewalk Improvements final plan (Exhibit T).
- 43. Section 17.90.110(G) contains standards for civic spaces within developments. The intent of civic space is to connect buildings to the public realm and create comfortable and attractive gathering places and outdoor seating areas for customers and the public. The development code requires developments to provide a civic space area that is at least three percent of the building ground floor area and in no instance is less than 64 square feet. The ground floor area of the proposed use is 9,381 square feet which results in a total need for approximately 281 square feet of civic space. The proposal includes a total of 320 square feet of civic space in the form of a hardscaped area at the northeast corner of the building adjacent to Pleasant Street. The civic space is proposed to contain planters and two benches.

- 44. Section 17.90.110(H) contains standards regarding lighting. The applicant shall follow all Dark Sky Ordinance requirements as outlined in Chapter 15.30 of this document.
- 45. Section 17.90.110(I) contains standards regarding safety and security intended to promote natural surveillance of public spaces. The proposed parking area is located adjacent to the east elevation of the proposed building. The east elevation (Exhibit C, Sheet A3.11) identifies windows along the east façade, which enables both surveillance of the parking area from inside as well as surveillance of the interior from the parking area. In addition, the proposal includes windows on the north, south, and west façades, which provides visibility to the adjacent sidewalks.
- 46. Section 17.90.110(I.3) requires street address numbers. The applicant shall provide street address numbers measuring a minimum of six (6) inches high, which clearly locate buildings and their entries for patrons and emergency services. The applicant shall verify the location(s) of the address with the Building Official and emergency service providers.
- 47. Section 17.90.110(J) contains standards regarding external storage and screening intended to promote land use compatibility and aesthetics, particularly where development abuts public spaces. The proposal includes a garbage/recycle enclosure in the parking area along the Highway 26 frontage. The enclosure is an 8 foot by 10 foot concrete block enclosure. Per the submitted narrative (Exhibit B), "trash and recycling storage is in a split face CMU enclosure in the southeast corner of the site. Enclosure CMU matches the color of the concrete block veneer base of the building." In addition, the metal roof of the trash enclosure matches the metal roofing color for the primary structure. Hoodview Disposal and Recycling typically requires the applicant to verify the level of service to determine if the trash/recycling enclosure is adequate size. The applicant shall verify the location of the trash and recycling enclosure with Hoodview.
- 48. The proposal includes mechanical and fire equipment at the southeast corner of the building along Highway 26. The equipment will be screened from the public right-of-way by an 8 foot tall wood stained fence.

17.92 – Landscaping and Screening

- 49. The subject property is zoned Central Business District, C-1. Section 17.92.20 requires that a minimum of 10 percent of the site be landscaped in the C-1 zoning district. The submitted Landscape Plan (Exhibit C, Sheet L1.01) details 3,250 square feet of landscaped area, which equates to a total of 15 percent of the site.
- 50. Section 17.92.10(C) requires significant plant and tree specimens to be preserved to the greatest extent practicable and integrated into the design of a development. Per the Existing Site Survey (Exhibit C, Sheet G0.11), there are two (2) existing 8-inch DBH conifers located in the southwest portion of the site. The Demolition Plan (Exhibit C, Sheet C0.3) indicates that these trees will be removed to allow for construction of the proposed building. The subject property is only 0.47 acres; the proposed building and parking area take up almost the entire site. The building size would need to be reduced significantly in order to retain the trees. Thus, staff determined that retention of the two trees is not practicable. The applicant will be planting

- additional trees in the parking area and installing additional landscaping around the entire building.
- 51. Section 17.92.30 requires trees to be planted along public street frontages. There are existing street trees on Highway 26 and the east end of the subject property's Pleasant Street frontage. Medium sized street trees spaced 30 feet on center will be required for both Ten Eyck Road and Pleasant Street. The Landscape Plan (Exhibit C, Sheet L1.01) details street trees along all three frontages of the subject property. The proposed street trees along Pleasant Street and Ten Eyck Road are 2-inch caliper Red Rage Tupelo (Nyssa sylvatica 'Hayman's Red'). All street trees shall be a minimum of 1.5-inches in caliper measured 6 inches above the ground and shall be planted per the City of Sandy standard planting detail. All street trees shall be staked and tied with loose twine so as not to damage the trunk; the twine shall be removed after one growing season (or a maximum of 1 year). Per the Public Works Director (Exhibit R), street tree and landscaping placement shall conform to the sight distance recommendations in the traffic impact analysis submitted by the applicant.
- 52. The parking lot is proposed to contain additional Red Rage Tupelo (*Nyssa sylvatica* 'Hayman's Red') trees. The Landscape Plan (Exhibit C, Sheet L1.01) details a structural tree at the ends of each parking bay with the exception of the parking bay directly in front (east) of the building. The applicant shall update the Landscape Plan to detail one structural tree at the north and south ends of the parking bay east of the building.
- 53. Section 17.92.40 requires that all landscaping be irrigated, either with a manual or automatic system. The Landscape Plan (Exhibit C, Sheet L1.01) states: "all landscape areas within the site including turf, shrub beds and tree areas shall be irrigated with an automatic irrigation system. The irrigation system shall be adjusted to meet the water requirements of the individual plant material." As required by Section 17.92.140, the developer shall be required to maintain all vegetation planted in the development for two years from the date of completion, and shall replace any dead or dying plants.
- 54. Section 17.92.50 specifies the types and sizes of plant materials for landscaping areas. The development code requires that all deciduous trees shall be at least 1.5 inches caliper and all coniferous trees shall be a minimum of 5 feet in height at planting. All shrubs/plants are required to be a minimum of 1 gallon in size or 2 feet in height at planting. The Landscape Plan Plant Schedule (Exhibit C, Sheet L4.00) identifies proposed trees at 2-inch caliper as well as a mix of shrubs, grasses, and vines, all of which meet the minimum 1 gallon in size requirement.
- 55. Section 17.92.130 details performance bond standards as they associate to required landscaping. The applicant has the option to defer the installation of street trees and/or landscaping for weather-related reasons. Staff recommends the applicant utilize this option rather than install trees and landscaping during the dry summer months. If the applicant chooses to postpone street tree and/or landscaping installation, the applicant shall post a performance bond equal to 120 percent of the cost of the landscaping, assuring installation within 6 months. The cost of street trees shall be based on the street tree plan and at least \$500 per tree. The cost of landscaping shall be based on the average of three estimates from three landscaping contractors; the estimates shall include as separate items all materials, labor, and other costs of the required action, including a two-year maintenance and warranty period.

17.98 – Parking, Loading, and Access Requirements

- 56. Section 17.98.20 contains off-street parking requirements. As detailed on the Site Plan (Exhibit C, Sheet A1.01), the Clackamas County Health Clinic site is proposed to contain 14 parking spaces and two bicycle parking spaces. Two bicycle parking spaces are proposed to be located under the overhang adjacent to the primary entrance. At the time of this land use application submittal medical offices/clinics were required to contain one vehicle parking space per 300 square feet of retail area, plus one parking space per two employees on the largest shift, and 5 percent or 2 bicycle parking spaces, whichever is greater. Per the submitted narrative (Exhibit B), the building will have approximately 8,345 square feet of clinic space (excluding hallways, restrooms, mechanical rooms, etc.). Based on this square footage of clinic area, the site is required to provide 28 parking spaces. The narrative states that the largest shift will have 12 employees requiring 6 additional parking spaces for a total of 34 required parking spaces. The property is in the C-1 zone, which at the time of this land use application submittal the development code allowed for a 25 percent reduction in required parking spaces; thus, 26 parking spaces are required with the allowed parking reduction. In addition to the 14 parking spaces on site, there are six (6) additional spaces shared with the adjacent property to the east. The applicant has also entered into a seven (7) year lease agreement for the use of 17 parking spaces for employees in the Immanuel Lutheran Church parking lot east of the subject property on the south side of Pleasant Street. Moreover, since the applicant submitted this land use application, the parking chapter of the development code (Chapter 17.98) was modified to remove minimum parking requirements for the C-1 zone.
- 57. ORS 447.233 requires one accessible parking space for each 25 parking spaces up to 100 parking spaces. The Site Plan (Exhibit C, Sheet A1.01) details one proposed ADA parking space on the east side of the proposed building. The accessible parking space is detailed at 9 feet in width with an 8-foot-wide access aisle on the passenger side of the ADA space as required by ORS.
- 58. Section 17.98.60 contains specifications for parking lot design and size of parking spaces. Standard parking spaces shall be at least 9 feet by 18 feet and compact spaces shall be at least 8 feet by 16 feet. The proposal provides a total of 14 parking spaces on site, including seven (7) standard spaces at 9 feet wide by 18 feet deep, six (6) compact parking spaces at 8 feet wide by 16 feet deep, and one (1) ADA space as specified above. The parking spaces are 90-degree spaces with a minimum 25-foot-wide drive aisle in compliance with the development code.
- 59. Section 17.98.100 has specifications for driveways. The site has one existing driveway on Ten Eyck Road and two existing driveways on Pleasant Street. The applicant is proposing to remove the existing driveway on Ten Eyck Road and to remove the existing west driveway on Pleasant Street. The existing driveway on Ten Eyck road is a safety concern due to the spacing to the intersection of Ten Eyck Road and Highway 26. Per the City Engineer (Exhibit N), the proposed driveway access on Pleasant Street shall be a concrete commercial driveway approach constructed in conformance with the applicable City of Sandy driveway details and meeting PROWAG requirements.
- 60. Section 17.98.120 contains requirements for landscaping and screening in parking areas. Since the proposal is for more than four parking spaces, compliance with the standards of Section 17.98.120 is required. The location of the parking lot already exists. The area between the parking areas and the right-of-way is at least 5 feet in planting depth. As detailed on the Site Plan (Exhibit C, Sheet A1.01), the proposed parking areas are divided into bays of less than 20 spaces.

Landscaping is proposed at the ends of some but not all parking rows. The parking row directly east of the proposed building is flanked by hardscaped areas – the proposed civic space to the north and a hardscaped area for the fire vault to the south. These areas could still contain structural trees in tree wells. The applicant shall update the Landscape Plan to detail one structural tree at the north and south ends of the parking bay east of the building.

- 61. Section 17.98.120 (F) contains requirements for wheel stops to protect landscaping and pedestrians. The submitted Site Plan (Exhibit C, Sheet A1.01) details wheel stops in every parking space.
- 62. Section 17.98.130 requires all parking and vehicular maneuvering areas to be paved with asphalt or concrete. The applicant is proposing to remove and replace the existing parking area. Per the narrative (Exhibit B) preamble asphalt will be used for the new parking lot surface in compliance with the development code.
- 63. Section 17.98.140 contains requirements for drainage. Chapter 17.84 of this document details the stormwater requirements.
- 64. Section 17.98.160 contains requirements for bicycle parking facilities. The requirement is bicycle parking at 5 percent of the vehicle parking or two bicycle parking spaces, whichever is greater. The Site Plan (Exhibit C, Sheet A1.01) details two (2) bicycle parking spaces under the overhang adjacent to the primary building entrance.

15.30 - Dark Sky Ordinance

- 65. Chapter 15.30 contains the City of Sandy's Dark Sky Ordinance. The applicant submitted a Photometric Plan (Exhibit C, Sheet E1.02) that details foot candles on the subject property and 10 feet beyond the property lines. All outdoor lighting systems must be designed so that the area 10 feet beyond the property line of the premises receives no more than .25 (one quarter) of a foot-candle of light from the proposed listing system. The Photometric Plan details readings that exceed 0.25 (one-quarter) of a foot-candle 10 feet beyond the south property line along Highway 26. The applicant shall adjust the location and/or illumination of the S1 lighting fixture located in the middle of the four (4) compact parking spaces on the south edge of the parking lot such that the area 10 feet beyond the property line does not exceed 0.25 of a foot candle. The applicant shall submit the revised photometric analysis for staff review and approval.
- 66. The submitted Photometric Plan (Exhibit C, Sheet E1.02) and lighting cut-sheets (Exhibit D) detail all lights at 3,500 Kelvins or less and downward facing. **All exterior lights shall be full cut-off and shall not exceed 4.125 Kelvins.**

15.44 – Erosion Control Regulations

67. All erosion control and grading shall comply with Section 15.44 of the Municipal Code. All onsite earthwork activities including any retaining wall construction shall follow the requirements of the most current edition of the Oregon Structural Specialty Code (OSSC). If the proposal includes a retaining wall, the applicant shall submit additional details on the proposed retaining wall for staff review and approval. The earthwork activities shall be observed and documented under the supervision of a geotechnical engineer.

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68. Site grading shall not in any way impede, impound, or inundate the surface drainage flow from the adjoining properties without a proper collection system. All the work within the public right-of-way and within the paved area should comply with American Public Works Association (APWA) and City requirements as amended. The applicant shall submit a grading and erosion control permit and request an inspection of installed devices prior to any additional grading onsite. The grading and erosion control plan shall include a revegetation plan for all areas disturbed during construction of the subdivision. All erosion control and grading shall comply with Section 15.44 of the Municipal Code.

RECOMMENDATION

Staff recommends the Planning Commission **approve** the requested Design Review, adjustment and variance associated with the Clackamas County Health Clinic as modified by the conditions of approval listed below.

Staff recommends the Planning Commission **approve** a Type II Adjustment to allow the proposed building to not include a base material on 18 percent of the building's facades.

Staff recommends the Planning Commission **approve** a Special Variance to Section 17.90.110(E.2) to not provide 40 percent window coverage along street frontages.

Staff recommends the Planning Commission **approve** the applicant's request for a design deviation to Section 17.90.110(B.3.d.1) to allow the use of Nichiha siding.

Staff recommends the Planning Commission **approve** the requested Design Deviation to Section 17.90.110(D.4) to not provide a corner entrance.

Staff recommends the Planning Commission **approve** the requested Design Deviation to Section 17.90.110(D.7) to not provide a public entrance facing a street or civic space.

CONDITIONS OF APPROVAL

A. Prior to any onsite grading or earthwork:

- 1. The applicant shall submit a grading and erosion control permit and request an inspection of installed devices prior to any additional grading onsite. The grading and erosion control plan shall include a re-vegetation plan for all areas disturbed during construction. All erosion control and grading shall comply with Section 15.44 of the Municipal Code.
- B. Prior to building permit submittal, the applicant shall complete the following and receive necessary approvals as described below:
 - 1. Provide staff with one copy of the final and complete plan set, including elevations, for the proposal to be included in the case file for future reference.
 - 2. Update the plan set to indicate conformance with the sidewalk tree grate and lighting details identified in Appendix F of the development code. Additionally, the streetscapes on Pleasant

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Street and Ten Eyck Road shall conform to the Curb & Sidewalk Improvements final plan (Exhibit T).

- 3. Update the Landscape Plan to detail one structural tree at the north and south ends of the parking bay east of the building.
- 4. Submit a revised photometric plan that adjusts the location and/or illumination of the S1 lighting fixture located in the middle of the four (4) compact parking spaces on the south edge of the parking lot such that the area 10 feet beyond the property line does not exceed 0.25 of a foot candle.
- 5. If the proposal includes a retaining wall, the applicant shall submit additional details on the proposed retaining wall for staff review and approval. All retaining walls shall have architectural finish.
- 6. Provide clarification on the proposed redwood and weathered copper colors. If they are paint colors, the applicant shall choose alternate colors from the City's approved color palette, or shall apply for a Design Deviation.
- 7. Choose a metal roof color from the City's approved metal roof color palette, or apply for a Design Deviation.

C. The Final Construction Plans shall include the following:

- 1. Stormwater management shall conform to the requirements in the City of Portland Stormwater Management Manual and the requirements in Sections 13.18 and 13.20 of the Sandy Municipal Code. The water quality shall be designed based on 0.19 in/hr rate for 5 minutes time of concentration. While the water quality shall be designed for 2, 5, 10, and 25-year storm events and not 25 and 100 year storm events only. A detailed final report stamped by a licensed professional shall be resubmitted for review with the final construction plans.
- 2. The final construction plans shall be submitted to the Sandy Fire Marshal for review and approval to ensure that the proposed vault has adequate fire protection.

D. Prior to the certificate of occupancy for the site, the applicant shall complete the following:

- 1. All required improvements shall be installed or financially guaranteed.
- 2. The applicant and the City of Sandy shall record cross-over easements for vehicular and pedestrian traffic and a shared parking agreement. The terms of that agreement shall be negotiated by the two parties.
- 3. Submit a signed copy of the parking space lease agreement with Immanuel Lutheran Church.
- 4. Abandon the existing driveway approach and improve the Ten Eyck Road frontage of the site including but not limited to: curbs, sidewalks, storm drainage, streetlighting, street trees per the requirements in Chapter 15.20 and Section 17.84.30 of the Sandy Municipal Code. Coordinate with the Clackamas County Department of Transportation and Development to determine the required section for Ten Eyck Road. This may include relocating the existing fire hydrant at the

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intersection of Ten Eyck Road and Pleasant Street to install ADA compliant access ramps. Ten Eyck Road is a minor arterial street. Both Clackamas County and the City of Sandy require minimum 6-foot-wide sidewalks on arterial streets. The County standard and the proposed ultimate section for Ten Eyck Road will only permit a curb tight sidewalk in the available right-of-way. The applicant shall be responsible for these improvements and is welcome to use the approved plan set (Exhibit T) for this work.

- 5. Remove the existing west driveway approach onto Pleasant Street. Improve the Pleasant Street frontage of the site including but not limited to: curbs, sidewalks, storm drainage, streetlighting, and street trees per the requirements in Chapter 15.20 and Section 17.84.30 of the Sandy Municipal Code. The sidewalk shall be five feet wide separated from the curb with a five-footwide planter strip including street trees specified by the City on 30 foot centers. Alternatively, the sidewalk may be curb-tight, minimum 8 feet in width with street trees specified by the City in tree wells on 30 foot centers. Where the existing driveway is removed on Pleasant Street, it shall be replaced with sidewalks and an ADA ramp shall be constructed at the intersection with Ten Eyck Road to current PROWAG requirements as required by Clackamas County.
- 6. The proposed driveway access on Pleasant Street shall be a concrete commercial driveway approach constructed in conformance with the applicable City of Sandy driveway details and meeting PROWAG requirements.
- 7. Improve the Highway 26 frontage of the site in compliance with ODOT requirements including modifying the existing slip/right-turn lane to reduce the curb radius at the corner to accommodate east-west and north-south ADA compliant ramps if required by ODOT. This may require right-of-way dedication at the NE corner of Highway 26 and Ten Eyck Road. The applicant shall dedicate sufficient right-of-way at the SW corner of the site to accommodate the required street section and pedestrian improvements including ADA-compliant ramp(s) at the intersection with Highway 26. The exact dedication area shall be determined during construction plan review. The applicant shall be responsible for providing legal descriptions and sketch maps of the dedication area, dedicating the right-of-way using the City's standard documents, and paying all recording costs.
- 8. The new fire department connection (FDC) shall be relocated to the Highway 26 side of the proposed fire vault to be as close as possible to the existing fire hydrant located in that area.
- 9. Update the address of the site to be addressed from Pleasant Street and pay the re-addressing fee (\$206) to the City of Sandy. Install street address numbers measuring a minimum of six (6) inches high, which clearly locate buildings and their entries for patrons and emergency services. The applicant shall verify the location(s) of the address with the Building Official and emergency service providers.
- 10. Install all landscaping, building improvements, and other site improvements as detailed in this final order.
- 11. Install mail delivery facilities in conformance with 17.84.100 and the standards of the USPS. The applicant shall submit a mail delivery plan to the City and USPS for review and approval prior to installation of mailboxes.

E. General Conditions of Approval

- 1. Design review approval shall be void after two (2) years from the date of the Final Order unless the applicant has submitted plans for building permit approval.
- 2. The applicant shall follow all Dark Sky Ordinance requirements as outlined in Chapter 15.30. All exterior lights shall be full cut-off and shall not exceed 4,125 Kelvins.
- 3. All street trees shall be a minimum of 1.5-inches in caliper measured 6 inches above the ground and shall be planted per the City of Sandy standard planting detail. All street trees shall be staked and tied with loose twine so as not to damage the trunk; the twine shall be removed after one growing season (or a maximum of 1 year). Street tree and landscaping placement shall conform to the sight distance recommendations in the traffic impact analysis submitted by the applicant.
- 4. The applicant shall be responsible for the installation of all improvements detailed in Section 17.100.310, including fiber facilities. SandyNet requires the developer to work with the City to ensure that broadband infrastructure meets the design standards and adopted procedures as described in Section 17.84.70.
- 5. All new utility lines and franchise utilities for future development shall be installed underground and in conformance with City standards.
- 6. The applicant shall call the PGE Service Coordinators at 503-323-6700 when the developer is ready to start the project.
- 7. All landscaping shall be continually maintained, including necessary watering, weeding, pruning, and replacing.
- 8. As required by Section 17.92.140, the developer shall be required to maintain all vegetation planted in the development for two years from the date of completion, and shall replace any dead or dying plants during that period.
- 9. All parking, driveway and maneuvering areas shall be constructed of asphalt, concrete, or other approved material.
- 10. The applicant will be required to obtain a separate permit for any proposed signage.
- 11. The applicant shall verify the location of the trash and recycling enclosure with Hoodview.
- 12. Preliminary plat approval does not connote utility or public improvements plan approval which will be reviewed and approved separately upon submittal of public improvement construction plans. Utility and right-of-way improvement plans are submitted with the land use application solely for conformance with the submittal requirements in Section 17.100.60(D). Land use approval does note connote approval of public improvement plans.
- 13. All on-site earthwork activities including any retaining wall construction shall follow the requirements of the most current edition of the Oregon Structural Specialty Code (OSSC). If the proposal includes a retaining wall, the applicant shall submit additional details on the proposed retaining wall for staff review and approval. The earthwork activities shall be observed and documented under the supervision of a geotechnical engineer.

- 14. Site grading shall not in any way impede, impound, or inundate the surface drainage flow from the adjoining properties without a proper collection system. All the work within the public right-of-way and within the paved area should comply with American Public Works Association (APWA) and City requirements as amended.
- 15. If the applicant chooses to postpone street tree and/or landscaping installation, the applicant shall post a performance bond equal to 120 percent of the cost of the landscaping, assuring installation within 6 months. The cost of street trees shall be based on the street tree plan and at least \$500 per tree. The cost of landscaping shall be based on the average of three estimates from three landscaping contractors; the estimates shall include as separate items all materials, labor, and other costs of the required action, including a two-year maintenance and warranty period.
- 16. Successors-in-interest of the applicant shall comply with site development requirements prior to the issuance of building permits.
- 17. Comply with all other conditions or regulations imposed by the Sandy Fire District, or state and federal agencies. Compliance is made a part of this approval and any violations of these conditions and/or regulations may result in the review of this approval and/or revocation of approval.



LAND USE APPLICATION FORM

(Please print or type the information below)

Planning Department 39250 Pioneer Blvd. Sandy OR 97055 503-668-4886

CITY OF SANDY, OREGON	CITY OF SANDY, OREGON			
Name of Project Sand	y Health Clinic			
Location or Address	39831 Hwy 26, Sandy, OR	97055		
Map & Tax Lot Numb	oer T_2S, R_4E, S	ection_13; Tax Lot(s)_24	E13AD01001	
Plan Designation C-1	Plan Designation C-1 Zoning Designation Commercial Acres 0.47			
Request:				
clinic. Project will construction of a r	consist of demolitionew health clinic bu	orith (2) Type II Adjustment on of existing one-story ilding, and associated significant of the property listed above spects true, complete and contents in the property listed above spects in the property listed ab	building, ite work.	
Applicant Steve Kelly		Owner	-	
Address 2051 Kaen R	and	Address		
City/State/Zip		City/State/Zip		
Phone	on City, OR 97045	Phone		
Email	503-655-8591 Email Email stevekel@clackamas.us			
Signature Signature Signature				
If signe	ed by Agent, owner's w	ritten authorization must be	attached.	
File No.	Date	Rec. No.	Fee \$	
Type of Review (circle	e one): Type I Ty	ре II Туре III Тур	pe IV	

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SUPPLEMENTAL APPLICATION DESIGN REVIEW / CONDITIONAL USE PERMIT

(Please print or type the information below)

Planning Department 39250 Pioneer Blvd. Sandy OR 97055 503-668-4886

Name of Project	
Products Manufactured	
Hours of Operation	
No. of Employees per Shift	
Total Site Size	Total Bldg. Square Footage
Project Summary	

G:\Forms All Departments\Planning\Form Updates 2014\Applications\Supplemental Design Review & CUP Form.Doc

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${\bf USES~WITHIN~BUILDING~(SQUARE~FOOTAGE)}\\$

Offices:	Shop:	Storage:		
Kitchen:	Laundry:	Rest Rooms:		
Other:				
OCCUPANCY & CONSTRUCTION TYPE (List all occupancies by square footage)				
UBC Occupancy Rating:				
UBC Type of Construction:				
Will fire sprinklers be installed in the building? [] Yes [] No				

SITE ANALYSIS DATA				
Туре	Lot Coverage (Square Feet)	Lot Coverage (Percent of Site)		
Buildings				
Parking Lots and Driveways				
Private Walks & Pedestrian Ways				
Landscaping – Improved Area				
Landscaping – Natural areas				
Storm Water Detention, Retention & Bioswale Areas				
Other (describe)				
Other (describe)				

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CONSTRUCTION MATERIAL DETAILS
Color & Type of Siding Materials:
Color & Type of Trim Materials:
Color & Type of Roof Materials:
Color & Type of Exterior Doors:
Color & Type of Exterior Stairs, Balconies & Railings:
Trash & Recycling Enclosure (describe type, colors, height):
Type of Lighting Fixtures (describe):
Pole:
Wall-Mounted:
Fencing (height and type):
Mailboxes (location and type):
Private Pedestrian Walkways (type of surfacing):
Recreational Amenities (describe type and location):
Other Site Elements (describe):

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	TYPE OF IRRIGATION SYSTEM	
Describe type and brand of irrigation system to be installed. Formal irrigation plans must be submitted with construction plans.		
	PROPOSED SOIL AMENDMENTS	
escribe soil o	conditions and proposed plans for soil treatment & amendments:	
	ADDITIONAL LANDSCAPING INFORMATION	

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LANDSCAPE MATERIALS				
Quantity	Type – Include botanical and common names. Plants must be keyed to landscape plan.	Size	Height	Spacing

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Sandy Health Clinic 39831 Highway 26 Sandy, OR 97055

Type II Design Review Project Narrative

February 12, 2020

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Application forms

Attached separately Land Use Application Form, Supplemental Application Form, and fee

Design Review Drawings Attached separately

Contacts

Developer Clackamas County Steve Kelly

2051 Kaen Road <u>stevekel@clackamas.us</u>

Oregon City, OR 97045 503.655.8591

Architect Ankrom Moisan Architects Lori Kellow, Project Manager

38 NW Davis Street <u>lorik@ankrommoisan.com</u>

Suite 300 503.245-7100 Portland, OR 97209

503.245.7100 Scott Soukup, Project Architect

scotts@ankrommoisan.com

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City Design Review Development Services Planning

Division City of Sandy 39250 Pioneer Blvd

Sandy, OR 97055

Kelly O'Neill Jr., Planning & Building Dir.

koneill@cityofsandy.com 503.489.2163

Emily Meharg, Associate Planner

emeharg@cityofsandy.com

503.783.2585

Land Use / Design Reviews

Application Requested Type II Design Review

(2) Type II Adjustments, please see Sections B.3 and E.2 in the Building Design Standards, below. Sections are highlighted in red for clarity.

Pre-Application Conference Conference held December 18th, 2019

Design Review Application Submittal: Feb 12, 2020

Site Description

Site Address: Sandy Health Clinic

39831 HWY 26 Sandy, OR 97055

Jurisdiction: Sandy, OR / Clackamas County

Map & Tax Lot Numbers: T2S, R4E, Section 13; Tax Lot 24E13AD01001

Parcel Number: 01830433

Total Site Area: 0.47 acres (20,473sf)

Zoning: C-1 (Central Business District)

Current title Owner: Clackamas County Health Centers

2051 Kaen Road, 3rd Floor Oregon City, OR 97045

Street classification

Hwy 26 / Mt. Hood Highway Major Arterial roadway

SE Ten Eyck Road Minor Arterial roadway

Pleasant Street Local Street

Project Description

Overview

The project site is located between Hwy 26 / Mt. Hood Highway and Pleasant Street, at the corner of SE Ten Eyck Road. The site is generally flat, with an approximate 2 ½-foot slope rising from the north to south at the sidewalk along Hwy 26, and an approximate 2 ¾-foot slope rising from the north to the south at the sidewalk along Pleasant Street. The parcel currently has a vacant building that was previously a distribution center, and an auto dealership. Zoning is C-1. The building height limit is 45 feet.

The proposed construction is for a one story Type VB, sprinklered health clinic, with primary care, behavioral health, and dental services. The reception / waiting area will be accessed from the parking lot, between the Sandy police station and the new building, at the northeast corner of the building. There will be other access points facing Hwy 26 and Pleasant Street. The Sandy clinic will operate Monday through Friday with hours of 8am-6pm. For the proposed development, 6 of the required spaces of the off-street parking will be shared on site with the Sandy Police Station. A parking agreement with Immanuel Lutheran Church, northeast of the property, will accommodate all staff parking and overflow visitor parking.

Total gross floor area: 9,381 gsf

Parking stalls: 22 stalls including 1 accessible van space and 6 compact spaces.

Bike storage: 5% or 2 whichever is greater

2 total (2 ½ feet x 6 feet x 7 feet vertical) for commercial use

Project Code Summary

Occupancy: B - Business

Construction type summary: One-story

Type VB

NFPA 13 Sprinkler System throughout.

Max code height: 60 ft maximum

Building is approximately 28 ft high

Zoning Requirements Narrative

Zoning: C-1 (Central Business District)

Land Use Review process: Preapplication Conference (December 18th, 2019)

Type II Design Review procedure

17.42.00: This district is intended to provide the community with a mix of retail,

personal services, offices, and residential needs of the community and its trade area in the city's traditional commercial core. This district is not intended for intensive automobile or industrial uses. This district is intended to provide the principal focus for civil and social functions within

the community.

This commercial district is intended for civic uses and to provide all basic services and amenities required to keep the downtown the vital center of our community. While the district does not permit new low density building types, it is not intended to preclude dwelling units in buildings containing commercial activities. All development and uses shall be consistent with the intent of the district, as well as compatible with the space, access and exposure constraints and opportunities of the central

city.

Zoning Requirements Narrative

Please see attached drawings for additional clarification regarding compliance.

Topic Requirement		Chapter	Compliance	
Permitted Uses	Commercial in buildings with up to 30,000 sf of gross floor area and without drive-through facilities	17.42.10.B.2. g	Medical facilities permitted outright.	
Development Standards	Lot Area: No minimum Lot Dimensions: No minimum Setbacks: No minimum; maximum 10 ft Lot Coverage: No maximum Landscaping: 10% minimum Structure Height: 45 ft maximum	17.42.30.A Lot Area: Comply Lot Dimensions: Comply Setbacks: Comply Lot Coverage: Comply Landscaping: 14% provid Structure Height: 28 ft		
Special Setbacks	Property abutting a more restrictive zoning district shall have the same yard setback as required by the abutting district. Pleasant Street: Across street from more restrictive R-2., requiring 10 ft minimum setback and additional 10 ft added for each 10 ft increment in building height over 35 ft.	17.42.30.B	10 ft minimum setback provided from north property line.	
Landscaping Preservation	Significant plant and tree specimens should be preserved to the greatest extent practicable.	17.92.10.C	Significant trees are currently at the sidewalks along Hwy 26 and Pleasant Street and will be preserved in the design. Trees will be preserved during construction by construction fences.	
Planter Sizing	Planter and boundary areas used for required plantings shall have a minimum diameter of 5 feet.	17.92.10.D	Planted areas are 5 feet wide, minimum.	
Planting Vision Clearance	Plantings or other screening is not permitted within vision clearance areas.	17.92.10.E	Tall plantings and screens are kept back from the street intersections.	
Pedestrian Amenities	Up to 35% of the total required landscaped area may be developed into pedestrian amenities.	17.92.10.F	Area developed for pedestrian amenities are the civic space near the main entry and the internal courtyard / break area. Spaces are less than 35% total landscaped area.	
Landscaping Extent	All areas not occupied by paved roadways, walkways, patios, or buildings shall be landscaped.	17.92.10.K	Complies.	

Topic	Requirement	Chapter	Compliance
•	-	_	_
Minimum Landscaping and Screening Improvements	10% required for C-1 zone (2,047 SF)	17.92.20	15% (2,999 SF) provided.
Required Tree Plantings	1 medium canopy street tree at 30 feet on center. 1 medium parking lot tree per 8 cars (8 / 21 total cars = 2.6 trees)	17.92.30	Compliant street trees and parking lot trees provided. See Landscape Plan L1.01.
Irrigation	Landscaping to be irrigated to sustain viable plant life	17.92.40	Electric solenoid controlled underground irrigation system, with low point self drain will be used.
Types and Sizes of Plant Materials	A. 75% landscape area to be combination of trees, shrubs, or evergreen ground cover. B. Use native plant materials acclimatized to the Pacific Northwest. C. Trees with a mature spread of crown greater than 15 feet should have trunks with maintained clear condition of over 5 feet of clear wood. D. Deciduous trees to be balled and burlapped, minimum of 7 feet in overall height or 1 ½" in caliper 6" above the ground.	17.92.50	A. Complies B. Complies C. Complies D. Complies
	E. Coniferous trees to be minimum of 5 feet above the ground at time of planting. F. Shrubs to be a minimum of 1 gal in size or 2 feet in height. G. Hedges to form a continuous, solid visual screen within 2 years after planting. H. Vines for screening to be minimum of 1 gal. or 30" in height. I. Groundcover to be fully rooted and well branched or leafed and provide complete coverage in one year. J. Turf areas to be species normally grown as permanent lawns in western Oregon. K. Landscaped areas may include architectural features or artificial ground cover not to exceed 25% of the required landscaped area.		E. Complies F. Complies G. No hedges in design. H. Vines not used for site screening. I. Complies J. No turf in design. K. Design includes benches, boulders, aluminum planters, wood fences, dark brown mulch, stone pavers, and a monument sign that do not exceed 25% of the landscaped area.
Landscaping Between Public Right-of-Way and Property Lines	Except for portions allowed for parking, loading, or traffic maneuvering, a required setback area abutting a public street and open area between property and roadway in the public street to be landscaped.	17.92.70	Area between property line and public streets are landscaped except where there is a sidewalk or driveway.

Topic	Requirement	Chapter	Compliance
Screening	A. Height and Opacity B. Chain Link Fencing C. Height Measurement D. Berms	17.92.90	A. Wood fencing is 8'-0" tall and at least 80% opaque. B. Not in project. C. Screen height is measured from the finished grade of screened improvements.
C	For the section of th	17.02.100	D. Not in project.
Screening of Service Facilities	Equipment and trash and recycling areas to be screened.	17.92.100	The site transformer and condensing unit are screened, and the trash enclosure is CMU to match the building base material.
Off Street Parking	For medical office / clinic use:	17.98.20	
Requirements	Number of car parking spaces: 1 per 300 sf, plus 1 per 2 employees Number of bike parking spaces: 5% or 2 whichever is greater		Car parking spaces: 8,345 GSF / 300 = 27.81 stalls 25% reduction for C-1 zone = 20.86 (21) required stalls. 6 of these required stalls are shared with the police station. 12 staff, 6 staff parking stalls at Immanuel Lutheran Church parking lot via agreement with church. Bike parking spaces: 27 stalls x 5% = 1.4 2 required bike spaces area provided near the main entry.
Accessible (ADA) Parking Stalls	1 accessible parking space required for parking facility with 1 to 25 stalls.	2010 ADA Standards Table 208.2	1 van accessible parking stall provided

Topic	Requirement	Chapter	Compliance	
•		-	-	
Parking Lot Design and Size	No more than 35% of parking stalls shall be compact spaces.	17.98.60	6 compact spaces / 22 total spaces = 27%, therefore ok	
	90 degree parking aisle width: 20 feet single sided one-way, 25 feet double sided two-way		All parking it 90 degree. 25 foot aisle provided off of Pleasant Street. 17'-3" aisle provided at south end of site, near compact parking stalls. This is larger than the approximately 14 foot aisle that currently exists at the south end of the 6 shared stalls. If a stall needs to be removed to provide a 20 foot space, there will be one parking stall fewer than required. Because these stalls are compact, 17'-3" will be adequate for cars to back up into the aisle.	
On-Site Circulation	Groups of more than 3 parking spaces shall be provided with adequate aisles or turnaround areas so that all vehicles enter the right-of-way in a forward manner.	17.98.70	Complies	
Driveway	Driveway to an off-street parking area shall be a minimum of 20 feet for a two-way drive for the first 20 feet of the driveway. Shall not traverse a slope in excess of 15% at any point along the driveway length.	17.98.100	Complies	
Landscaping and Screening	Between, and at the end of each parking bay, there shall be planters that have a minimum width of 5 feet and a minimum length of 17 feet for a single depth bay and 34 feet for a double bay. Each planter shall contain one major structural tree and ground cover.	17.98.120.D	Complies. Please see Site Plan and Landscape Plan	
Paving	Parking areas, driveways, aisles, and turnarounds shall be paved with concrete, asphalt, or comparable surfacing, constructed to city standards for off-street vehicle areas.	17.98.130.A	Permeable asphalt will be used for the parking lot surface.	

Topic	Requirement	Chapter	Compliance
Drainage	Provide adequate provisions for collection of drainage waters to eliminate sheet flow of water onto sidewalks, public rights-of-way, and abutting private	17.98.140	Permeable asphalt will be used for the parking lot surface.
Lighting	property. Artificial lighting to be provided at parking.	17.98.150	See sheet E1.02 – Electrical Site Lighting Plan.
Bicycle Parking Facilities	A. Location B. Bicycle Parking Space Dimensions	17.98.160	A. Bike parking is located next to the primary building entrance, visible from the reception/waiting room, separated from the vehicular parking area, with direct access to the public right-ofway. B. Each of the (2) bike
	C. Security D. Signing		parking spaces are 2 ½ feet by 6 feet, covered with a vertical clearance of 12 feet. C. Bikes can be locked to a provided bike rack with both cable and U-shaped locks, permitting the frame and one wheel to be secured. Bike rack is anchored to the ground and sheltered under a roof overhang. D. Bike parking is visible from the entry and public right-of-way, so no signage
A. minimum area of loading zone 250 sf. B. Loading berth to be 10 feet by minimum with a height clearance feet. C. Loading areas to be screened public view with the same screen parking lots. F. Off-street loading facilities not when buildings abut a public aller way that loading can be conducted the alley in accordance with applit traffic and parking ordinances.		17.98.190	is needed. F. Loading will be conducted from clinic side of parking lot. Loading will be conducted off clinic hours. Deliveries anticipated at this clinic will be smaller in quantity due to requirements of the building program.

Building Design Standards Narrative

	IDY DESIGN STANDARDS .90.110)	PROPOSAL RESPONSE
A	Site Layout and Vehicle Access	
A.1.	Lot shall abut or have cross access to a dedicated public street	The lot has access to Hwy 26/Mt Hood Hwy and Pleasant Street
A.3.	Off street parking located to the rear or side of buildings, with no portion of the lot within required setbacks or within 10-feet of the public right-of-way. Driveways for ingress or egress shall be limited to one per 150 ft.	Parking lot is an extension of the police station parking lot and sited between the two buildings. All portions of the lot are within required setbacks.
A.4.	Adjacent parking lots shall be connected to one another when the City determines it is practicable to do so.	Parking lot is an extension of the police station parking lot and sited between the two buildings.
A.5.	Urban design details shall be used to calm traffic and protect pedestrians in parking areas.	Sidewalk crossing the driveway will be a different paving material than the parking lot.
A.8.	Raised walkways or painted crossings from public street sidewalk to building entrance(s).	There is a different paving material used from the sidewalk to the main building entrance.
A.12.	Free-standing buildings shall be connected to one another with a seamless pedestrian network that provides access to building entrances and civic spaces.	The sidewalks along Pleasant Street and Hwy 26 connect to the health clinic and police station entrances.
B and	Building Facades, Materials, I Colors	
B.1.	Articulation	 1) All elevations facing public streets are broken into 30 feet or shorter. Each plane projects or recedes 24 inches from the adjacent plane. 2) Wall planes incorporate visually contrasting and complimentary trim.
B.2.	Pedestrian Shelters	 a) Shelter over building's main entry is provided by roof projecting over entry. Metal canopies are provided over the other entries. b) Canopies, roof overhangs, and alcoves are provided. c) Overhang over entry projects 5 feet to protect pedestrians. d) Shelter over main entry is extension of roof with same roof slope.
В.3.	Building Materials	a) Building is architecturally unified by palette of earth tone material colors, continuous gabled roof form, and repeated dormer/pop-out language.

12

B.4. Colors	 b) Building has a 36-inch base of Ebony colored, split-faced concrete block veneer with running bond pattern. Type II adjustment, based on notes from pre-application meeting: Percentage of gaps in base material is 18%. Please see Base Material Diagram on Sheet G1.30. c) Foundation is sheathed with the split-faced concrete block veneer. d) Siding above veneer base consists of horizontal wood textured fiberboard sizing (redwood color), vertical ribbed fiberboard panels (1 5/8" ribs and weathered copper color) e) Building elevations facing a public right-of-way or civic space incorporate the following features: Wood door and window trim and fascia Metal canopies Metal brackets – these are cosmetic metal applied to the ends of the extruded framed walls, labeled "break metal trim" on the exterior elevations f) Materials required on elevations visible from abutting public streets turn the corner and are consistent around the full perimeter of the building. a) Color palette is warm earth tones and wood look. Material colors are noted in B.3. section above. Exterior paint for trim and fascia is Miller Historic
	color H119 Sturgis Grayb) Black color is only used for metal canopies
C Roof Pitch, Materials, and	c) No day-glow or reflective colors are used.
Parapets	
C.1. Gabled Roof Pitch C.2. Secondary Roof Forms	 Primary roof forms are 6:12, except at the east roof. East roof is 4 feet wider than the rest of the roofs, so the pitch is 5 5/8:12 to allow the ridges to be at the same level. 6:12 pitch is only required for new buildings with a span of 50 feet or less. Portion of roofing at 5 5/8:12 is 62'-8" wide due to building program needs. Secondary roof forms are 4 1/2:12. Secondary roof forms comprise of 13% of roof area.
	(1,514 secondary roof area, 11,624 total roof area)
C.3. Gable	Gable ends of the roof face Hwy 26, the intersection of Hwy 26 and SE Ten Eyck Road, and the parking lot. The gable facing the parking lot, and part of the main entrance, is visible from Pleasant Street.
C.4. Secondary Roof Form Quantities	• (2) secondary roof forms on the 76'-7" length of roof facing Ten Eyck. (2) secondary roof forms on the 114'-11" length of roof facing Pleasant Street.
C.5. Roof Materials	Roofing is a standing seam metal roof, color is gray.
C.6. Roof and Wall Mounted Equipment	There is no rooftop equipment other than an approximately 12" tubular skylight centered on the roof. Mechanical and electrical equipment is internal in dedicated rooms and in the attic space.

D	Building Orientation and	
D.1.	rances Orientation	More than 50% of the building's street frontage is within 10 feet or a sidewalk or civic space. The edges of the building are as close to the property line as allowed. No off-street parking is placed between the building and adjacent streets.
D.2.	Parking Placement	Not applicable
D.3.	Ground Floor Connection	Ground floor spaces are connected to the sidewalks along Pleasant Street and Hwy 26.
D.4.	Corner Entrance	There is a door into the building within 40 feet of the corner at the intersection of Ten Eyck and Hwy 26, and an door into the building within 40 feet of the corner at the intersection of Ten Eyck and Pleasant Street. A direct corner entrance is not practical due to the nature of the building's use as a health clinic and its relation to the parking lot. The corners have detailing consistent with the Sandy Style.
D.6.	Activated Pedestrian Environment on two Elevations	There is an entrance with a pedestrian shelter facing Hwy 26 and facing Pleasant Street, however, due to the nature of the clinic, these are staff entrances, not customer entrances. The patient entrance faces the parking lot. Patients may leave the other (2) doors, but these are primarily intended for clinic staff.
D.7.	Primary Entry Spacing	Due to the nature of the medical clinic, entrances are spaced further than 30' apart. There is no public retail as a part of this building.
D.8.	Primary Entrance Architecture	The roof overhangs the main entrance by 5' and there is a bench and landscaped elements to emphasize it. There is a metal canopy over the other entrances, with landscaping near these entries.
Е	Windows	
E.1.	Unified Design	There are only 4 different sizes of windows in this design, and their headers, sills, and mullions are in consistent planes in relation to each other.
E.2.	Ground Floor Windows	Type II adjustment: Percentage of window area on Hwy 26 elevation is 27%. Percentage of window area on SE Ten Eyck Road elevation is 28%. Percentage of window area facing Pleasant street is 23%. Due to the use of the building as a clinic, it is not practical to have 40% windows on (2) of these frontages. The building GSF is just shy of 10,000 SF, which only requires 25% window area, which we are meeting on (2) frontages. a) Typical windows have a sill 3 feet above grade. b) Windows are square and vertically oriented. c) Windows are divided into panes that are smaller than 6 feet.
		• d) Windows have 4 inch wood trim around them.

		b) Storefront windows are located at the entrance. The rest of the windows are vinyl.
F	Landscaping and Streetscape	
Des	sign	
F.1.	Landscape and Screening General Standards (Chapter 17.92)	Please see Chapter 17.92 sections in the Zoning Requirements Narrative above.
F.2.	Streetscape Design Conflicts	There are no conflicts with the Streetscape design and Landscaping and Screening General Standards.
G	Civic Space	
G.1.	Percent of Ground Floor Area	The civic space near the main entry to the clinic is 3% of the ground floor area. (320 SF / 9,381 SF).
G.2.	Dimensions and Area	The civic space is 19'-8" across and 320 SF.
G.3.	Туре	The civic space is an extension of the sidewalk in front of the clinic entrance, with benches for seating, and doubles as a zone for patients to await pick-up.
G.4.	Location	The civic space is located roughly mid-block, near the entrance, which will be a zone with higher pedestrian activity.
G.5.	Adjacency to Right-of-Way	The civic space is directly adjacent to the sidewalk on Pleasant Street, with rectangular stone pavers to identify it. This space is not gated or otherwise closed to public access.
Н	Lighting	
H.1.	Chapter 15.30, Dark Sky Ordinance	Streetscape lighting conforms to the Downtown Sandy Streetscape Design and Dark Sky Ordinance. See sheet E1.02 – Electrical Site Lighting Plan. The area 10 feet beyond the property line receives no more than 0.25 of a foot-candle of light and the new lighting does not exceed 4,125 Kelvins.
H.2.	Integration	Site lighting is situated to highlight the architectural design of the clinic and to compliment the site lighting at the nearby police station.
H.3.	Safety	Exterior can lights are integrated into the metal canopies over the entries. Walkways, parking lot, and entries are illuminated at 1.5-2.0 foot candles: see sheet E1.02 – Electrical Site Lighting Plan.
I	Safety and Security	
I.1.	Windows	Windows are located around the full perimeter of the building, facing pedestrian, parking, and loading areas.
I.2.	Interior Surveillance	Windows are located so interior activity by be seen from all public right-of-ways, however, due to the nature of the building as a medical clinic, internal shades will be used to provide patients and staff with privacy as needed. The police station is also right

		next to the clinic, which will aid in surveillance of the site.
I.3.	Street Address	 8" tall street address numbers are near south entry, near the intersection of Hwy 26 and Pleasant Street, shown on sheet A3.11, elevation 6.
I.4.	On-Site Lighting Orientation	Site lighting is placed to provide adequate surveillance of on-site activities. See sheet E1.02 – Electrical Site Lighting Plan.
J	External Storage and Screening	
J.1.	Exterior Storage	There will be no exterior storage of merchandise or materials.
J.3.	Equipment Screening	Mechanical condensing unit and transformer are screened from view with wood landscape fencing. Garbage storage is screened within trash enclosure.
J.4.	Trash Screening	Trash and recycling storage is in a split face CMU enclosure in the southeast corner of the site. Enclosure CMU matches the color of the concrete block veneer base of the building. And the metal roof of the trash enclosure matches the metal roofing color.

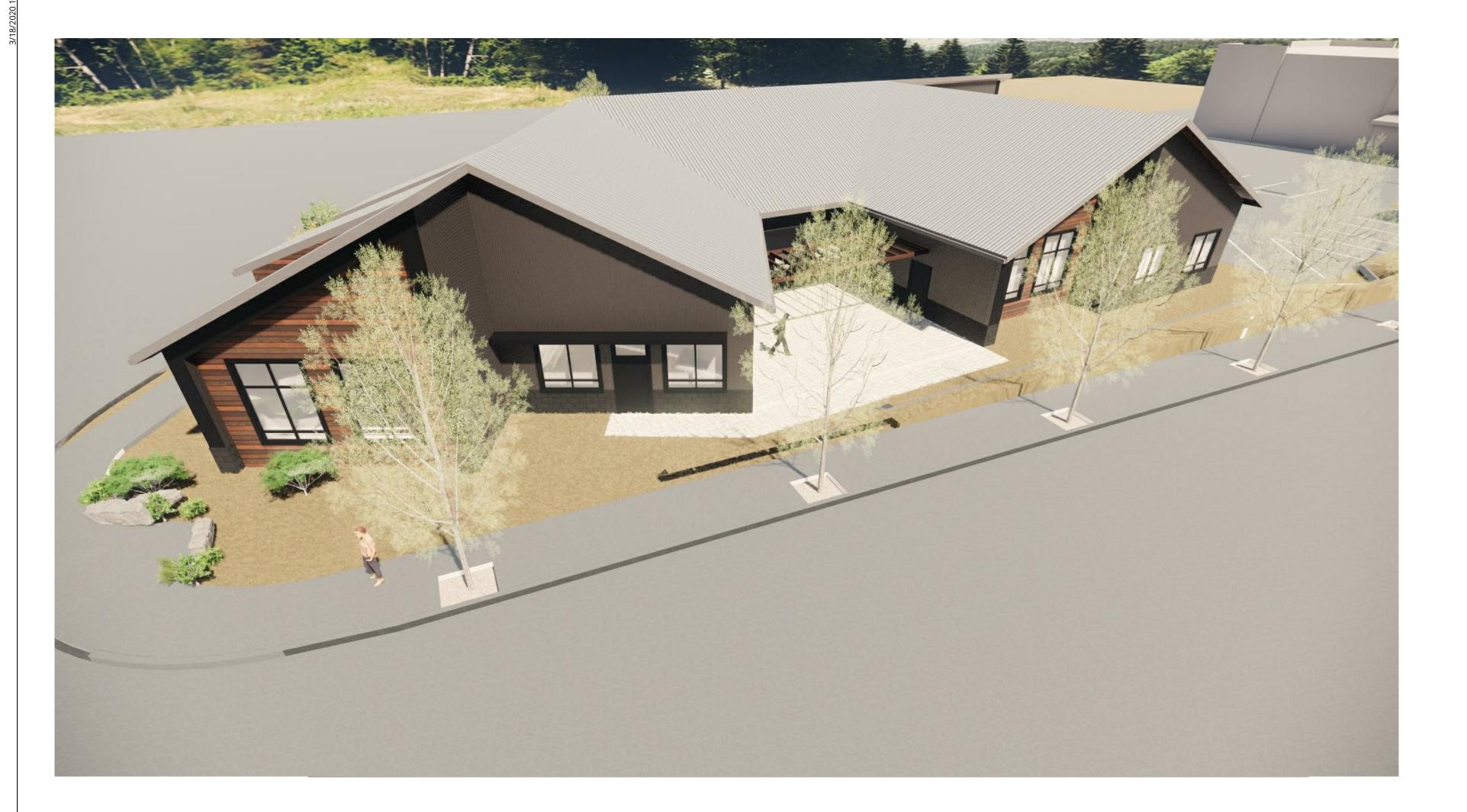


EXHIBIT C

TABLE OF CONTENTS

Cover Sheet

A. Site Analysis:

- -1:60 Vicinity Map -Existing Site Survey
- -500 Foot Radius

B. Site Plan:

- -Site Demolition Plan -Site Hardscape Plan
- -Site Plan

C. Grading Plan

D. Utility Plan

E. Architectural Drawings: -Building Elevations -Building Sections

- -Floor Plan
- -Roof Plan
- -Trash Enclosure / Site Details
- -Window Area and Base Calculations

- -(3) Renderings

 F. Exterior Lighting Plan:
 -Electrical Symbol Legend
- -Lighting Fixture Schedule -Electrical Site Plan
- -Photometric Site Plan
- G. Landscape Plan (Landscape Plan + (3) Supplemental Information Sheets)
 H. Other Submissions:

- -Light Fixture Cut Sheets: S1 through S7
 -Exterior Building Materials
 -Traffic Impact Analysis Letter
 -Stormwater Utility Narrative

VICINITY MAP



NOT FOR

CONSTRUCTION

38 NORTHWEST DAVIS, SUITE 300 PORTLAND, OR 97209 503.245.7100

1505 5TH AVE, SUITE 300 SEATTLE, WA 98101 206.576.1600

1014 HOWARD STREET SAN FRANCISCO, CA 94103 415.252.7063

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CENTER

REASON FOR ISSUE

COVER SHEET

DESIGN REVIEW

PROJECT NUMBER 192530 DATE 4.3.2020

CS

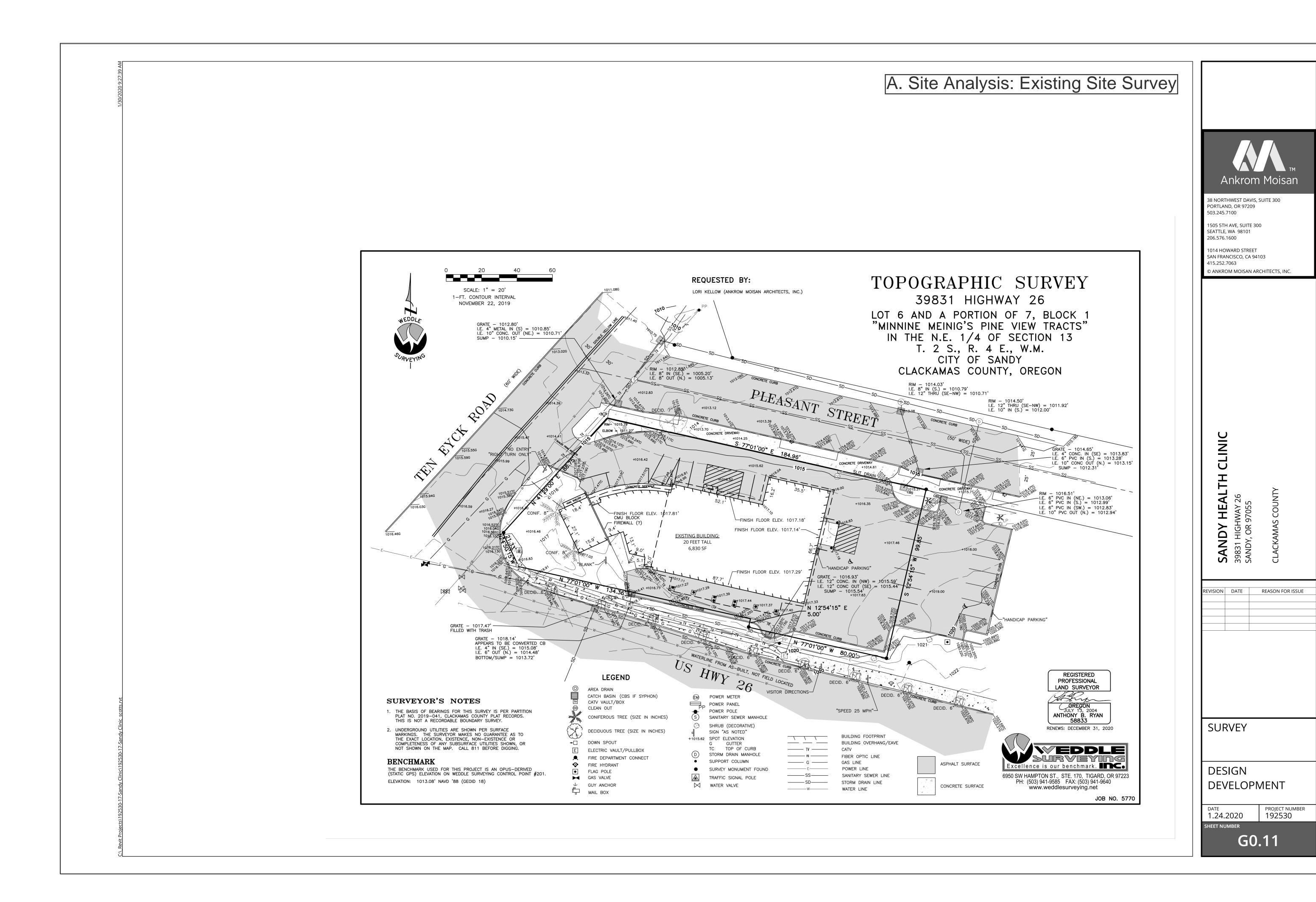
AMA TEAM

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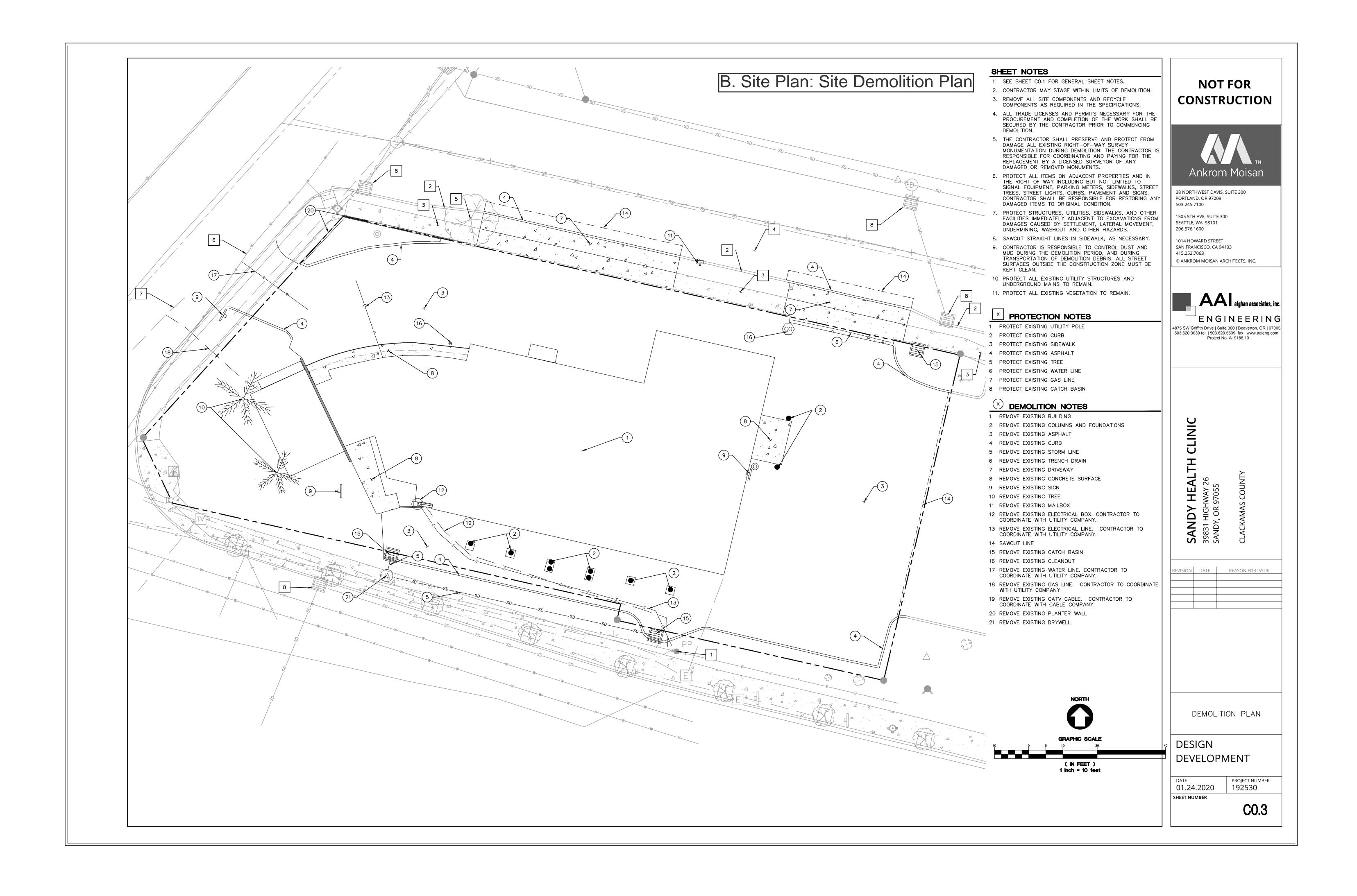
OWNER TEAM

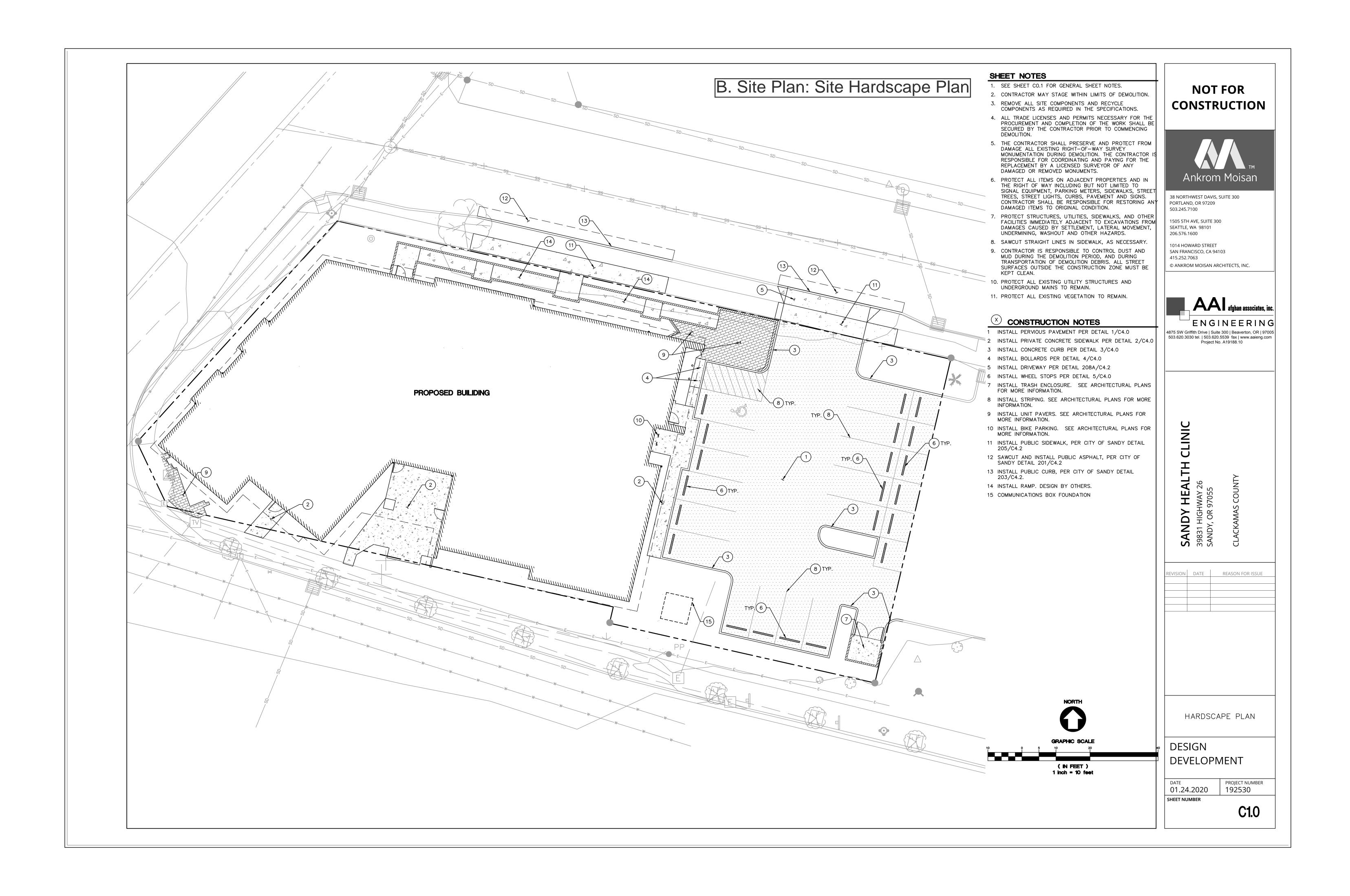
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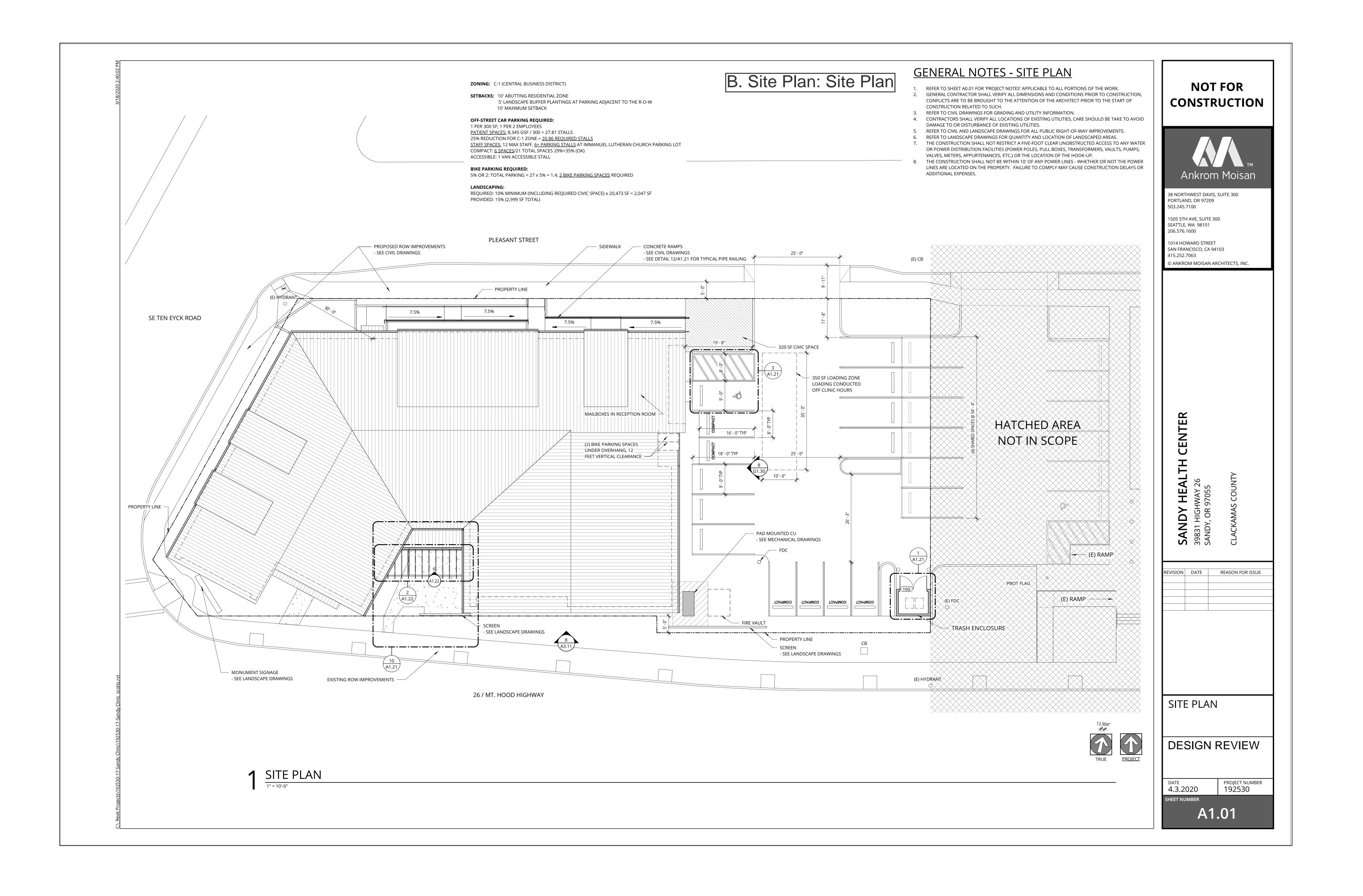
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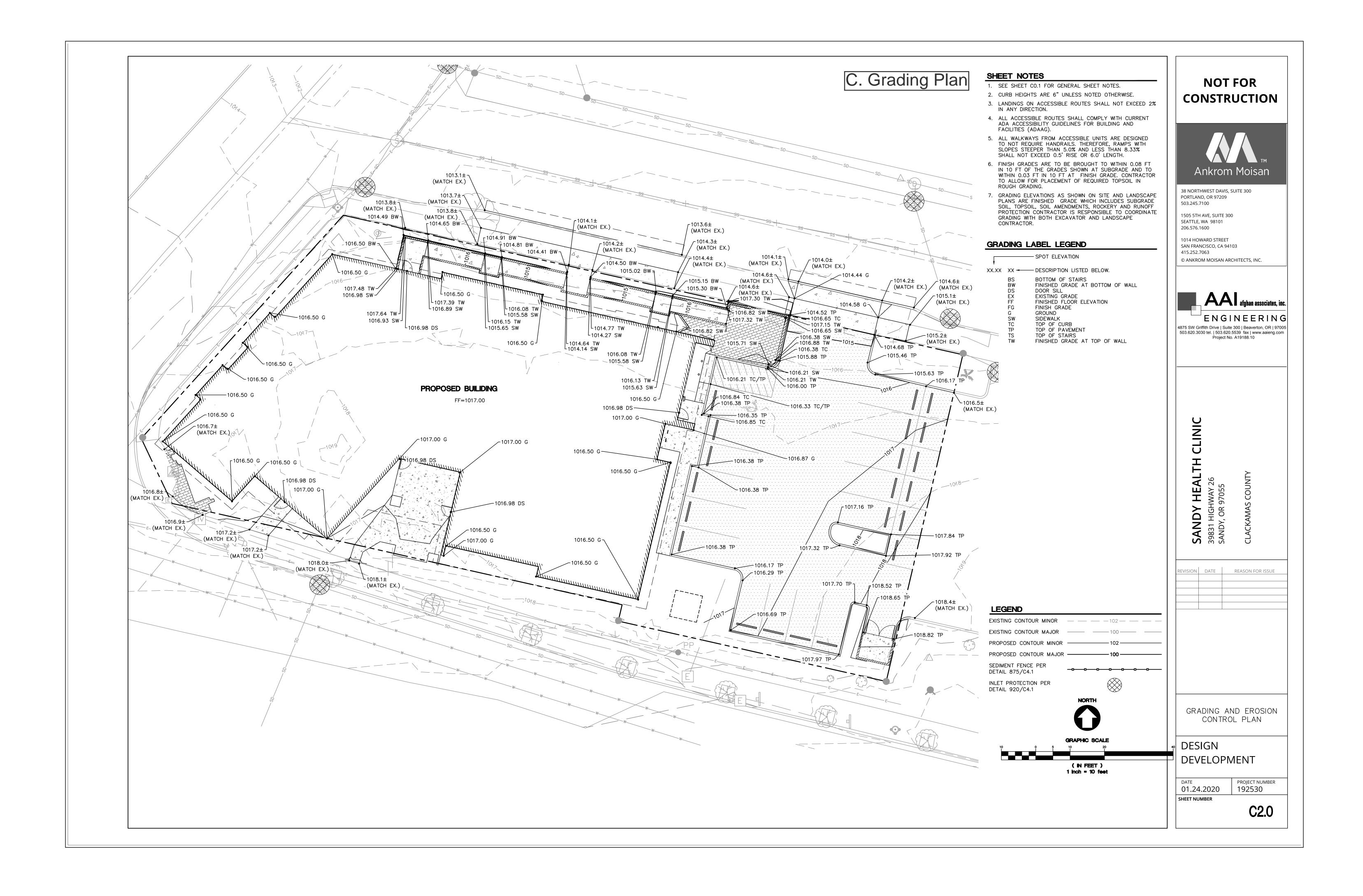


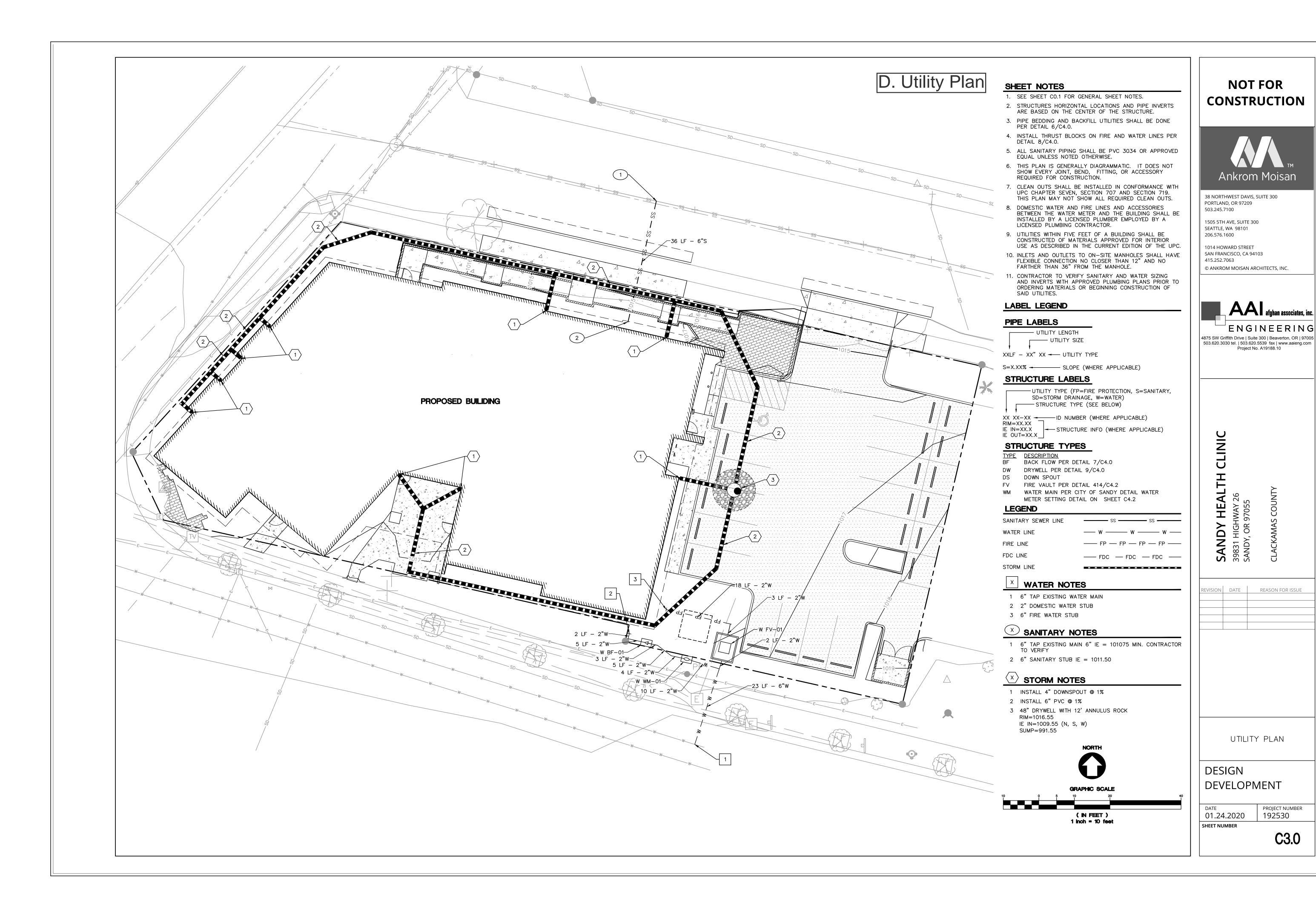
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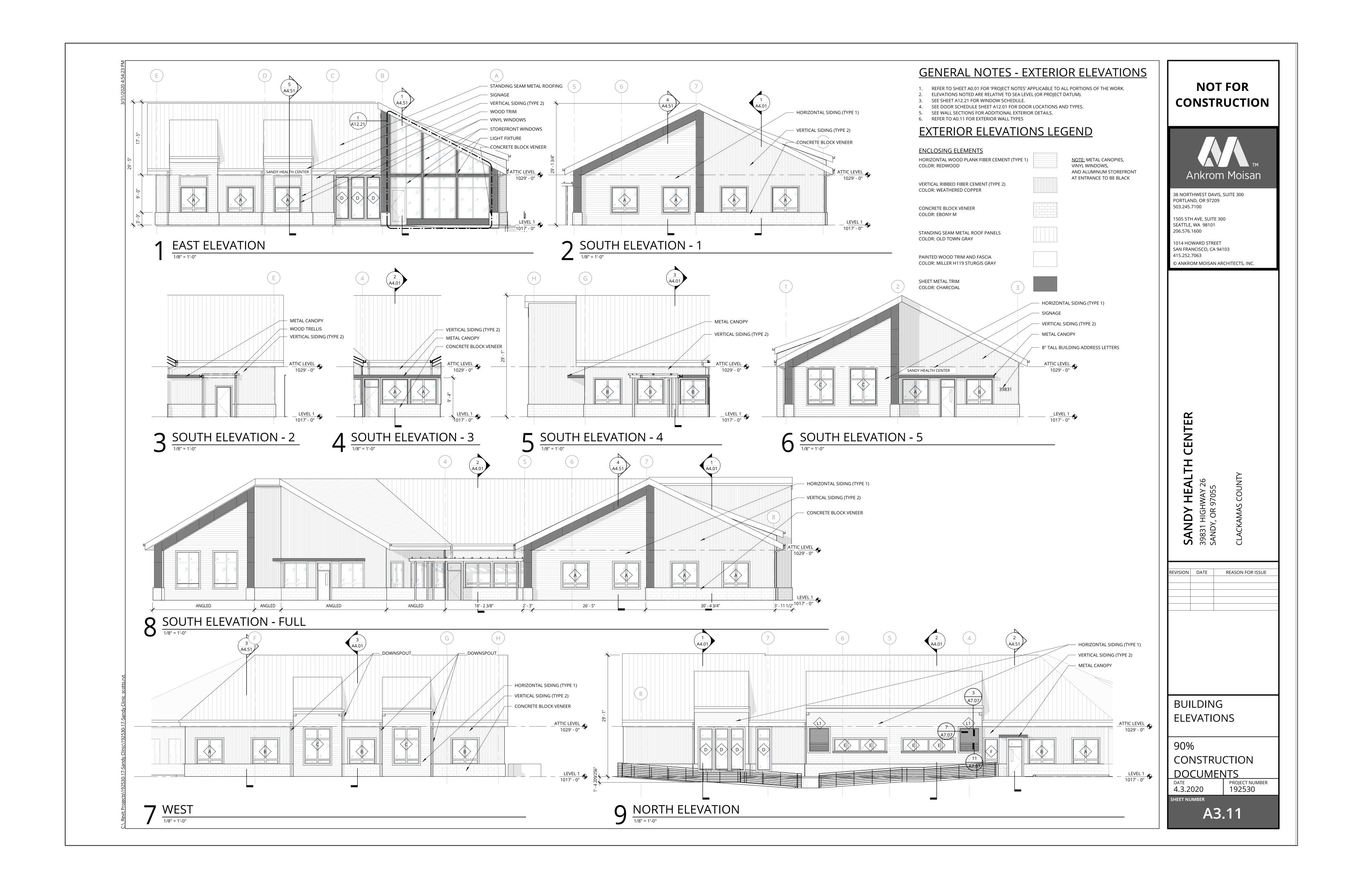


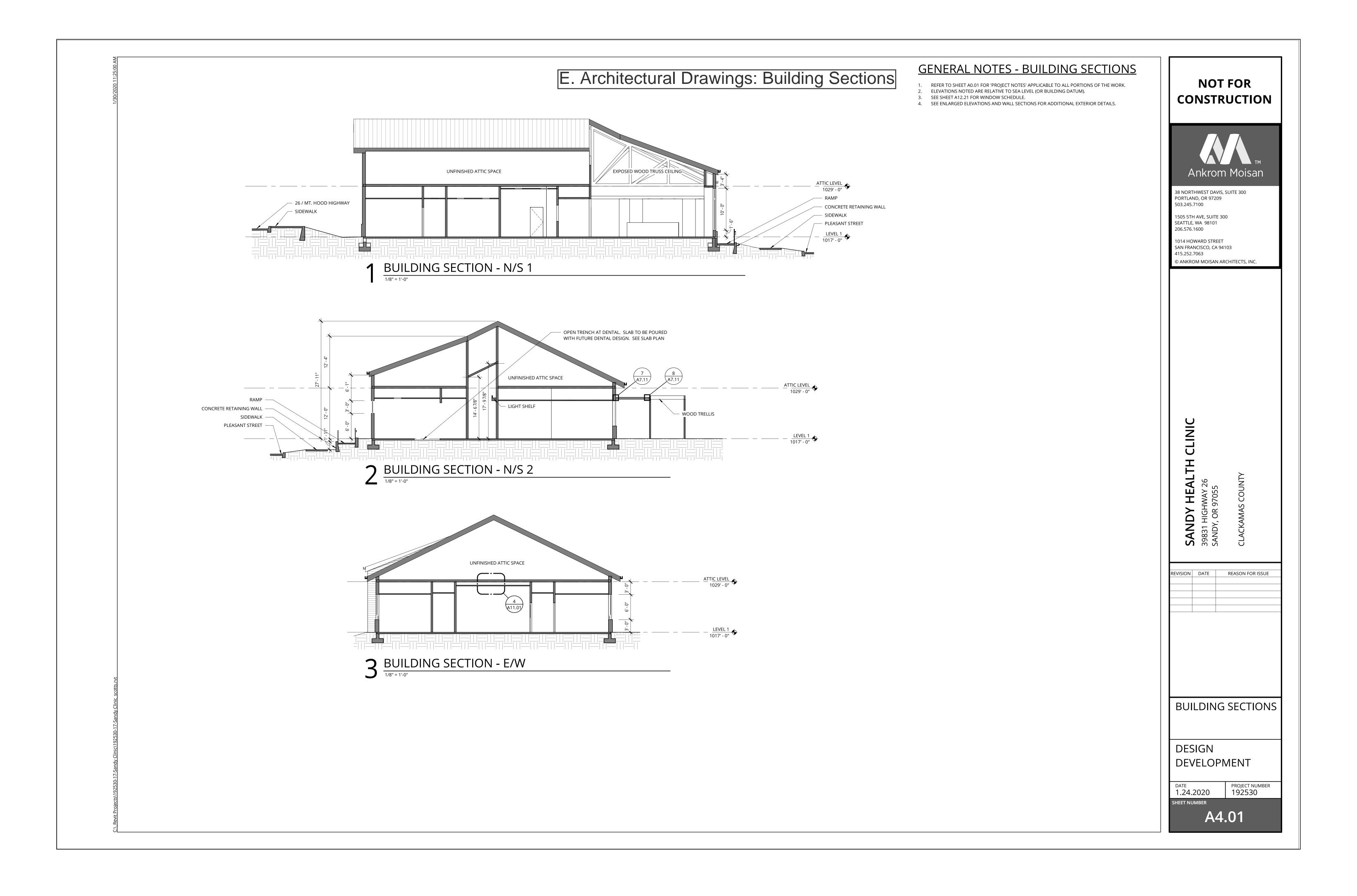


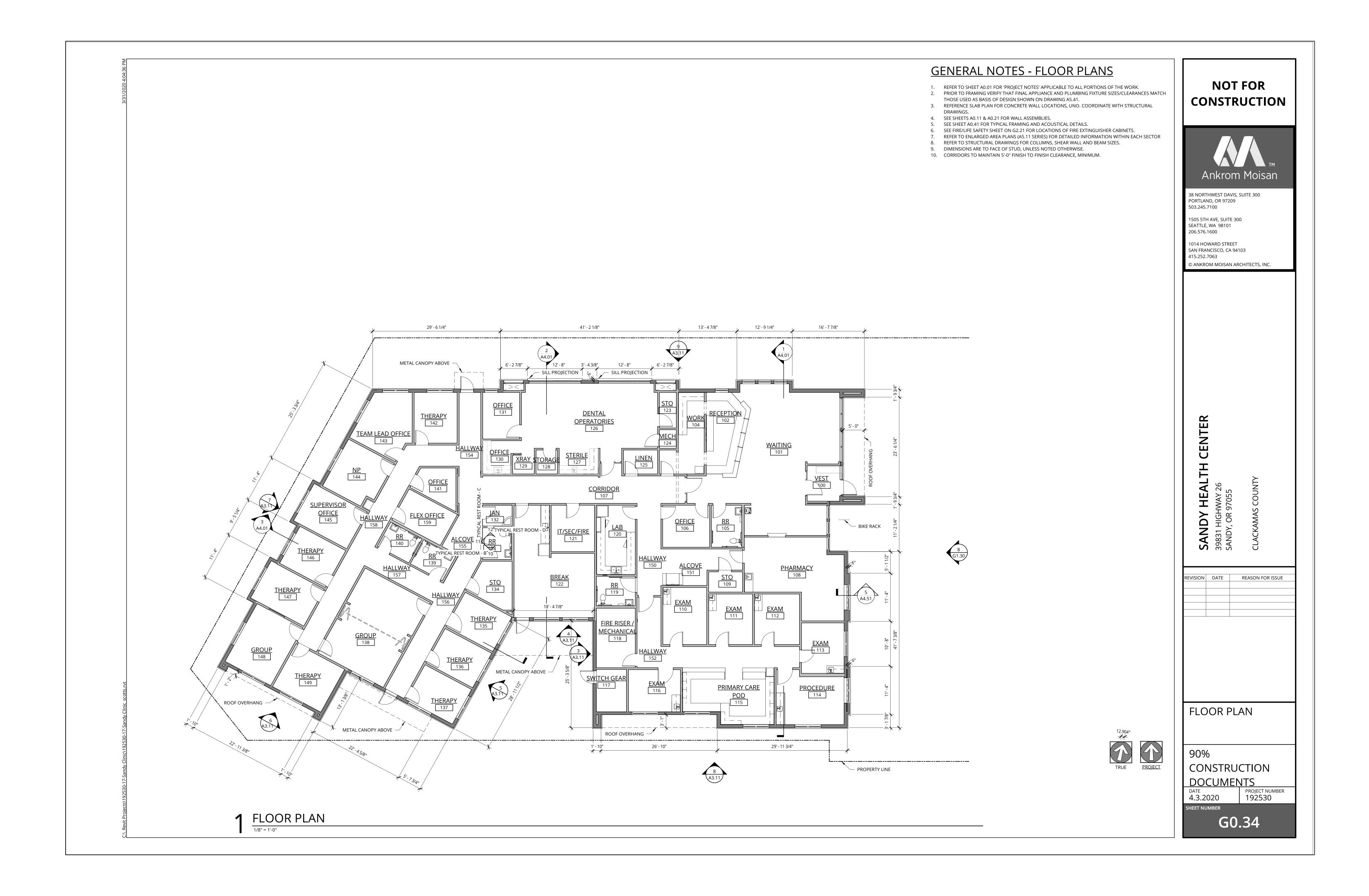


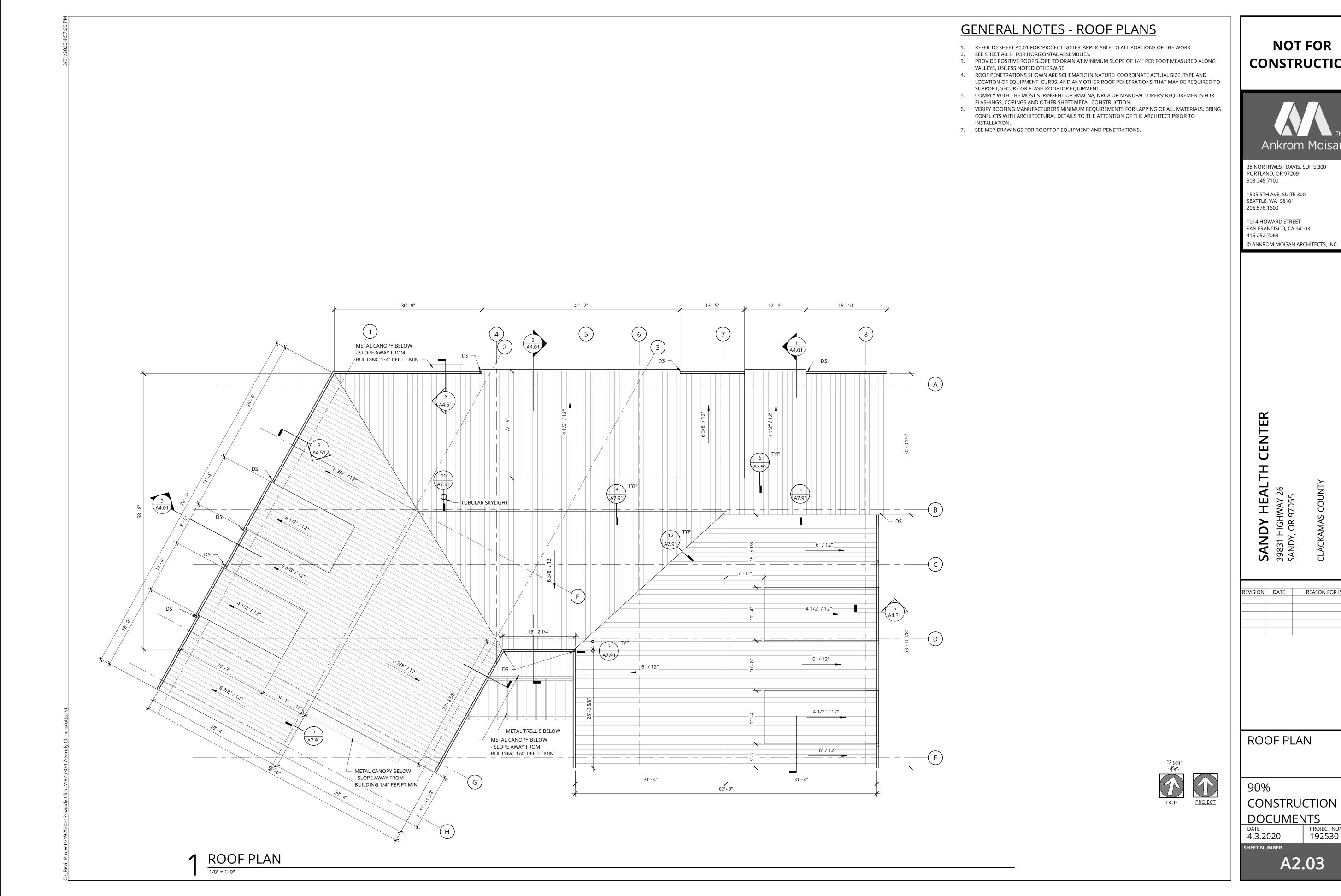














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SANDY HEALTH 39831 HIGHWAY 26 SANDY, OR 97055

REASON FOR ISSUE

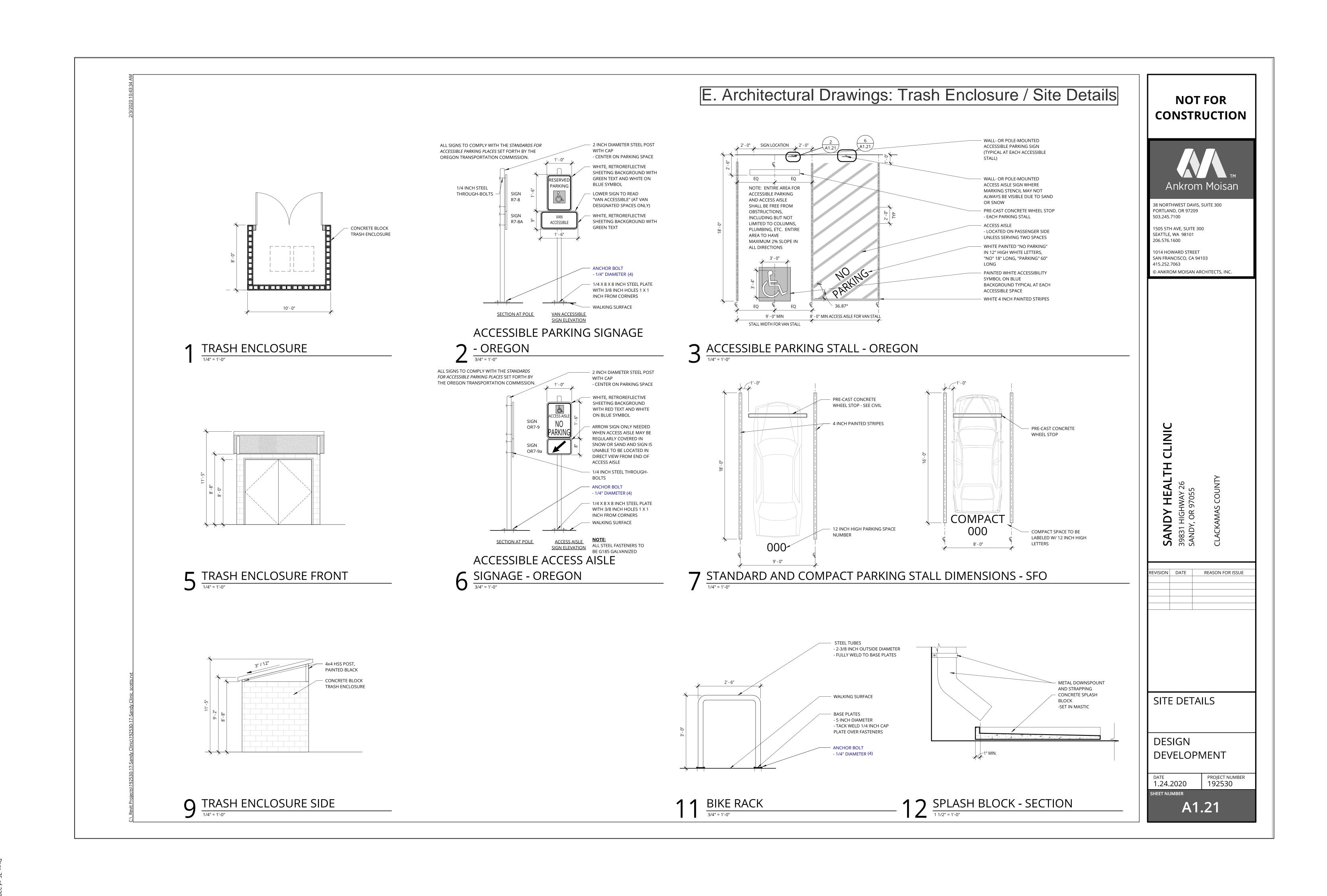
ROOF PLAN

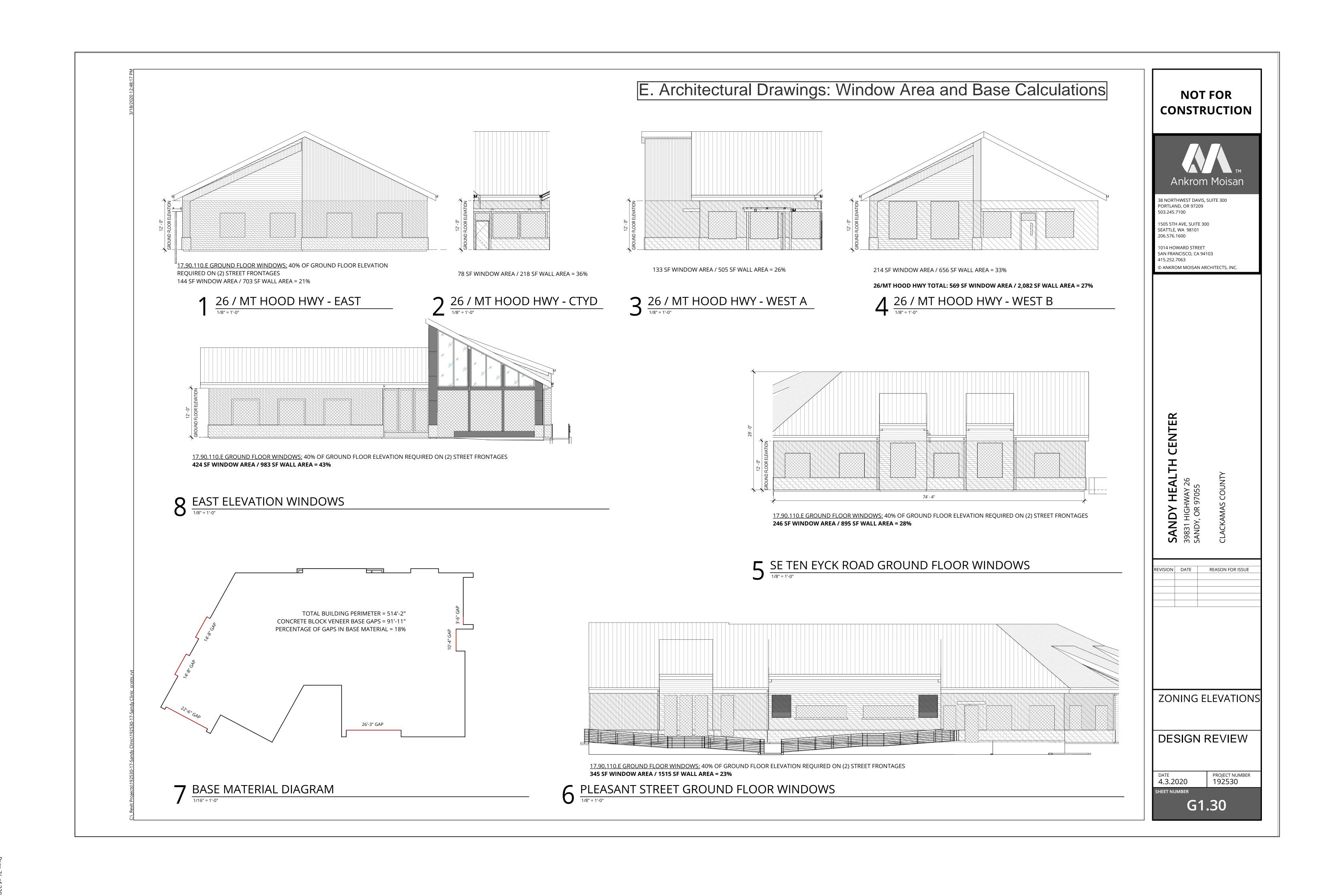
CONSTRUCTION DOCUMENTS

DATE PROJECT NUMBER

4.3.2020 192530

A2.03





- THE ELECTRICAL INSTALLATION SHALL BE IN ACCORDANCE WITH ALL LOCAL, STATE, AND NATIONAL CODES. THE CURRENT ADOPTED EDITION OF THE ELECTRICAL CODE SHALL BE THE STANDARD FOR THE ELECTRICAL INSTALLATION. VERIFY WITH LOCAL OFFICIALS WHEN PERMITS ARE
- OBTAINED. NOTIFY DESIGN TEAM OF ANY DESCREPANCIES BETWEEN THE PROJECT MANUAL OR DRAWINGS AND THE GOVERNING CODE. INSTALLATION SHALL FOLLOW ALL REQUIREMENTS OF THE ADAAG – AMERICANS WITH
- DISABILITIES ACT. REFER TO PROJECT MANUAL AND PROJECT CODE REVIEW SHEET FOR LIST OF ALL APPLICABLE CODES.

GENERAL NOTES - ELECTRICAL

- COORDINATE LOCATION/INSTALLATION OF MECHANICAL AND ELECTRICAL WORK WITH ALL OTHER TRADES. NO ASPECT OF A SYSTEM INSTALLATION OR ITS ROUGH-IN SHALL COMMENCE UNTIL PROPER AND TIMELY COORDINATION WITH ALL TRADES ASSOCIATED WITH THE INSTALLATION HAS OCCURRED. ITEMS TO BE COORDINATED SHALL INCLUDE BUT NOT BE LIMITED TO: BUILDING STRUCTURE, SHEET METAL, ALL PIPING SYSTEMS, LIGHT FIXTURES, CONDUITS, CABLE TRAYS, ETC. REFER TO ALL GENERAL, MECHANICAL, AND ELECTRICAL DRAWINGS AND SPECIFICATIONS FOR THIS PROJECT. ANY REWORK OF INSTALLED EQUIPMENT OR SYSTEMS WILL BE AT THE CONTRACTORS EXPENSE.
- NOTE THAT THE ELECTRICAL DRAWINGS ARE ONLY A PORTION OF THE COMPLETE SET OF PLANS CONTRACT DOCUMENTS. THE COMPLETE SET CONTRACT OF DOCUMENTS SHALL BE USED TO DEFINE THE ELECTRICAL SCOPE OF WORK. THIS SHALL INCLUDE, BUT NOT BE LIMITED TO, USING THE ARCHITECTURAL PLANS FOR DIMENSIONS AND DETAILS; EQUIPMENT PLANS FOR ROUGH-IN REQUIREMENTS, AND THE MECHANICAL PLANS FOR EQUIPMENT SIZES

INSTALLATION NOTES - ELECTRICAL

ELECTRICAL ABBREVIATIONS

ABOVE FINISHED FLOOR

ATS AUTOMATIC TRANSFER SWITCH

CURRENT TRANSFORMER

EXISTING ITEM TO REMAIN

ELECTRICAL CONTRACTOR

FACP FIRE ALARM CONTROL PANEL

MC MECHANICAL CONTRACTOR

MDP MAIN DISTRIBUTION PANEL

N NEW DEVICE IN EXISTING LOCATION

NOTE: NOT ALL SYMBOLS APPLY TO THIS PROJECT

MCB MAIN CIRCUIT BREAKER

FSD FIRE SMOKE DAMPER

KVA KILO-VOLT-AMPERES

MLO MAIN LUGS ONLY

GND GROUND

KW KILOWATTS

EMERGENCY LIGHT FIXTURE

NEW LOCATION OF EXISTING ITEM

ROUGH IN FOR FUTURE DEVICE FAAP FIRE ALARM ANNUNCIATOR PANEL

CIRCUIT BREAKER

CEILING

DEVICE MOUNTED +8" ABOVE

COUNTER TOP (VERIFY LOCATION)

NOT IN CONTRACT

CONTRACTOR INSTALLED

REPLACED WITH NEW

SCCR SHORT CIRCUIT CURRENT RATING

UPS UNINTERRUPTIBLE POWER SUPPLY

TAMPER PROOF DEVICE

EXISTING ITEM TO BE REMOVED

EXISTING ITEM TO BE REMOVED AND

EXISTING ITEM TO BE REMOVED AND

OWNER INSTALLED

NTS NOT TO SCALE

ON CENTER OFCI OWNER FURNISHED

OFOI OWNER FURNISHED,

RELOCATED

TELEVISION

VOLT-AMPERES

WIREGUARD COVER

WEATHERPROOF DEVICE

WEATHER RESISTANT DEVICE

INDICATES MOUNTING HEIGHT CENTER LINE OF DEVICE TO FINISHED FLOOR

TYP TYPICAL

WR

VOLTS

- CONTRACTOR SHALL FAMILIARIZE THEMSELVES WITH EXISTING CONDITIONS PRIOR TO
- ALL 120V-1 PHASE CIRCUITS EXCEEDING 100 FEET TO CENTER OF LOAD SHALL HAVE CONDUCTORS INCREASED TO ACCOUNT FOR VOLTAGE DROP.
- RACEWAYS AND BOXES ARE SHOWN DIAGRAMMATICALLY ONLY AND INDICATE THE GENERAL AND APPROXIMATE LOCATION. THE LAYOUT DOES NOT NECESSARILY SHOW THE TOTAL NUMBER OF RACEWAYS OR BOXES FOR THE CIRCUITS REQUIRED, NOR ARE THE LOCATIONS OF INDICATED RUNS INTENDED TO SHOW THE ACTUAL ROUTING OF THE
- ALL LIGHT FIXTURES, SWITCHES, DEVICES, ETC. ARE SHOWN IN PREFERRED LOCATION. E.C. RESPONSIBLE FOR MODIFYING CONDUIT, HANGERS, CIRCUITING, ETC. TO PROVIDE A COMPLETE AND OPERATIONAL SYSTEM.
- ALL RECEPTACLE CIRCUITS SHALL HAVE DEDICATED NEUTRALS PER CODE. PROVIDE A DEDICATED GREEN INSULATED GROUND CONDUCTORS TO ALL DEVICES. THE CONDUIT SYSTEM SHALL NOT BE USED AS THE ONLY EQUIPMENT GROUNDING
- DO NOT INSTALL DEVICES BACK TO BACK ON OPPOSITE SIDES OF WALL. MAINTAIN
- MINIMUM OF 8" DISTANCE BETWEEN WHEREVER APPLICABLE. BALANCE THE LOAD ON PANELS AS EVENLY AS POSSIBLE DURING INSTALLATION. CIRCUIT NUMBERING SHOWN ON PLANS MAY BE ADJUSTED.
- PROVIDE FINAL TYPED PERMANENT PANEL DIRECTORY AT PROJECT COMPLETION. CONTRACTOR SHALL BE RESPONSIBLE FOR OPENINGS IN ALL WALLS CREATED BY THEIR WORK. PENETRATIONS SHALL BE SEALED IN ACCORDANCE WITH THE RATINGS OF THE AFFECTED WALL. REFER TO ARCHITECTURAL CODE PLAN FOR RATED WALLS.

FIRE DETECTION AND ALARM SYMBOLS

- MANUAL FIRE ALARM PULL STATION
- SMOKE DETECTOR
- SMOKE DETECTOR WITH 520Hz SOUNDER BASE
- SMOKE DETECTOR WALL MOUNTED
- DUCT SMOKE DETECTOR
- CARBON MONOXIDE DETECTOR
- HEAT DETECTOR
- HORN WALL MOUNTED
- COMBINATION HORN WITH STROBE WALL MOUNTED
- COMBINATION HORN WITH STROBE CEILING MOUNTED
- COMBINATION SPEAKER WITH STROBE WALL MOUNTED
- COMBINATION SPEAKER WITH STROBE CEILING MOUNTED
- STROBE WALL MOUNTED
- STROBE CEILING MOUNTED
- SPEAKER WALL MOUNTED
- SPEAKER CEILING MOUNTED
- FIRE FIGHTER TELEPHONE STATION
- ADDRESSABLE INPUT MODULE
- SPRINKLER WATER FLOW SWITCH ADDRESSABLE INPUT MODULE
- SPRINKLER VALVE TAMPER SWITCH ADDRESSABLE INPUT MODULE
- SPRINKLER POST INDICATOR VALVE TAMPER ADDRESSABLE INPUT MODULE
- ADDRESSABLE OUTPUT MODULE
- SD FAN SHUT DOWN RELAY ADDRESSABLE OUTPUT MODULE
- MAGNETIC DOOR HOLD SURFACE MOUNTED
- MAGNETIC DOOR HOLD FLOOR MOUNTED
- SMOKE DAMPER
- COMBINATION FIRE/SMOKE DAMPER
- FIRE ALARM ANNUNCIATOR PANEL
- FIRE ALARM CONTROL PANEL + EMERGENCY COMMUNICATIONS PANEL

GENERAL SYMBOLS

- E──∃ CONDUIT SLEEVE
- CONDUIT UP, REFER TO TAG ON DRAWING FOR SIZE
- CONDUIT DOWN, REFER TO TAG ON DRAWING FOR SIZE
- CIRCUIT HOMERUN, CONCEALED CONDUIT OR CABLE
- CIRCUIT HOMERUN, UNDER FLOOR CONDUIT OR CABLE
- # KEYNOTE
- GROUND FAULT CIRCUIT INTERRUPTER TCC TEMPERATURE CONTROL CONTRACTOR \ SIM DETAIL DRAWING REFERENCE TAG, SIM-SIMILAR, TYP-TYPICAL, OPP-OPPOSITE SHEET REFERENCE



SECTION CUT REFERENCE TAG, SIM-SIMILAR, TYP-TYPICAL, OPP-OPPOSITE



- KITCHEN EQUIPMENT TAG NUMBER, REFER TO KITCHEN EQUIPMENT CONNECTION SCHEDULE
- EQUIPMENT IDENTIFICATION TAG. REFER TO EQUIPMENT CONNECTION SCHEDULE



A101 1 INTERIOR ELEVATION DRAWING REFERENCE TAG

POWER SYMBOLS

- SINGLE RECEPTACLE, WALL MOUNT +18", OR AS NOTED
- DUPLEX RECEPTACLE, CEILING MOUNT
- DUPLEX RECEPTACLE, TAMPER-RESISTANT, WALL MOUNT +18", OR AS NOTED
- DUPLEX RECEPTACLE, SURFACE RACEWAY, WALL MOUNT +18", OR AS NOTED
- DUPLEX GFCI RECEPTACLE, TAMPER-RESISTANT, WALL MOUNT +18", OR AS NOTED
- DUPLEX RECEPTACLE, MOUNTED WITHIN WATER COOLER HOUSING, VERIFY HEIGHT. CONNECT TO GFCI. CIRCUIT BREAKER OR REMOTE WALL DEVICE.
- DUPLEX GFCI RECEPTACLE WITH WEATHER-PROOF IN-USE COVER, TAMPER-RESISTANT, WALL MOUNT +24", OR AS NOTED
- QUADRAPLEX RECEPTACLE, TAMPER-RESISTANT, WALL MOUNT +18", OR AS NOTED
- QUADRAPLEX GFCI RECEPTACLE, TAMPER-RESISTANT, WALL MOUNT +18", OR AS NOTED
- DUPLEX RECEPTACLE IN FLOORBOX, TAMPER-RESISTANT. REFER TO SCHEDULE.
- QUADRUPLEX RECEPTACLE IN FLOORBOX, TAMPER-RESISTANT. REFER TO FLOOR BOX, COMBINATION POWER AND DATA ENCLOSURE. QUANTITY OF CABLES AS
- NOTED. DEVICES AS NOTED. REFER TO SCHEDULE. SPECIAL RECEPTACLE, WALL MOUNT +18", OR AS NOTED, REFER TO ELECTRICAL EQUIPMENT CONNECTION SCHEDULE FOR RECEPTACLE TYPE
- SPECIAL RECEPTACLE, CEILING MOUNT, REFER TO ELECTRICAL EQUIPMENT CONNECTION SCHEDULE FOR RECEPTACLE TYPE
- EQUIPMENT CONNECTION, REFER TO ELECTRICAL EQUIPMENT CONNECTION
- EQUIPMENT CONNECTION, WALL MOUNT +18", OR AS NOTED, REFER TO ELECTRICAL EQUIPMENT CONNECTION SCHEDULE FOR CONNECTION TYPE
- BLANK FACE GFCI DEVICE, WALL MOUNT +48", OR AS NOTED
- MOTORIZED DOOR OPERATOR CONTROL STATION, WALL MOUNT, +48", OR AS NOTED

SCHEDULE FOR CONNECTION TYPE

- DOOR PUSH BUTTON (WEATHERPROOF), +48" OR AS NOTED
- GYM EQUIPMENT CONTROLLER, WALL MOUNT +48", OR AS NOTED JUNCTION BOX WITH PULL STRING WALL MOUNT.
- REFER TO PLAN OR DETAIL FOR MOUNTING HEIGHT
- HAND DRYER, WALL MOUNT +48", OR AS NOTED
- UTILITY TRANSFORMER
- UTILITY METER
- SURGE PROTECTIVE DEVICE
- SAFETY DISCONNECT SWITCH
- EMERGENCY PUSH BUTTON
- PANELBOARD SURFACE MOUNTED
- PANELBOARD RECESSED IN WALL
- VARIABLE FREQUENCY DRIVE

TELECOMMUNICATIONS INFRASTRUCTURE SYMBOLS

- VOICE / DATA OUTLET: 4" SQUARE JUNCTION BOX, MUD RING, AND 1"C WITH PULL ROPE TO ACCESSIBLE CEILING SPACE OR MDF ROOM
- WIRELESS ACCESS POINT- SEE SCHEDULE FOR EXACT REQUIREMENTS
- CABLE TELEVISION LOCATION RADIO GRADE CABLING
- - SEE RISER DIAGRAM AND SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS

TEMPERATURE CONTROLS SYMBOLS - DEVICES PROVIDED BY T.C.C

- THERMOSTAT JUNCTION BOX ROUGH-IN. WALL MOUNTED +48" OR AS NOTED. EXTEND 3/4" CONDUIT TO ACCESSIBLE LOCATION ABOVE CEILING.
- HUMIDISTAT JUNCTION BOX ROUGH-IN. WALL MOUNTED +48" OR AS NOTED. EXTEND 3/4" CONDUIT TO ACCESSIBLE LOCATION ABOVE CEILING.
- CARBON DIOXIDE SENSOR JUNCTION BOX ROUGH-IN. WALL MOUNTED +48" OR AS

IOTED. EXTEND 3/4" CONDUIT TO ACCESSIBLE LOCATION ABOVE CEILING.

TERMPERATURE SENSOR JUNCTION BOX ROUGH-IN. WALL MOUNTED +48" OR AS NOTED. EXTEND 3/4" CONDUIT TO ACCESSIBLE LOCATION ABOVE CEILING.

LIGHTING SYMB F. Exterior Lighting Plan RECESSED LIGHT FIXTURE, LETTER INDICATES SWITCH LEG (TYPICAL), SHADING

INDICATES EMERGENCY LIGHT (TYPICAL)

- ROUND LIGHT FIXTURE SURFACE MOUNTED
- PENDANT MOUNTED LIGHT FIXTURE
- ROUND APERTURE RECESSED DOWNLIGHT FIXTURE, ARROW INDICATES WALLWASH
- SQUARE APERTURE RECESSED DOWNLIGHT FIXTURE, ARROW INDICATES WALLWASH

SQUARE LIGHT FIXTURE - SURFACE MOUNTED

- SURFACE MOUNTED STRIP FIXTURE
- • LINEAR PENDANT MOUNTED FIXTURE
- ├─────── INDUSTRIAL STRIP LIGHT FIXTURE

WALL MOUNTED STRIP LIGHT FIXTURE

- COVE LIGHT FIXTURE
- EMERGENCY LIGHT FIXTURE, WALL MOUNT, +96" OR AS NOTED
- EMERGENCY LIGHT FIXTURE, CEILING MOUNT
- EXIT SIGN, WALL MOUNT +96", SHADED AREAS INDICATE NUMBER OF FACES, ARROWS INDICATE SIGN ARROWS
- EXIT SIGN, CEILING MOUNT, SHADED AREAS INDICATE NUMBER OF FACES, ARROWS INDICATE SIGN ARROWS
- EXTERIOR LIGHT FIXTURE, WALL MOUNT +10', OR AS NOTED
- INTERIOR LIGHT FIXTURE, WALL MOUNT
- EXTERIOR POLE MOUNTED LIGHT FIXTURE, REFER TO LIGHT FIXTURE SCHEDULE
 - BOLLARD LIGHT FIXTURE
- EXTERIOR FLOOD LIGHT FIXTURE, REFER TO LIGHT FIXTURE SCHEDULE
- CEILING FAN
- SINGLE POLE SWITCH, WALL MOUNT +48", OR AS NOTED. LETTER INDICATES SWITCH LEG
- THREE WAY SWITCH, WALL MOUNT +48", OR AS NOTED,
- PILOT LIGHT SWITCH, WALL MOUNT +48", OR AS NOTED, \$ c LETTER INDICATES SWITCH LEG
- DIMMER SWITCH, WALL MOUNT +48", OR AS NOTED,
- LOW VOLTAGE MULTI-BUTTON SWITCH, WALL MOUNT +48", OR AS NOTED, LETTER INDICATES SWITCH LEG, REFER TO LIGHTING CONTROLS SCHEDULE

OCCUPANCY SENSOR, WALL MOUNT +48", NUMBER INDICATES TYPE, LETTER

- INDICATES SWITCH LEG, REFER TO LIGHTING CONTROLS SCHEDULE
- OCCUPANCY SENSOR, CEILING MOUNT, NUMBER INDICATES TYPE, LETTER INDICATES SWITCH LEG, REFER TO LIGHTING CONTROLS SCHEDULE
- DAYLIGHTING SENSOR, CEILING MOUNT, NUMBER INDICATES TYPE, LETTER INDICATES SWITCH LEG, REFER TO LIGHTING CONTROLS SCHEDULE
- LIGHTING CONNECTION, REFER TO LIGHTING FIXTURE SCHEDULE FOR
- EMERGENCY TRANSFER DEVICE
- LIGHTING CONTACTOR
- PHOTOCELL

GROUNDING AND BONDING SYMBOLS

ROOM/ZONE CONTROLLER, MOUNT ABOVE ACCESSIBLE CEILING

- <u>•</u> GROUND BAR

TELECOMMUNICATIONS MAIN GROUND BAR

- TELECOMMUNICATIONS GROUND BAR
 - SEE RISER DIAGRAM AND SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS

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EVISION DATE REASON FOR ISSUE

ELECTRICAL SYMBOL LEGEND AND GENERAL

DEVELOPMENT

1.24.2020

PROJECT NUMBER

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E0.01

LIGHTING FIXTURE SCHEDULE 1. ALL FIXTURES SHALL BE U.L. OR SIMILARLY LISTED. REFER TO ARCHITECTURAL DOCUMENTS FOR EXACT MOUNTING LOCATIONS, DETAILS, AND CONFIGURATIONS OF ALL LUMINAIRES. IF ARCHITECTURAL DRAWINGS DO NOT CLARIFY EXACT MOUNTING LOCATION OR DETAIL, CONTRACTOR SHALL 2 ISSUE AN RFI FOR ARCHITECT TO SPECIFICALLY CLARIFY PRIOR TO FIXTURE ROUGH-IN. 3. VERIFY COMPATIBILITY OF LIGHT FIXTURES WITH CEILING MATERIAL, ADJACENT CONSTRUCTION, AND ADJACENT FINISHES PRIOR TO SHOP DRAWINGS SUBMITTAL AND NOTIFY THE ARCHITECT OF ANY CONFLICTS WITH THE PROPOSED... 4. CONTRACTOR IS RESPONSIBLE FOR ALL MISCELLANEOUS HARDWARE NECESSARY TO INSTALL AND SUPPORT THE LUMINAIRES. ADJUSTABLE INTERIOR AND EXTERIOR LIGHT FIXTURES SHALL BE TARGETED AND ADJUSTED BY THE CONTRACTOR UNDER THE OBSERVATION AND IN COMPLIANCE WITH RECOMMENDATIONS OF THE ARCHITECT. ALL LABOR AND MATERIAL 5. COSTS MADE NECESSARY BY THIS REQUIREMENT SHALL BE INCLUDED. 6. CONTRACTOR IS RESPONSIBLE FOR OBTAINING AND FILLING OUT ALL UTILITY REBATE FORMS FOR OWNER. TYPE MANUFACTURER MODEL VOLTAGE LOAD-VA LAMP TYPE APPROVED EQUALS 6IN LED RECESSED DOWNLIGHT, OPEN, 3500K AS APPROVED BY ENGINEER 120 V 25 VA 25 VA 3.5IN LED RECESSED DOWNLIGHT, TRIMLESS, OPEN, 3000K 120 V AS APPROVED BY ENGINEER 120 V SIMILAR TO TYPE D2. RECESSED ADJUSTABLE DOWNLIGHT. 25 VA AS APPROVED BY ENGINEER 2X2 RECESSED LED TROFFER, 3500K, STANDARD OUTPUT 120 V AS APPROVED BY ENGINEER 35 VA 2X2 RECESSED LED TROFFER, 3500K, HIGH OUTPUT 120 V AS APPROVED BY ENGINEER 35 VA 12FT CONTINUOUS LENGTH, 4IN WIDE APERTURE RECESSED LINEAR LED, STANDARD OPTICS, 3500K 120 V AS APPROVED BY ENGINEER 2X2 RECESSED LED EDGE-LIT TROFFER, 3500K, STANDARD OUTPUT 120 V 35 VA AS APPROVED BY ENGINEER AS APPROVED BY ENGINEER 4FT LED SURFACE MOUNTED LENSED WRAP, 3500K 120 V 40 VA LED UTILITY STRIP FIXTURE. 4' LENGTH, CHAIN MOUNTED. 3500K 120 V AS APPROVED BY ENGINEER 40 VA AS APPROVED BY ENGINEER 4FT LED COVE LIGHT MOUNTED INSIDE OF COVE SHELF, CONTINUOUSLY, 3000K 120 V 40 VA LOW PROFILE LINEAR NICHE DOWNLIGHT. 3000K, 90 CRI 120 V AS APPROVED BY ENGINEER 40 VA 120 V 40 VA 120 V 30 VA 120 V 30 VA 4FT LED UTILITY STRIP, SURFACE MOUNTED. 3500K AS APPROVED BY ENGINEER AS APPROVED BY ENGINEER 32IN LED PENDANT, 2700K, CLOTH SHADE AS APPROVED BY ENGINEER LED GLASS PENDANT, CABLE HUNG, 3000K LED SITE AREA LIGHT, 25FT OVERALL HEIGHT, 5" SQUARE STEEL POLE, 3000K, FORWARD THROW, 120 V 0 VA AS APPROVED BY ENGINEER 6IN ROUND LED LIGHTING BOLLARD, 9" HEIGHT, 3000K 120 V AS APPROVED BY ENGINEER 2IN LED CYLINDER, DOWN DISTRIBUTION, WALL MOUNT, WET LOCATION LISTED, 3000K 120 V 0 VA AS APPROVED BY ENGINEER 120 V 6IN CYLINDER LED DOWNLIGHT, PENDANT THREAD MOUNT, WET LOCATION LISTED, 3000K 0 VA AS APPROVED BY ENGINEER ADJUSTABLE LED FLOOD LIGHT PROJECTOR. MOUNTED ALONG UNDERSIDE OF EXTERIOR SOFFIT. 120 V AS APPROVED BY ENGINEER 0 VA ADJUSTABLE AIM KNUCKLE MOUNT, WIDE FLOOD OPTIC, 3000K, REMOTE POWER SUPPLY LINEAR LED TAPE LIGHT. OUTDOOR RATED. 3000K, 200 LUMENS/FT. APPROXIMATELY 12'-9" IN TOTAL 120 V 24 VA AS APPROVED BY ENGINEER I FNGTH LED SLIM STEP LIGHT, 3000K 0 VA AS APPROVED BY ENGINEER 3FT LED WALL MOUNT VANITY LIGHT, ABOVE RESTROOM MIRROR, 3500K 120 V 35 VA AS APPROVED BY ENGINEER 120 V 20 VA W2 4FT VERTICAL LINEAR WALL MOUNT, SOLID FRONT WITH SIDE OPTICS, 3500K AS APPROVED BY ENGINEER W3 LED WALL SCONCE, GLASS SHADE, 3000K 120 V 20 VA AS APPROVED BY ENGINEER 2IN LED ACCENT, FLOOD DISTRIBUTION, HINGE MOUNT ADJUSTABLE AIM, ALTERNATE UP/DOWN, 120 V 20 VA AS APPROVED BY ENGINEER LED EXIT SIGN, UNIVERSAL MOUNTING 120 V 5 VA RED LED

	EQUIPMENT CONNECTION SCHEDULE								
ABE	BREVIATIONS:								
1	NEMA 1 ENCLOSURE	INT	INTEGRAL WITH EQUIPMENT FROM FACTORY						
3R	NEMA 3R ENCLOSURE	MMS	MANUAL MOTOR STARTER WITH FUSES						
4	NEMA 4 ENCLOSURE	NFD	NON-FUSED DISCONNECT SWITCH						
4X	NEMA 4X ENCLOSURE	RD	RETURN AIR DUCT DETECTOR						
во	PROVIDED BY OTHERS	RSR	RUN STATUS RELAY , NORMALLY OPEN						
СВ	CIRCUIT BREAKER IN PANEL	SD	SUPPLY AIR DUCT DETECTOR						
CSE	COMBINATION STARTER/DISCONNECT	SSP	START/STOP PUSHBUTTON WITH PILOT						
CP	CORD AND PLUG PROVIDED WITH UNIT	SS	START/STOP PUSHBUTTON						
ECB	B ENCLOSED CIRCUIT BREAKER	ST	SHUNT TRIP						
FAR	FIRE ALARM SHUTDOWN RELAY	TOR	TIME DELAY OFF RELAY						
FDS	FUSED DISCONNECT SWITCH	TS	TOGGLE SWITCH WITH PLUG FUSE						
GF	GROUND FAULT CIRCUIT INTERRUPTION	TS-L	LOCKABLE TOGGLE SWITCH WITH PLUG FUSE						
HOA	A HAND-OFF-AUTO	VFD	VARIABLE FREQUENCY DRIVE						
			NO CONTROL C						

	ELEC	TRICAL C	HARACTE	ERISTI	<u>cs</u>		DISCONNECT			CONTROLS		
<u>TAG</u>	VOLTAGE	PHASE	MOTOR HP	<u>KW</u>	MCA	TYPE	SIZE (AMPS)	<u>NEMA</u> RATING	FUSE SIZE (AMPS)	STARTER	DESCRIPTION	REMARKS
AHU-1	208 V	3			63	INT	-	-	-	-	-	CONNECT TO SINGLE POINT POWER CONNECTION AT MECHANICAL UNIT. COORDINATE WITH APPROVED SHOP DRAWINGS.
B-1	120 V	1	-	-	5.88	TS-L	20	1	20	-	-	PROVIDE WITH LOCKABLE TOGGLE SWITCH COVER PER NEC. PROVIDE EMERGENCY-OFF PUSHBUTTON CONTROLS.
B-2	120 V	1	-	-	5.88	TS-L	20	1	20	-	-	PROVIDE WITH LOCKABLE TOGGLE SWITCH COVER PER NEC. PROVIDE EMERGENCY-OFF PUSHBUTTON CONTROLS.
CAB-1	120 V	1			5	TS-L	20		20			
CP-1	120 V	1		-	2	TS	20	1	20	-	-	-
CU-1	208 V	3	-	-	121.4	FDS	200	3R	150	-	-	UNIT PROVIDED WITH NON-FUSED DISCONNECT.
CU-2	208 V	1			16.5	FDS	30	3R	25	-	-	
EF-1	120 V	1	1/2	-	11.52	TS-L				0	-	
P-1	208 V	3	3	-	12.94	FDS	30	1	-	-	-	CONNECT TO SINGLE POINT POWER CONNECTION AT MECHANICAL UNIT. COORDINATE WITH APPROVED SHOP DRAWINGS.
P-2	208 V	3	3	-	12.94	FDS	30	1	-	-	-	CONNECT TO SINGLE POINT POWER CONNECTION AT MECHANICAL UNIT. COORDINATE WITH APPROVED SHOP DRAWINGS.
WH-1	120 V	1	-	-	5.88	TS-L	20	1	20	-	-	-

NOT FOR CONSTRUCTION



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SANDY HEALT

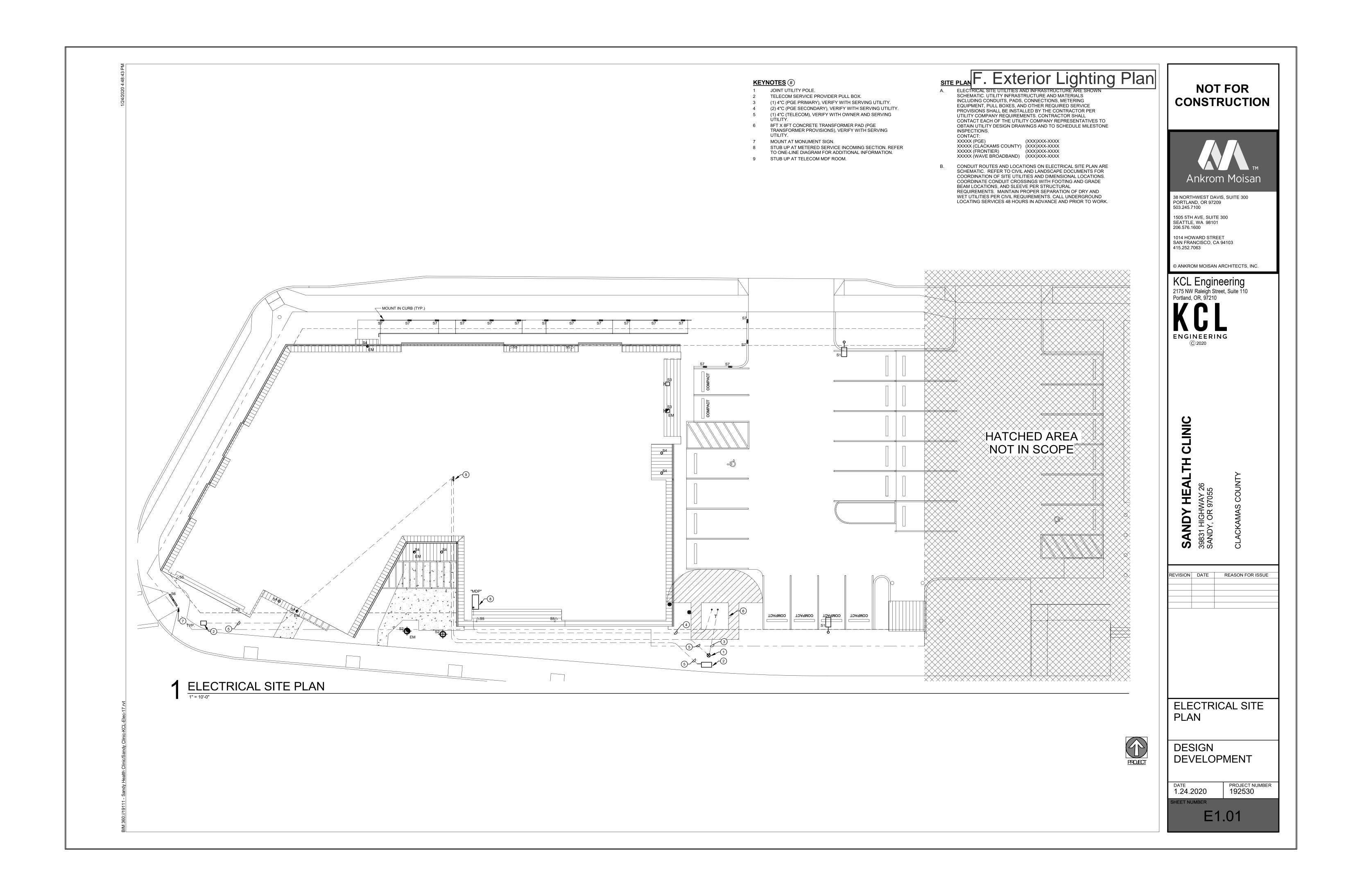
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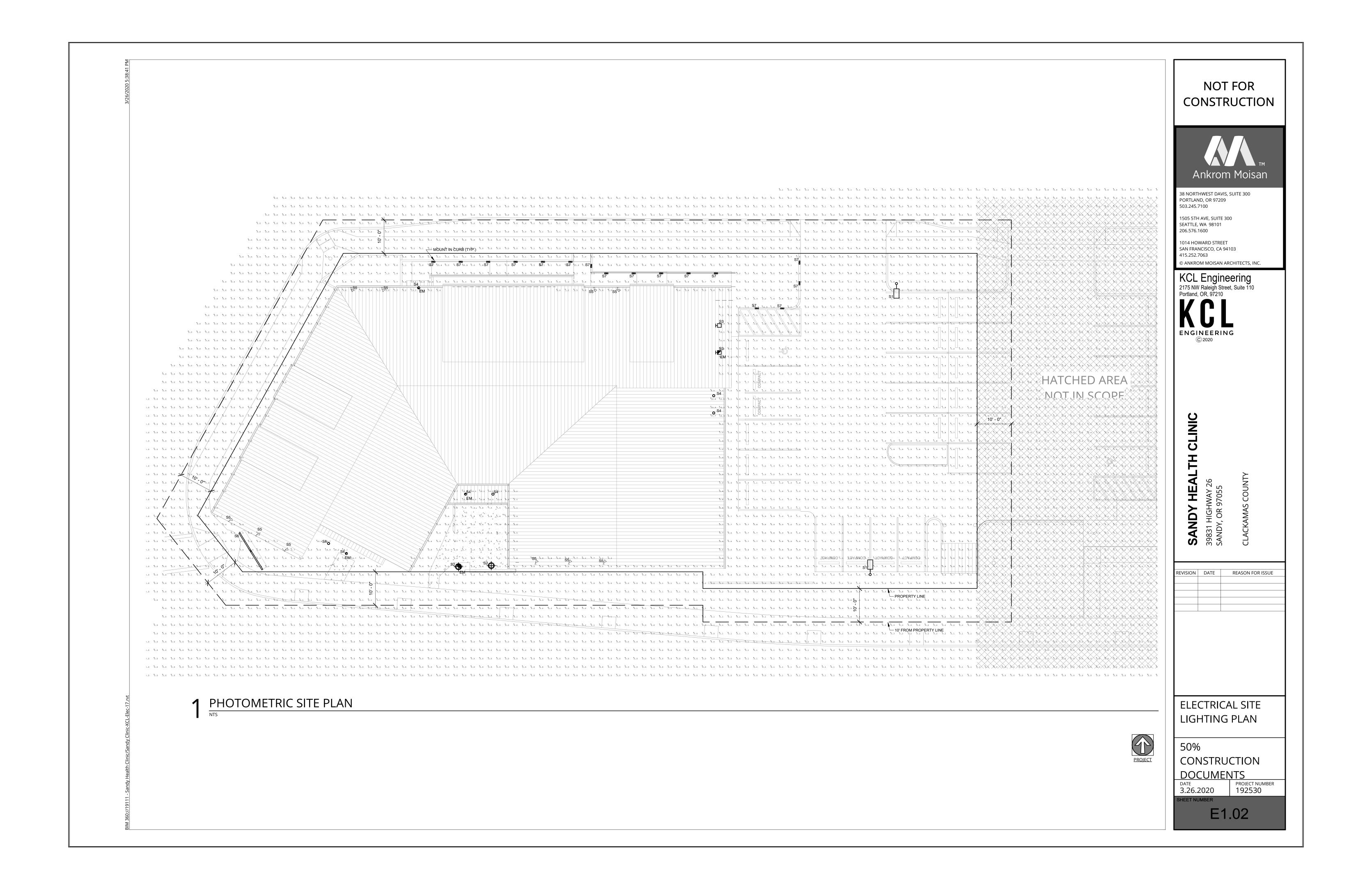
ELECTRICAL SCHEDULES

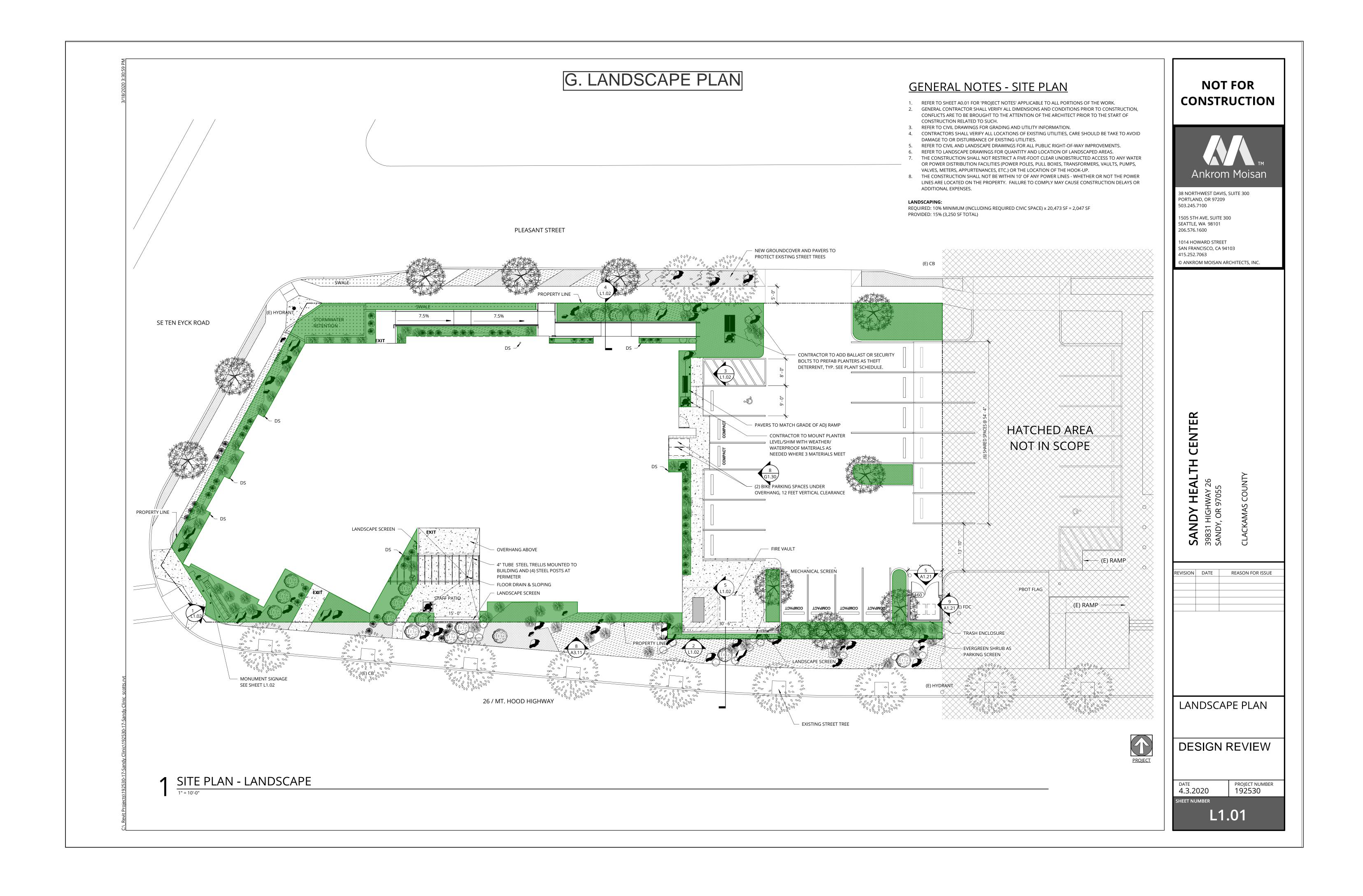
CONSTRUCTION DOCUMENTS

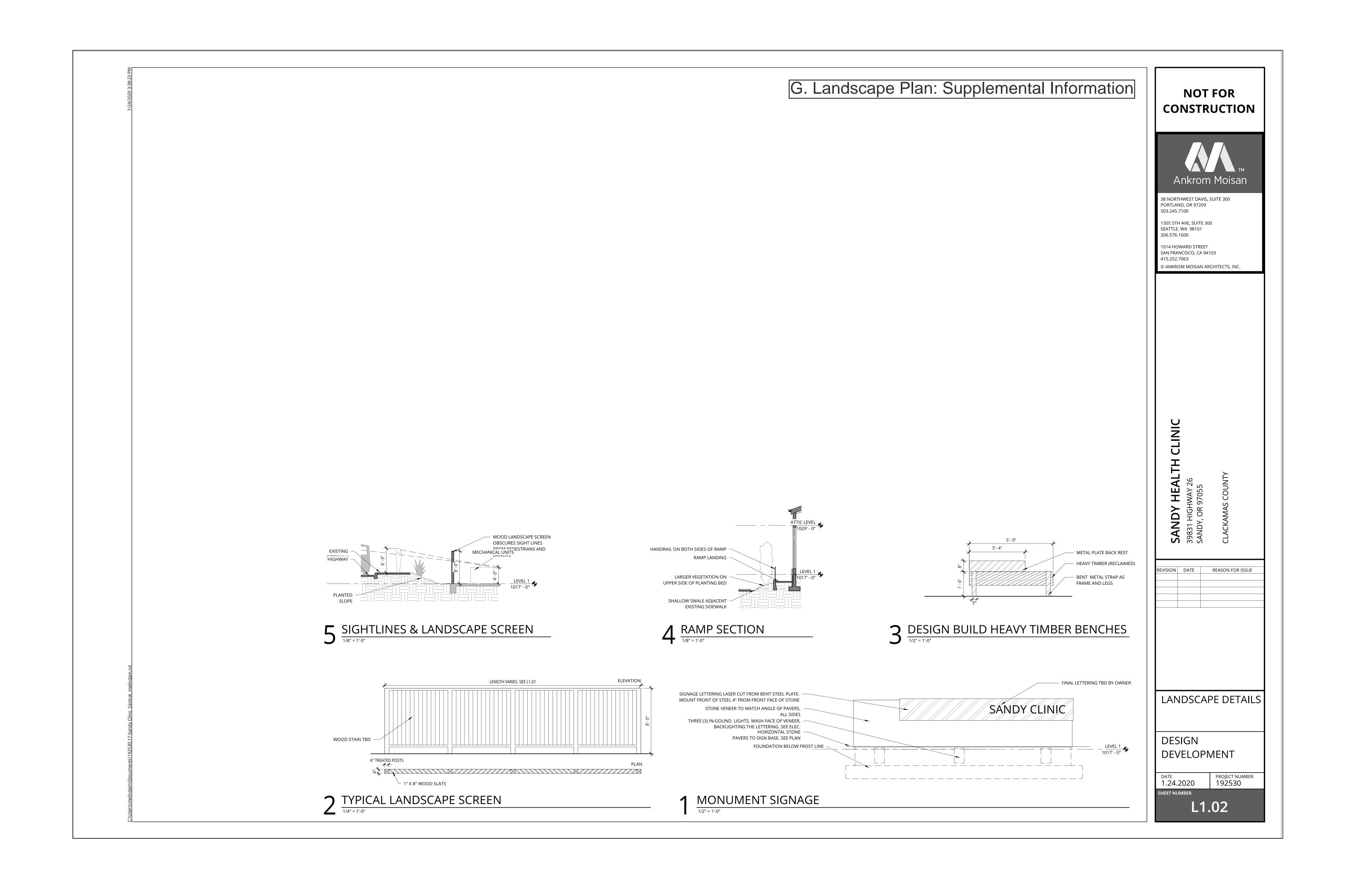
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GENERAL PLANTING NOTES:

- A. DO NOT WILLFULLY PROCEED WITH PLANTING OPERATIONS WHEN IT IS OBVIOUS THAT UNKNOWN OBSTRUCTIONS AND GRADE DIFFERENCES EXIST THAT MAY NOT HAVE BEEN KNOWN DURING THE DESIGN PROCESS. BRING SUCH CONDITIONS IMMEDIATELY TO ATTENTION OF OWNER'S AUTHORIZED REPRESENTATIVE FOR RESOLUTION. ASSUME FULL RESPONSIBILITY FOR COSTS INCURRED AND REQUIRED MODIFICATIONS DUE TO LACK OF PROVIDING SUCH NOTIFI CATION.
- B. ENSURE THAT FINISH GRADE ELEVATIONS OF PLANTING AREAS ARE SET AT THE PROPER ELEVATIONS RELATIVE TO PAVING FINISH SURFACE ELEVATIONS, UTILITY COVERS AND CURBS. SHRUB PLANTING AREAS AT 2" BELOW AND LAWN 1" BELOW ADJACENT GRADE. NOTIFY OWNER OF ANY DISCREPANCIES.
- C. ASSURE POSITIVE DRAINAGE IN ALL PLANTING AREAS TO DRAIN AWAY FROM BUILDING, 2% MINIMUM.
- D. PLANT MATERIAL, I.E. TREES, SHRUBS VINES, ESPALIERS AND GROUNDCOVERS, MUST BE APPROVED BY OWNER'S AUTHORIZED REPRESENTATIVE PRIOR TO INSTALLATION. PLANT MATERIAL INSTALLED WITHOUT OWNER'S AUTHORIZED REPRESENTATIVE'S APPROVAL MAY BE SUBJECT TO REMOVAL AND REPLACEMENT WITH RELATED COSTS BORNE BY CONTRACTOR.
- E. FINAL LOCATIONS OF PLANT MATERIALS ARE SUBJECT TO APPROVAL OF THE OWNER'S AUTHORIZED REPRESENTATIVE PRIOR TO INSTALLATION. PERFORM THE FOLLOWING BEFORE BEGINNING PLANTING PIT
- E.A. SHRUBS PLACE ACTUAL PLANT CONTAINERS ON-SITE IN "FINAL" LOCATIONS.
- E.B. TREES PAINT OR STAKE CENTER OF TREE.
- F. PLANTING SHALL NOT BE PERFORMED UNTIL PRE-PLANTING SOIL AMENDMENTS ARE COMPLETE AND APPROVED BY THE OWNER'S REPRESENTATIVE.
- G. TOPSOIL SHALL BE UTILIZED IN ALL PLANTING AREAS TO THE MAXIMUM EXTENT FEASIBLE. TOPSOIL REMOVED DURING CONSTRUCTION ACTIVITY SHALL BE CONSERVED FOR LATER USE ON AREAS REQUIRING REVEGETATION
- H. SOIL AMENDMENTS SHALL BE PROVIDED IN ACCORDANCE WITH THE RECOMMENDATIONS OF THE FERTILITY ANALYSIS PROVIDED BY AND APPROVED SOIL TESTING LAB. SOILS SHALL BE THOROUGHLY LOOSENED TO A DEPTH OF NOT LESS THAN 8 INCHES AND SOIL AMENDMENT SHALL BE THOROUGHLY INCORPORATED INTO THE SOIL OF ALL LANDSCAPE AREAS TO A DEPTH OF AT LEAST 6 INCHES BY TILLING, DISKING OR OTHER SUITABLE PROCESSES.
- I. IF CONFLICTS ARISE BETWEEN ACTUAL SIZE OF PLANTING AREAS ON-SITE AND THOSE AREAS INDICATED ON DRAWINGS, CONTACT OWNER'S AUTHORIZED REPRESENTATIVE FOR RESOLUTION. FAILURE TO MAKE SUCH CONFLICTS KNOWN TO OWNER'S AUTHORIZED REPRESENTATIVE IN A TIMELY FASHION MAY RESULT IN CONTRACTOR'S LIABILITY TO RELOCATE PLANT MATERIALS OR AT WORST CASE, BECOME UNABLE TO CHARGE OWNER FOR PLANT MATERIAL ALREADY PLANTED.
- J. SHRUB AND GROUNDCOVER AREAS TO RECEIVE A 3-INCH DEEP LAYER MULCH TO BE SUBMITTED FOR APPROVAL FROM LANDSCAPE ARCHITECT.
- K. IRRIGATION: ALL LANDSCAPE AREAS WITHIN THE SITE INCLUDING TURF, SHRUB BEDS AND TREE AREAS SHALL BE IRRIGATED WITH AN AUTOMATIC IRRIGATION SYSTEM. THE IRRIGATION SYSTEM SHALL BE ADJUSTED TO EMIT THE WATER REQUIREMENTS OF THE INDIVIDUAL PLANT MATERIAL.
- L. PROVIDE ROOT CONTROL BARRIERS FOR ALL TREES PLANTED WITHIN 5' OF A HARDSCAPE EDGE SUCH AS PAVING, WALLS, STEPS, ETC. REFER TO PLANTING DETAILS FOR ADDITIONAL INFORMATION.
- M. INSTALL PLANT MATERIAL WITH ITS BEST SIDE FACING PREDOMINATE VIEW OF PUBLIC.
- N. PROVIDE THE PROPER SETBACK BETWEEN UTILITIES AND TREES CONTACT CITY INSPECTOR FOR REQUIRED SETBACKS IN THE CASE THAT THE DRAWINGS ARE NOT CLEAR.
- O. ALL WORK WITHIN THE PUBLIC RIGHT OF WAY UNDER SEPARATE PERMIT. SHOWN FOR REFERENCE ONLY. REFER TO APPROVED RIGHT OF WAY DRAWINGS PRIOR TO CONSTRUCTION.
- P. REFERENCE CIVIL AND ARCHITECTURAL DRAWINGS FOR ADDITIONAL SITEWORK INFORMATION. .
- Q. ALL WORK IN AREAS OF TREE PRESERVATION TO BE DONE USING HAND TOOLS WITH CARE TAKEN TO AVOID DAMAGE TO EXISTING TREE ROOTS, OR UNDER DIRECT SUPERVISION OF A PROJECT ARBORIST.
- R. DO NOT MULCH AGAINST TREE TRUNK.
- S. PLANT PER PLAN BUT NO CLOSER THAN 5 FEET TO TRUNK. ADJUST LOCATION OF NEW PLANTS AS NEEDED TO AVOID TREE ROOTS IF ENCOUNTERED DURING HAND DIGGING.
- T. USE TEMPORARY DRIP IRRIGATION TO WATER PLANTS UNTIL ESTABLISHED, SEE IRRIGATION PLAN FOR MORE
- U. UNUSED.
- V. PER VMC 20.925.115, PRIOR TO OCCUPANCY, A LICENSED LANDSCAPE ARCHITECT SHALL CERTIFY THAT ALL REQUIRED LANDSCAPING HAS BEEN PLANTED PER THE APPROVED PLANS.

PREPARATION, INSTALLATION AND MAINTENANCE OF NEW TREES

- ALL TREES SHALL BE SECURELY STAKED OR GUYED.
- 2. ALL PLANT MATERIAL SHALL BE FREE OF ANY DEFECTS, OF NORMAL HEALTH, HEIGHT, LEAF DENSITY AND SPREAD APPROPRIATE TO THE SPECIES AS DEFINED BY THE AMERICAN ASSOCIATION OF NURSERYMEN (AAN) STANDARDS. ALL TREES SHALL BE BALL AND BURLAP OR EQUIVALENT.
- 3. ALL LANDSCAPING SHALL BE MAINTAINED FREE FROM DISEASE, PESTS, WEEDS AND LITTER.

MAINTENANCE NOTES FOR EXISTING TREES

- WASH OFF FOLIAGE WHICH BECOMES SOILED DURING CONSTRUCTION.
- 2. WATER TREES AND OTHER VEGETATION WHICH ARE TO REMAIN AS NECESSARY TO MAINTAIN THEIR HEATH DURING THE COURSE OF THE WORK. RATE AND FREQUENCY OF APPLICATION TO BE DETERMINED BY PROJECT ARBORIST.
- ALL PRUNING SHALL BE PERFORMED BY A CURRENT ARBORIST LICENSED WITHIN THE STATE/COUNTY/CITY WHERE THE WORK IS TO BE COMPLETED.

STORMWATER PLANTING NOTES

- STORMWATER FACILITIES TO BE PLANTED & CONSTRUCTED PER CITY OF
- PLANT QUANTITIES MEET REQUIREMENTS OF CITY OF SANDY. CONTRACTOR IS RESPONSIBLE FOR VERIFYING THAT THE REQUIRED
- CITY/COUNTY STORMWATER FACILITY INSPECTIONS HAVE BEEN PERFORMED AND APPROVED PRIOR TO PLACEMENT OF THE ENGINEERED SOIL. 4. REFER TO CIVIL ENGINEERS PLANS FOR SOIL (FG) FINISHED GRADE
- ELEVATION AND DEPTH PRIOR TO INSTALLING THE APPROVED TOPSOIL, IRRIGATION AND PLANTING ABOVE DRAIN ROCK AND PIPING.

LANDSCAPE CALCULATION NOTES

LANDSCAPE CALCULATIONS DESCRIPTION

SIGHT DISTANCE TRIANGLE: ALL SHRUBS WITHIN THE SIGHT DISTANCE TRAINGLES SHALL BE MAINTAINED SO THAT FOLIAGE HEIGHT ABOVE PAVEMENT DOES NOT EXCEED 2.5 FT. STREET TREES WITHIN SIGHT DISTANCE TRIANGLE SHALL BE LIMBED UP TO A HEIGHT OF 10 FT. CONSISTENT WITH ALL A300 STANDARDS TO PROVIDE SIGHT

5 FOOT L1 BUFFER REQUIRED: 2" CALIPER TREES, 1 PER 30 LINEAR FEET, TYP

10 FOOT L2 BUFFER REQUIRED: 2" CALIPER TREE, 1 PER 30 LINEAR FEET, 3" CONTINUOUS HEDGE - 3 GALLON CONTAINER WITH 18" MINIMUM SPREAD, TYP

STREET TREES: 2" CALIPER AT 30" ON CENTER, TYP

FIELD ADJUST EXISTING PLANTING TO ACCOMODATE NEW WORK

INFILL PLANTING, FIELD ADJUST TO AVOID EXISTING TREE ROOTS

PARKING LOT PLANTING REQUIREMENTS: A MINIMUM OF 10% OF THE PARKING LOT IS

TO BE LANDSCAPED, INCLUDING 1 TREE PER 10 PARKING STALLS AND 1 SHRUB FOR 30 SF OF THE REQUIRED LANDSCAPE AREA, TYP

PLANT SCHEDULE

*						
TREES	BOTANICAL/COMMON NAMES	CONT	CAL	SIZE	C	ΣΤΥ
	NYSSA SYLVATICA 'HAYMAN'S RED'/ RED RAGE TUPELO	B&B	2" CAL			9
	ILEX CRENATA 'STEEDS' UPRIGHT HOLLY			6'H/7	GAL	18
SHRUBS	BOTANICAL/COMMON NAMES	SIZE	HT.			QTY
	OSMANTHUS XFORTUNEI 'SAN JOSE' SWEET TEA OLIVE	3 GAL				10
ORNAMENTAL GRASSES	BOTANICAL/COMMON NAMES	SIZE				QTY
	OPHIOPOGON PLANISCAPUS 'NIGRESCENS' / MONDO GRASS	3 GA	L			66
E mart	DELPHINIUM ELATUM/ MAGIC FOUNTAIN MIX	3 GA	L			28
*	CAREX MORROWII 'AUREA-VARIEGATA'/ VARIEGATED JAPANESE SEDGE	3 GA	AL.			55
VINES	BOTANICAL/COMMON NAMES	SIZE				QTY
	CLEMATIS ARMANDII 'APPLE BLOSSOM/ WHITE EVERGREEN CLEMATIS	1 GA	L			2
STORMWATER/ SWALE PLANTING	G MIX BOTANICAL/COMMON NAMES					SF
	/ATER BNUPTA / SLOUGH SEDGE EFFUSUS / SOFT RUSH			1 GAL 1 GAL	50% @ 12" oc 50% @ 12" oc	220

PAVERS AND STONE

RECTANGULAR TUMBLED STONE PAVERS, RUNNING PATTERN ORIENTATION AT ANGLE INDICATED. 525 sf NATIVE ROCK TO MATCH BOULDERS - ARCHITECT TO APPROVE SAMPLE

AREA/QTY



MD (12" - 24") AND LG (24" - 36") BOULDERS, NATIVE ROCK MATERIAL- AS NOTED ON L1.01 ARCHITECT TO APPROVE SAMPLE

MULCH 4" DARK BROWN BARK MULCH BY INSTALLER, LOCAL SUPPLIER, NO DYES - FOR ALL BEDS AT PLANT INSTALLATION

GROUND COVERS	BOTANICAL/COMMON NAMES	CONT.	SPACING	AREA
	POTENTILLA N. 'NANA'/CINQUEFOIL	1 gal	18" o.c.	2600 sf
	EUONYMUS 'COLORATUS' / PURPLEWINTERCREEPER	1 gal	18" o.c.	1500 sf

FURNITURE



PLANTERS QTY 4 powder-coated aluminum planters,27" x 27" x 28"-color TBD, 1 gal evergreen shrub in center (selected by installer), seasonal annuals by owner. BOD: Ore, Inc Cube https://ore.design/products/cube/

BENCHES QTY 2 single-sided heavy-timber benches with steel back rest and legs, scaled dimensions as shown, QTY: 1 double -sided heavy-timber benches with steel back rest and legs, scaled dimensions as shown,Contractor design/build

G. Landscape Plan: Supplemental Information

NOT FOR CONSTRUCTION



38 NORTHWEST DAVIS, SUITE 300 PORTLAND, OR 97209 503.245.7100

1505 5TH AVE, SUITE 300 SEATTLE, WA 98101 206.576.1600

1014 HOWARD STREET SAN FRANCISCO, CA 94103 415.252.7063

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HEALHWAY 26

97055 SANDY I 39831 HIGHV SANDY, OR 9

VISION	DATE	REASON FOR ISSU

LANDSCAPE NOTES -PLANT SCHEDULE

DESIGN DEVELOPMENT

PROJECT NUMBER 1.24.2020 192530

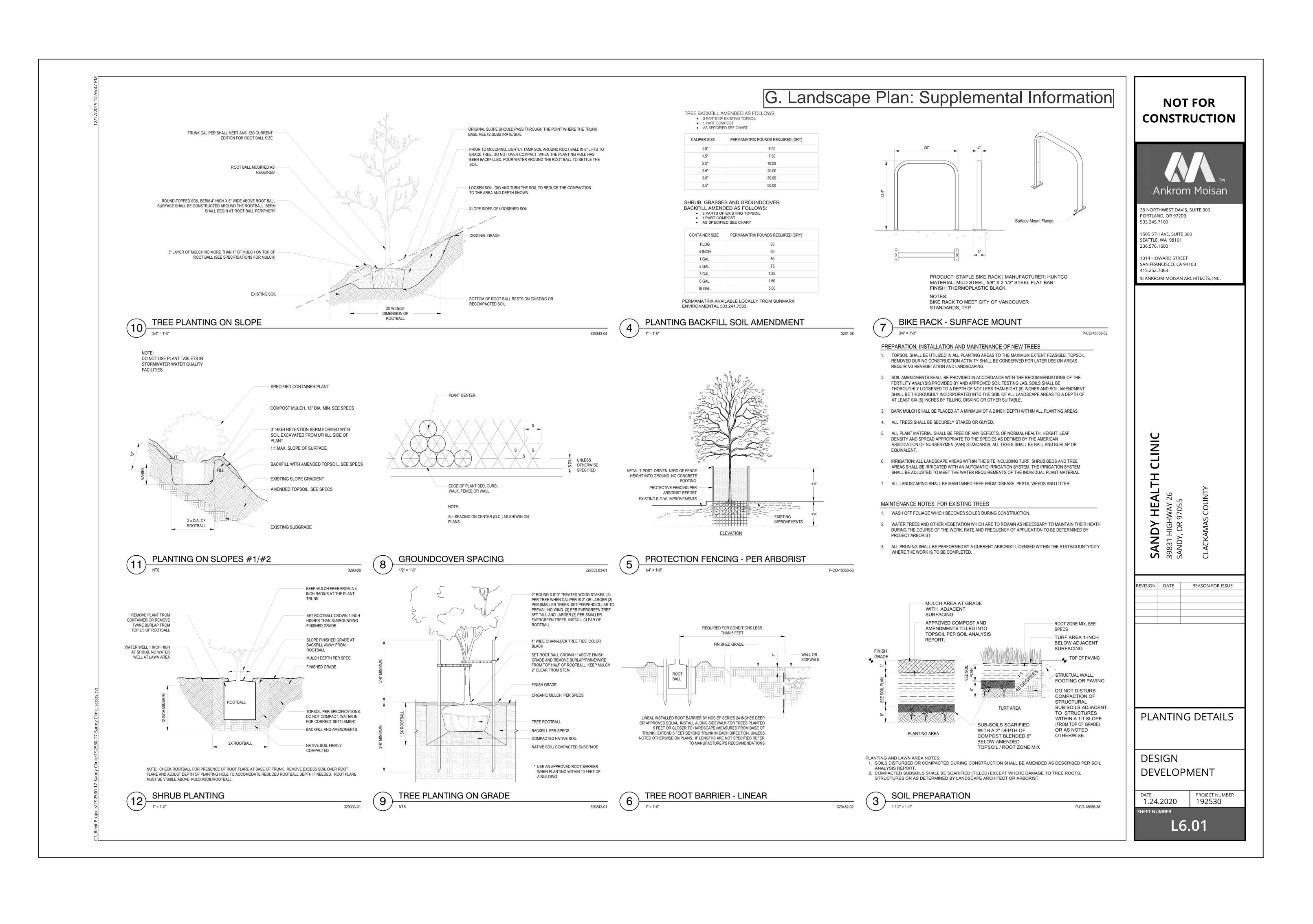


EXHIBIT D



D-Series Size 1

LED Area Luminaire





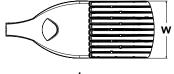




Specifications

- p	
EPA:	1.01 ft ² (0.09 m ²)
Length:	33" (83.8 cm)
Width:	13" (33.0 cm)
Height H1:	7-1/2" (19.0 cm)
Height H2:	3-1/2"
Weight	27 11.

(max):





S1 Fixture

H. Sandy Health Clinic
- Exterior Light Fixture
Cut-Sheets

Introduction

The modern styling of the D-Series is striking yet unobtrusive - making a bold, progressive statement even as it blends seamlessly with its environment. The D-Series distills the benefits of the latest in LED technology into a high performance, high efficacy, long-life luminaire.

The outstanding photometric performance results in sites with excellent uniformity, greater pole spacing and lower power density. It is ideal for replacing up to 750W metal halide in pedestrian and area lighting applications with typical energy savings of 65% and expected service life of over 100,000 hours.



Ordering Information

EXAMPLE: DSX1 LED P7 40K T3M MVOLT SPA NLTAIR2 PIRHN DDBXD

DSX1 LED					
Series	LEDs	Color temperature Dist	stribution	Voltage	Mounting
DSX1 LED	Forward optics	30K 3000 K T1 :	1S Type I short T5VS Type V very short	MVOLT ³	Shipped included
	P1 P4 P7	40K 4000 K T2	2S Type II short T5S Type V short	120 ⁴	SPA Square pole mounting
	P2 P5 P8	50K 5000 K T2	2M Type II medium T5M Type V medium	208 4	RPA Round pole mounting
	P3 P6 P9	T3:	3S Type III short T5W Type V wide	240 ⁴	WBA Wall bracket
	Rotated optics	T3	3M Type III medium BLC Backlight control ²	277 4	SPUMBA Square pole universal mounting adaptor 5
	P10 ¹ P12 ¹	T4	4M Type IV medium LCCO Left corner cutoff ²	347 4	RPUMBA Round pole universal mounting adaptor 5
	P11 ¹ P13 ¹	TF	FTM Forward throw RCCO Right corner cutoff ²	480 4	Shipped separately
			medium		KMA8 DDBXD U Mast arm mounting bracket adaptor (specify finish) ⁶

Control op	tions			Other options		Finish (required)	
Shipped in NLTAIR2 PIRHN PER PER5 PER7 DMG	nstalled nLight AIR generation 2 enabled ⁷ Network, high/low motion/ambient sensor ⁸ NEMA twist-lock receptacle only (controls ordered separate) ⁹ Five-pin receptacle only (controls ordered separate) ^{9,10} Seven-pin receptacle only (controls ordered separate) ^{9,10} 0-10v dimming wires pulled outside fixture (for use with an external control, ordered separately) ¹¹ Dual switching ^{12,13,14}	PIR PIRH PIR1FC3V PIRH1FC3V FAO	High/low, motion/ambient sensor, 8-15' mounting height, ambient sensor enabled at 5fc ^{15, 16} High/low, motion/ambient sensor, 15-30' mounting height, ambient sensor enabled at 5fc ^{15, 16} High/low, motion/ambient sensor, 8-15' mounting height, ambient sensor enabled at 1fc ^{15, 16} Bi-level, motion/ambient sensor, 15-30' mounting height, ambient sensor enabled at 1fc ^{15, 16} Field adjustable output ¹⁴	HS SF DF L90 R90	ped installed House-side shield ¹⁷ Single fuse (120, 277, 347V) ⁴ Double fuse (208, 240, 480V) ⁴ Left rotated optics ¹ Right rotated optics ¹ ped separately Bird spikes ¹⁸ External glare shield	DDBXD DBLXD DNAXD DWHXD DWHXD DDBTXD DBLBXD DNATXD DWHGXD	Dark bronze Black Natural aluminum White Textured dark bronze Textured black Textured natural aluminum Textured white



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COMMERCIAL OUTDOOR

Ordering Information

Accessories

DLL127F 1.5 JU Photocell - SSL twist-lock (120-277V) 19 DLL347F 1.5 CUL JU Photocell - SSL twist-lock (347V) 19 DLL480F 1.5 CUL JU Photocell - SSL twist-lock (480V) 19 DSHORT SBK U Shorting cap 19

House-side shield for P1, P2, P3, P4 and P5¹¹ DSX1HS 30C U DSX1HS 40C U House-side shield for P6 and P717 DSX1HS 60C U House-side shield for P8, P9, P10, P11 and P1217

Square and round pole universal mounting bracket (specify finish)²⁰ PUMBA DDBXD U* KMA8 DDBXD U Mast arm mounting bracket adaptor (specify finish) ⁶

DSX1EGS (FINISH) U External glare shield

For more control options, visit DTL and ROAM online.

- NOTES

 1 P10, P11, P12 or P13 and rotated optics (L90, R90) only available together.

 2 Not available with HS.

 3 M/OLIT driver operates on any line voltage from 120-277V (50/60 Hz).

 4 Single fuse (SF) requires 120V, 277V or 347V. Double fuse (DF) requires 208V, 240V or 480V.

 5 Universal mounting brackets intended for retrofit on existing, pre-drilled poles only. 1.5 G vibration load rating per ANCI C136.31.

 6 Must order future with SPA option. Must be ordered as a separate accessory; see Accessories information. For use with 2-3/8" mast arm (not included).

 7 Must be ordered with PIRHN. Sensor cover available only in dark bronze, black, white and natural aluminum colors.

 8 Must be ordered with PIRHA. Sensor cover available only in dark bronze, black, white and natural aluminum colors.

 8 Must be ordered with PIRHA. Sensor cover available only a Vaint this line.

 9 Photocell ordered and shipped as a separate line item from Acuity Brands Controls. See accessories. Not available with DS option. Shorting cap included.

 10 In ROAM mode required, it must be ordered and shipped as a separate line item from Acuity Brands Controls. Node with integral dimming.

 11 DNG not available with PIRHN, PERS, PERP, PIR, PIRH, PIRHC-3V or PIRHH-C3V.

 12 Provides SOfficture operation via (2) independent drivers. Not available with PER, PERS, PER7, PIR or PIRH. Not available P1, P2, P3, P4 or P5.

 13 Requires (2) separately switched circuits with isolated neutrol. See Outdoor Control Technical Guide for details.

 14 Reference Anktoin Sensor table on page 4.

 15 Reference controls options table on page 4 to see functionality.

 16 Not available with DLC LCCO and RCCO distribution. Also available as a separate accessory; see Accessories information.

 18 Must be ordered with future for factory pre-drilling.

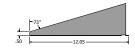
 19 Requires luminaire to be specified with PER, PERS or PER7 option. See PER Table on page 3.

 20 For retrofit use only.

Options

EGS - External Glare Shield

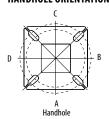


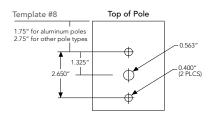




Drilling

HANDHOLE ORIENTATION





Tenon Mounting Slipfitter**

Tenon O.D.	Mounting	Single Unit	2 @ 180	2 @ 90	3 @120	3 @ 90	4 @ 90
	SPA/RPA	AS3-5 190	AS3-5 280	AS3-5 290	AS3-5 320	AS3-5 390	AS3-5 490
2-3/8"	SPUMBA	AS3-5 190	AS3-5 280	AS4-5 290	AS3-5 320	AS4-5 390	AS4-5 490
	RUPUMBA	AS3-5 190	AS3-5 280		AS3-5 320		
	SPA/RPA	AST25-190	AST25-280	AST25-290	AST25-320	AST25-390	AST25-490
2-7/8"	SPUMBA	AST25-190	AST25-280		AST25-320		
	RUPUMBA	AST25-190	AST25-280		AST25-320		
	SPA/RPA	AST35-190	AST35-280	AST35-290	AST35-320	AST35-390	AST35-490
4"	SPUMBA	AST35-190	AST35-280	AST35-290	AST35-320	AST35-390	AST35-490
	RUPUMBA	AST35-190	AST35-280		AST35-320		

		-		Ł.,	_T_	Y	-1-
Mounting Option	Drilling Template	Single	2 @ 180	2@90	3 @ 90	3 @ 120	4@90
Head Location		Side B	Side B & D	Side B & C	Side B, C & D	Round Pole Only	Side A, B, C & D
Drill Nomenclature	#8	DM19AS	DM28AS	DM29AS	DM39AS	DM32AS	DM49AS

	Drilling Template	Minimum Acceptable Outside Pole Dimension					
SPA	#8	2-7/8"	2-7/8"	3.5"	3.5"	3"	3.5"
RPA	#8	2-7/8"	2-7/8"	3.5"	3.5"	3"	3.5"
SPUMBA	#5	2-7/8"	3"	4"	4"	3.5"	4"
RPUMBA	#5	2-7/8"	3.5"	5"	5"	3.5"	5"



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Lumen Ambient Temperature (LAT) Multipliers

Use these factors to determine relative lumen output for average ambient temperatures from 0-40 $^{\circ}\text{C}$ (32-104 $^{\circ}\text{F}$).

Am	Ambient						
0°C	32°F	1.04					
5°C	41°F	1.04					
10°C	50°F	1.03					
15°C	50°F	1.02					
20°C	68°F	1.01					
25°C	77°F	1.00					
30°C	86°F	0.99					
35°C	95°F	0.98					
40°C	104°F	0.97					

Projected LED Lumen Maintenance

Data references the extrapolated performance projections for the platforms noted in a 25°C ambient, based on 10,000 hours of LED testing (tested per IESNA LM-80-08 and projected per IESNA TM-21-11).

To calculate LLF, use the lumen maintenance factor that corresponds to the desired number of operating hours below. For other lumen maintenance values, contact factory.

Operating Hours	Lumen Maintenance Factor
0	1.00
25,000	0.96
50,000	0.92
100,000	0.85

	Motion Sensor Default Settings									
Option	Dimmed State	High Level (when triggered)	Phototcell Operation	Dwell Time	Ramp-up Time	Ramp-down Time				
PIR or PIRH	3V (37%) Output	10V (100%) Output	Enabled @ 5FC	5 min	3 sec	5 min				
*PIR1FC3V o	r 3V (37%) Output	10V (100%) Output	Enabled @ 1FC	5 min	3 sec	5 min				

Electrical Load

								nt (A)		
	Performance Package	LED Count	Drive Current	Wattage	120	208	240	277	347	480
	P1	30	530	54	0.45	0.26	0.23	0.19	0.10	0.12
	P2	30	700	70	0.59	0.34	0.30	0.25	0.20	0.16
	P3	30	1050	102	0.86	0.50	0.44	0.38	0.30	0.22
	P4	30	1250	125	1.06	0.60	0.52	0.46	0.37	0.27
Forward Optics (Non-Rotated)	P5	30	1400	138	1.16	0.67	0.58	0.51	0.40	0.29
	P6	40	1250	163	1.36	0.78	0.68	0.59	0.47	0.34
	P7	40	1400	183	1.53	0.88	0.76	0.66	0.53	0.38
	P8	60	1050	207	1.74	0.98	0.87	0.76	0.64	0.49
	P9	60	1250	241	2.01	1.16	1.01	0.89	0.70	0.51
	P10	60	530	106	0.90	0.52	0.47	0.43	0.33	0.27
Rotated Optics	P11	60	700	137	1.15	0.67	0.60	0.53	0.42	0.32
(Requires L90 or R90)	P12	60	1050	207	1.74	0.99	0.87	0.76	0.60	0.46
	P13	60	1250	231	1.93	1.12	0.97	0.86	0.67	0.49

		Controls Options		
Nomenclature	Descripton	Functionality	Primary control device	Notes
FAO	Field adjustable output device installed inside the lumiaire; wired to the driver dimming leads.	Allows the lumiaire to be manually dimmed, effectively trimming the light output.	FAO device	Cannot be used with other controls options that need the 0-10V leads
DS	Drivers wired independantly for 50/50 luminaire operation	The luminaire is wired to two separate circuits, allowing for 50/50 operation.	Independently wired drivers	Requires two seperately switched circuits. Consider nLight AIR as a more cost effective alternative.
PER5 or PER7	Twist-lock photocell recepticle	Compatible with standard twist-lock photocells for dusk to dawn operation, or advanced control nodes that provide 0-10V dimming signals.	Twist-lock photocells such as DLL Elite or advanced control nodes such as ROAM.	Pins 4 & 5 to dimming leads on driver, Pins 6 & 7 are capped inside luminaire
PIR or PIRH	Motion sensors with integral photocell. PIR for 8-15' mounting; PIRH for 15-30' mounting	Luminaires dim when no occupancy is detected.	Acuity Controls SBGR	Also available with PIRH1FC3V when the sensor photocell is used for dusk-to-dawn operation.
NLTAIR2 PIRHN	nLight AIR enabled luminaire for motion sensing, photocell and wireless communication.	Motion and ambient light sensing with group response. Scheduled dimming with motion sensor over-ride when wirelessly connected to the nLight Eclypse.	nLight Air rSDGR	nLight AIR sensors can be programmed and commissioned from the ground using the CIAIRity Pro app.



Lumen Output

Lumen values are from photometric tests performed in accordance with IESNA LM-79-08. Data is considered to be representative of the configurations shown, within the tolerances allowed by Lighting Facts Contact factory for performance data on any configurations not shown here.

Forward 0	ptics																		
LED C.	Drive	Power	System	Dist.		30K (3000 K, 70 CRI)			40K (4000 K, 70 CRI)						50K				
LED Count	Current	Package	Watts	Туре	Lumens	(3000 B	U U	G	LPW	Lumens	(4000 B	K, 70 CKI	G	LPW	Lumens	(5000 B	K, 70 CRI) U	G	LPW
				T1S	6,457	2	0	2	120	6,956	2	0	2	129	7,044	2	0	2	130
				T2S	6,450	2	0	2	119	6,949	2	0	2	129	7,037	2	0	2	130
				T2M	6,483	1	0	1	120	6,984	2	0	2	129	7,073	2	0	2	131
				T3S	6,279	2	0	2	116	6,764	2	0	2	125	6,850	2	0	2	127
				T3M T4M	6,468	1	0	2	120 117	6,967	1	0	2	129	7,056	1	0	2	131
				TFTM	6,327 6,464	1	0	2	120	6,816 6,963	1	0	2	126 129	6,902 7,051	1	0	2	131
30	530	P1	54W	T5VS	6,722	2	0	0	124	7,242	3	0	0	134	7,334	3	0	0	136
				TSS	6,728	2	0	1	125	7,248	2	0	1	134	7,340	2	0	1	136
				T5M	6,711	3	0	1	124	7,229	3	0	1	134	7,321	3	0	2	136
				T5W	6,667	3	0	2	123	7,182	3	0	2	133	7,273	3	0	2	135
				BLC	5,299	1	0	1	98	5,709	1	0	2	106	5,781	1	0	2	107
				LCCO RCCO	3,943	1	0	2	73	4,248	1	0	2	79 79	4,302	1	0	2	80
				T1S	3,943 8,249	2	0	2	118	4,248 8,886	2	0	2	127	4,302 8,999	2	0	2	129
				T2S	8,240	2	0	2	118	8,877	2	0	2	127	8,989	2	0	2	128
				T2M	8,283	2	0	2	118	8,923	2	0	2	127	9,036	2	0	2	129
				T3S	8,021	2	0	2	115	8,641	2	0	2	123	8,751	2	0	2	125
				T3M	8,263	2	0	2	118	8,901	2	0	2	127	9,014	2	0	2	129
				T4M	8,083	2	0	2	115	8,708	2	0	2	124	8,818	2	0	2	126
30	700	P2	70W	TFTM T5VS	8,257 8,588	3	0	0	118 123	8,896 9,252	3	0	0	127 132	9,008 9,369	3	0	0	129
				T5S	8,588	3	0	1	123	9,252	3	0	1	132	9,369	3	0	1	134
				T5M	8,573	3	0	2	122	9,236	3	0	2	132	9,353	3	0	2	134
				T5W	8,517	3	0	2	122	9,175	4	0	2	131	9,291	4	0	2	133
				BLC	6,770	1	0	2	97	7,293	1	0	2	104	7,386	1	0	2	106
				LCC0	5,038	1	0	2	72	5,427	1	0	2	78	5,496	1	0	2	79
				RCCO	5,038	1	0	2	72	5,427	1	0	2	78	5,496	1	0	2	79
				T1S T2S	11,661 11,648	2	0	2	114 114	12,562 12,548	3	0	3	123 123	12,721 12,707	3	0	3	125
				T2M	11,708	2	0	2	115	12,546	2	0	2	123	12,777	2	0	2	125
				T3S	11,339	2	0	2	111	12,215	3	0	3	120	12,370	3	0	3	121
				T3M	11,680	2	0	2	115	12,582	2	0	2	123	12,742	2	0	2	125
				T4M	11,426	2	0	3	112	12,309	2	0	3	121	12,465	2	0	3	122
30	1050	P3	102W	TFTM	11,673	2	0	2	114	12,575	2	0	3	123	12,734	2	0	3	125
				TSVS	12,140	3	0	1	119	13,078	3	0	1	128	13,244	3	0	11	130
				T5S T5M	12,150 12,119	3	0	2	119 119	13,089 13,056	3	0	2	128 128	13,254 13,221	3	0	2	130
				T5W	12,119	4	0	3	118	12,970	4	0	3	127	13,134	4	0	3	129
				BLC	9,570	1	0	2	94	10,310	1	0	2	101	10,440	1	0	2	102
				LCCO	7,121	1	0	3	70	7,671	1	0	3	75	7,768	1	0	3	76
				RCCO	7,121	1	0	3	70	7,671	1	0	3	75	7,768	1	0	3	76
				T1S	13,435	3	0	3	107	14,473	3	0	3	116	14,657	3	0	3	117
				T2S	13,421	3	0	3	107	14,458	3	0	3	116	14,641	3	0	3	117
				T2M T3S	13,490 13,064	3	0	3	108	14,532 14,074	3	0	3	116 113	14,716 14,252	3	0	3	118
				T3M	13,457	2	0	2	103	14,074	2	0	2	116	14,681	2	0	2	117
				T4M	13,165	2	0	3	105	14,182	2	0	3	113	14,362	2	0	3	115
30	1250	P4	125W	TFTM	13,449	2	0	3	108	14,488	2	0	3	116	14,672	2	0	3	117
20	1230	""	ILJVV	T5VS	13,987	4	0	1	112	15,068	4	0	1	121	15,259	4	0	1	122
				T5S	13,999	3	0	1	112	15,080	3	0	1	121	15,271	3	0	1	122
				T5M T5W	13,963 13,872	4	0	3	112 111	15,042 14,944	4	0	3	120 120	15,233 15,133	4	0	3	122
				BLC	11,027	1	0	2	88	11,879	1	0	2	95	12,029	1	0	2	96
				LCCO	8,205	1	0	3	66	8,839	1	0	3	71	8,951	1	0	3	72
				RCCO	8,205	1	0	3	66	8,839	1	0	3	71	8,951	1	0	3	72
				T1S	14,679	3	0	3	106	15,814	3	0	3	115	16,014	3	0	3	116
				T2S	14,664	3	0	3	106	15,797	3	0	3	114	15,997	3	0	3	116
				T2M	14,739	3	0	3	107	15,878	3	0	3	115	16,079	3	0	3	117
				T3S T3M	14,274 14,704	2	0	3	103	15,377 15,840	3	0	3	111	15,572 16,040	3	0	3	113 116
				T4M	14,704	2	0	3	107	15,496	3	0	3	112	15,692	3	0	3	114
20	1400		42011	TFTM	14,695	2	0	3	106	15,830	3	0	3	115	16,030	3	0	3	116
30	1400	P5	138W	T5VS	15,283	4	0	1	111	16,464	4	0	1	119	16,672	4	0	1	121
				TSS	15,295	3	0	1	111	16,477	4	0	1	119	16,686	4	0	1	121
				T5M	15,257	4	0	2	111	16,435	4	0	2	119	16,644	4	0	2	121
				T5W PLC	15,157	4	0	3	110	16,328	4	0	3	118	16,534	4	0	3	120
				BLC LCCO	12,048 8,965	1	0	3	87 65	12,979	1	0	3	94 70	13,143 9,780	1	0	3	95 71
				RCCO	8,965	1	0	3	65	9,657 9,657	1	0	3	70	9,780	1	0	3	71
					5,703			,	33	,,,,,,,			,	,,,	2,700			,	



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Lumen Output

Lumen values are from photometric tests performed in accordance with IESNA LM-79-08. Data is considered to be representative of the configurations shown, within the tolerances allowed by Lighting Facts. Contact factory for performance data on any configurations not shown here.

Forward O	ptics																		
LED Count	Drive	Power	System	Dist.			30K K, 70 CRI)				40K K, 70 CRI					50K K, 70 CRI)	
	Current	Package	Watts	Type	Lumens	В	U	G	LPW	Lumens	В	U	G	LPW	Lumens	В	U	G	LPW
				T1S	17,654	3	0	3	108	19,018	3	0	3	117	19,259	3	0	3	118
				T2S	17,635	3	0	3	108	18,998	3	0	3	117	19,238	3	0	3	118
				T2M	17,726	3	0	3	109	19,096	3	0	3	117	19,337	3	0	3	119
				T3S	17,167	3	0	3	105	18,493	3	0	3	113	18,727	3	0	3	115
				T3M	17,683	3	0	3	108	19,049	3	0	3	117	19,290	3	0	3	118
				T4M	17,299	3	0	3	106	18,635	3	0	4	114	18,871	3	0	4	116
40	1250	P6	163W	TFTM	17,672	3	0	3	108	19,038	3	0	1	117	19,279	3	0	4	118
				T5VS T5S	18,379	4	0	2	113	19,800	4	0	2	121 122	20,050	4	0	2	123
				T5M	18,394 18,348	4	0	2	113	19,816 19,766	4	0	2	122	20,066 20,016	4	0	2	123 123
				T5W	18,228	5	0	3	112	19,636	5	0	3	120	19,885	5	0	3	123
				BLC	14,489	2	0	2	89	15,609	2	0	3	96	15,806	2	0	3	97
				LCCO	10,781	1	0	3	66	11,614	1	0	3	71	11,761	2	0	3	72
				RCCO	10,781	1	0	3	66	11,614	1	0	3	71	11,761	2	0	3	72
				T1S	19,227	3	0	3	105	20,712	3	0	3	113	20,975	3	0	3	115
				T2S	19,206	3	0	3	105	20,690	3	0	3	113	20,952	3	0	3	114
				T2M	19,305	3	0	3	105	20,797	3	0	3	114	21,060	3	0	3	115
				T3S	18,696	3	0	3	102	20,141	3	0	3	110	20,396	3	0	4	111
				T3M	19,258	3	0	3	105	20,746	3	0	3	113	21,009	3	0	3	115
				T4M	18,840	3	0	4	103	20,296	3	0	4	111	20,553	3	0	4	112
40	1400	P7	183W	TFTM	19,246	3	0	4	105	20,734	3	0	4	113	20,996	3	0	4	115
	1.00		10311	T5VS	20,017	4	0	1	109	21,564	4	0	1	118	21,837	4	0	1	119
				T5S	20,033	4	0	2	109	21,581	4	0	2	118	21,854	4	0	2	119
				T5M	19,983	4	0	2	109	21,527	5	0	3	118	21,799	5	0	3	119
				T5W	19,852	5	0	3	108	21,386	5	0	3	117	21,656	5	0	3	118
				BLC LCCO	15,780 11,742	2	0	3	86 64	16,999 12,649	2	0	3	93 69	17,214 12,809	2	0	3	94 70
				RCCO	11,742	2	0	3	64	12,649	2	0	3	69	12,809	2	0	3	70
				T1S	22,490	3	0	3	109	24,228	3	0	3	117	24,535	3	0	3	119
				T2S	22,466	3	0	4	109	24,202	3	0	4	117	24,509	3	0	4	118
				T2M	22,582	3	0	3	109	24,327	3	0	3	118	24,635	3	0	3	119
				T3S	21,870	3	0	4	106	23,560	3	0	4	114	23,858	3	0	4	115
				T3M	22,527	3	0	4	109	24,268	3	0	4	117	24,575	3	0	4	119
				T4M	22,038	3	0	4	106	23,741	3	0	4	115	24,041	3	0	4	116
60	1050	P8	207W	TFTM	22,513	3	0	4	109	24,253	3	0	4	117	24,560	3	0	4	119
00	1030	го	207 W	T5VS	23,415	5	0	1	113	25,224	5	0	1	122	25,543	5	0	1	123
				TSS	23,434	4	0	2	113	25,244	4	0	2	122	25,564	4	0	2	123
				T5M	23,374	5	0	3	113	25,181	5	0	3	122	25,499	5	0	3	123
				T5W	23,221	5	0	4	112	25,016	5	0	4	121	25,332	5	0	4	122
				BLC	18,458	2	0	3	89	19,885	2	0	3	96	20,136	2	0	3	97
				LCC0	13,735	2	0	3	66	14,796	2	0	4	71	14,983	2	0	4	72
				RCCO	13,735	2	0	3	66	14,796	2	0	4	71	14,983	2	0	4	72
				T1S T2S	25,575	3	0	3	106 106	27,551 27,522	3	0	3	114 114	27,900	3	0	3	116 116
				T2M	25,548 25,680	3	0	3	106	27,522	3	0	3	115	27,871 28,014	3	0	3	116
				T3S	24,870	3	0	4	103	26,791	3	0	4	111	27,130	3	0	4	113
				T3M	25,617	3	0	4	106	27,597	3	0	4	115	27,130	3	0	4	116
				T4M	25,061	3	0	4	104	26,997	3	0	4	112	27,339	3	0	4	113
,	42		24	TFTM	25,602	3	0	4	106	27,580	3	0	4	114	27,929	3	0	4	116
60	1250	P9	241W	T5VS	26,626	5	0	1	110	28,684	5	0	1	119	29,047	5	0	1	121
				T5S	26,648	4	0	2	111	28,707	5	0	2	119	29,070	5	0	2	121
				T5M	26,581	5	0	3	110	28,635	5	0	3	119	28,997	5	0	3	120
				T5W	26,406	5	0	4	110	28,447	5	0	4	118	28,807	5	0	4	120
				BLC	20,990	2	0	3	87	22,612	2	0	3	94	22,898	2	0	3	95
				LCC0	15,619	2	0	4	65	16,825	2	0	4	70	17,038	2	0	4	71
				RCCO	15,619	2	0	4	65	16,825	2	0	4	70	17,038	2	0	4	71



Lumen Output

Lumen values are from photometric tests performed in accordance with IESNA LM-79-08. Data is considered to be representative of the configurations shown, within the tolerances allowed by Lighting Facts. Contact factory for performance data on any configurations not shown here.

Rotated Op	otics																												
LED Count	Drive	Power	System	Dist.			30K K, 70 CRI)				40K K, 70 CRI					50K K, 70 CRI												
	Current	Package	Watts	Type	Lumens	В	U	G	LPW	Lumens	В	U	G	LPW	Lumens	В	U	G	LPW										
				T1S	13,042	3	0	3	123	14,050	3	0	3	133	14,228	3	0	3	134										
				T2S	12,967	4	0	4	122	13,969	4	0	4	132	14,146	4	0	4	133										
				T2M	13,201	3	0	3	125	14,221	3	0	3	134	14,401	3	0	3	136										
				T3S	12,766	4	0	4	120	13,752	4	0	4	130	13,926	4	0	4	131										
				T3M	13,193	4	0	4	124	14,213	4	0	4	134	14,393	4	0	4	136										
				T4M	12,944	4	0	4	122	13,945	4	0	4	132	14,121	4	0	4	133										
60	530	P10	106W	TFTM	13,279	4	0	4	125	14,305	4	0	4	135	14,486	4	0	4	137										
				TSVS	13,372	3	0	1	126	14,405	4	0	1	136	14,588	4	0	1	138										
				T5S T5M	13,260 13,256	3	0	2	125 125	14,284 14,281	3	0	2	135	14,465	3	0	2	136 136										
				T5W	13,236	4	0	3	125	14,281	4	0	3	135	14,462 14,332	4	0	3	135										
				BLC	10,906	3	0	3	103	11,749	3	0	3	111	11,898	3	0	3	112										
				LCCO	7,789	1	0	3	73	8,391	1	0	3	79	8,497	1	0	3	80										
				RCCO	7,779	4	0	4	73	8,380	4	0	4	79	8,486	4	0	4	80										
				T1S	16,556	3	0	3	121	17,835	3	0	3	130	18,061	4	0	4	132										
				T2S	16,461	4	0	4	120	17,733	4	0	4	129	17,957	4	0	4	131										
				T2M	16,758	4	0	4	122	18,053	4	0	4	132	18,281	4	0	4	133										
				T3S	16,205	4	0	4	118	17,457	4	0	4	127	17,678	4	0	4	129										
				T3M	16,748	4	0	4	122	18,042	4	0	4	132	18,271	4	0	4	133										
				T4M	16,432	4	0	4	120	17,702	4	0	4	129	17,926	4	0	4	131										
60	700	P11	137W	TFTM	16,857	4	0	4	123	18,159	4	0	4	133	18,389	4	0	4	134										
00	700	FII	13/W	T5VS	16,975	4	0	1	124	18,287	4	0	1	133	18,518	4	0	1	135										
				T5S	16,832	4	0	1	123	18,133	4	0	2	132	18,362	4	0	2	134										
				T5M	16,828	4	0	2	123	18,128	4	0	2	132	18,358	4	0	2	134										
				T5W	16,677	4	0	3	122	17,966	5	0	3	131	18,193	5	0	3	133										
				BLC	13,845	3	0	3	101	14,915	3	0	3	109	15,103	3	0	3	110										
				LCC0	9,888	1	0	3	72	10,652	2	0	3	78	10,787	2	0	3	79										
				RCCO	9,875	4	0	4	72	10,638	4	0	4	78	10,773	4	0	4	79										
				T1S T2S	22,996 22,864	4	0	4	111 110	24,773 24,631	5	0	- 4 - 5	120 119	25,087 24,943	5	0	5	121 120										
														T2M	23,277	4	0	4	112	25,075	4	0	4	121	25,393	4	0	4	120
				T3S	22,509	4	0	4	109	24,248	5	0	5	117	24,555	5	0	5	119										
				T3M	23,263	4	0	4	112	25.061	4	0	4	121	25,378	4	0	4	123										
				T4M	22,824	5	0	5	110	24,588	5	0	5	119	24,899	5	0	5	120										
				TFTM	23,414	5	0	5	113	25,223	5	0	5	122	25,543	5	0	5	123										
60	1050	P12	207W	T5VS	23,579	5	0	1	114	25,401	5	0	1	123	25,722	5	0	1	124										
				TSS	23,380	4	0	2	113	25,187	4	0	2	122	25,506	4	0	2	123										
				T5M	23,374	5	0	3	113	25,181	5	0	3	122	25,499	5	0	3	123										
				T5W	23,165	5	0	4	112	24,955	5	0	4	121	25,271	5	0	4	122										
				BLC	19,231	4	0	4	93	20,717	4	0	4	100	20,979	4	0	4	101										
				LCC0	13,734	2	0	3	66	14,796	2	0	4	71	14,983	2	0	4	72										
				RCCO	13,716	4	0	4	66	14,776	4	0	4	71	14,963	4	0	4	72										
				T1S	25,400	4	0	4	110	27,363	4	0	4	118	27,709	4	0	4	120										
				T2S	25,254	5	0	5	109	27,205	5	0	5	118	27,550	5	0	5	119										
				T2M	25,710	4	0	4	111	27,696	4	0	4	120	28,047	4	0	4	121										
				T3S	24,862	5	0	5	108	26,783	5	0	5	116	27,122	5	0	5	117										
				T3M T4M	25,695	5	0	5	111	27,680	5	0	5	120 118	28,031	5	0	5	121 119										
				TFTM	25,210 25,861	5	0	5	112	27,158 27,860	5	0	5	121	27,502 28,212	5	0	5	122										
60	1250	P13	231W	T5VS	26,043	5	0	1	113	28,056	5	0	1	121	28,411	5	0	1	123										
				TSS	25,824	4	0	2	112	27,819	5	0	2	120	28,172	5	0	2	123										
				T5M	25,818	5	0	3	112	27,813	5	0	3	120	28,165	5	0	3	122										
				T5W	25,586	5	0	4	111	27,563	5	0	4	119	27,912	5	0	4	121										
				BLC	21,241	4	0	4	92	22,882	4	0	4	99	23,172	4	0	4	100										
				LCCO	15,170	2	0	4	66	16,342	2	0	4	71	16,549	2	0	4	72										
				RCCO	15,150	5	0	5	66	16,321	5	0	5	71	16,527	5	0	5	72										



4+ Capable Luminaire

This item is an A+ capable luminaire, which has been designed and tested to provide consistent color appearance and system-level interoperability.

- All configurations of this luminaire meet the Acuity Brands' specification for chromatic consistency
- This luminaire is A+ Certified when ordered with DTL® controls marked by a shaded background. DTL DLL equipped luminaires meet the A+ specification for luminaire to photocontrol interoperability1
- This luminaire is part of an A+ Certified solution for ROAM® or XPoint™ Wireless control networks, providing out-of-the-box control compatibility with simple commissioning, when ordered with drivers and control options marked by a shaded background¹

To learn more about A+,

visit www.acuitybrands.com/aplus.

- 1. See ordering tree for details.
- A+ Certified Solutions for ROAM require the order of one ROAM node per luminaire. Sold Separately: Link to Roam; Link to DTL DLL

FEATURES & SPECIFICATIONS

INTENDED USE

The sleek design of the D-Series Size 1 reflects the embedded high performance LED technology. It is ideal for many commercial and municipal applications, such as parking lots, plazas, campuses, and streetscapes.

CONSTRUCTION

Single-piece die-cast aluminum housing has integral heat sink fins to optimize thermal management through conductive and convective cooling. Modular design allows for ease of maintenance and future light engine upgrades. The LED drivers are mounted in direct contact with the casting to promote low operating temperature and long life. Housing is completely sealed against moisture and environmental contaminants (IP65). Low EPA (1.01 ft²) for optimized pole wind loading.

FINISH

Exterior parts are protected by a zinc-infused Super Durable TGIC thermoset powder coat finish that provides superior resistance to corrosion and weathering. A tightly controlled multi-stage process ensures a minimum 3 mils thickness for a finish that can withstand extreme climate changes without cracking or peeling. Available in both textured and non-textured finishes.

OPTICS

Precision-molded proprietary acrylic lenses are engineered for superior area lighting distribution, uniformity, and pole spacing. Light engines are available in standard 3000 K, 4000 K and 5000 K (70 CRI) configurations. The D-Series Size 1 has zero uplight and qualifies as a Nighttime Friendly™ product, meaning it is consistent with the LEED® and Green Globes™ criteria for eliminating wasteful uplight.

ELECTRICAL

Light engine configurations consist of high-efficacy LEDs mounted to metal-core circuit boards to maximize heat dissipation and promote long life (up to L85/100,000 hours at 25°C). Class 1 electronic drivers are designed to have a power factor >90%, THD <20%, and an expected life of 100,000 hours with <1% failure rate. Easily serviceable 10kV surge protection device meets a minimum Category C Low operation (per ANSI/IEEE C62.41.2).

STANDARD CONTROLS

The DSX1 LED area luminaire has a number of control options. Dusk to dawn controls can be utilized via optional NEMA twist-lock photocell receptacles. Integrated motion sensors with on-board photocells feature field-adjustable programing and are suitable for mounting heights up to 30 feet.

nLIGHT AIR CONTROLS

The DSX1 LED area luminaire is also available with nLight® AIR for the ultimate in wireless control. This powerful controls platform provides out-of-the-box basic motion sensing and photocontrol functionality and is suitable for mounting heights up to 40 feet. Once commissioned using a smartphone and the easy-to-use CLAIRITY app, nLight AIR equipped luminaries can be grouped, resulting in motion sensor and photocell group response without the need for additional equipment. Scheduled dimming with motion sensor over-ride can be achieved when used with the nLight Eclypse. Additional information about nLight Air can be found here.

INSTALLATION

Included mounting block and integral arm facilitate quick and easy installation. Stainless steel bolts fasten the mounting block securely to poles and walls, enabling the D-Series Size 1 to withstand up to a 3.0 G vibration load rating per ANSI C136.31. The D-Series Size 1 utilizes the AERIS™ series pole drilling pattern (template #8). NEMA photocontrol receptacle are also available.

LISTINGS

UL Listed for wet locations. Light engines are IP66 rated; luminaire is IP65 rated. Rated for -40°C minimum ambient. U.S. Patent No. D672,492 S. International patent pending.

 $\label{lem:decomposition} Design Lights Consortium @ (DLC) \ Premium \ qualified \ product \ and \ DLC \ qualified \ product.$

Not all versions of this product may be DLC Premium qualified or DLC qualified. Please check the DLC Qualified Products List at www.designlightsorg/QPL to confirm which versions are qualified.

International Dark-Sky Association (IDA) Fixture Seal of Approval (FSA) is available for all products on this page utilizing 3000K color temperature only.

WARRANTY

 $\hbox{\it 5-year limited warranty. Complete warranty terms located at:} \\ www.acuitybrands.com/CustomerResources/Terms_and_conditions.asp$

Note: Actual performance may differ as a result of end-user environment and application.

All values are design or typical values, measured under laboratory conditions at 25 $^{\circ}$ C.

Specifications subject to change without notice.



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COMMERCIAL OUTDOOR

LED bollard - 180° distribution

S2 Fixture

BEGA

Application

LED bollard with shielded 180° light distribution. This luminaire is designed to provide one sided illumination of ground surfaces. Provided with mounting system that allows the luminaire to be adjusted independent of anchor bolt orientation.

Materials

Luminaire housing and base constructed of die-cast and extruded marine grade, copper free (\leq 0.3% copper content) A360.0 aluminum alloy Borosilicate glass lens

Reflector made of pure anodized aluminum High temperature silicone gasket

Mechanically captive stainless steel fastener

NRTL listed to North American Standards, suitable for wet locations Protection class IP 65

Weight: 9.7 lbs

Electrical

Operating voltage 120-277VAC Minimum start temperature -20° C 7.2 W LED module wattage System wattage 10.0 W Controllability 0-10V dimmable Ra > 80 411 lumens (3000K) Color rendering index Luminaire lumens Lifetime at Ta = 15° C 77,000 h (L70) Lifetime at Ta = 55° C 53,000 h (L70)

LED color temperature

4000K - Product number + **K4** 3500K - Product number + **K35** 3000K - Product number + **K3** 2700K - Product number + **K27**

BEGA can supply you with suitable LED replacement modules for up to 20 years after the purchase of LED luminaires - see website for details

Finish

All BEGA standard finishes are matte, textured polyester powder coat with minimum 3 mil thickness.

Available colors Black (BLK) White (WHT) RAL: Bronze (BRZ) Silver (SLV) CUS:

H. Sandy Health Clinic Type: - Exterior Light Fixture BEG4 Cut-Sheets

Project: Modified:





 LED
 A
 B
 Anchorage

 77752
 7.2W
 6½
 8%
 79817

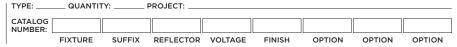
BEGA 1000 BEGA Way, Carpinteria, CA 93013 (805) 684-0533 info@bega-us.com

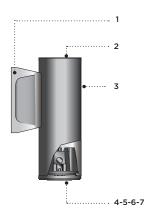
Due to the dynamic nature of lighting products and the associated technologies, luminaire data on this sheet is subject to change at the discretion of BEGA North America. For the most current technical data, please refer to bega-us.com © copyright BEGA 2018



SY300 SERIES SYRIOS - LED

3" WALL UP OR DOWN LIGHT





- Cast aluminum driver housing, includes galvanized steel wall mount pressure plate.
- 2- Cast aluminum top cover.
- 3- Extruded aluminum cylindrical housing.
- 4- Fully sealed cast aluminum light assembly.
- 5- Sealed cast aluminum lens frame.
- 6- Clear tempered glass lens.
- 7- Faceted specular aluminum reflector.

All stainless steel hardware.



MATERIALS

Syrios LED is made of corrosion resistant 356 aluminum alloy with a copper (CU) content of less than 0.1%.

The main housing is made of seamless extruded aluminum, with an integrally sealed LED light module designed for optimal heat dissipation, and lighting performance.

Syrios LED SY300 series is standard with 29° optic. See options section for alternate selection.

ELECTRICAL

DRIVER Standard driver is 0-10V dimming-ready (dims to 10%) with: 120-277 multi-volt compatibility (50-60Hz), operating

temperature range of -30°C/-22°F to 55°C/131°F, output over voltage protection, output over current protection and output short circuit protection with auto-recovery.

LED Standard 4000K /80CRI. Optional 2700K, 3000K & 3500K.
Optional Amber LED for turtle sensitive areas.
Wavelengths: 584.5nm to 597nm.

LIFE

60,000hrs $\rm L_{70}B_{50}$ (based on IESNA TM-21 Test Method and LM-80 data). Up to 95,000hrs $\rm L_{70}B_{50}$ (calculated projection from LM-80 data).

FINISH

Five-stage preparation process including preheating of cast aluminum parts for air extraction, and an environmentally friendly alloy sealant. Polyester powder coating is applied through an electrostatic process and oven cured for long term finish.

MOUNTING

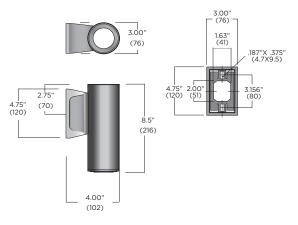
Maximum weight: 2.5lbs (1.1kg)

The mounting plate is designed to fit on a 2X4" (51x102) rectangular electrical box using 3.156" (80) C/C mounting holes.

Optional trimming plate for octagonal jbox (option MT4).

CERTIFICATION

Tested to UL1598 and CSA 22.2 #250. ETL listed wet location. Rated IP66. CE Certification on request.



Una 2019 Bayo

LUMINIS | Toll free: 866.586.4647 Fax: 514.683.8872 Email: info@luminis.com 260 Labrosse, Pointe-Claire (QC) Canada H9R 5L5

Luminaires may be altered for design improvement or discontinued without prior notice.

LUMINIS.COM

SY300 SERIES **SYRIOS - LED ADA Compliant**

LUMINAIRE SELECTION

MODEL#	LED LIGHT	SELECTION				REFLECTORS*	VOLTAGE	FINISH
	SUFFIX	INPUT WATTS	DELIVERED LUMENS	CRI	CCT ºK			STANDARD COLORS
□ SY300	□ L1L10 AMBER L □ L1L1K2	13W ED IDA - Dark Sky 2A 7W	924 / Approved 124	80 AMBER	4000	R30 Flood optics 29 ⁸ (standard) R40 Wide flood optics *Depending on direction fixture is installed add U (for uplight) or D (f downlight) to suffix for all reflector listed above. (i.e. R40U is 42 ⁹ upligh	r	□ WHT Snow white □ BKT Jet black □ BZT Bronze □ MST Matte silver □ GRT Titanium gray □ DGT Gun metal □ CHT Champagne (Refer to color chart)
								OPTIONAL COLORS
	VERY NA □ L1L5N	RROW DISTRIBU R 10W	TION 539	80	4000	☐ R9 Very narrow optic Field angle 21° (12,018 candela)	. 3 5	CS Custom color RAL RAL# color NATATORIUM SUITED COLORS NWHT White NBKT Black

OPTIONS

ELECTRICAL		LIGHT & OF	PTICS
☐ FS ☐ REML2-50	Fuse 7W remote emergency battery backup for LED, 90 min. Remote mount 50ft - 12" (305) square enclosure with access cover'	Alternate □ K27 □ K3 □ K35	CCT ⁹ K LED (LCF: Lumen conversion factor) 2700K CCT 80 CRI (LCF: 0.91) ³ IDA - Dark Sky Approved 3000K CCT 80 CRI (LCF: 0.94) IDA - Dark Sky Approved 3500K CCT 80 CRI (LCF: 0.983) ³
MOUNTING		NOTE: O	ther CCT & higher CRI available, please consult factory.
□ SWK □ MT4	Adaptor box for surface 3/4" conduit feed Trimming plate for octagonal box		
ACCESSORIES			
□ SL □ LSL □ SNT □ HL	Solite lens Linear spread lens 1.5" (38) snoot ² Hexcell louver		

- The remote enclosure must be interior.
 To prevent reflections, interior painted black when a light color finish is selected (ex. WHT, MST, GRT and CHT).
 Please consult factory when selecting K27 or K35 in conjuction with R9 (very narrow optics).

LUMINIS_®

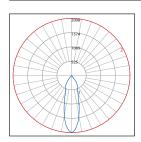
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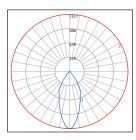
SY300 SERIES SYRIOS - LED ADA Compliant

TYPICAL PHOTOMETRY SUMMARY



Descriptive Information

SY300-L1L10-R30 Total Lms: 924 Lumens Total Input Watts: 13 W Efficacy: 74 Lumens/Watt BUG: B1-U0-G0 CCT/CRI: 4000K/80 Maximum Candela: 2098 @ 0 deg



Descriptive Information

SY300-L1L10-R40
Total Lms: 846 Lumens
Total Input Watts: 13 W
Efficacy: 68 Lumens/Watt
BUG: B1-U0-G0
CCT/CRI: 4000K/80
Maximum Candela: 1317 @ 0 deg

Descriptive Information



SY300-L1L5NR-R9
Total Lms: 539 Lumens
Total Input Watts: 10 W
Efficacy: 53 Lumens/Watt
BUG: B1-U0-G0
CCT/CRI: 4000K/80
Maximum Candela: 12018 @ 0 deg

Please visit our web site www.luminis.com for complete I.E.S. formatted download data.



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Luminaires may be altered for design improvement or discontinued without prior notice.

3

H. Sandy Health Clinic - Exterior Light Fixture **Cut-Sheets**



Catalog Number

Gotham Architectural Downlighting LED Downlights





Solid-State Lighting



OPTICAL SYSTEM

- Relf-flanged semi-specular, matte-diffuse or finishing trim Patented Bounding Ray™ optical design (U.S. Patent No. 5,800,050)
- 45° cutoff to source and source image
- Top-down flash characteristic Polycarbonate lens integral to light engine

MECHANICAL SYSTEM

- Heavy-gauge aluminum construction
- Ceiling mount and wall mount for direct installation to 4" octagonal or square junction box
- Pendant mount entry for 3/8" National Pipe Thread stem; wires supplied by others
- Unique mounting mechanism at top of cylinders for easy one-person installation EDXB driver includes 3-foot DMX signal cable when ordered with FCM or WM
- mounting option. Fixture includes 10-foot DMX signal cable when ordered with PM or ACC mounting option.
- ACC180 provided with 15' 5-wire cord for power and 0-10V dimming

ELECTRICAL SYSTEM

- Fully serviceable and upgradeable LED light engine 70% lumen maintenance at 60,000 hours
- Tested according LM-79 and LM-80 standards
- 2.5 SDCM; 85 CRI typical, 90+ CRI optional
- Overload and short circuit protected
- Dimming wires supplied by others

LISTINGS

Fixtures are CSA certified to meet US and Canadian standards; wet location, covered ceiling. ENERGY STAR® certified product.

WARRANTY

5-year limited warranty. Complete warranty terms located at: www.acuitybrands.com/CustomerResources/Terms_and_conditions.aspx

Note: Actual performance may differ as a result of end user environment and application. All values are design or typical values, measured under laboratory conditions at



EXAMPLE: EVO CYL 35/10 6AR MWD LSS MVOLT EZ1 FCM DWHG

Series	Туре	Colo temp	r perature		ninal en values			Aperture/ Trim colo		Trim type	е	Distri	bution ²	Finis	sh
EVO	CYL	27/ 30/ 35/ 40/ 50/	2700 K 3000 K 3500 K 4000 K 5000 K	10 15 20 25 30	1000 lumens 1500 lumens 2000 lumens 2500 lumens 3000 lumens	35 40 45	3500 lumens 4000 lumens 4500 lumens	6AR 6PR 6WTR 6GR 6WR ¹ 6BR ¹ 6WRAMF ¹	Clear Pewter Wheat Gold White Black White anti- microbial	(blank) W	Downlight Wallwash	ND MD MWD WWD	Very narrow (0.5 s/mh) Narrow (0.7 s/mh) Medium (0.9 s/mh) Medium wide (1.0 s/mh) Wide (1.2 s/mh)	LSS LD LS	Semi- specular Matte- diffuse Specular

Voltage	Driver ³		Mountin	g	Options		
MVOLT 120 277	GZ10 GZ1 EZ10 EZ1 EZB EDAB4 EDXB4 EXA15 EXAB5 ECOS246	0-10V driver dims to 10% 0-10V driver dims to 1% eldoLED 0-10V ECOdrive. Linear dimming to 10% min. eldoLED 0-10V ECOdrive. Linear dimming to 1% min. eldoLED 0-10V SOLOdrive. Linear dimming to 1% min. eldoLED 0-10V SOLOdrive. Logarithmic dimming to <1%. eldoLED SOLOdrive DALI. Logarithmic dimming to <1%. eldoLED POWERdrive DMX with RDM (remote device management). Square Law dimming to <1%. Includes termination resistor. Refer to DMXR Manual. XPoint Wireless, eldoLED 0-10V ECOdrive. Linear dimming to 1%. Refer to XPoint tech sheet. XPoint Wireless, eldoLED 0-10V SOLOdrive. Logarithmic dimming to <1%. Refer to XPoint tech sheet. Lutron® Hi-Lume® 2-wire forward-phase driver. 120V only. Minimum dimming level 1%. Minimum lumen 1000/Maximum lumen 3000. Lutron® Hi-Lume® 3-wire or EcoSystem® dimming driver. Minimum dimming level 1%. Minimum lumen 1000/Maximum lumen 4500.	FCM WM7 PM ACC®	Ceiling mount Wall mount Pendant 3/8" thread mount 10ft aircraft cable and cord mount 15ft aircraft cable and cord mount	SF CRI90 NPP16D ⁹ NPP16DER ⁹ NPS80EZ ^{9,10} NPS80EZER ^{9,10}	Single fuse. Specify voltage. High CRI (90+) nLight® network power/relay pack with 0-10V dimming for non-eldoLED drivers (GZ10, GZ1). nLight® network power/relay pack with 0-10V dimming for non-eldoLED drivers (GZ10, GZ1). ER controls fixtures on emergency circuit. Sensor Switch® nLight® dimming pack controls 0-10V eldoLED drivers (EZ_). Sensor Switch® nLight® dimming pack controls 0-10V eldoLED drivers. ER controls fixtures on emergency circuit operation (EZ_).	 ctural Colors - Paint ¹¹ Matte white (standard) Dark bronze Black Gloss white Medium bronze Natural aluminum Sandstone Charcoal grey Tennis green Bright red Steel blue

EVO-CYL-6 PAGE 1 OF 4 GOTHAM ARCHITECTURAL DOWNLIGHTING | P 800.315.4982 | gothamlighting.com

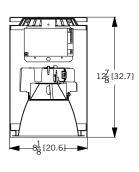
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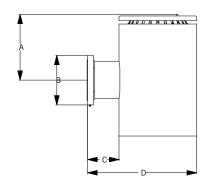


FEATURES

DIMENSIONAL DATA

All dimensions are inches (centimeters) unless otherwise noted.





Reflector aperture: 6-1/4 (15.9) Housing diameter: 8-1/8 (20.3) Wall mount dimensions

A = 7-3/4 (19.7)

B = 5-5/16 (13.5)C = 3-1/4 (8.3)

D = 10-7/8 (27.6)

ELECTRICAL

WATTAGE CONSUMPTION MATRIX											
LUMENS	LM ACTUAL	WATTAGE	LUMENS per WATT								
1000	1,059	11.8	90.1								
1500	1,572	18.5	85.0								
2000	2,058	23.2	88.9								
2500	2,612	29.5	88.5								
3000	3,077	36.6	84.1								
3500	3,591	42.1	85.3								
4000	4,046	48.1	84.2								
4500	4,555	46.9	97.1								

ACCESSORIES

NOTES

ACCESSORIES order as separate catalog numbers (shipped separately)

CYS12 3/8" stem and canopy with 5° "hang straight" swivel CRS¹²

3/8" stem and canopy with 45° swivel

CYSX12 $3/8\mbox{"}$ stem and canopy with $5\mbox{°}$ "hang straight" swivel. Use this nomenclature

when ordering EDXB driver

CRSX¹²

3/8" stem and canopy with 45° swivel. Use this nomenclature when

ordering EDXB driver

SDT 347/120 75VA13 347V Step-down transformer

ORDERING NOTES

- Not available with finishes.
- Not available with wallwash trim type. Refer to TECH-240 for compatible dimmers.
- Not available with nLight® and XPoint options.
- XPoint® CMRB ships separately.
- Specify voltage 120V.
- 7. Access panel (supplied by others) recommended for use with nLight® and XPoint®.
- White cord with white housings. All others black cord.

- Specify voltage. For use with generator supply EM power. Will require an emergency hot feed and normal hot feed.
- 10. Interface remote mounted.
- Additional architectural colors available; see www.lithonia.com/archcolors. 11.
- Color and length of stem must be specified (from 6" to 240" in even increments in maximum sections of 48"). Ex.: CYSO6 DWHG. Ceiling attachment for interior use. Consult factory for exterior use. Wire not included.
- 13. Transformer must be field-installed to an accessible remote-mounted junction box.



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10% beam 78.1°

 Diameter
 FC

 8.9
 13.0

 12.2
 7.0

 15.4
 4.4

 18.7
 3.0

 21.9
 2.2

 FC
 Diameter

 56.2
 8.9

 30.2
 12.2

 18.8
 15.4

 12.9
 18.7

10% beam 77.3°

 Diameter
 FC

 8.8
 6.7

 12.0
 3.6

 15.2
 2.2

 18.4
 1.5

 21.6
 1.1

33.4 17.9 11.2 7.6 5.5

5.7 65.1 7.7 35.0 9.8 21.8 11.8 14.9 13.9 10.8

Height 8.0 10.0 12.0 14.0 16.0

Mounting Center

Beam 112.4 60.4 37.7 25.7

8eam 66.7 35.9 22.4 15.3 11.1

8.0 10.0 12.0 14.0 16.0 5.7 7.8 9.9 12.0 14.1

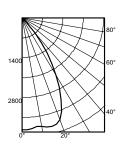
Height 8.0 10.0 12.0 14.0 16.0 Beam 130.1 70.0 43.6



Distribution Curve Distribution Data Output Data Coefficient of Utilization Illuminance: Single Luminaire 30" Above Floor

EVO 35/40 6AR LS

INPUT WATTS: 48.1, DELIVERED LUMENS: 4046, LM/W=84.1 , 1.03 S/MH, TEST NO. LTL27768

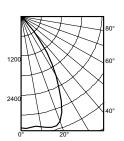


	Ave	Lumens	Zone	Lumens	% Lamp	pw	50%	30%	10%	50%	30%	10%	50%	30%	10%
0	3935		0° - 30°	2904.3	71.8	0	119	119	119	116	116	116	111	111	111
5	3901	371	0° - 40°	3830.0	94.7	1	111	108	106	109	106	104	105	103	101
15	3944	1106	0° - 60°	4043.4	99.9	2	103	99	96	101	98	95	98	95	93
25	3172	1427	0° - 90°	4046.3	100.0	3	96	91	87	95	90	87	92	88	85
35	1508	926	90° - 180°	0.0	0.0	4	90	84	80	89	84	80	87	82	79
45	221	203	0° - 180°	4046.3	*100.0	5	84	78	74	83	78	74	81	77	73
55	6	10	*	Efficiency		6	79	73	69	78	73	68	77	72	68
65	2	2				7	74	68	64	73	68	64	72	67	63
75	1	1				8	70	64	60	69	63	60	68	63	59
85	0	0				9	66	60	56	65	60	56	64	59	56
90	0					10	62	56	52	62	56	52	61	56	52

EVO 35/35 6AR LS

INPUT WATTS: 42.1, DELIVERED LUMENS: 3591, LM/W=85.3, 1.05 S/MH, TEST NO. LTL27767

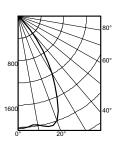
20%



						рс		80%			70%			50%	
	Ave	Lumens	Zone	Lumens	% Lamp	pw	50%	30%	10%	50%	30%	10%	50%	30%	10%
0	3400		0° - 30°	2579.3	71.8	0	119	119	119	116	116	116	111	111	111
5	3390	324	0° - 40°	3399.8	94.7	1	111	108	106	109	106	104	105	103	101
15	3497	981	0° - 60°	3586.3	99.9	2	103	99	96	101	98	95	98	95	93
25	2830	1274	0° - 90°	3590.5	100.0	3	96	91	87	95	90	87	92	88	85
35	1335	820	90° - 180°	0.0	0.0	4	90	84	80	89	84	80	87	82	79
45	193	177	0° - 180°	3590.5	*100.0	5	84	78	74	83	78	74	81	77	73
55	5	9	*	Efficiency		6	79	73	69	78	72	68	77	72	68
65	2	2				7	74	68	64	73	68	64	72	67	63
75	1	1				8	70	64	60	69	63	59	68	63	59
85	1	1				9	66	60	56	65	60	56	64	59	55
90	1					10	62	56	52	62	56	52	61	56	52

EVO 35/20 6AR LS

INPUT WATTS: 23.2, DELIVERED LUMENS: 2058, LM/W=88.7, 1.02 S/MH, TEST NO. LTL27777



						pc		80%			70%			50%	
	Ave	Lumens	Zone	Lumens	% Lamp	pw	50%	30%	10%	50%	30%	10%	50%	30%	10%
0	2018		0° - 30°	1498.5	72.8	0	119	119	119	116	116	116	111	111	111
5	1997	190	0° - 40°	1958.0	95.1	1	111	108	106	109	107	105	105	103	101
15	2053	576	0° - 60°	2056.6	99.9	2	103	99	96	102	98	95	98	95	93
25	1618	733	0° - 90°	2058.3	100.0	3	96	92	88	95	91	87	92	89	86
35	749	459	90° - 180°	0.0	0.0	4	90	85	81	89	84	80	87	83	79
45	105	94	0° - 180°	2058.3	*100.0	5	84	79	74	83	78	74	82	77	73
55	3	5	*	Efficiency		6	79	73	69	78	73	69	77	72	68
65	1	1				7	74	68	64	74	68	64	72	67	64
75	0	0				8	70	64	60	69	64	60	68	63	60
85	0	0				9	66	60	56	66	60	56	65	60	56
90	0					10	62	57	53	62	56	53	61	56	53

LUMEN OUTPUT MULTIPLIER - CRI				
CRI	FACTOR			
80 CRI	1			
90 CRI	0.79			

LUMEN OUTPUT MULTIPLIER - CCT				
CRI	FACTOR			
5000 K	1.101			
4000 K	1.035			
3500 K	1			
3000 K	0.973			
2700 K	0.938			

LUMEN OUTPUT MULTIPLIER - TRIM FINISH								
FINISH	CLEAR (AR)	PEWTER (PR)	WHEAT (WTR)	GOLD (GR)	WHITE (WR/WRAMF)	BLACK (BR)		
Specular (LS)	1.00	0.88	0.83	0.95	N/A	N/A		
Semi-specular (LSS)	0.95	0.84	0.79	0.90	N/A	N/A		
Matte-diffuse (LD)	0.85	0.73	0.69	0.80	N/A	N/A		
Paint	N/A	N/A	N/A	N/A	0.87	0.73		

PHOTOMETRY NOTES

- Tested in accordance with IESNA LM-79-08.
- Tested to current IES and NEMA standards under stabilized laboratory conditions.
- CRI: 85 typical.

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CONTROLS

Solid-State Lighting



Choose Wall Controls.

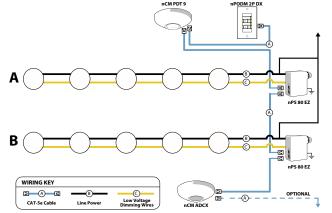
nLIGHT o ers multiple styles of wall controls – each with varying features and user experience.



Push-Button WallPod Traditional tactile buttons and LED user feedback



Graphic WallPod Full color touch screen provides a sophisticated look and feel



EXAMPLE

Group Fixture Control*

*Application diagram applies for fixtures with eldoLED drivers only.

nPS 80 EZ Dimming/Control Pack (qty 2 required)
nPODM 2P DX Dual On/Off/Dim Push-Button WallPod
nCM ADCX Daylight Sensor with Automatic Dimming Control
nCM PDT 9 Dual Technology Occupancy Sensor

Description: This design provides a dual on/off/dim wall station that enables manual control of the fixtures in Row A and Row B separately. Additionally, a daylight harvesting sensor is provided so the lights in row B can be configured to dim automatically when daylight is available. An occupancy sensor turns off all lights when the space is vacant.

nLight® Control Accessories: .com/nLight for complete listing of nLight controls. Order as separate catalog number. Visit v WallPod stations Model number Occupancy sensors Model number nPODM [color] Small motion 360°, ceiling (PIR / dual tech) nCM 9 / nCM PDT 9 nPODM DX [color] nCM 10 / nCM PDT 10 On/Off & Raise/Lower Large motion 360°, ceiling (PIR / dual tech) Graphic Touchscreen nPOD GFX [color] Wide view (PIR / dual tech) nWV 16 / nWV PDT 16 Wall Switch w/ Raise/Lower (PIR / dual tech) Photocell controls nWSX LV DX / nWSX PDT LV DX Model number Dimming nCM ADCX Cat-5 cables (plenum rated) Model number 10', CAT5 10FT CAT5 10FT J1 15', CAT5 15FT CAT5 15FT J1

4 Capable Luminaire

This item is an A+ capable luminaire, which has been designed and tested to provide consistent color appearance and out-of-the-box control compatibility with simple commissioning.

- All configurations of this luminaire meet the Acuity Brands' specification for chromatic consistency
- This luminaire is part of an A+ Certified solution for nLight* control networks when ordered with drivers marked by a shaded background*
- This luminaire is part of an A+ Certified solution for nLight control networks, providing advanced control functionality
 at the luminaire level, when selection includes driver and control options marked by a shaded background*

To learn more about A+, visit $\underline{www.acuitybrands.com/aplus}.$

*See ordering tree for details



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EVO-CYL-6 PAGE 4 OF 4

BULLETTO

Compact Adjustable Flood Light Projector

Concept: Compact projector flood light for high intensity applications.

Materials: Aluminum body and joints for maximum heat dissapation powder coated in Ferrite Grey or Bronze finish. Modular body for toolless maintenance. Lens cover assymbly for simple toolless field interchangability of accessories.

Optics: NSP, SP, FL, MW, and WF use high efficiency LED Chip on Board. Equipped with collimating optic with angle specific holigraphic spread lens filters.

Mounting: Adjustable up to 180° on the vertical plane with aim locking set screw. Brass strain release gland with $\frac{1}{2}$ " NPT thread nipple that can be screwed directly to recieving mounting.

Installation: Pre-cabled with 10' Belden direct burial 18ga 3 Conductor Cable for Connection to remote power supply.

Finish: Ferrite Grey / Bronze

Power Supply: Remote Class 2, 120V-277VAC power supply required, see page 2 for options.

Wattage: 10W (NSP) / 12W (SP/FL/MF/WF)

Color Temperature: 2700°K / 3000°K / 3500°K / 4000°K

CRI: Ra84

Delivered Lumens: 3000°K

Lumen Maintenance (L70): 50,000hrs

Calculation for LED fixtures are based on measurements that comply with IES LM-80.

CRI: Ra84 Voltage: 24V DC IK Rating: IK10 IP Rating: IP66

Certifications: cULus Class 2 Wet Location Listed

Tested in accordance with LM-79-08

^ATitle 24 commercial installation compliant.

Warranty: 5 year limited warranty





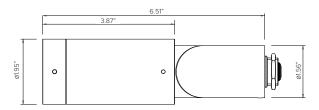






PRODUCT CODE	DRIVER	FINISH	ОИТРИТ	OPTICS	COLOR TEMP	+	POWER SUPPLY
BLT — BULLETTO	RP — Remote Power	FE - Ferrite Grey	L1 — 10W	NS — Narrow Spot 10°	27 — 2700K		See page 4
		BZ — Bronze	L2 — 12W	SP — Spot 15°	30 — 3000K		
				FL — Flood 25°	35 — 3500K		
				MF— Medium Wide Flood 30°	40 — 4000K		
				WF — Wide Flood 43°			

Views



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BULLETTO

INTERNAL OPTICAL ACCESSORIES:					
Maximum of two optical accessories per fixture.					
1E3798 Chromatic filter Red. Dimensions Ø50mm					
1E3799 Chromatic filter Green. Dimensions Ø50mm					
1E3800	Chromatic filter Blue. Dimensions Ø50mm				
1E3801	Chromatic filter Yellow. Dimensions Ø50mm				
1E3802	Chromatic filter Magenta. Dimensions Ø50mm				
1E3790	'Blade of Light' linear spread lens filter. Dimensions Ø50mm				
1E3792	Honeycomb filter. Dimensions Ø50mm				







EXTERNAL OPTICAL ACCESSORIES:				
		Description		
1E3788	1E3806	Asymmetric screen. Powder coated finish.		









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BULLETTO

Tree Mounting Accessories

TREE STRAP (REQUIRED) - CHOOSE 1 Durable 1.5 inch wide Nylon webbing Strap is flexible with High Strength Aircraft Aluminum V-ring Buckle in Gunmetal finish and lock in loop to prevent slipping. The strap is made to coordinate with up to 6 fixtures and 1 wiring connection box Tree strap for trees 39" in circumference or smaller. TTS0101 Olive Green nylon strap with Gunmetal cinch buckle Tree strap for trees 39" in circumference or smaller. TTS0102 Coyote Brown nylon strap with Gunmetal cinch buckle. Tree strap for trees 39" in circumference or smaller. TTS0103 Graphite Grey nylon strap with Gunmetal cinch buckle



Shown with Tree Strap and Fixture Bracket

INSTALLATION STRAP (OPTIONAL)

TREE STRAP CONNECTION BOX (OPTIONAL)

 $\hbox{Durable 1.5 inch wide Nylon webbing Strap is flexible with Metal Cinch Buckle}$ and lock in loop to prevent slipping. The installation strap comes with 4 each 12 inch Rubber Loops used to hold up to 6 fixtures in place while positioning and tightening the Tree Strap in position during Installation or Maintenance and then removed for operation. For use with 2 or more fixtures.

TTIS010	1

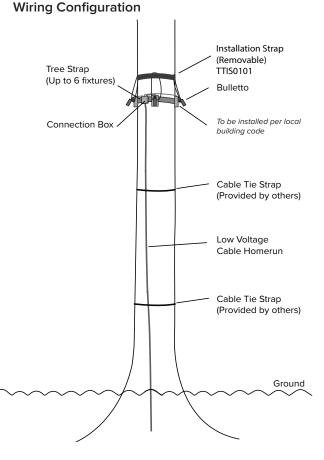
Installation strap flexible nylon webbing strap with metal cinch buckle and lock in loop. Includes 4 each 150mm rubber loops to hold up to 6 fixtures in place while positioning.

FIXTURE BR	FIXTURE BRACKETS (REQUIRED) - CHOOSE 1					
Buckle style Brackets for securing onto Tree Straps.						
		Description				
TTBLT0101	TTBLT0102	Bracket with stainless steel mounting screws.				

FIXTURE BR	FIXTURE BRACKETS (REQUIRED) - CHOOSE 1						
Buckle style B	Buckle style Brackets for securing onto Tree Straps.						
		Description					
TTBLT0101	TTBLT0102	Bracket with stainless steel mounting screws.					

	nection boxes are used to connect all multiple low voltage continuous cable connection down the tree, additional cable hers.						
TTCB0103	Tree Strap Connection Box. Grey ABS plastic with internal terminal block for wire connections and 3 each ½" knockouts for connections to 1 or 4 fixtures. Dimensions: 3½" x 2½" x 1¾".						
TTCB0104	Tree Strap Connection Box. Grey ABS plastic with internal terminal block for wire connections and 4 each ½" knockouts for connections to 5 or 6 fixtures. Dimensions: 4½" x 2½" x 1¾".						

MESH WIRE P	ROTECTION COVER (OPTIONAL)				
A braided polyethylene terepthalate (PET) monofilament yarn resistant to chemical degradation, UV radiation, and abrasion. TTSJS01 Carbon Grey 3/8" SJO Cord Sleeve (Sold per foot)					
TTSJS03	Forest Green 3/8" SJO Cord Sleeve (Sold per foot)				



Adjust the strap every 6 months in order to prevent tree damage.

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Power Supply (REQUIRED)	Туре	Wattage	Input/Output Voltage	Dimmable	IP Rating	Output	Dimensions	
DEL60PWM	ELECTRONIC PWM DRIVER STANDALONE, UL LISTED ENCLOSURE PROVIDED BY OTHERS	60W	120-277V / 24V	0-10V/PWM 10%	IP67	UR CLASS 2	5.9" × 2.09" × 1.38"	
DEL90PWM	ELECTRONIC PWM DRIVER STANDALONE, UL LISTED ENCLOSURE PROVIDED BY OTHERS	90W	120-277V / 24V	0-10V/PWM 10%	IP67	UR CLASS 2	6.73" X 2.48" X 1.48"	
DEDD10010	ELECTRONIC 0-10V DRIVER STANDALONE, UL LISTED ENCLOSURE PROVIDED BY OTHERS	100W	120-277V / 24V	0-10V DIMMING 0.1%	IP20	UR CLASS 2	6.02" × 1.97" × 0.91"	
DELV40124D	ELECTRONIC LOW VOLTAGE TRANSFORMER	40W	120-277V / 24V	PHASE (120V ONLY) /0-10V DIMMING < 1%	Y) /0-10V IP66 / NEMA4		11" × 4" × 2.21"¹	
DELV60124D	ELECTRONIC LOW VOLTAGE TRANSFORMER	60W	120-277V / 24V	PHASE (120V ONLY) /0-10V DIMMING < 1%	IP66 / NEMA4 ENCLOSURE	UL CLASS 2	11" × 4" × 2.21"	
DELV96124D	ELECTRONIC LOW VOLTAGE TRANSFORMER	96W	120-277V / 24V	PHASE (120V ONLY) /0-10V DIMMING < 1%	IP67 / NEMA4 ENCLOSURE	UL CLASS 2	11" × 4" × 2.21"¹	
DELX601241CPWM	ELECTRONIC PWM DRIVER	60W	120-277V / 24V	0-10V/PWM 10%	IP67 / NEMA3R ENCLOSURE	UL CLASS 2	10" × 10" × 4"1	
DELX901241CPWM	ELECTRONIC PWM DRIVER	90W	120-277V / 24V	0-10V/PWM 10%	IP67 / NEMA3R ENCLOSURE	UL CLASS 2	10" × 10" × 4"1	
DELX1802242CPWM	ELECTRONIC PWM DRIVER	2X90W	120-277V / 24V	0-10V/PWM 10%	IP67 / NEMA3R ENCLOSURE	UL CLASS 2	12" x 12" x 4"¹	
DELX2703243CPWM	ELECTRONIC PWM DRIVER	3X90W	120-277V / 24V	0-10V/PWM 10%	IP67 / NEMA3R ENCLOSURE	UL CLASS 2	12" × 12" × 4" ¹	
PS060	LUTRON HI-LUME PREMIER 0.1% CONSTANT VOLTAGE DRIVER WITH UL LISTED ENCLOSURE		UNIVERSAL 120-277 VAC	HI-LUM DIMMABLE 0.1%	IP20/NOM CERTIFIED	UL CLASS 2	10.5" × 5.5" × 2"1	
QOMELED1002410BK	QTRAN QOM-eLED CONSTANT VOLTAGE DRIVER WITH UL LISTED ENCLOSURE	100W	100-277V / 24V	0-10V	IP67	UR CLASS 2	15.4" X 9.22" X 4.90"	
QOMELED2002410BK	QTRAN QOM-eLED CONSTANT VOLTAGE DRIVER WITH UL LISTED ENCLOSURE	2X100W	100-200V / 24V	0-10V	IP67	UR CLASS 2	15.4" X 9.22" X 4.90"	
QOMDRIVE10024VBKDMX	QTRAN QOM-DRIVE-PS CONSTANT VOLTAGE DRIVER WITH UL LISTED ENCLOSURE	100W	100-277V / 24V	DMX CONTROL 4 CHANNEL	IP67	UR CLASS 2	15.4" X 9.22" X 4.90"	
QOMDRIVE20024VBKDMX	QTRAN QOM-DRIVE-PS CONSTANT VOLTAGE DRIVER WITH UL LISTED ENCLOSURE	2X100W	100-200V / 24V	DMX CONTROL 4 CHANNEL	IP67	UR CLASS 2	15.4" X 9.22" X 4.90"	

MAX FIXTURES PER DRIVER

ure	Driver Wattage										
	Fixture <i>N</i> attage		40W	60W	96W	90W	2X90W	3X90W	100W	2X100	
Fixt Wat	10W	3	4	7	7	14	21	8	16		
	12W	2	4	6	6	12	18	6	13		

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¹ Dimensions include enclosure with mounting bracket.

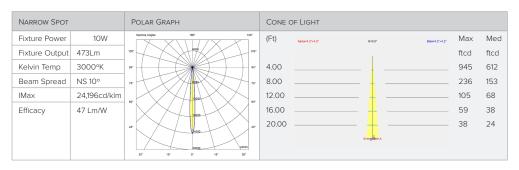
* Constant voltage drivers 50/60HZ, voltage regulated with short circuit protection. Operating temperature -40 C- 80° C

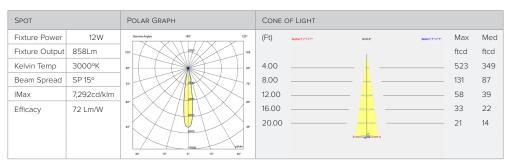
* Installation of power supply must be compliant to Class 2 installation standards. Refer to NEC and local building code requirements.

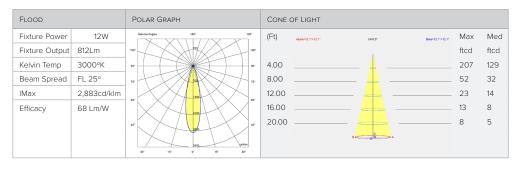
* Consult factory for additional driver options (ie: DMX, DALI, wattage, size, shape, Lutron, ELDO, or others).

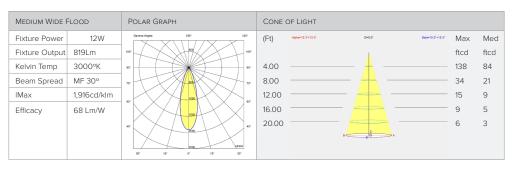
BULLETTO

Photometry





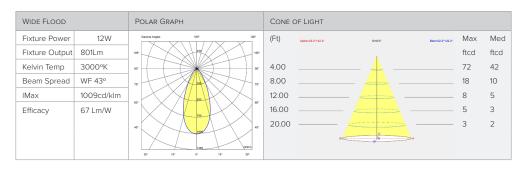




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BULLETTO

Photometry Cont.



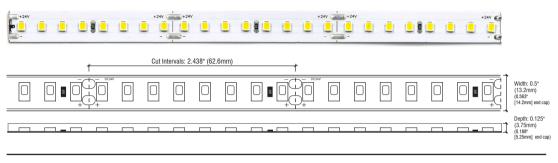
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PERFORMANCE 500 (OUTDOOR)

PQ-SERIES | RUBBER COATED | LINEAR LED LIGHTING

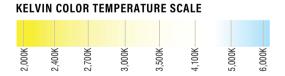
ixture Type:	
Project:	
I ocation:	



MODEL:	PQ27K-WR-24V	PQ3K-WR-24V	PQ35K-WR-24V	PQ41K-WR-24V	PQ5K-WR-24V		
Kelvin	2700K	3000K	3500K	4100K	5000K		
Lumens	508 lm/ft	530 lm/ft	551 lm/ft	572 lm/ft	594 lm/ft		
Rating	IP67	IP67	IP67	IP67	IP67		

PRODUCT FEATURES

- 90+ CRI
- Dimmable
- 50,000 hour life
- 5-year warranty
- · UL-listed for indoor and outdoor use
- . 3M™ Industrial adhesive backing
- For use with 24V power supplies



SPECIFICATIONS

Series	PQ - Performance 500 (Outdoor)
Input Voltage	24V DC / Constant Voltage
Watts per Foot	2.6W/ft @ Maximum Run Length
Beam Spread	120°
Max Run Length	Unlimited, power every 25ft
Production Intervals	2.438" (62.6mm)
End Cap Dimensions	0.563" (14.2mm) × 0.188" (5.25mm)
Tape Dimensions	0.5" (13.2mm) × 0.125" (3.75mm)
CRI	90+
Diode	2835
Dimming Options	PWM, Triac, 0-10V, DMX, Hi-lume
Temp Range	-40°F (-40°C) to 149°F (65°C)

TOTAL WATTAGE USED AT EACH LENGTH

1ft	2ft	3ft	4ft	5ft	6ft	7ft	8ft	9ft	10ft	11ft	12ft	13ft
4.1	7.4	11.3	15.3	19.0	22.5	25.8	28.6	32.3	35.5	37.8	41.0	42.7
14ft	15ft	16ft	17ft	18ft	19ft	20ft	21ft	22ft	23ft	24ft	25ft	26ft
45.0	48.2	50.3	52.4	54.8	56.5	58.8	60.7	61.7	62.5	63.1	63.8	n/a

Conforms to ANSI/UL Standard 2108 Certified to CAN/CSA Standard C22.2 No. 250.0











Questions/Support | 800-789-3810 | quotes@kelvix.com

042219DM

ZEDGE LINE

Professional Small Scale LED Steplight

S7 Fixture

H. Sandy Health Clinic
- Exterior Light Fixture
Cut-Sheets

Concept: Recessed wall mounted LED fixture.

Materials: Die-cast anodized aluminum body and external frame; powder coated frame

Source: LED High Efficiency Board.

Optic: Polycarbonate opal screen. Floor Washer frame allows for uniform

optical distribution on the floor and excellent visual comfort.

Mounting: To be completed with a special nylon outer casing fitted for parallel connection.

 $\mbox{\bf Driver:}$ Integrated 4/1 driver (Non-dimmable / 0-10V / Reverse Phase / Forward Phase). Dimmable to 1%.

Finish: Ferrite Grey

Wattage: 9W

Color Temperature: 2700° K / 3000° K / 3500° K / 4000° K

CRI: Ra84

Delivered Lumens: 2700°K 3000°K 3500°K 4000°K

230Lm 245Lm 251Lm 257Lm

Lumen Maintenance (L70): 50,000hrs

Calculation for LED fixtures are based on measurements that comply with IES LM-80.

Voltage: Universal Voltage 120-277V AC 50/60Hz

IK Rating: IK10
IP Rating: IP66

Certifications: cULus Listed Wet Location

Tested in accordance with LM-79-08

^A Title 24 commercial installation compliant.

^B Consult factory for marine grade cataphoresis treatment.

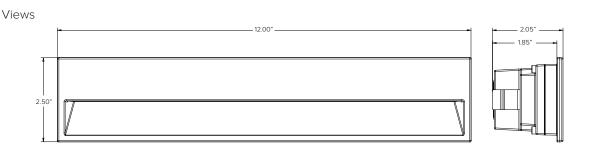
Warranty: 5 year limited warranty

Designed in collaboration with Gensler as Product Design Consultant









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TARGETTI

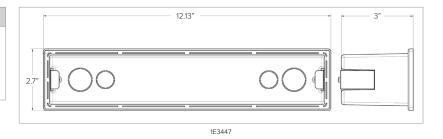
ZEDGE LINE

INSTALLATION (REQUIRED)

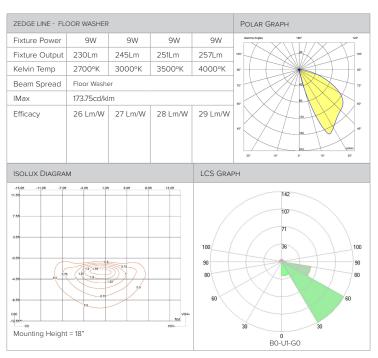
1E3447

Nylon outer casing fitted for parallel connection with feed through-wiring. 34" and ½" knock-outs made for EMT connectors and conduit entry. Suitable for concrete pour, drywall, or stucco applications.

Dimensions: 12.13"W x 2.7"H x 3"D



Photometry



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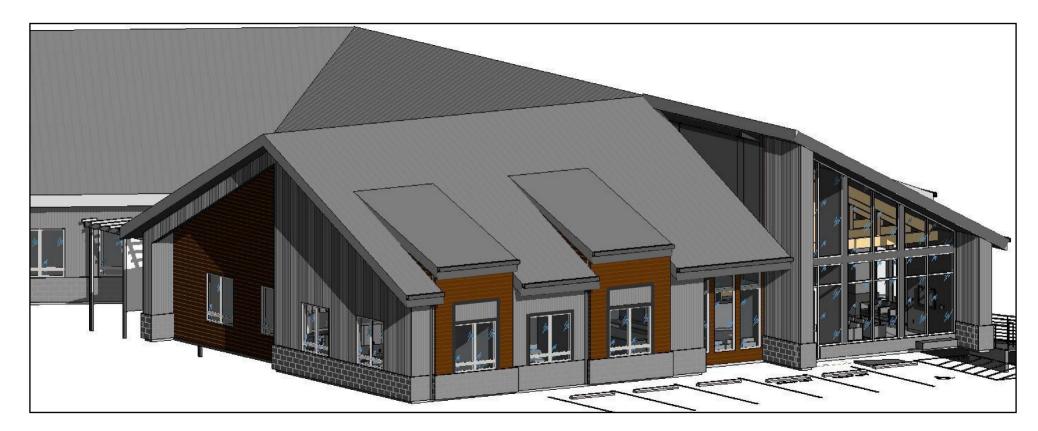


EXHIBIT F

BASE CONCRETE BLOCK VENEER AND TRASH ENCLOSURE BLOCK



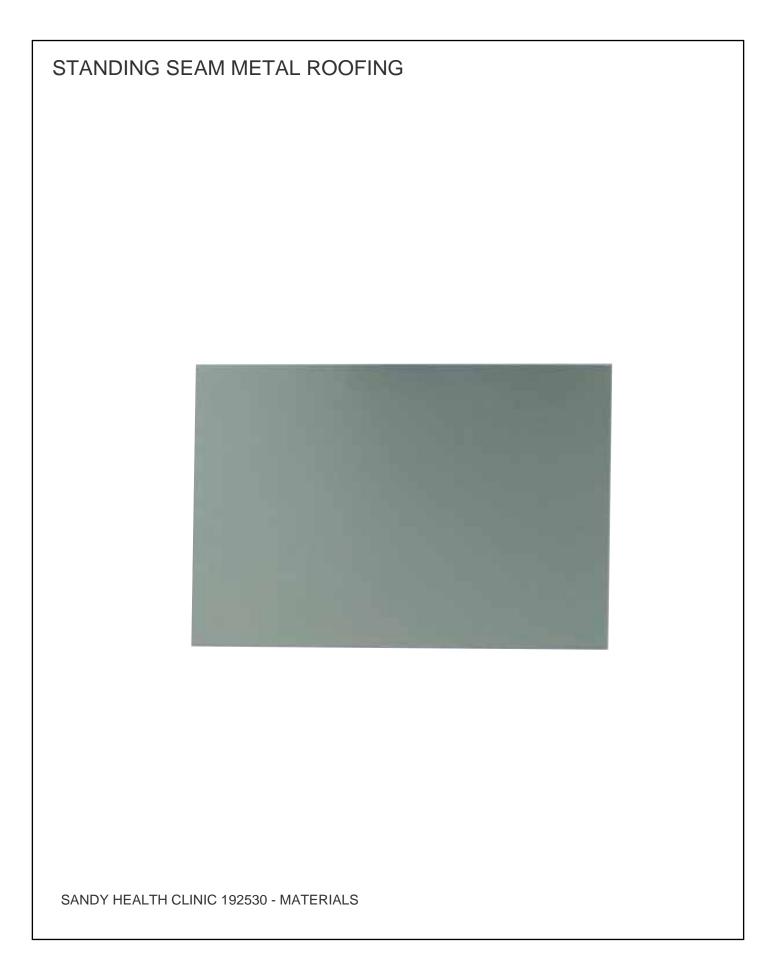
SANDY HEALTH CLINIC 192530 - MATERIALS

HORIZONTAL FIBER-CEMENT SIDING

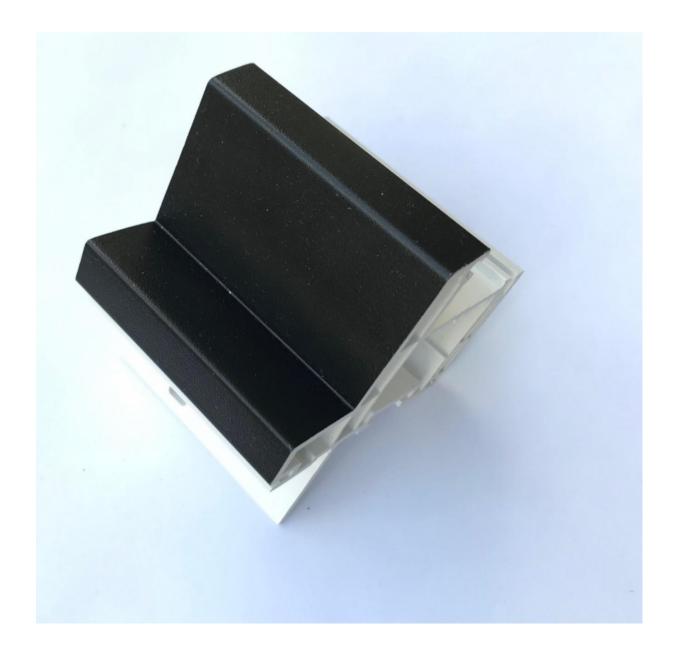


SANDY HEALTH CLINIC 192530 - MATERIALS

VERTICAL FIBER-CEMENT PANEL SIDING PROPOSED MATERIAL WILL BE PAINTED TO MATCH WEATHERED COPPER COLOR SWATCH SHOWN HERE, BUT WILL HAVE VERTICAL TEXTURE AS SHOWN. SANDY HEALTH CLINIC 192530 - MATERIALS



WINDOW COLOR



SANDY HEALTH CLINIC 192530 - MATERIALS

STURGIS GRAY TRIM AND FASCIA PAINT

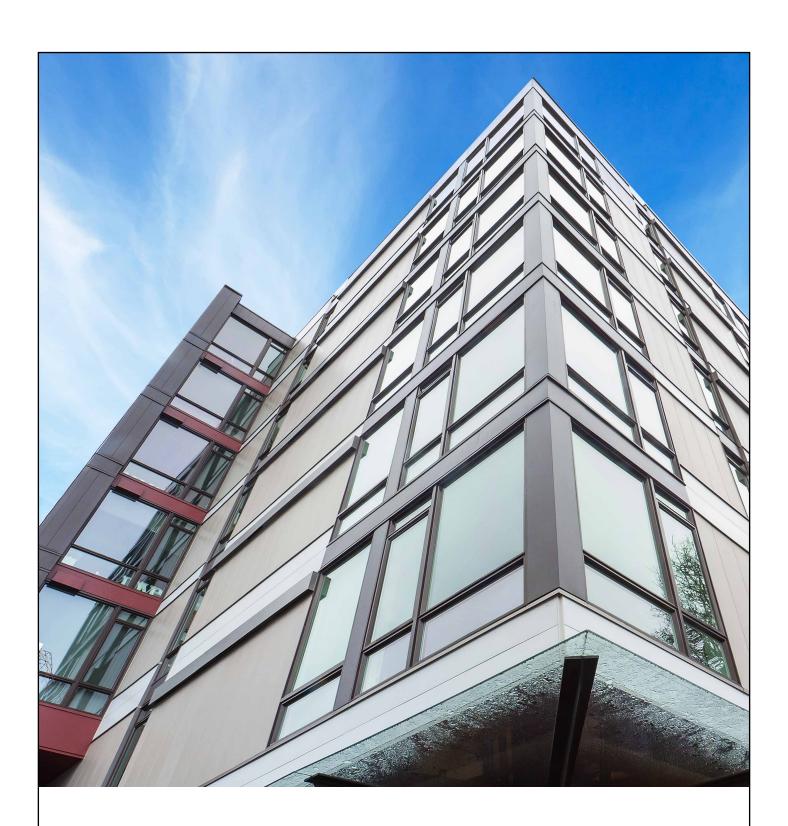


SANDY HEALTH CLINIC 192530 - MATERIALS











ARCHITECTURAL WALL PANELS | AWP3030 | JULY 2019

Vertical Installation Guide

AWP-3030 VERTICAL INSTALLATION GUIDE

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GENERAL

This guide is intended to provide the key information needed to successfully install Nichiha's 3030mm Architectural Wall Panels (AWP3030) in a vertical application. Further installation information and technical resources such as animated instructional videos, Technical Bulletins, three-part specifications, product testing and certifications, architectural details in AutoCAD, Revit, and PDF versions, and other technical documents are available on our website: nichiha.com/resource-center.

Install products in accordance with the latest installation guidelines and all applicable building codes and other laws, rules, regulations, and ordinances. Review all installation instructions and other applicable product documents before installation. This install guide's effective date is July 2019.

PRODUCT INSPECTION

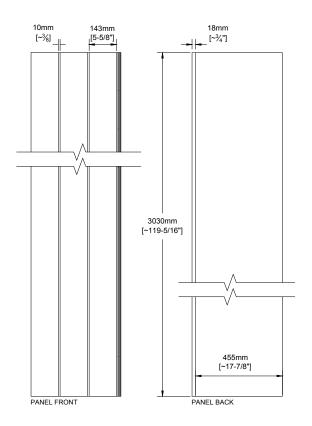
Inspect all products thoroughly prior to installation. Do not install any product which may have been damaged in shipment or appears to have a damaged or irregular finish. Should you have a question or problem with your order, contact your local dealer or Nichiha Customer Service, toll-free, at 1.866.424.4421. Keep the products dry prior to installation. It is best to store the products indoors, otherwise keep them covered. Do not stack pallets more than two high.

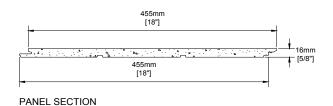
BASICS OF THE AWP3030 SYSTEM

Nichiha AWP3030 dimensions are 455mm (h) \times 3,030mm (l) \times 16mm (t). It is important to keep in mind the actual metric dimensions when considering panel layout, placement of control and compression joints, and with respect to sizing window and door openings. Approximate Imperial dimensions are 17-7/8" (h) \times 119-5/16" (l) \times 5/8" (t).

AWP3030 panel edges are shiplapped on the long edges and a factory sealant gasket is included on one edge, providing a factory seal on all vertical joints. AWP attachment hardware engages the long edges, holding the panels off the substrate surface by 10mm (~3/8") and creating a closed-joint, drained/back-ventilated rainscreen system with concealed fastening. When accounting for the overall thickness of the AWP system, add this 10mm plus the thickness of the panel (16mm) for total system thickness of 26mm.

AWP3030 may be installed horizontally or vertically. See also *Horizontal Installation Guide AWP1818*, AWP3030.





VINTAGE WOOD PANEL DETAIL SCALE: 1" = 1'-0"

LIMITATIONS AND TECHNICAL DESIGN REVIEWS

Natural limitations on product usage are inherent to any cladding product's design, physical characteristics, and attachment system. Nichiha AWP are intended as a low-to-mid-rise cladding product.

Any project of more than three stories or 45 feet, as well as those located in high wind coastal areas (Exposure Categories C and D with Basic Wind Speed in excess of 130 mph), or those with any wall assembly not described in *Framing & Sheathing Requirements*, require a technical review by Nichiha to evaluate feasibility via our Technical Design Review process.

By evaluating a project's unique criteria and design, we can reference independently test-derived and calculated wind load performance data for our products to determine whether and how the panels can safely be installed on the project. Contact your local rep or Nichiha technical department for details or to initiate a Technical Design Review.

AWP are not to be used in any applications/uses not specified or described in this installation guide or other Nichiha technical documents. Any such use shall not be backed by the manufacturer's product warranty.

Do not use AWP on open screen walls.

Insulated Concrete Forms (ICFs) require additional measures.

Installation of AWP products on modular structures that are factory-constructed and then transported to a final site are not approved; and further, excluded from the Limited Product Warranty, per Section 2.F.

AWP installed as soffit is **not** covered by the Nichiha Product Warranty or the Nichiha Finish Warranty. Refer to pages 38-41 of the *AWP Horizontal Installation Guide*.

Please contact Nichiha Technical Services for assistance.

SAFETY

As with any natural stone, masonry, or concrete based product, when cutting, drilling, sawing, sanding, or abrading fiber cement cladding, proper safety measures must be taken due to the potential for airborne silica dust, an OSHA-identified hazardous substance that can pose serious medical risks.

Always wear safety glasses and a NIOSH/OSHA approved respirator with a rating of N, O, or P 100. Carefully follow the respirator manufacturer's instructions as well as applicable governmental safety regulations concerning silica. Refer to Nichiha's SDS for more information.

Always cut fiber cement panels outside and with a dust-collecting HEPA system. Do not cut the products in an enclosed area.

Use a dust-reducing circular saw with diamond-tipped or carbide-tipped fiber cement saw blades.

Always clean panels after cutting. Fiber cement dust can bind to the panel finish. Vacuum dust with a HEPA-filtered vacuum.

FRAMING AND SHEATHING REQUIREMENTS

Prior to Nichiha installation, closely inspect exterior wall substrate and correct any problems. Walls that are out of plumb, for example, can negatively impact the installation quality of AWP. Nichiha Spacer may be used in conjunction with panel attachment hardware if necessary to ensure a smooth, even substrate.

With conventional stud spacing, 7/16" or thicker APA rated OSB or Plywood sheathing *must* be used as the fastening base for Vertical AWP3030 as the panel size module will not align with framing. If nail-base insulation sheathing is considered, please contact Nichiha Technical as additional measures may be required. Alternatively, studs or furring may be spaced at 45.5cm (17-7/8") o.c. to allow fastening of AWP hardware directly to framing.

Refer to our third party building code certification(s) and/or state/local approvals for allowable wind design pressures at nichiha.com/resource-center.

Nichiha AWP cladding may be installed on vertical walls only. No tilted/sloped walls, nor true radius/curved walls. Vertical AWP installations are not compatible with PEMBs. AWP may be installed on wood or steel framing, concrete/masonry with furring, and Structural Insulating Panels (SIP) meeting the following requirements:

WOOD STUDS

Structural Sheathing Method Size: minimum 2"x4" studs Spacing: 16" o.c. max

Sheathing: APA rated exterior grade minimum

7/16" plywood/OSB required

Custom Stud/Furring Spacing Method

Size: minimum 2"x4" studs Spacing: 45.5cm (17-7/8") o.c.

Sheathing: APA rated exterior grade minimum 7/16" plywood/OSB, 1/2" or 5/8" gypsum

METAL STUDS

Structural Sheathing Method

Gauge: minimum 18 Spacing: 16" o.c. max

Sheathing: APA rated exterior grade minimum 7/16"

plywood/OSB required

Custom Stud/Furring Spacing Method

Gauge: minimum 18

Spacing: 45.5cm (17-7/8") o.c.

Sheathing: APA rated exterior grade minimum 7/16"

plywood/OSB, 1/2" or 5/8" gypsum

CONCRETE/MASONRY

Furring is required for installation of AWP over concrete and masonry structures.

Wood Furring: pressure treated lumber 2"x4", oriented vertically, spaced 45.5cm (17-7/8") o.c. max with additional vertical furring segments at Vertical Starter Track locations to enable 9" o.c. fastener spacing for track.

Metal Furring: hat channel, c-stud, or z-furring, minimum 18 gauge with 1"- 2" flanges, oriented vertically, spaced 45.5cm (17-7/8") o.c. max. with additional vertical furring segments at Vertical Starter Track locations to enable 9" o.c. fastener spacing for track.

Sheathing: exterior grade minimum 7/16" plywood/ OSB required with furring spacing other than 45.5cm (17-7/8") o.c.

STRUCTURAL INSULATING PANELS (SIP)

SIPs should be installed in accordance with manufacturer's instructions and local building codes. Additional special Nichiha installation requirements for SIPs are discussed in the Fasteners and Installing the First Course sections to follow.

For buildings greater than one story, contact Technical Department for assistance.

CONTINUOUS INSULATION

When exterior/continuous insulation is to be used with AWP3030 in vertical applications, please contact Nichiha *Technical Services* for assistance. Framing/sheathing/furring alternatives will be necessary.

Also refer to the Technical Bulletin:

Continuous Insulation and AWP available at
Nichiha.com/resource-center.

VERTICAL AWP3030 OVER C.I. ATTACHMENT REQUIREMENTS

When adding a furring grid* to enable AWP installation over c.i., the following general criteria are applicable:

Special attention must be paid to supporting the Vertical Starter Track, which bears the weight of AWP3030 in vertical applications. The clips do not share the dead loads for vertical panels.

- 1. Shaped metal furrings (Z, hat channel, C, etc.)
 - Minimum 18 gauge
 - Aligned vertically
 - Spaced 16" o.c. (max)
 - Min. 7/16" APA Rated OSB or Plywood

- or -

- 2. Pressure treated lumber
 - Minimum 2x (1.5") thickness
 - Aligned vertically
 - Spaced 16" o.c. (max.)
 - Min. 7/16" APA Rated OSB or Plywood

- or -

- 3. Shaped metal furrings (one layer)
 - Minimum 18 gauge
 - Aligned vertically at 17-7/8" o.c.
 - Additional vertical furring segments at Vertical Starter Track locations to enable
 9" o.c. fastener spacing for track

- or -

- 4. Shaped metal furrings (two layers)
 - (Z, hat channel, C, etc.)
 - Layer One
 - Minimum 18 gauge
 - Aligned horizontally
 - Spaced per engineer's design
 - Layer Two
 - Minimum 18 gauge
 - Aligned vertically at 17-7/8" o.c.
 - Additional vertical furring segments at Vertical Starter Track locations to enable 9" o.c. fastener spacing for track.

- or -

5. CL-TALON® 300

- Base Track and Wall Mount T-Tracks (vertical) at 16" o.c. (aligned with framing), and Therme Clips spaced per project loading requirements
- Wall Mount Supports (horizontal) at 16" o.c.

*Consult a structural engineer to design the furring system to manage the AWP system dead load of minimum 4 psf and also meet the project wind load design criteria. Furring must account for expected building compression. Nichiha does not provide fastener design for anchoring the furring to structure. Refer to IBC 2015 Table 2603.12.2 for more info.

ACCESSORY ATTACHMENTS

Nichiha Double and Single Flange Sealant Backers and metal trims, such as H-Mold and Corner Key, must be fastened to furring, blocking, or 18 gauge flat stock. Sealant backers must be fastened every 12"-14" vertically, so any use of flat stock must accommodate this fastening schedule.

Outside corners may be wrapped with 18 gauge flat stock fabricated to fit the corner. Attach the stock to furring on both sides of the corner. Corner Clips are used to secure Nichiha factory panel Corners and may be fastened to the flat stock wrapping, as can metal trim corners.

IBC 2015 TABLE 2603.12.2

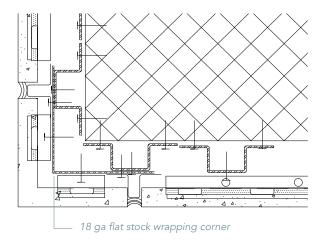
The model building code for 2015 includes information in Chapter 26 about foam plastic insulation/sheathing and furring minimum fastening requirements. Table 2603.12.2 shows various configurations depending upon framing gauge and spacing, fastener size and spacing, thickness of insulation and cladding weight. As an example, according to the table, 3 inches is the maximum thickness of foam sheathing on which a furring can be added directly on top, spaced at 16" o.c. and fastened with #8 screws every 12"-16" (into 18 gauge wall framing), that can support a cladding weight of 3 psf.

ENERGY CODE FRIENDLY MARKET OPTIONS

A number of engineered third party systems exist that are designed to solve the conflicts between energy code compliance and the safe installation of exterior claddings over continuous insulation.

Nichiha has direct experience with these products:

- Bracket and rail systems:
 - Cascadia Clips®
 - FERO Cladding Support
 - ISO Clip
 - Knight Wall MFI®
- CL-TALON®
- Knight Wall CI® and HCI™ Systems
- SMARTci GreenGirts



GENERAL

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WEATHER RESISTIVE BARRIERS

A weather resistive barrier (WRB) is required when installing Nichiha panels over stud walls and SIPs. For CMU/concrete assemblies, Nichiha defers to local code requirements. Use an approved WRB as defined by the 2015 IBC. Refer to local building codes.

A permeable WRB is highly recommended when installing Nichiha panels for residential applications.

Permeable WRB is required for all commercial applications. A fluid applied WRB is acceptable.

Sheathings and insulations with an integrated code-compliant WRB such as ZIP System® and DensElement $^{\text{TM}}$ are acceptable.

All openings must have appropriate flashing to prevent moisture penetration. Follow manufacturer's guidelines and all local building codes.

STORAGE & HANDLING

AWP are a finished product and care must be taken to protect them against damage prior to and during installation. Panels must be stored flat and kept dry. Indoor storage is best. Refer to the storage information included on product pallets.

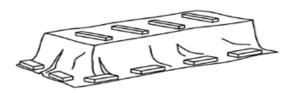
Ensure panels are completely dry before installing. Direct contact between the panels and the ground must be avoided at all times. It is necessary to keep panels clean during the installation process.

Cut panels face down.

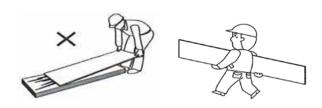
Always clean panels with a clean, soft, dry cloth after cutting. Dust can bind to the finish.

When sidewalks are poured after awp installation, take steps to cover/protect panels near grade.

Cement dried on AWP cannot be removed.



Always cover pallets with a tarp or store indoors!



Don't unpackage and re-stack panels! Always carry panels on edge!

FASTENERS

All Applications

Fasteners must be corrosion resistant. Stainless steel or corrosion resistant screws such as hot-dipped zinc or ceramic coated are recommended. Comply with all local building codes for fastener requirements.

Number 10, pan-head screws (HD .365") were used as clip fasteners for AWP wind load testing. The minimum size for clip fasteners is #8. Ultimate Clip and Starter Track screws must have a pan, wafer, or hex type full head.

Min. Number 7 finish screws with a bugle or flat head (min. head diameter 0.255") are appropriate for face fastening locations. These must penetrate framing per the minimum requirements below. Refer to the *Face Fastening Best Practices* section for face fastening procedure.

When installing AWP with the Structural Sheathing Method, ensure clip fasteners are at least 1" in length to fully penetrate the plywood or OSB. Wherever possible when face fasteners are needed, screws must be long enough to penetrate all the way through the sheathing and into the framing.

For the Custom Stud Spacing Method, the fasteners must always penetrate the studs or furring with minimum 1" penetration for wood or 1/2" for metal.





FACE FASTENING BEST PRACTICES

To minimize the appearance of face fasteners, utilize the following steps:

- 1. Apply low adhesive tape such as painters tape to the panel at face fastening locations.
- 2. Pre-drill panels 1" from the cut edge to be face fastened. Use a countersink drill bit with chamfer matching the head diameter of the bugle-head type screws to be used for face fastening.
- 3. Fill counter-sunk fastener holes with exterior cementitious filler, such as MH Ready Patch® and later dab touch-up paint with cotton swabs or artist brush.
- 4. Remove the painter's tape only after applying patch and touch up paint.





INSTALLATION HARDWARE & ACCESSORIES



ULTIMATE VERTICAL STARTER TRACK

Ultimate Vertical Starter Track serves as the foundational support for the AWP system while also providing faster and greater ease of installation. With Vertical AWP3030, the Starter Track carries the entirety of the dead loads and is required for each course.

FA 710 T Vertical Starter Track – 10mm rainscreen



ULTIMATE CLIP II

Ultimate Clips are secured to the vertical panels' shiplaps, securing AWP to the wall while holding their back surface off the substrate to create the 10mm (3/8") rainscreen space. In vertical applications, clips do not support panel weight.

JEL 778 CLIP Compatible with all 3030mm AWP - 10mm rainscreen

Joint Tab Attachments included with Ultimate Clips are not needed for vertical panel installations.



CORRUGATED SPACER

At termination points where Ultimate Clips cannot be used, Nichiha Corrugated Spacer is required to maintain the rainscreen space and prevent panel deflection at face fastening locations such as window jambs and outside corners.

FS 1005 SPACER - 5mm rainscreen

FS 1010 SPACER - 10mm rainscreen





Nichiha Sealant Backers provide exact spacing for expansion and termination joints and the recommended depth of sealant (75-80%).





Single Flange Sealant Backer: FHK 1015 – 10mm rainscreen

Double Flange Sealant Backer: FH 1015 – 10mm rainscreen



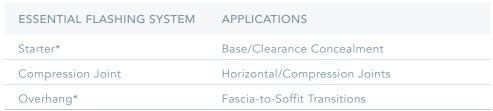
METAL TRIM OPTIONS

Nichiha metal trim provides aesthetically pleasing design options for corners, openings, and transitions.



TRIM	APPLICATIONS
Corner Key	Outside Corners
H-Mold	Vertical Joints
Open Outside Corner	Outside Corners
J-Mold	Terminations
Inside Corner	Inside Corners







* Inside and outside corner segments are available.

PLANNING AND PANEL LAYOUT

To ensure a successful installation, it is important to first plan how the panels will be laid out, where horizontal/ compression joints will be located for each course, and line of sight regarding inside corners decided.

Reminder: AWP3030 actual dimensions are metric: 455mm (h) x 3,030mm (l). Imperial equivalents: 17-7/8'' (h) x 119-5/16'' (l).

Horizontal/Compression Joints (Page 25):

1/2" (min.) Horizontal, flashed break detail to allow for building compression at floor lines. *Horizontal joints may not be staggered*.

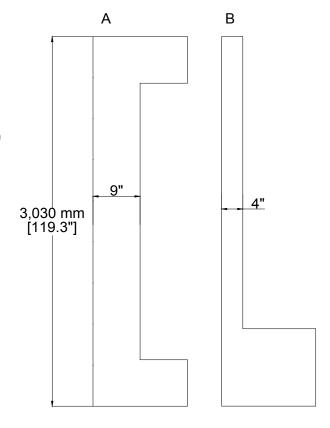
Inside Corner Line of Sight (Page 20): Sealant joints at inside corners can be placed out of view from the primary line of sight of a wall. Place the sealant joint on the less-viewed corner wall. Alternatively, utilize Inside Corner metal trim.

Cut Panels: In general, it is best to avoid cutting AWP to short or narrow strips and segments of less than 9". The hard minimum width or height is 4". Adjust the layout or use alternate materials when needed to avoid cutting AWP smaller than 4".

Specifically, when an individual panel is taller than a window or other opening and is used over the head or under the sill, do not cut it to less than 9" in width along the opening jamb. (see image A)

When an opening is taller than an individual panel and two or more are needed to cap over the header or cup the sill, do not cut the panel to less than 4" in width along the jamb. (see image B)

Design Wind Pressures: Refer to our code approval documents when determining the best vertical panel installation method for a specific project. The Structural Sheathing Method and Custom Stud Spacing methods result in different allowable design pressures, dependent upon thickness of wood sheathing or type/gauge of custom spaced studs/furring. Refer also to Limitations, Technical Reviews section regarding Nichiha's technical review process.



AWP3030 - VERTICAL: INSTALLING THE FA 710 T VERTICAL STARTER TRACK

All Applications

Without custom stud/furring spacing outlined in the Framing & Sheathing Requirements section, 7/16" or thicker APA rated OSB or plywood sheathing MUST be used to enable vertical installation of AWP3030. Plywood/OSB shall be secured to building framing in compliance with best practices and local building codes. In any case, Vertical Starter Track must be secured to framing and never sheathing alone as it fully carries the weight of the vertical panels. With respect to nail-base insulation sheathings, please contact Nichiha Technical Services for guidance.

Starter Track must remain continuous. Staggering of horizontal joints is not permitted.

MINIMUM CLEARANCES

The Starter must be level and attached at a minimum of 6" above finished soil grade or per local building codes (use a laser level to verify). When installing over a hard surface such as driveways or sidewalks, a 2" clearance is acceptable.

Keep AWP at least 1" above roofs.

Essential Starter Flashing may be installed prior to the Starter Track to conceal the clearance gap above hardscape and decking. Beginning with outside and inside corner segments, fasten trim at each stud location or every 10" o.c. to sill plate. Fasten inside and outside corner segments to framing on both sides of the trim, keeping at least 1" from trim vertical edges. Main segments will slide into/overlap the corner trim. Position Starter Track to leave 1/4" clearance between the panel edge and trim/flashing.



Vertical Starter Track fastened every 6" to 9" to framing.

INSTALLING STARTER TRACK 13

ALL APPLICATIONS

To fully secure Vertical Starter Track, use corrosion resistant screws of sufficient length to ensure full penetration of the sheathing and into framing by 1" for wood or 1/2" for metal. Starter must be level.

WOOD & METAL STUDS

Vertical Starter Track must be secured every 6"-9" into the sill plate or to the studs and, if applicable, halfway between into the wood sheathing.

CONCRETE/MASONRY

When installing over concrete construction, the wall must be furred out with pressure treated lumber, metal hat channel, or z-furring. Install APA rated 7/16" OSB or plywood to furring when the spacing is other than 45.5cm (17-7/8") o.c.. Starter Track must be secured at each furring location and halfway between into the sheathing or blocking at an overall fastener spacing of 6"-9" o.c.

STRUCTURAL INSULATING PANELS (SIP)

Secure Starter Track every 6"-9" o.c. max into the sill/horizontal base framing of the SIP.

NAIL-BASE INSULATION SHEATHING

Contact Nichiha Technical Services for guidance.



14 INSTALLING STARTER TRACK

GENERAL PANEL & ACCESSORY BASICS

PANEL SELECTION

Nichiha AWP are packaged with two panels in a pack, which are placed on pallets consisting of two stacks. Due to alternating patterns of texture and color between individual panels as well as how the panels are manufactured and packaged, it is best to install all panels from each individual stack before taking and installing panels from the second stack on the same pallet. Do not alternate installing from one stack and the second, which may result in undesirable patterns.



SEALING CUT PANEL EDGES

When cutting AWP, it is best to cut with the panel face down, except when cutting brick finish panels as it is easier to follow the simulated mortar lines.

Cut and exposed panel edges must be primed or sealed with fiber cement sealer (e.g. DryLock®) or paint such as Kilz Premium® or Kilz Max®. Do not use supplied Illumination Touch-Up paint. (Fig. 14)

Be sure to clean panels with a dry, soft, clean cloth after cutting to prevent dust from bonding to the finish.

CUTTING ULTIMATE CLIPS

JEL778 Ultimate Clips are 26" long. Where full length clips can be used, they are required. However, there may be conditions where clips must be cut to accommodate panels in smaller areas or segments such as short columns, pilasters, or insets/recesses.

Notches on the upward panel engagement flanges indicate where clips can be cut evenly into thirds. These 1/3 segments can be further reduced evenly into two or four pieces each with weep holes serving as dividing points. The smallest segment must include at least one downward panel engagement flange. Always use the widest clip segment possible. Cut with a non-ferrous saw blade on a band or chop saw.



SEALANT

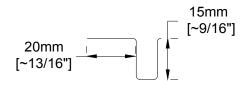
Sealants to be used with AWP must match the following requirements:

- Comply with ASTM C920
- Have a Class of 35, 50, or 100/50 (minimum 35% joint movement)
- Be a polyurethane, polyurethane hybrid, or Adfast Adseal 4580
- Provide two-sided adhesion at joints

OSI® QUAD® may not be used for Nichiha expansion joints:

- It is a class 25 product.
- QUAD® MAX is acceptable since it is a Class 50.

Refer to the Technical Bulletin: *Sealants* available at *nichiha.com/resource-center*.



Single Flange Sealant Backer (FHK1015R) (Galvalume)

SEALANT JOINTS/CAULKING

Fasten Single Flange Sealant Backers at inside corners (one wall at corner), along window and door jambs, and transition points with other cladding. Fasten to framing, blocking or plywood/OSB sheathing at 12"-14" o.c. with the 3/8" bump/ sealant portion butting the corner or jamb.

Sealant complying with ASTM C920, Class 35 (min.) is required where Single and/or Double Flange Sealant Backer is used.

Refer to the sealant manufacturer's instructions or requirements.

Place low-adhesive tape (masking or painter's) over the panel along the areas requiring sealant joints for a clean caulk line. Fill the gap between the panels with a color-matched/coordinating ASTM C920, Class 35 (min.) sealant. The Nichiha Sealant Backer allows for the proper depth of sealant (75-80%).

Before removing tape, press the surface of the sealant with a caulk spatula or similar tool to ensure an even surface.

Remove masking tape before sealant cures.

If excess sealant adheres to panel, remove completely using a putty knife or soft cloth.

GENERAL PANEL & ACCESSORY BASICS 17

AWP3030 - VERTICAL INSTALLATION

Without custom stud/furring spacing outlined in Framing & Sheathing Requirements section, 7/16" or thicker APA rated OSB or plywood sheathing MUST be used to enable vertical installation of AWP3030. Wall surfaces must be flat.

Use corrosion resistant screws of sufficient length to ensure full penetration of wood sheathing (Structural Sheathing Method), or the 17-7/8" o.c. studs with the Custom Stud/Furring Spacing Method (minimum penetration 1" into wood, 1/2" into metal), to secure Ultimate Clips. Face fasteners must be at least 1-1/2" in length.

Single Flange Sealant Backer and metal trim should be installed before panels. Refer to *Inside Corners*, *Windows & Doors* and *Outside Corners* sections.

AWP installation proceeds by working from left to right.

If starting at an inside corner, predetermine which wall will include the Single Flange Sealant Backer. Consider the location to minimize the visibility of the sealant line. Clad the higher visibility wall without the sealant joint first so that the adjoining wall panels can terminate to it with the Single Flange Sealant Backer detail. Or utilize Inside Corner metal trim.

Prior to installing the first vertical panel, add 10mm corrugated Spacer at the left edge of the wall at the starting point. The Spacer should extend upwards to where the panel will end.

Looking at an AWP3030 oriented horizontally, remove the bottom ship-lapped edge and then rotate the panel 90 degrees clockwise to set the short panel edge on the FA 710T Vertical Starter Track. The freshly cut and sealed edge should butt to the corner/starting point and will cover the 10mm Spacer. Be sure to clean dust from cut panels with a dry, soft cloth or HEPA vacuum.

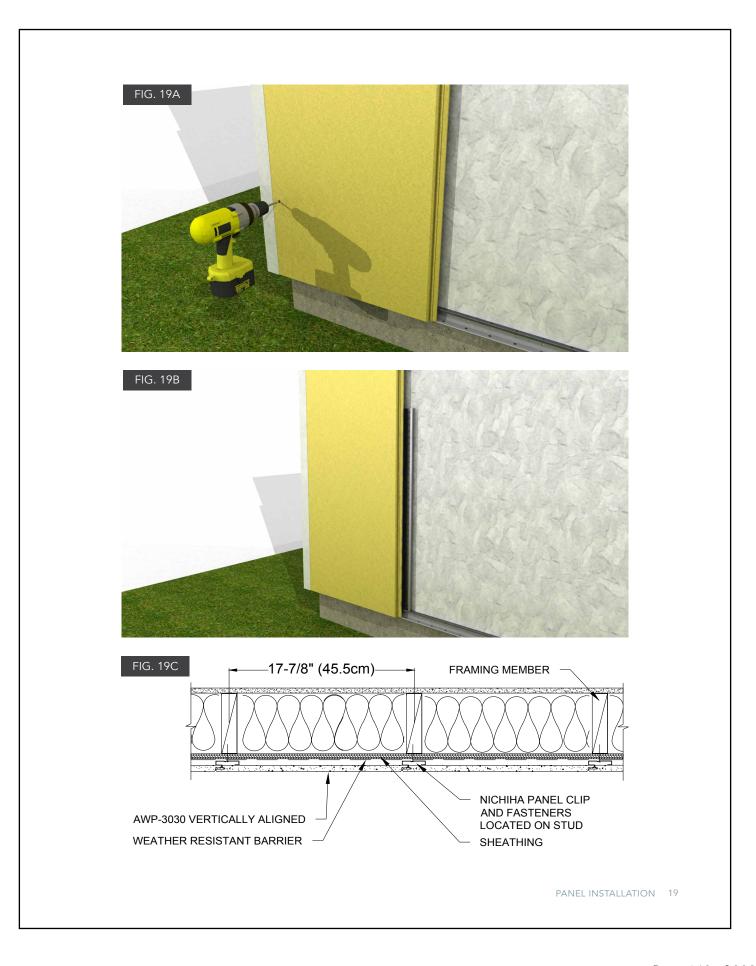
Pre-drill panels after applying low-adhesive tape to be removed after patching/touch-up. Fasten every 12"-16" o.c., spaced vertically, with a minimum 1" distance from the edge (*Fig. 19a*).

Fill counter-sunk fastener holes with exterior cementitious filler, such as MH Ready Patch® and later dab touch-up paint with cotton swabs. Remove painter's tape.

Whenever possible, use face fastening screws long enough to penetrate all the way through the sheathing and into the framing by 1/2" for metal, 1" into wood. Refer to the *Touch-up Paint*, *Minor Repairs* sections for more info on patching face fasteners.

On the right, factory edge, add four Ultimate Clips evenly spaced along the full AWP3030 panel, with the first at the Starter Track edge. Add four fasteners per clip, evenly spaced (*Fig. 19b*). In the Structural Sheathing Method, the clips will be fastened only to the plywood/osb sheathing. With the Custom Stud Spacing, the clips will align with vertical framing and the fasteners will be secured to the studs or furring (*Fig. 19c*).

Working from left to right, install the next panel with its ship-lapped edges intact. A rubber mallet or block may be used to seat panels firmly in place and tighten together on vertical panel joints. Do not hammer directly on the panels as direct contact may cause cracks, gouges, or chipping. Install four Ultimate Clips as with the first panel, each with four screws. (Continued p. 20)



Continue likewise until reaching a termination or transition point. The factory edge must be removed from the last panel, and this cut edge must be face fastened over 10mm Spacer. Space the fasteners every 12"-16" o.c. vertically, with a minimum 1" distance from the edge. Again, whenever possible, use face fastening screws long enough to penetrate all the way through the sheathing and into the framing, 1/2" into metal, 1" into wood. Refer to Face Fastening Best Practices for info on patching face fasteners.

To begin a second course of panels, install appropriate horizontal joint flashing or Essential Compression Joint Flashing above the top edge of the bottom/first course of panels. Then repeat the steps beginning with FA 710 T Vertical Starter Track a minimum 1/2" above the top edge of the first course of panels (See Horizontal/Compression Joint section). Horizontal joints may not be staggered.

INSIDE CORNERS, WINDOWS & DOORS All Applications

Appropriate flashing should be used to prevent moisture penetration on all inside corners, doors, and windows. Refer to local building codes for best practices.

Cut and exposed panel edges must be coated with exterior acrylic latex paint.

INSIDE CORNERS

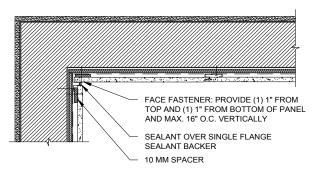
Single Flange Sealant Backer (FHK 1015):

Decide primary line of sight in order to minimize visibility of the sealant joint.

Install the panel on the front wall (more visible) first. Ensure panel is butted up tight to the inside corner wall. Fasten the Single Flange Sealant Backer onto the side wall right up against the front wall panel's edge at 12"-14" o.c. to framing, plywood/osb sheathing, or blocking.

Add 10mm Spacer over the fastening flange of the Sealant Backer.

Install side wall panel, with factory edge removed and sealed, directly against the sealant backer, over the Spacer, and secure with face fasteners*. Fill space with ASTM C920, Class 35 (min.) sealant.



20 PANEL INSTALLATION

Inside Corner Metal Trim: Install Nichiha Inside Corner metal trim directly against the inside corner sheathing. Fasten metal trim every 12"-16" in a staggered fashion on alternating flanges.

Remove the shiplapped edges that will be inserted into the trim, treating the cuts, and install panels normally, butting to the Inside Corner trim in moderate contact.

Trim Boards: Install trim boards at inside corner first and then add Single Flange Sealant Backer. Add ASTM C920, Class 35 (min.) compliant sealant to the gap.

*Face fasteners should fully penetrate OSB or plywood sheathing and into the framing whenever possible. Refer to Face Fastening Best Practices for info on patching face fasteners.

WINDOWS AND DOORS

Window Sills (J-Mold optional): For recessed windows, add a flashing where the panels will terminate so that the top edge is covered or capped.

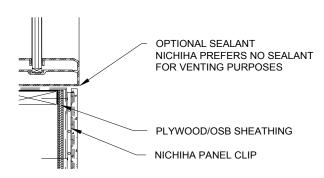
As needed, cut the panel to the required height to fit below the window sill, leaving a 1/4" gap between the top of the cut panel edge and the window sill or trim board.

Cut panel edges must be sealed with 100% acrylic latex exterior primer or paint, such as Kilz Premium or Kilz Max. Clean any dust off the panels with a dry, soft clean cloth.

Fasten Ultimate Clips along the sides of the panel to sheathing, framing, or furring with a clip positioned within an inch of the top end of the panel meeting the sill and the lowest clip at Vertical Starter Track edge.

If the top edge of the panel is fully sheltered under the sill, it is not necessary to seal the 1/4" gap. For better system performance, Nichiha recommends the vented approach.

If desired, install J-Mold trim, fastened every 12"-16", under the sill prior to panels.



PANEL INSTALLATION 21

WINDOW/DOOR JAMBS

A minimum gap of 1/4" is required when butting panels into windows, doors, and trim boards. Refer to window/door manufacturer guidelines for spacing trims around windows.

Single Flange Sealant Backer: Install the Single Flange Sealant Backer first, butting to the door/window jamb or trim pieces prior to installing the panels.

The Single Flange Sealant Backer must be fastened a minimum of 12" to 14" o.c. to framing, plywood/osb sheathing, or blocking.

Add 10mm Corrugated Spacer along the jamb.

Remove appropriate ship-lapped edge of panel, clean off dust with soft, dry cloth, and treat cut edge.

Install panels, face fastening through Spacer along the jamb edge every 12"-16", keeping a minimum 1" from panel edge. Use face fasteners long enough to penetrate framing.

Fill gap with recommended sealant.

J-Mold: Pre-install J-Mold trim, fastening every 12"-16", with a 1/4" gap between it and the jamb or per window/door manufacturer instructions

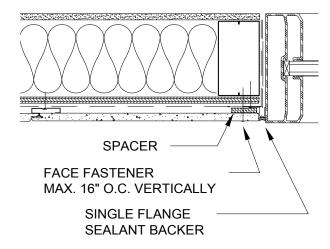
After installing the next-to-last panel, measure from the edge of the face of this panel to the J-Mold edge (the 90 degree angle edge). From this measurement, subtract 1/4" and cut the last panel to this width. Paint or prime cut edges and clean off dust from panel.

Install 10mm Corrugated Spacer next to the metal trim.

Install panels by inserting the cut edge into the metal channel and then shifting the panel over onto the side Ultimate Clips along the adjacent panel, fitting ship-lapped edges together.

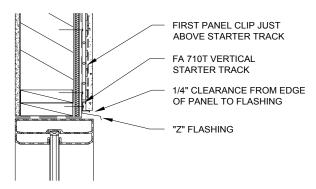
Face fasten through Spacer along the jamb edge every 12"-16". Use face fasteners long enough to penetrate framing.

Lastly, add foam backer rod and sealant to the 1/4" gap, if applicable, between the J-Mold and jamb.



WINDOW/DOOR HEADERS

Starter Track: When starting a course of vertical panels above a window or door, add flashing and FA 710 T Vertical Starter Track at the header, installed with fasteners every 6"-9" into the opening header.



OUTSIDE CORNERS

There are two primary outside corner installation options for vertical AWP3030:

Trim Boards: Fiber Cement, Wood, or PVC

Metal (Open Outside, Corner Key) or Vinyl Trim Channels.

Appropriate flashing must be used as required to prevent moisture penetration at outside corners.

FIBER CEMENT & PVC TRIM BOARDS

Nichiha manufactures a full line of fiber cement trim boards - NichiTrim[™], which are available in the Southeast U.S. Refer to Nichiha.com for more information.

When panels are to be butted to fiber cement, wood or other trim pieces, use Nichiha Single Flange Sealant Backer.

Add 10mm Spacer, remove the appropriate panel shiplap, and face fasten panel edge every 12"-16", vertically, keeping 1" from edge. Apply sealant to joint width. Sealant must be compliant with ASTM C920, Class 35 (min).

METAL & VINYL TRIM

(including Nichiha Corner Key and Open Outside Corner trim)

When installing Nichiha AWP3030 in a vertical orientation, pre-fasten corner trim channels, securing trim to framing every 12"-16", alternating/staggering the fasteners on both flanges.

At the starting point, such as an outside corner, remove the panel edge, add 10mm Spacer to the wall at the corner. Set the panel on the Starter and into the corner trim channel, and then face fasten the panel as described at the beginning of the AWP3030 Vertical Panel Installation section.

Working from left to right, when reaching the next outside corner, follow the steps for the appropriate trim profile:

Corner Key: After installing the next-to-last panel, measure from the edge of the face of this panel to the Corner Key edge (the 90 degree corner angle edge). From this measurement, subtract 1-3/8" and cut the panel to this width. Paint or prime cut edges and clean off dust from panel. (Fig. 24a)

Open Outside Corner: After installing the next-tolast panel, measure from the edge of the face of this panel to the Open Outside Corner edge (the 90 degree angle edge). From this measurement, subtract 1/4" and cut the panel to this width. Paint or prime cut edges and clean off dust from panel.

Install 10mm Spacer next to the metal trim. Install panels by inserting the cut edge into the metal channel, rotating into the wall plane, and then shifting the panel over onto the side clips along the adjacent panel, fitting ship-lapped edges together.

Face fasten panels through Spacer along the corner edge every 12"-16"(Fig. 24b). Use face fasteners long enough to penetrate framing.

Fit panels into channel trim so that panel edges are not exposed.

Nichiha metal trim pieces are each 10' in length. To cut metal trim, use a non-ferrous carbide miter saw blade. When butting/stacking metal trim pieces, add a bead of polyurethane sealant at the seam/joint.

Metal trim can be pre-finished when purchased to match Nichiha Color Xpressions color(s). Otherwise, for field painting primed metal trim refer to Tamlyn's XtremeTrim Painting Guide.





NON-90 DEGREE CORNERS

Corners other than 90 degrees can be achieved with custom metal trim, butting panels to trim board with a minimum 1/4" sealant gap, or with the use of Double Flange Sealant Backer to set cut panel edges at the desired corner angle. Please contact Nichiha Technical.

VERTICAL CONTROL/EXPANSION JOINTS

All Applications

Because thermal expansion occurs in the long (3030mm) dimension of the panels, Vertical Control/ Expansion Joints are not required for vertical installations of AWP3030.

HORIZONTAL/COMPRESSION JOINTS

All Applications

The module of Vertical AWP3030 necessitates a continuous Horizontal/Compression Joint every 119-5/16" (repeating after each course).

Do not stagger horizontal joints.

Do not span floor lines with panels.

INSTALLING A HORIZONTAL COMPRESSION JOINT

Install Essential Compression Joint Flashing or heavy gauge z-shaped metal flashing or drip cap over the top edge of the course of panels terminating under the Horizontal Compression Joint location. Fasten Essential Flashing at each stud



location.

Install Vertical Starter Track over the flashing and check for level. Place Vertical Starter at least 1/2" above the course below and 1/4" above flashing/ trim. A best practice is to add flashing tape to cover the fasteners of the flashing, sealing it to the WRB.

Continue to install panels according to these guidelines with compression joints every 119-5/16" (max).



LARGE OPENINGS

All Applications

Install Vertical Starter Track at the wall base in keeping with standard instructions on both sides of the opening.

Install Vertical Starter Track at the head of the opening, either the width of the opening or all the way across the wall.

Add panels per the standard procedure as in a typical *Window or Door Opening* for the jamb conditions.

Do not span floor lines with panels. Plan for a *Horizontal/Compression Joint* at the head of the opening or above, at the same level where the panels along the sides of the opening terminate, assuming the garage or other large opening is shorter than full length panels.

PENETRATIONS, RAILINGS, AND SIGNAGE

Openings for small penetrations for pipes or conduits may be cut through a panel with the hole sealed with ASTM C920 compliant sealant. For larger penetrations greater than 1-1/2", it is best to block or frame out the opening. Treat the penetration like a small window.

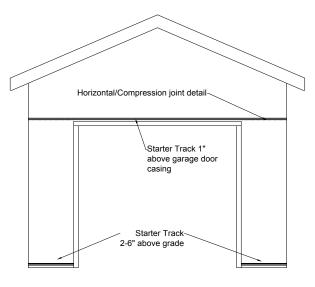
Along the jambs of the opening install Single Flange Sealant Backer. Cut panel edge as needed to butt to Sealant Backer and add recommended sealant.

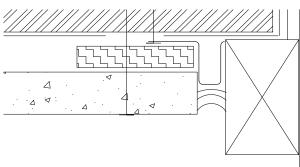
Underneath the opening block out, terminate panel with 1/4" gap. Sealant here is optional, depending on the depth of the blocking.

Above the penetration, add flashing and install FS1010 Spacer as needed for face fastening panel edge at framing locations. Ensure minimum 1/4" gap between bottom of panel edge and penetration blocking.

Keep any face fasteners 1" away from panel edges.

If installing railings or signage over AWP, ensure fasteners are secured through to framing or other structural support. Do not fasten any attachments solely to panels. Add a small spacer (up to 10mm) between the signage and AWP to prevent moisture pooling on top of the attachment and seeping between it and the AWP, becoming trapped.



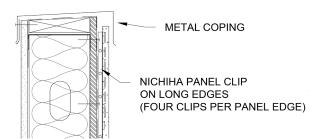


LAST COURSE

All Applications

Cut panels (horizontally) to properly fit at the roof line under soffit or parapet cap (or at the proper transition point). Ensure Ultimate Clips along factory edges are secured no more than 1"-2" from the top of the panels.

Cover top panel row edge with roof cap/coping, where applicable.



GABLE & OVERHANG

Allow a minimum of 1" clearance (as per local building codes) above the roof line.

At the top, cut the panel to follow the slope of the gable or overhang.

When installing soffit, the wall panels should be installed first, with the soffit installed over the panels.

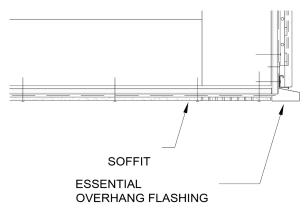
Seal all cut panel edges with 100% acrylic primer or paint. Do not leave any panel edges exposed.

Essential Overhang Flashing may be used at the base of overhangs/bump-outs or porte-cocheres.

Prior to panel installation, fasten Overhang Flashing at each stud location, beginning with corner segments. Main segments will slide under/overlap corner segments.

Use Joint Clip segments to join main segments together. After first piece is secured, add a Joint Clip, fastening through both it and the first main segment. The next main segment will slide behind the Joint Clip.

Position Overhang so that its bottom/return flange butts to or overlaps soffit. The bottom return portion must extend beyond the face of the fascia substrate.



Essential Overhang Flashing & Joint Clip



TRANSITIONS WITH HORIZONTAL AWP

On projects also utilizing horizontally-installed AWP, expansion and compression type joints will be required as there is no way to naturally joint horizontal and vertical AWP directly.

VERTICAL JOINTS

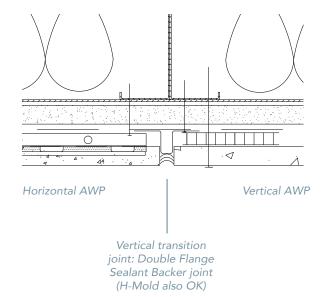
A Double Flange Sealant Backer or H-Mold trim is necessary at vertical joints/transitions between horizontally oriented panels and vertically oriented ones.

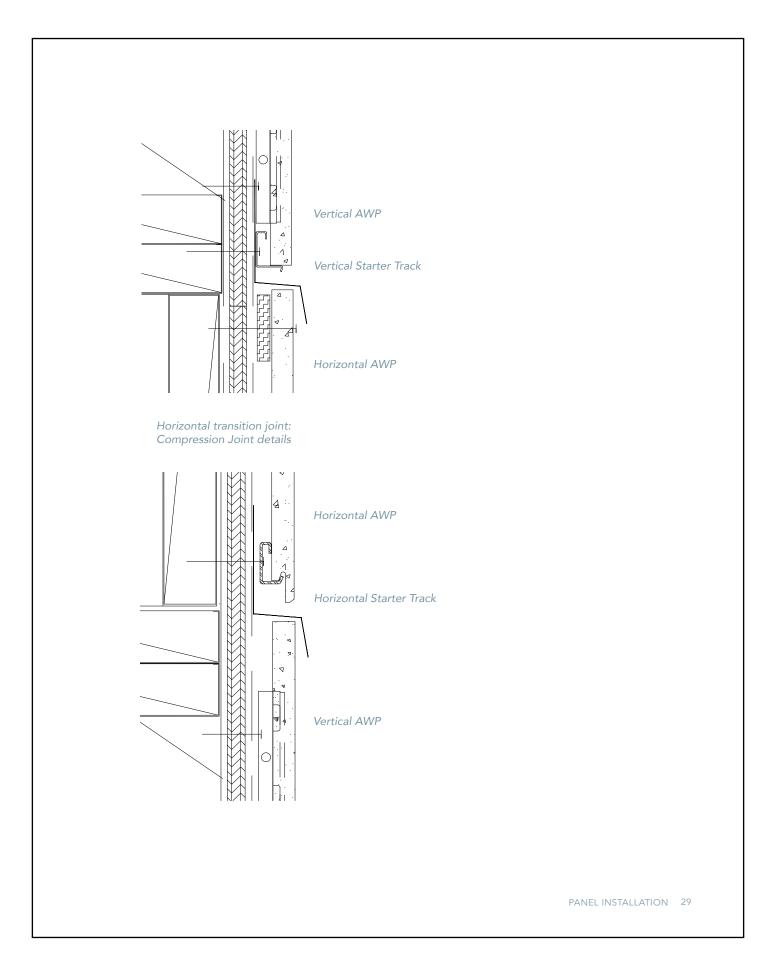
HORIZONTAL JOINTS

A horizontal/compression joint style detail is necessary to transition between horizontal and vertical AWP. Please refer to *Horizontal/Compression Joints* on page 25.

Horizontal panel to Vertical panel Transitions
Face fasten the top, cut edge of the horizontal AWP,
cap it with Essential Compression Joint Flashing
or Z-flashing. To then install vertical AWP, add
the Vertical Starter Track following the standard
procedure and fastening requirements.

Vertical panel to Horizontal panel Transitions
Install vertical panels to the desired transition level
and cap with Essential Compress Joint Flashing or
Z-flashing. Install the Horizontal Starter Track 1-1/4"
above the flashing, following the standard procedure
and fastening requirements. Refer to the Horizontal
Installation Guide AWP1818, AWP3030.





CLEANING & MAINTENANCE

CLEANING PANELS

After completion of the installation or for periodic maintenance, it may be necessary to clean panels.

When cleaning panels, use no more than 400 psi of water pressure at 10" to 12" away. Do not pressure wash custom color panels.

To clean heavily soiled areas, a mild household detergent and/or soft bristle brush may be required.

Do not allow any detergent/cleaner to dry on panels. Rinse immediately after cleaning.

REMOVAL OF EXTERIOR ACRYLIC LATEX PAINT

Wet Paint Removal - While the paint is still wet, flush the area with clean water, using mild abrasion with a clean cloth or soft brush.

Semi-Dry Paint Removal - If paint has set, but not dried, flush and clean as above, followed by light scrubbing with alcohol to remove any remaining paint residue. Rinse with water and a clean cloth.

Dry Paint Removal - Please refer to paint-removal guide in the next section.

PAINT TOUCH-UP

Touch up paint must be exterior grade 100% acrylic latex and can be color matched by taking a panel sample to your local paint or home improvement store.

A small amount of touch-up paint is supplied with your custom color panel order. Do not use touch-up paint for edge treatment/sealing due to the limited quantity provided.

Utilize low-adhesive tape to isolate patching and touchup locations such as face fastened areas. Where face fasteners have been used and patched by cementitious filler, use a cotton swab to lightly dab touch-up paint.

For scratches, use a cotton swab for small ones or 1" foam brush for longer ones, again using a dabbing motion rather than brushing in order to minimize the amount of paint applied.

OTHER PAINT & GRAFFITI REMOVAL

The following products have been tested on Nichiha panels to aid in the removal of graffiti type markings.* These citrus-based products can also be used for basic panel cleaning purposes. The panels were sprayed with an indoor/outdoor aerosol spray paint and left to dry overnight, and then the paint removal products were applied following the manufacturer's guidelines.

All products tested achieved good results. However, the outcome may vary depending on the amount of paint that needs to be removed. Be sure to follow all manufacturer's guidelines and first test in an inconspicuous area before working on a larger area.

Do NOT use these cleaners with custom color panels. *Nichiha is not liable for any damage caused by the use of these cleaners.

CITRISTRIP

www.citristrip.com

Products tested:

Citristrip Striping Gel - One Quart container Citristrip Stripping Aerosol - 18 oz. spray can

GOOF OFF GRAFFITI REMOVER

www.goof-off.com

Products tested:

Goof Off Aerosol - 16 oz. spray can Goof Off - 22 oz. trigger spray bottle

TAGAWAY

www.tagaway.com

Product tested:

Tagaway - 32 oz. trigger spray bottle

REPAIRING MINOR DAMAGE

Isolate the blemish with a low adhesive tape such as painters tape. This will help protect the surrounding area of the panel and aide in creating a more polished, clean repair.

Lightly brush/abrade the surface within the taped off area in order to remove any loose material.

Carefully fill and smooth the resultant prepped area with cementitious patching material such as MH Ready Patch. Allow to dry/cure fully.

Gently smooth the patch and then apply touch-up paint to the affected area. Allow touch-up paint to dry and remove the tape.

CLEANING & MAINTENANCE 31

PANEL REPLACEMENT

Set the depth of the circular saw blade slightly deeper than the panel so the saw blade does not cut into the building wrap or sheathing.

Make cuts into the damaged panel and break into pieces for easier removal.

Remove damaged panel.

If necessary, cut new panel to appropriate height.

Looking at the panel oriented horizontally, cut the top ship-lapped edge off the panel (*Figure 33a*).

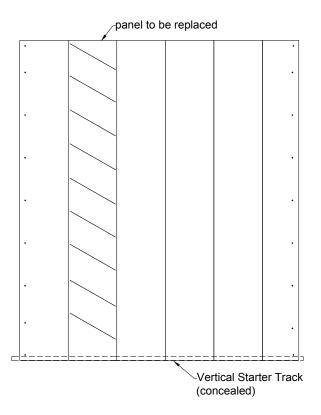
Clean off dust and seal the cut edge.

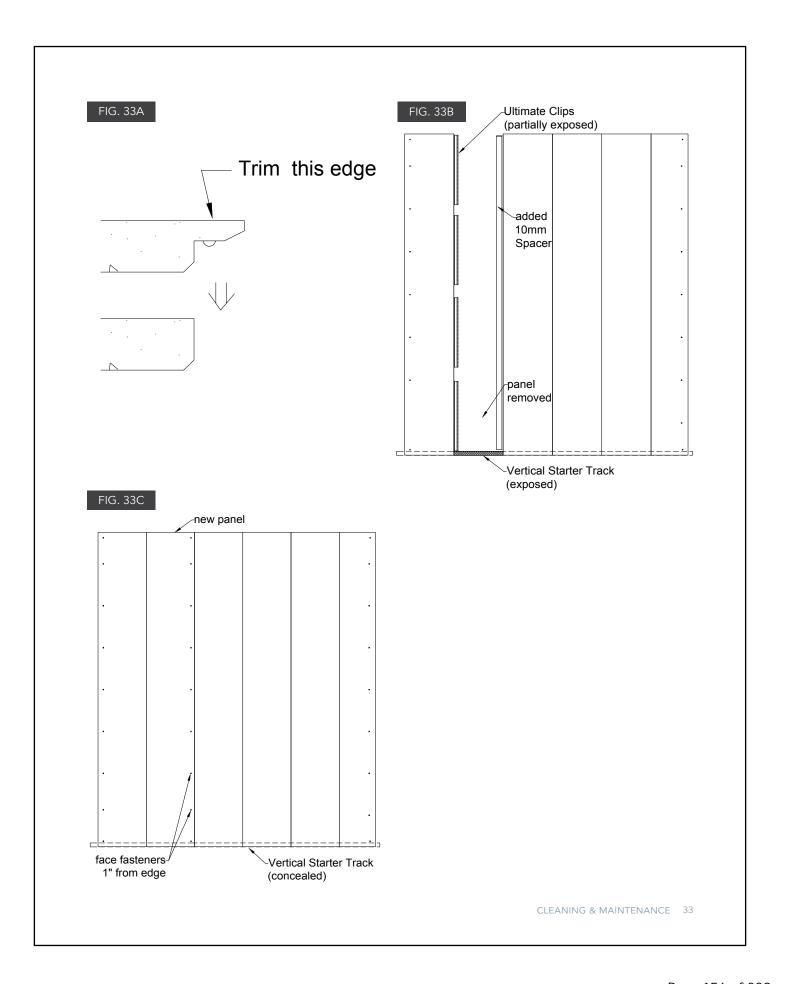
Add 10mm Spacer along the right side of the uncovered wall surface. (*Figure 33b*)

Set the new panel in place on the Vertical Starter Track with the intact factory edge fitting on the exposed clips on the left side of the uncovered space.

Pre-drill and face fasten the right edge of panel through the Spacer with a screw every 12"-16" into framing, furring, or blocking (*Figure 33c*). When only wood sheathing is available for the face fasteners, reduce the screw spacing to 6"-8" o.c.

Fill countersunk screw heads per *Paint Touch Up* and *Minor Repairs*.





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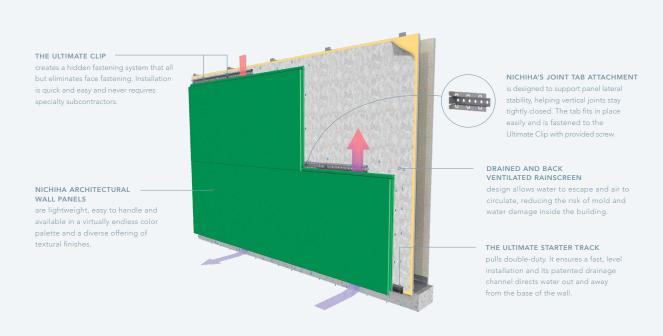
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34 RESOURCES

Never underestimate the power of REALLY GOOD TOOLS.

Whether you are an architect, a builder or a contractor, Nichiha wants to ensure that you have all the information you need to make your project go as smoothly as possible. The way we see it, we are partners. Our website offers a comprehensive collection of technical information, installation videos, Architectural details, in-depth specifications and everything you'll ever need to know about installing Nichiha products.



DESIGN REVIEW GUIDE

Download our quick reference guide to get an overview on our Architectural Wall Panels. nichiha.com/resource-center.



ARCHITECTURAL DETAILS

Take a closer look and download our conceptual detail drawings.

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Our in-house technical team is here to assist. If you have questions, comments or concerns, call or email us.

1.866.424.4421 or technicalservices@nichiha.com

RESOURCES 35

THE POWER OF POSSIBILITIES AND PARTNERSHIPS

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- ILLUMINATION SERIES PANELS 15-year limited warranty* on panels, 15-year limited warranty* on finish.
- ARCHITECTURAL WALL PANELS (Brick, Block, Stone, Wood, Kurastone) 15-year limited warranty* on panels, 15-year limited warranty* on finish.
- Tamlyn warrants defective-free products for a period of 10 years for the original purchaser. Please visit tamlyn.com for detailed information on terms, conditions and limitations.

*See Nichiha warranties for detailed information on terms, conditions and limitations. Visit nichiha.com for easy downloadable warranties or call tollfree 1.866.424.4421 for a copy.

CERTIFICATION & TESTING







Code Compliance CCRR-0299

Florida Approval





Miami-Dade 8140-2029 NOA 18-0522.05

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March 12, 2020

Steve Kelly, Project Coordinator Clackamas County Community Development 2051 Kaen Road, Suite 245 Oregon City, Oregon 97045

Via email: stevekel@clackamas.us

Regarding: Traffic Impact Analysis Letter

Sandy Health Clinic 39831 Highway 26 Sandy, Oregon 97055 PBS Project 71524.000

Dear Mr. Kelley:

This document represents a traffic impact analysis (TIA) letter to meet the submittal requirements of the City of Sandy (City) and the Oregon Department of Transportation (ODOT). The TIA letter was scoped based on consultation with the City, ODOT, and Clackamas County.

PROJECT DESCRIPTION

The Clackamas County Health Department proposes to consolidate its health services from two offices in Sandy, Oregon, to one location. The existing offices are Sandy Health Clinic located at Sandy High School, 37400 Bell Street, and Clackamas County Health Center, located downtown at 38872 Proctor Boulevard. The proposed site has an existing 6,300-square-foot building used as a warehouse that will be replaced with a 9,600-square-foot health clinic. See Figure 1 for a vicinity map.

The site is within the city under its jurisdiction at the northeast corner of the Highway 26/Ten Eyck Road intersection, and the site will send all of its trips through the Highway 26/Ten Eyck Road intersection. See Figure 2 for the site plan. ODOT recommended a TIA be prepared to determine if a right-turn lane is warranted on westbound Highway 26 at Ten Eyck Road. The intersection of Highway 26 and Ten Eyck Road is under ODOT authority, and Ten Eyck Road is under Clackamas County jurisdiction. After contacting each of the three public agencies and holding discussions with Ankrom Moisan Architects, Inc., the TIA was focused toward two intersections: Highway 26 and Ten Eyck Road, related to the need for a westbound right-turn lane, and Pleasant Street and Ten Eyck Road, related to queueing on southbound Ten Eyck Road.

ODOT recommended that, as a condition of approval, a TIA be prepared to determine if a right-turn lane is warranted for westbound Highway 26 at Ten Eyck Road. If a right-turn lane is not warranted, ODOT recommended that the sidewalk along the site frontage of Highway 26 be extended into the roadway in such a way as to eliminate the existing slip lane and define the bicycle lane through the intersection. Figure 3 shows the existing lane configuration of the intersections in the vicinity of the project.

City staff directed PBS to include an evaluation of queueing on Ten Eyck Road to verify it will not block access to Pleasant Street.

415 W 6TH STREET, SUITE 601 * VANCOUVER, WA 98660 * 360.695.3488 MAIN * 866.727.0140 FAX * PBSUSA.COM

TRAFFIC VOLUMES

PBS contracted with All Traffic Data to collect AM and PM peak hour traffic counts for the adjacent streets at Ten Eyck Road/Highway 26, Ten Eyck Road/Pleasant Street (east leg), and Ten Eyck Road/Pleasant Street (west leg). The traffic data were collected on February 19, 2020. Detailed traffic volume reports are provided in Appendix A

Background Growth

Background growth is a generic increase in traffic volumes that either is not attributable to specific developments in process (in process) or is attributable to influences outside the study area. No in process projects contributed trips to the studied intersections. A linear background growth rate of 2.0% per year was applied to 2020 peak hour volumes between public roadways at the studied intersections. The 2.0% growth rate was based on evaluation of ODOT count volumes on Highway 26 through traffic east and west of Ten Eyck Road between 2013 and 2018. The background growth volumes are included in the 2022 build-out year and the 2029 forecast year traffic volumes. Details of the background growth rate estimate are in Appendix A

Seasonal Adjustment Factor

A 28% seasonal adjustment factor (SAF) was applied to the through movements on Highway 26 based on the ODOT *Analysis Procedure Manual*, Version 2 (APMv2). Since no ODOT automatic traffic recorders (ATRs) are located within the study area, and no ATRs offer similar characteristics or reasonable adjustments, the seasonal trend table method was used. The 28% SAF was calculated based on the average of the commuter and summer trends, adjusting from the mid-February counts to the seasonal peak period. Figure 5 presents the 30th highest-volume hour, including the SAF. Details of the seasonal adjustment factor estimate are in Appendix A

Baseline Volumes

The 2020, 2022, and 2029 baseline volumes represent the study area traffic volumes without the Sandy Health Clinic project development trips. The baseline volumes are calculated as the sum of existing traffic, background growth, and season factors. The 2029 forecast year is used to compare to the City of Sandy's Transportation System Plan (TSP).

TRIP GENERATION AND DISTRIBUTION

The following section relies on data provided in the Institute of Transportation Engineers' (ITE) *Trip Generation Manual*, 10th Edition (2017). Detailed trip generation calculations are provided in Appendix B.

Trip Generation

The proposed Sandy Health Clinic will utilize a site with an existing building. The net new trips to the site are based on the replacement of a 6,300-square-foot building used for food distribution and warehouse space with a 9,600-square-foot health clinic. Figure 2 provides a site plan for the proposed site. Trip generation for both the existing warehouse and the proposed health clinic are based on an independent variable of 1,000 square feet in the respective buildings. The trip generation estimates were calculated using the ITE weighted average trip rates for the peak hours of the adjacent streets. Table 1 summarizes the trip generation calculations.

¹ Noting that Highway 26 is a primary route connecting between the Portland metro area and the Mount Hood winter recreation areas, the recreational summer-winter seasonal trend was also evaluated. Averaging the commuter and recreational summer-winter trends, while not an approved blend per ODOT APMv2, also yields a 28% SAF.

Table 1. Trip Generation

ITE Land Use:	Existing Warehouse Code 150		Propose Code		Not Nove Tring		
Independent Variable:	1,000 square feet		1,000 square feet		Net New Trips		
Size:	6.3		9.6				
Weekday ADT:	11		36	56	355		
Total Peak Hour Trips:	AM PM		AM	PM	AM	PM	
In:	1	0	27	9	26	9	
Out:	0	1	8	22	8	21	
Total:	1	1	35	31	34	30	

Findings: The proposed Sandy Health Clinic project will generate 355 net new trips on an average weekday, 34 net new trips in the AM peak hour of the adjacent streets, and 30 net new trips in the PM peak hour of the adjacent streets

Proposed Trip Distribution

The proposed distribution of new trips is based on a review of the land uses within the study area, consultation with Clackamas County health clinic staff, and on engineering judgment. See Appendix B for correspondence on the trip distribution of the existing Sandy Health Clinic. All trips from northwest, southwest, and west of the site intersect Highway 26, and approach from the west of the site on Highway 26. All traffic southeast, northeast, and east of the site will approach the site from the east on Highway 26. The overall distribution pattern is proposed as follows:

- 90% to and from the site via Highway 26 from the west
- 10% to and from the site via Highway 26 from the east

The distribution pattern above represents an external distribution of the net new trips entering and exiting the study area. The distribution and assignment of the project's net new trips are shown on Figure 4.

INTERSECTION OPERATIONS AND ROADWAY CAPACITY ANALYSES

Operation Standards

The City of Sandy Minimum Requirements for Traffic Analysis cites a minimum level of service (LOS) D for signalized intersections and for stop conditions. The LOS is based on the volume-to-capacity ratio (v/c) for signalized intersections when the development is in full service. ODOT has a mobility standard of a v/c ratio 0.85 for highways categorized as Freight Route on a statewide highway for locations inside an urban growth boundary and a posted speed equal to or under 35 miles per hour (mph).

Analysis Methodology

Traffic impacts were estimated to determine the extent of change in traffic conditions caused by future development. In order to make this determination, the following assumptions were employed:

- The individual peak hour volumes were analyzed for 2020 existing year, 2022 assumed full operation, and 2029 forecast year.
- The analysis is based on the AM and PM peak hour of the adjacent streets.
- No in-process trips were included in the analysis.

- An SAF of 28% increase was applied to the through movement volumes on Highway 26 based on the ODOT APMv2 Seasonal Trend Table method.
- The peak hour factor (PHF) for the overall intersection, as calculated from the count data, was applied for each analysis scenario.
- A minimum value of 2.0% was assumed for each movement in the future conditions for heavy vehicle percentage (HV%).
- As noted previously, trip generation, distribution, and assignment estimates for the project were prepared for the weekday AM and PM peak hours on the surrounding street system.
- Cumulative traffic impacts of the proposed project were determined by superimposing the project-generated traffic onto the baseline volumes for the weekday AM and PM peak hour at studied intersections. This sum is termed the "With Project" conditions.
- The LOS for the signalized intersection was calculated with Trafficware's Synchro software, Version 10, based on *Highway Capacity Manual* (HCM) 6th Edition (2016) methodologies. The ODOT protocol for Synchro analysis at signalized intersections was used to calculate the intersection v/c ratio.
 - o Signalized intersection results are reported as the v/c ratio for the intersection.
 - ODOT right-turn lane warrants at a signalized intersection are based on volume threshold and LOS standards per ODOT APMv2.
 - Traffic signal timing values were supplied by ODOT and used throughout the LOS analysis without modification.
 - The analysis includes a 2029 With Project alternative with a right-turn lane for westbound Highway 26.
- No site driveways were analyzed for this report.
- The queueing was estimated for 2029 conditions with and without the project using SimTraffic following ODOT APMv2.
- Right-turn lane impacts on bicycle safety are based on bicycle level of traffic stress (LTS) methodology, noted in APMv2.
- The results of the TIA were compared to the City's TSP related to the Highway 26/Ten Eyck Road improvements.

Level of Service Analyses

Table 2 presents the LOS analysis for the Highway 26/Ten Eyck Road intersection for 2020 existing conditions during the studied peak hours. Detailed LOS calculation reports are provided in Appendix C.

Table 2. Estimated Level of Service at Study Area Intersection for 2020 Existing Conditions

	AM Peak Hour			PM Peak Hour			
INTERSECTION	LOS	Delay (sec/veh)	Intersection v/c Ratio	LOS	Delay (sec/veh)	Intersection v/c Ratio	
Highway 26/Ten Eyck Road	Α	9.1	0.56	С	20.8	0.67	

Finding: The existing Highway 26/Ten Eyck Road intersection operates above LOS standards.

Table 3 presents the LOS analysis for the Highway 26/Ten Eyck Road intersection for 2022 without the project during the studied peak hours. Detailed LOS calculation reports are provided in Appendix C.

Table 3. Estimated Level of Service at Study Area Intersection for 2022 Without Project Conditions

			•				
		AM Peak	Hour	PM Peak Hour			
INTERSECTION	LOS	Delay (sec/veh)	Intersection v/c Ratio	LOS	Delay (sec/veh)	Intersection v/c Ratio	
Highway 26/Ten Eyck Road	Α	9.5	0.58	С	21.4	0.70	

Finding: In 2022 without the project, the Highway 26/Ten Eyck Road intersection will operate above LOS standards.

Table 4 presents the LOS analysis for the Highway 26/Ten Eyck Road intersection for 2022 with the project during the studied peak hours. Detailed LOS calculation reports are provided in Appendix C.

Table 4. Estimated Level of Service at Study Area Intersection for 2022 With Project Conditions

	AM Peak Hour			PM Peak Hour			
INTERSECTION	LOS	Delay (sec/veh)	Intersection v/c Ratio	LOS	Delay (sec/veh)	Intersection v/c Ratio	
Highway 26/Ten Eyck Road	В	10.2	0.59	C	21.8	0.72	

Finding: In 2022 with the project, the Highway 26/Ten Eyck Road intersection will operate above LOS standards.

Table 5 presents the LOS analysis for the Highway 26/Ten Eyck Road intersection for 2029 without the project during the studied peak hours. Detailed LOS calculation reports are provided in Appendix C.

Table 5. Estimated Level of Service at Study Area Intersection for 2029 Without Project Conditions

		AM Peak	Hour	PM Peak Hour			
INTERSECTION*	LOS	Delay (sec/veh)	Intersection v/c Ratio	LOS	Delay (sec/veh)	Intersection v/c Ratio	
Highway 26/Ten Eyck Road	В	11.1	0.64	C	24.0	0.81	

^{*}This TIA evaluates the intersection in its existing configuration. The northbound and southbound left-turn lanes called for in City of Sandy's TSP are not evaluated.

Finding: In 2029 without the project, the Highway 26/Ten Eyck Road intersection will operate above LOS standards.

Table 6 presents the LOS analysis for the Highway 26/Ten Eyck Road intersection for 2029 with the project during the studied peak hours. It includes the LOS results with a westbound right-turn lane. Detailed LOS calculation reports are provided in Appendix C.

Table 6. Estimated Level of Service at Study Area Intersection for 2029 With Project Conditions

		AM Peak Hour			PM Peak Hour			
INTERSECTION*	LOS	Delay (sec/veh)	Intersection v/c Ratio	LOS	Delay (sec/veh)	Intersection v/c Ratio		
Highway 26/Ten Eyck Road	В	12.0	0.66	C	24.4	0.84		
Highway 26/Ten Eyck Road With Westbound Right-Turn Lane	В	11.4	0.66	С	23.0	0.83		

^{*}This TIA evaluates the intersection in its existing configuration and with the addition of a westbound right-turn lane. The northbound and southbound left-turn lanes called for in City of Sandy's TSP are not evaluated.

Findings: In 2029 with the project, the Highway 26/Ten Eyck Road intersection will operate above LOS standards. The operation of the Highway 26/Ten Eyck Road intersection will not significantly improve with the installation of a westbound right-turn lane on Highway 26.

Recommendation: Do not install a right-turn lane for westbound Highway 26 at Ten Eyck Road.

The City's TSP calls for left-turn lanes on Ten Eyck Road and Wolf Drive at Highway 26, project M8. This improvement alternative was not evaluated for LOS, but the findings of the evaluation without M8 improvements did not support this relatively expensive improvement. The applicant is advised to maintain the existing 39-foot street width on Ten Eyck Road frontage to allow this improvement in the future.

Finding: Maintain the width of 39 feet on Ten Eyck Road to provide the necessary width a for future left-turn lane.

Queueing Analysis

Table 7 presents the 95th percentile queue analysis for the Highway 26/Ten Eyck Road and Ten Eyck Road/Pleasant Street intersections to verify the queuing at the intersections do not conflict with each other and that the existing lane storage is not exceeded. The queue analysis is based on procedures and settings outlined in ODOT APMv2 when using Trafficware's SimTraffic (Version 10) simulation software. Table 7 includes a column with queue model results with the inclusion of the southbound left-turn lane on Ten Eyck Road, City TSP project M8, between Highway 26 and the east segment of Pleasant Street. The detailed queuing reports are provided in Appendix C.

Table 7. 95th Percentile Queue Analysis for 2029 Conditions

	1 (1	Available		y AM Pea Queue (ft)		Weekday PM Peak Hour Queue (ft)		
Intersection	ntersection Lane/Lane S	Storage ² (ft)	2029 Without Project	2029 With Project	Add SB LT Lane	2029 Without Project	2029 With Project	Add SB LT Lane
	EB LT	120	100	125	125	225	250	250
	EB TH	400	250	275	275	375	475	450
	EB TH	1,000	200	200	200	350	425	400
	EB RT	100	75	50	50	175	175	175
Highway 26/Ten Eyck Road	WB LT	100	0	0	0	50	75	75
Lyck Rodd	WB TH	1,200	275	275	300	500	500	500
	WB TH+RT	1,200	250	250	275	475	475	475
	NB	275	175	175	175	275	250	250
	SB	120	200	200	175	325	350	225
Ten Eyck	EB	120	75	50	50	75	100	75
Road/Pleasant Street ¹	WB	120	0	50	50	50	125	75

BOLD font indicates the queue exceeds the noted available storage.

Eastbound left-turn queues at the Highway 26/Ten Eyck Road signal are anticipated to spill beyond the 120 feet of available storage lane during the PM peak hour, both without and with the project trips. The queue will block the eastbound-to-westbound Highway 26 connector and the ARCO gas station driveway. These queues will be made slightly longer by the addition of the Sandy Health Clinic project trips due to the eight new PM peak hour trips turning left on to Ten Eyck Road.

Eastbound through queues at the Highway 26/Ten Eyck Road signal are anticipated to spill beyond several driveways during the PM peak hour; with the addition of the Sandy Health Clinic project trips, the queues could spill beyond the Revenue Avenue intersection, likely due to the eastbound left-turn lane overflow.

Eastbound right-turn queues at the Highway 26/Ten Eyck Road signal are anticipated to spill beyond the 100 feet of available storage lane during the PM peak hour, both without and with the project trips. The Sandy Health Clinic project trips will have negligible effect on these queues.

Southbound queues at the Highway 26/Ten Eyck Road signal are anticipated to spill beyond the upstream intersections, both without and with the project trips. The queue will routinely block the driveways closest to the intersection (serving the lot on the northwest intersection corner) and the east segment of Pleasant Street. During the PM peak hour, the west segment of Pleasant Street also will be blocked by the southbound queue. The southbound queues will be made slightly longer by the addition of the Sandy Health Clinic project trips due to most trips turning right in the PM peak hour (19 of 21 trips). The queueing is much more impacted by the 2.0%

¹ The queue lengths are reported for both intersections.

² For exclusive turn lanes, the available storage noted represents the length of the full-width lane, exclusive of the taper or transition. For continuous lanes, the available storage noted represents the distance from the intersection stop bar to the next upstream intersection or major driveway.

growth for each turning movement over the next nine years. This may be overly conservative as the 2.0% growth used in this report was based on ODOT through movements on Highway 26.

Finding: The trips from the Sandy Health Clinic do not significantly contribute to the queueing. The addition of a southbound left-turn lane to Ten Eyck Road with the future City project may reduce southbound queues at Highway 26. However, the 95th percentile queues will still extend beyond the east segment of Pleasant Street.

Recommendation: Collect additional turning movement counts in the future to verify the intersection volumes, especially the eastbound left-turn and southbound traffic movements, are growing at 2.0% annually. Reevaluate the queuing with the next TSP update.

SAFETY EVALUATION

The safety evaluation focused on crash history at the existing intersection, bike evaluation, pedestrian safety, and Americans with Disabilities Act (ADA) access.

Traffic Safety

The proposed Sandy Health Clinic was evaluated for traffic safety based on the existing crash history of the Highway 26/Ten Eyck Road intersection, sight distance of driveways, and driveway/intersection spacing. The crash history was reviewed for the last available 5-year period (January 1, 2014 to December 31, 2018). The records show 11 crashes at the Highway 26/Ten Eyck intersection with 6 rear-end crashes (5 of those on Highway 26) and 2 right-angle crashes. None were related to the right-turning vehicles and none were related to bicycles. The intersection crash rate per million vehicles entering is 0.26. The mean critical crash rate for a four leg, signalized intersection in an urban area is 0.40. See Appendix D for the crash history and crash rate calculations.

Finding: The intersection crash rate is lower than the mean critical crash rate. No further investigation is necessary.

The stopping sight distance necessary is based on the existing speed limit and the likely travel speeds on the east leg of Pleasant Street. As a local street, Pleasant Street is assumed to have a 25 mph speed limit, but based on the dead end 450 feet to the east of the proposed site driveway and an intersection of Ten Eyck Road 150 feet to the west of the proposed site driveway, speeds are assumed to be 20 mph approaching the driveway. The stopping sight distance is adequate for looking east and west, but care should be taken not to install landscaping or signs along the site frontage that may restrict sight distance below 200 feet.

Finding: The proposed driveway will meet stopping sight distance standards.

Recommendation: The site plan should take care to maintain approximately 200 feet of sight distance in both directions on Pleasant Street.

The applicant proposes elimination of an existing site driveway on Ten Eyck Road. No crashes were noted associated with this driveway, but removing it will likely reduce risk of crashes in the future. The driveway's proximity to the Highway 26 signal (70 feet) may have contributed to past crashes. The same is true of eliminating the existing driveway on Pleasant Street that is close to Ten Eyck Road but is much less likely to be a safety concern due to the low traffic volume on Pleasant Street.

Finding: The project should improve safety on Ten Eyck Road and Pleasant Street by eliminating two driveways.

Bicycle Safety

The multi-model safety evaluation is related to the bike lane and motor vehicle lane interaction at the westbound bike lane approaching the Highway 26/Ten Eyck Road intersection. The existing and future conditions are evaluated using the bicycle LTS. The westbound Highway 26 approach to Ten Eyck Road has a 135-foot-long taper (slip lane) with a dropped bike lane. The right-turn lane is approximately 50 feet long with a dashed merge area approximately 50 feet long. Based on ODOT APMv2: "A roadway with no marked bike lanes and a right-turn lane will be a high stress location unless the right-turn lane is short and rarely used. This condition will also occur if a bike lane is dropped ahead of an intersection. If the turn lane is short (less than 75') then there is no impact on the LTS."

With the development, the right-turn volume will increase from 16 vehicles per hour to approximately 22 vehicles in the AM and PM peak hour. In the AM hours only one bicycle used the westbound bike lane. In the PM peak hours, no bicycles were counted. With the short length of the right-turn lane, there is no impact on the bicycle LTS.

Finding: No mitigation is necessary for the existing slip lane and bike lane due to short length of the lane and the low turning movement volumes.

Recommendation: Make no change to the existing westbound slip lane.

Pedestrian Safety

The current site does not have a sidewalk on the east side Ten Eyck Road. The City has a sidewalk project in process to install a sidewalk on Ten Eyck Road and Pleasant Street, referred to as the SE Ten Eyck Road & Pleasant Street Curb and Sidewalk Improvements. The project will provide ADA access to the Sandy Health Clinic.

The project may be modified to reduce the turning radius on northeast corner of the Highway 26/Ten Eyck Road intersection. This will reduce vehicle speeds and provide more area for ramp improvements at the corner. This should improve pedestrian safety.

Finding: The proposed modification to the City's sidewalk project will improve pedestrian safety but not reduce the width of Ten Eyck Road below 36-feet to allow future installation of a left-turn lane.

FINDINGS

The TIA findings are summarized below:

- The proposed Sandy Health Clinic project will generate 355 net new trips on an average weekday, 34 net new trips in the AM peak hour of the adjacent streets, and 30 net new trips in the PM peak hour of the adjacent streets.
- The existing Highway 26/Ten Eyck Road intersection operates above LOS standards.
- In 2022 without the project, the Highway 26/Ten Eyck Road intersection will operate above LOS standards.
- In 2022 with the project, the Highway 26/Ten Eyck Road intersection will operate at above LOS standards.
- In 2029 without the project, the Highway 26/Ten Eyck Road intersection will operate above LOS standards
- In 2029 with the project, the Highway 26/Ten Eyck Road intersection will operate above LOS standards.
- In 2029 with the project, the operation of the Highway 26/Ten Eyck Road intersection will not significantly improve with the installation of a westbound right-turn lane on Highway 26.

- The trips from the Sandy Health Clinic do not significantly contribute to queueing at the Highway 26/Ten Eyck Road intersection.
- Maintain the width of 39 feet on Ten Eyck Road to provide the necessary width a for future left-turn lane.
- The intersection crash rate is lower than the mean critical crash rate. No further investigation is necessary.
- The proposed driveway on Pleasant Street will meet stopping sight distance standards.
- The project should improve safety on Ten Eyck Road and Pleasant Street by eliminating two existing driveways (one on each roadway).
- No mitigation is necessary for the existing slip lane and bike lane due to its short length and low turning movements.
- The proposed modification to the City's sidewalk project will improve pedestrian safety but not reduce the width of Ten Eyck Road below 36-feet to allow future installation of a left-turn lane.

RECOMMENDATIONS

The TIA recommendations are summarized below:

- Do not install a right-turn lane for westbound Highway 26 at Ten Eyck Road.
- Make no change to the existing westbound slip lane.
- Collect additional turning movement counts in the future to verify the intersection volumes, especially the eastbound left-turn and southbound traffic volumes, are growing at 2.0% annually. Reevaluate the queuing with the next TSP update.
- The site plan should take care to maintain approximately 200 feet of sight distance in both directions on Pleasant Street.

CLOSING

Please feel free to contact me at 360.567.2117 or John.Manix@pbsusa.com with any questions or comments.

Sincerely,

John Manix, PE

Senior Traffic Engineer

Attachments: Figure 1. Vicinity Map

Figure 2. Site Plan

Figure 3. Trip Distribution and Assignment Figure 4. 2040 With Project Volumes

Appendix A. Traffic Counts

Appendix B. Trip Generation Calculations Appendix C. Level of Service Calculations

Appendix D. Crash History

OREGON

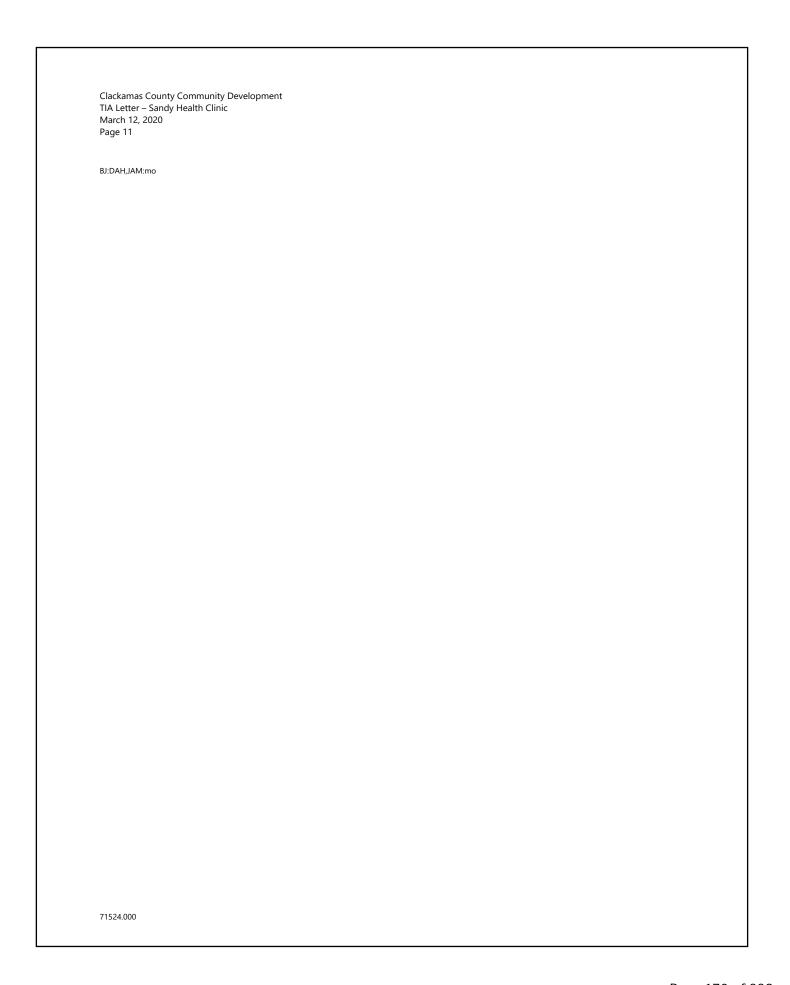
ONEGON

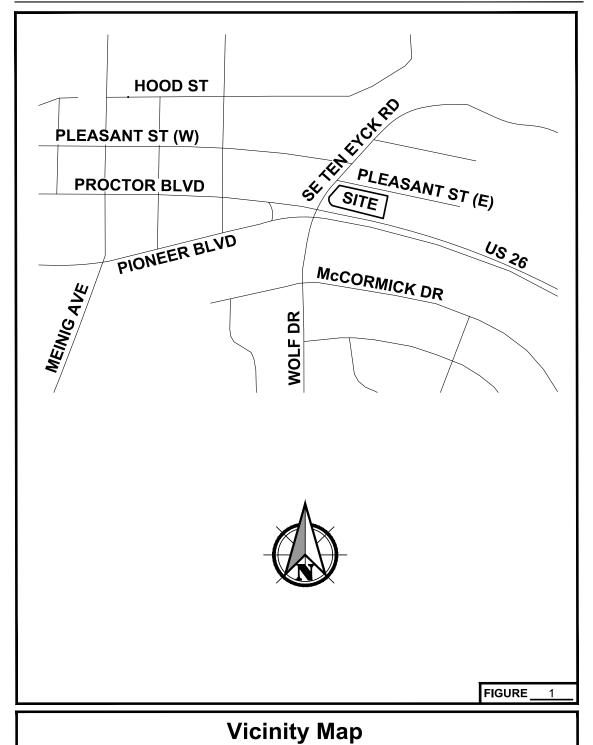
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EXPIRES: 12/31/20

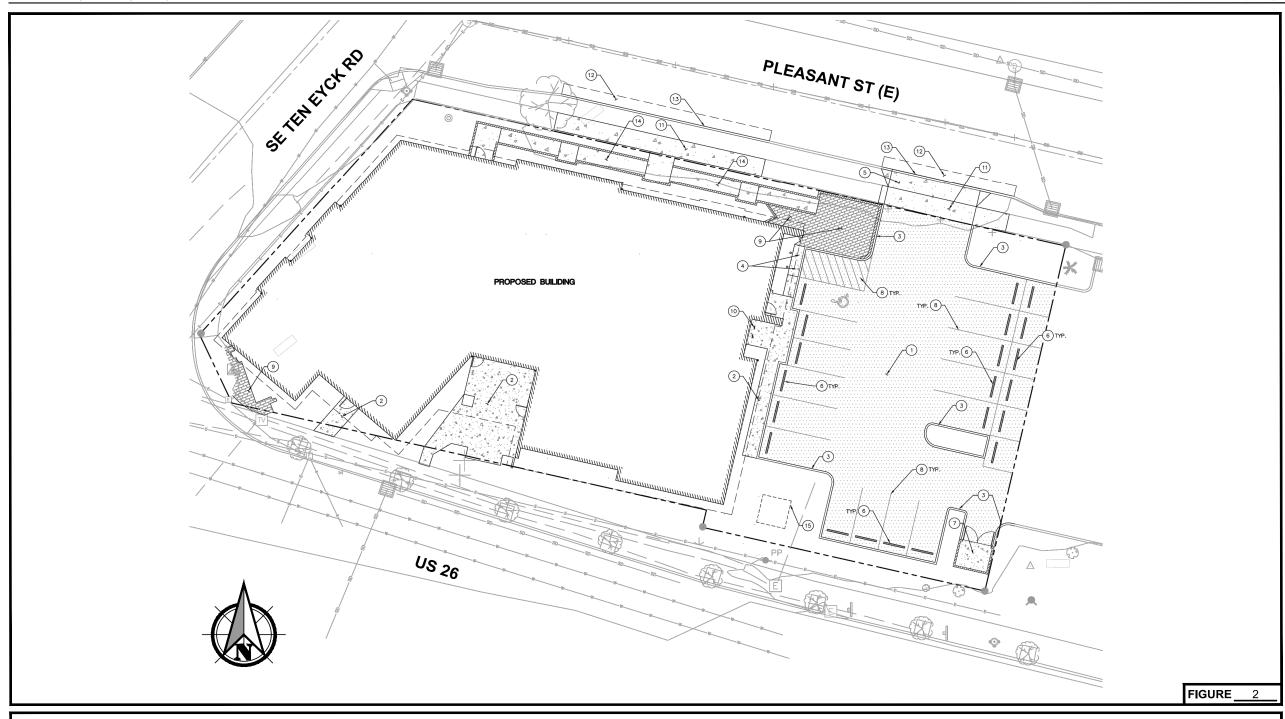




Sandy Health Clinic

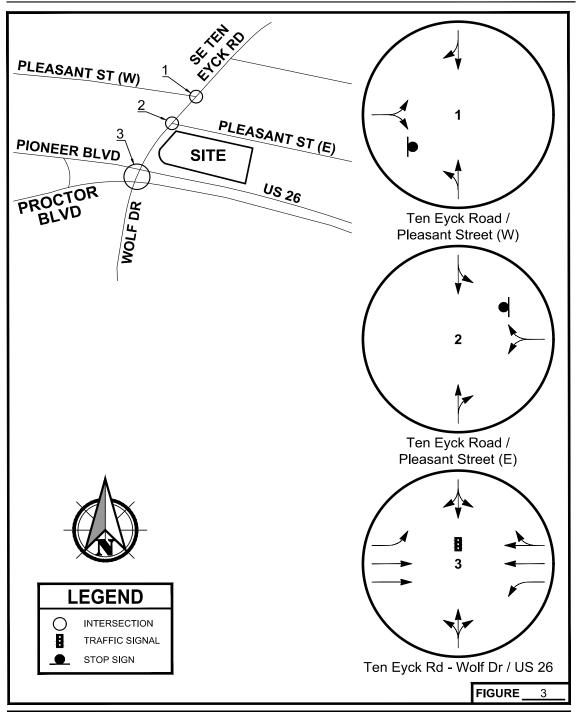
PBS

Traffic Impact Analysis
Clackamas County Community Development
Sandy, Oregon



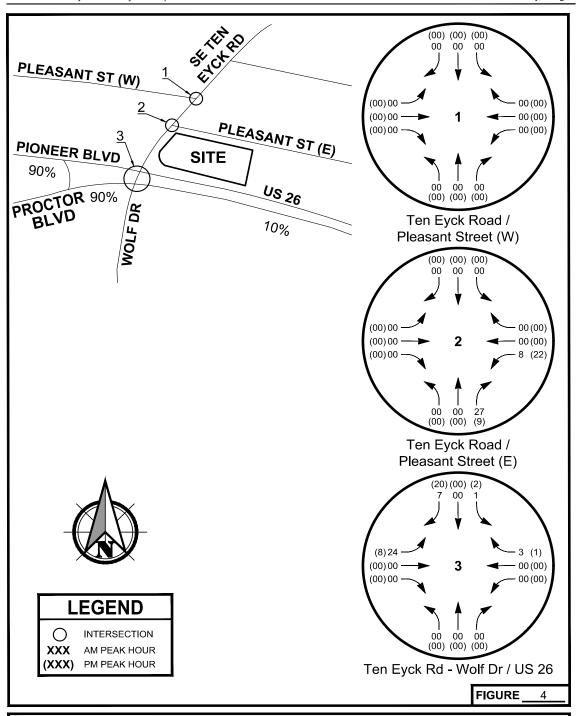
Site Plan
Sandy Health Clinic





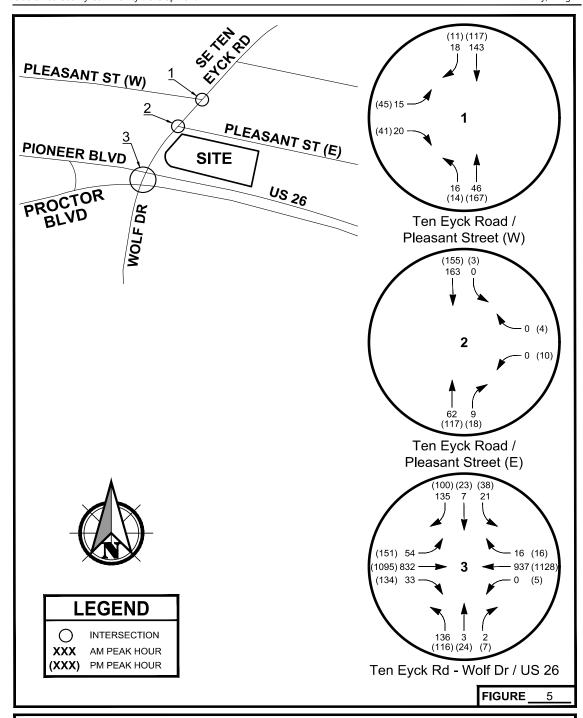
Existing Lane Configurations Sandy Health Clinic





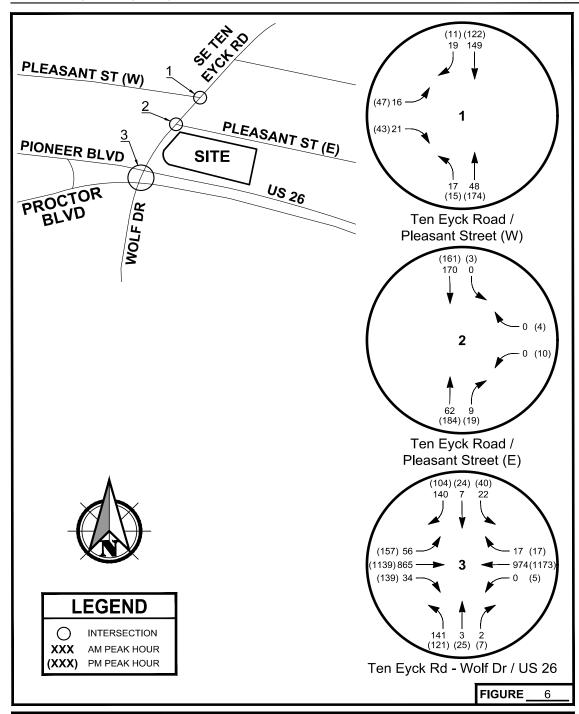
Trip Distribution & Assignment Sandy Health Clinic





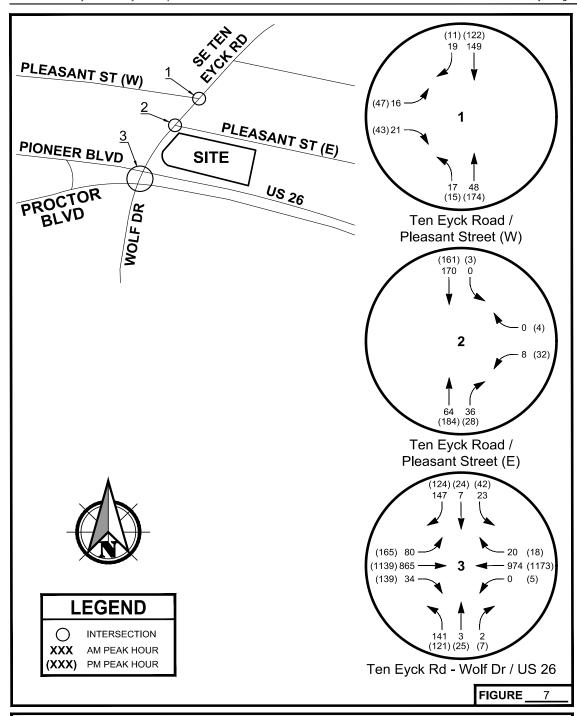
2020 Existing Traffic Volumes Sandy Health Clinic





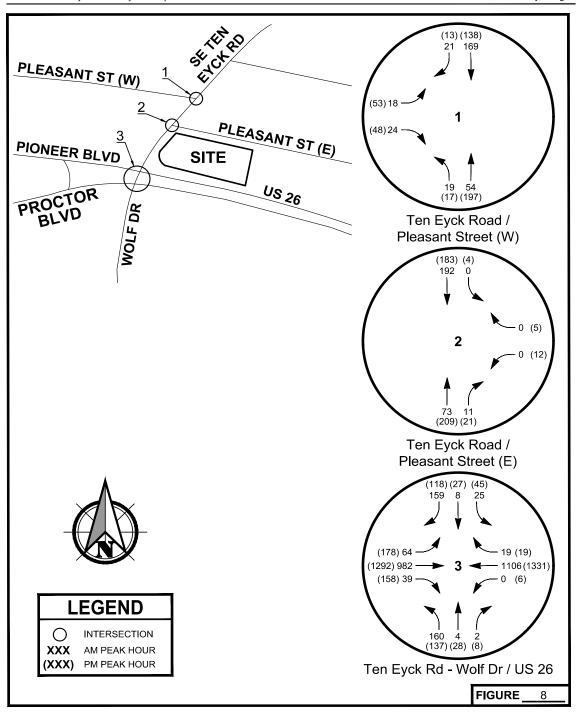
2022 Without Project Traffic Volumes Sandy Health Clinic





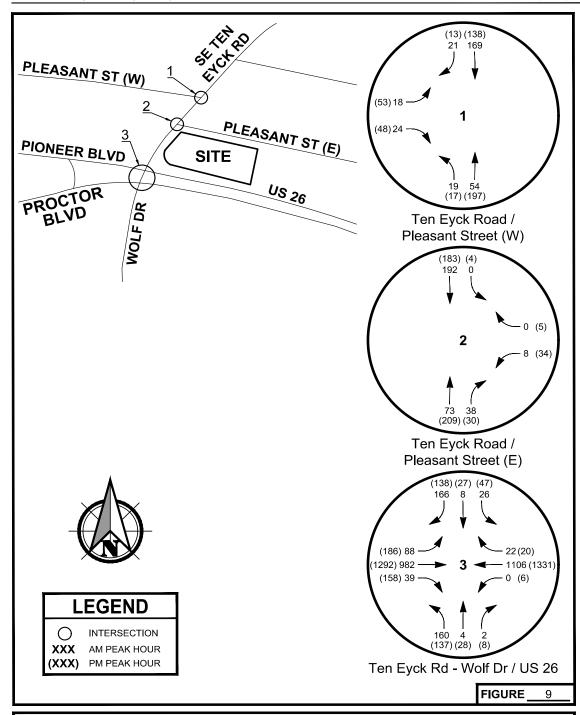
2022 With Project Traffic Volumes Sandy Health Clinic





2029 Without Project Traffic Volumes Sandy Health Clinic





2029 With Project Traffic Volumes Sandy Health Clinic



Appendix A Traffic Counts

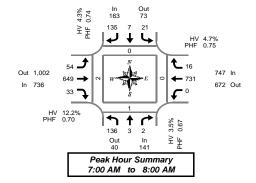
Total Vehicle Summary



Ten Eyck Rd & Hwy 26

Wednesday, February 19, 2020 7:00 AM to 9:00 AM

5-Minute Interval Summary 7:00 AM to 9:00 AM



Interval Start			bound vck Rd			South Ten E				Eastb Hw				Westh			Interval		Pedes		
Time		T	R	Bikes		T	R	Bikes	L	т 1	R	Bikes	L	Г т	R	Bikes	Total	North	South	East	West
7:00 AM	13	0	0	0	3	0	11	0	2	33	2	0	0	61	1	0	126	0	0	0	0
7:05 AM	16	0	0	0	0	0	15	0	6	39	0	0	0	78	1	0	155	0	1	0	1
7:10 AM	20	0	0	0	3	1	12	0	2	26	2	0	0	83	1	0	150	0	0	0	0
7:15 AM	15	0	0	0	2	0	18	0	2	53	3	0	0	85	1	0	179	0	0	0	0
7:20 AM	16	2	0	0	2	0	17	0	3	39	3	0	0	64	0	0	146	0	0	0	0
7:25 AM	9	0	0	0	1	0	12	0	2	39	4	0	0	74	3	0	144	0	0	0	0
7:30 AM	7	0	0	0	0	1	9	0	4	53	4	0	0	58	3	0	139	0	0	0	0
7:35 AM	12	1	0	0	0	1	13	0	10	57	4	0	0	45	1	0	144	0	0	0	0
7:40 AM	5	0	0	0	3	1	5	0	7	71	2	0	0	55	0	0	149	0	0	0	0
7:45 AM	8	0	0	0	3	1	13	0	4	86	3	0	0	49	3	1	170	0	0	0	1
7:50 AM	5	0	1	0	4	1	4	0	8	79	4	0	0	41	0	0	147	0	0	0	0
7:55 AM	10	0	1	0	0	1	6	0	4	74	2	0	0	38	2	0	138	0	0	0	0
8:00 AM	4	0	0	0	1	0	8	0	1	62	4	0	0	45	1	0	126	0	0	0	0
8:05 AM	12	0	0	0	1	0	9	0	3	55	1	0	0	43	0	0	124	0	0	0	1
8:10 AM	5	0	0	0	0	0	9	0	4	60	2	0	0	46	0	0	126	0	0	0	0
8:15 AM	4	0	0	0	0	0	8	0	4	47	1	0	1	51	0	0	116	0	0	0	0
8:20 AM	7	0	0	0	3	0	12	0	4	62	1	0	0	48	2	0	139	0	0	0	0
8:25 AM	6	2	1	0	1	0	11	0	3	61	4	0	0	40	0	0	129	0	0	0	0
8:30 AM	7	1	0	0	6	0	9	0	7	60	1	0	1	47	1	0	140	0	0	0	1
8:35 AM	12	0	0	0	1	1	7	0	5	53	3	0	0	53	2	0	137	0	0	0	0
8:40 AM	9	1	1	0	2	11	11	0	1	56	2	0	1	53	0	0	138	0	0	0	0
8:45 AM	15	0	0	0	5	0	13	0	10	64	3	0	0	51	1	0	162	0	0	0	0
8:50 AM	4	1	0	0	2	0	14	0	5	60	3	0	0	62	2	0	153	0	0	0	0
8:55 AM	3	0	0	0	3	1	4	0	3	60	2	0	0	45	0	0	121	2	0	0	0
Total Survev	224	8	4	0	46	10	250	0	104	1,349	60	0	3	1,315	25	1	3,398	2	1	0	4

15-Minute Interval Summary 7:00 AM to 9:00 AM

Interval		North	bound			South	bound			Eastb	ound			Westk	ound				Pedes	strians	
Start		Ten E	yck Rd			Ten E	yck Rd			Hw	/ 26			Hwy	/ 26		Interval		Cross	swalk	
Time	L	T	R	Bikes	L	T	R	Bikes	L	T	R	Bikes	L	T	R	Bikes	Total	North	South	East	We
7:00 AM	49	0	0	0	6	1	38	0	10	98	4	0	0	222	3	0	431	0	1	0	1
7:15 AM	40	2	0	0	5	0	47	0	7	131	10	0	0	223	4	0	469	0	0	0	0
7:30 AM	24	1	0	0	3	3	27	0	21	181	10	0	0	158	4	0	432	0	0	0	0
7:45 AM	23	0	2	0	7	3	23	0	16	239	9	0	0	128	5	1	455	0	0	0	1
8:00 AM	21	0	0	0	2	0	26	0	8	177	7	0	0	134	1	0	376	0	0	0	1
8:15 AM	17	2	1	0	4	0	31	0	11	170	6	0	1	139	2	0	384	0	0	0	0
8:30 AM	28	2	1	0	9	2	27	0	13	169	6	0	2	153	3	0	415	0	0	0	1
8:45 AM	22	1	0	0	10	1	31	0	18	184	8	0	0	158	3	0	436	2	0	0	0
Total Survey	224	8	4	0	46	10	250	0	104	1,349	60	0	3	1,315	25	1	3,398	2	1	0	4

Peak Hour Summary 7:00 AM to 8:00 AM

Ву			bound yck Rd				bound yck Rd				ound / 26				oound y 26		Total
Approach	In					Out	Total	Bikes	In	Out	Total	Bikes	In	Out	Total	Bikes	
Volume	141	40	181	0	163	73	236	0	736	1,002	1,738	0	747	672	1,419	1	1,787
%HV						4.3	3%			12.	2%			4.	7%		7.7%
PHF		0.67				0.	74			0.	70			0.	75		0.92

	Pedes	trians	
	Cross	swalk	
North	South	East	West
0	1	0	2

By Movement			bound yck Rd				bound yck Rd			Eastb Hwy				Westl	oound y 26		Total
Movement	L	T	R	Total	L	T	R	Total	L	T	R	Total	L	Т	R	Total	
Volume	136	3	2	141	21	7	135	163	54	649	33	736	0	731	16	747	1,787
%HV	3.7%	0.0%	0.0%	3.5%	23.8%	0.0%	1.5%	4.3%	5.6%	13.1%	6.1%	12.2%	0.0%	4.8%	0.0%	4.7%	7.7%
PHF	0.67	0.38	0.25	0.67	0.53	0.58	0.72	0.74	0.64	0.68	0.69	0.70	0.00	0.74	0.57	0.75	0.92

Rolling Hour Summary 7:00 AM to 9:00 AM

Interval		North	bound			South	bound			Eastb	ound			West	bound				Pedes	strians	
Start		Ten E	yck Rd			Ten E	yck Rd			Hw	y 26			Hw	y 26		Interval		Cross	swalk	
Time	L	T	R	Bikes	L	T	R	Bikes	L	T	R	Bikes	L	T	R	Bikes	Total	North	South	East	West
7:00 AM	136	3	2	0	21	7	135	0	54	649	33	0	0	731	16	1	1,787	0	1	0	2
7:15 AM	108	3	2	0	17	6	123	0	52	728	36	0	0	643	14	1	1,732	0	0	0	2
7:30 AM	85	3	3	0	16	6	107	0	56	767	32	0	1	559	12	1	1,647	0	0	0	2
7:45 AM	89	4	4	0	22	5	107	0	48	755	28	0	3	554	11	1	1,630	0	0	0	3
8:00 AM	88	5	2	0	25	3	115	0	50	700	27	0	3	584	9	0	1 611	2	0	0	2

Heavy Vehicle Summary



Clay Carney (503) 833-2740

Ten Eyck Rd & Hwy 26

Wednesday, February 19, 2020 7:00 AM to 9:00 AM

In 90 Peak Hour Summary 7:00 AM to 8:00 AM

Heavy Vehicle 5-Minute Interval Summary 7:00 AM to 9:00 AM

Interval Start		North Ten E					bound yck Rd			Eastb Hw	ound y 26				oound y 26		Interval
Time	L	T	R	Total	L	T	R	Total	L	T	R	Total	L	Т	R	Total	Total
7:00 AM	1	0	0	1	0	0	0	0	0	8	1	9	0	5	0	5	15
7:05 AM	1	0	0	1	0	0	0	0	0	13	0	13	0	2	0	2	16
7:10 AM	0	0	0	0	1	0	0	1	0	6	0	6	0	4	0	4	11
7:15 AM	2	0	0	2	0	0	0	0	1	7	0	8	0	3	0	3	13
7:20 AM	0	0	0	0	1	0	0	1	0	5	0	5	0	4	0	4	10
7:25 AM	0	0	0	0	0	0	0	0	0	5	0	5	0	2	0	2	7
7:30 AM	0	0	0	0	0	0	0	0	1	2	0	3	0	0	0	0	3
7:35 AM	0	0	0	0	0	0	0	0	1	3	0	4	0	2	0	2	6
7:40 AM	0	0	0	0	1	0	0	1	0	10	0	10	0	4	0	4	15
7:45 AM	0	0	0	0	1	0	1	2	0	8	0	8	0	4	0	4	14
7:50 AM	0	0	0	0	1	0	1	2	0	9	1	10	0	1	0	1	13
7:55 AM	1	0	0	1	0	0	0	0	0	9	0	9	0	4	0	4	14
8:00 AM	0	0	0	0	0	0	0	0	0	7	0	7	0	4	0	4	11
8:05 AM	0	0	0	0	0	0	1	1	0	5	1	6	0	5	0	5	12
8:10 AM	0	0	0	0	0	0	0	0	0	5	0	5	0	7	0	7	12
8:15 AM	0	0	0	0	0	0	0	0	0	2	0	2	0	5	0	5	7
8:20 AM	1	0	0	1	0	0	0	0	0	2	0	2	0	8	0	8	11
8:25 AM	0	0	1	1	0	0	0	0	0	8	0	8	0	6	0	6	15
8:30 AM	1	0	0	1	1	0	0	1	0	3	0	3	1	11	0	12	17
8:35 AM	1	0	0	1	0	0	1	1	1	3	0	4	0	2	1	3	9
8:40 AM	0	0	0	0	0	0	0	0	0	3	0	3	0	3	0	3	6
8:45 AM	0	0	0	0	0	0	0	0	0	4	0	4	0	3	0	3	7
8:50 AM	0	0	0	0	0	0	2	2	0	6	11	7	0	5	0	5	14
8:55 AM	0	0	0	0	0	0	0	0	0	8	0	8	0	3	0	3	11
Total Survey	8	0	1	9	6	0	6	12	4	141	4	149	1	97	1	99	269

Heavy Vehicle 15-Minute Interval Summary 7:00 AM to 9:00 AM

Interval		North	bound			South	bound			Eastb	ound			West	oound		
Start		Ten E	yck Rd			Ten E	yck Rd			Hw	/ 26			Hw	y 26		Interval
Time	L	Т	R	Total	L	Т	R	Total	L	Т	R	Total	L	T	R	Total	Total
7:00 AM	2	0	0	2	1	0	0	1	0	27	1	28	0	11	0	11	42
7:15 AM	2	0	0	2	1	0	0	1	1	17	0	18	0	9	0	9	30
7:30 AM	0	0	0	0	1	0	0	1	2	15	0	17	0	6	0	6	24
7:45 AM	1	0	0	1	2	0	2	4	0	26	1	27	0	9	0	9	41
8:00 AM	0	0	0	0	0	0	1	1	0	17	1	18	0	16	0	16	35
8:15 AM	1	0	1	2	0	0	0	0	0	12	0	12	0	19	0	19	33
8:30 AM	2	0	0	2	1	0	1	2	1	9	0	10	1	16	1	18	32
8:45 AM	0	0	0	0	0	0	2	2	0	18	1	19	0	11	0	11	32
Total Survey	8	0	1	9	6	0	6	12	4	141	4	149	1	97	1	99	269

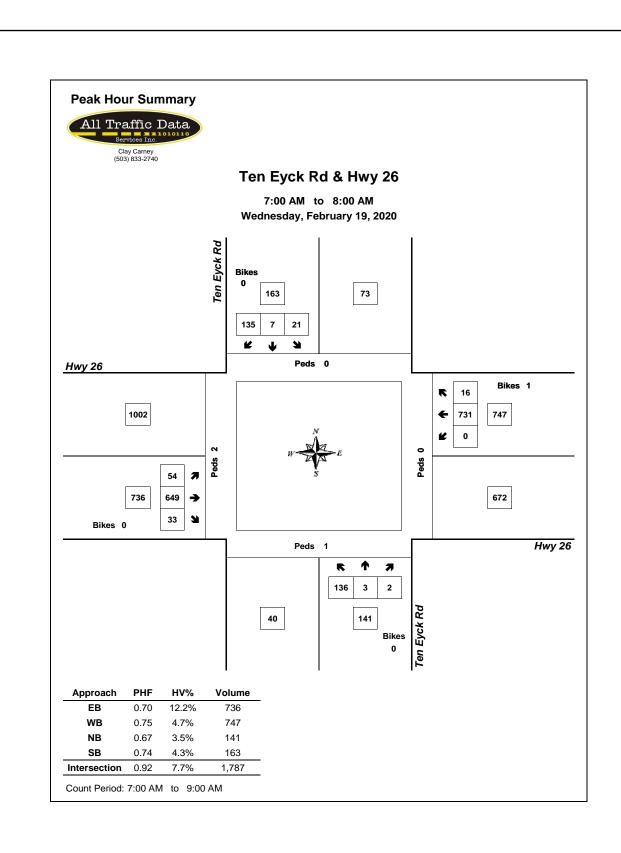
Heavy Vehicle Peak Hour Summary 7:00 AM to 8:00 AM

Bv		North	bound		South	bound		Easth	ound		West	bound	
,		Ten E	yck Rd		Ten E	yck Rd		Hw	y 26		Hw	y 26	Total
Approach	In	Out	Total	In	Out	Total	In	Out	Total	In	Out	Total	1
Volume	5	2	7	7	3	10	90	42	132	35	90	125	137
PHF	0.42			0.35			0.80			0.80			0.82

By		Northi Ten Ey	bound yck Rd				bound yck Rd				ound / 26			Westl			Total
Movement	L	Т	R	Total	L	Т	R	Total	L	Т	R	Total	L	Т	R	Total	
Volume	5	0	0	5	5	0	2	7	3	85	2	90	0	35	0	35	137
PHF	0.42	0.00	0.00	0.42	0.42	0.00	0.25	0.35	0.38	0.79	0.50	0.80	0.00	0.80	0.00	0.80	0.82

Heavy Vehicle Rolling Hour Summary 7:00 AM to 9:00 AM

Interval		North	bound			South	bound			Fasth	ound			West	nound		
Start			yck Rd				yck Rd				y 26			Hwy			Interval
Time	L	T	R	Total	L	Т	R	Total	L	T	R	Total	L	T	R	Total	Total
7:00 AM	5	0	0	5	5	0	2	7	3	85	2	90	0	35	0	35	137
7:15 AM	3	0	0	3	4	0	3	7	3	75	2	80	0	40	0	40	130
7:30 AM	2	0	1	3	3	0	3	6	2	70	2	74	0	50	0	50	133
7:45 AM	4	0	1	5	3	0	4	7	1	64	2	67	1	60	1	62	141
8:00 AM	3	0	1	4	1	0	4	5	1	56	2	59	1	62	1	64	132



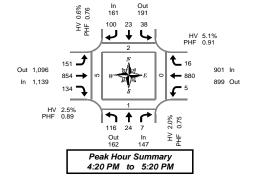
Total Vehicle Summary



Ten Eyck Rd & Hwy 26

Wednesday, February 19, 2020 4:00 PM to 6:00 PM

5-Minute Interval Summary 4:00 PM to 6:00 PM



Interval Start			bound yck Rd				bound yck Rd			Eastb Hwy	ound / 26			Westl Hwy			Interval		Cros	strians swalk	
Time	L	T	R	Bikes	L	Т	R	Bikes	L	T	R	Bikes	L	T	R	Bikes	Total	North	South	East	West
4:00 PM	5	4	0	0	2	2	14	0	11	55	19	0	1	76	2	0	191	0	0	0	1
4:05 PM	10	2	1	0	4	2	6	0	17	78	5	0	0	48	4	0	177	0	0	0	0
4:10 PM	4	3	0	0	0	1	7	0	13	73	12	0	0	68	1	0	182	4	0	0	2
4:15 PM	4	3	0	0	5	0	5	0	9	67	12	0	0	81	3	0	189	1	0	0	0
4:20 PM	6	2	0	0	1	1	5	0	11	73	8	0	0	81	0	0	188	2	0	0	0
4:25 PM	11	0	1	0	4	1	8	0	8	60	11	0	0	72	0	0	176	0	0	0	0
4:30 PM	6	2	0	0	3	0	4	0	9	70	17	0	0	64	2	0	177	0	0	0	0
4:35 PM	14	2	0	0	2	4	6	0	13	70	6	0	0	83	1	0	201	0	0	0	0
4:40 PM	11	2	0	0	0	6	10	0	19	80	12	0	11	75	1	0	217	0	0	0	0
4:45 PM	10	1	2	0	2	0	4	0	18	85	18	0	11	63	2	0	206	0	0	0	3
4:50 PM	12	6	0	0	3	2	15	0	11	55	12	0	11	61	3	0	181	0	1	0	1
4:55 PM	11	1	2	0	4	2	6	0	9	52	14	0	11	89	3	0	194	0	0	0	0
5:00 PM	12	4	1	0	4	2	9	0	18	60	7	0	11	85	0	0	203	0	0	0	0
5:05 PM	7	2	0	0	4	2	14	0	13	99	7	0	0	67	1	0	216	0	0	0	0
5:10 PM	7	1	0	0	6	2	7	0	8	72	13	0	0	71	3	0	190	0	0	0	1
5:15 PM	9	1	1	0	5	1	12	0	14	78	9	0	0	69	0	0	199	0	0	0	0
5:20 PM	8	1	0	0	4	1	4	0	12	59	8	0	11	68	1	0	167	0	0	0	0
5:25 PM	6	2	0	0	5	0	7	0	11	71	7	0	11	73	0	0	183	1	0	0	0
5:30 PM	4	0	0	0	1	3	9	0	14	77	10	0	0	62	2	0	182	0	0	0	0
5:35 PM	8	2	1	0	1	0	6	0	18	80	10	0	0	36	0	0	162	0	1	0	0
5:40 PM	5	1	0	0	4	11	10	0	16	57	4	0	0	48	0	0	146	11	0	0	0
5:45 PM	7	3	1	0	3	0	7	0	12	65	9	0	0	62	1	0	170	0	0	0	1
5:50 PM	12	1	0	0	2	0	8	0	13	72	5	0	0	63	1	0	177	1	0	0	2
5:55 PM	4	2	1	0	3	1	9	0	17	63	14	0	0	48	1	0	163	0	0	0	0
Total Survev	193	48	11	0	72	34	192	0	314	1,671	249	0	8	1,613	32	0	4,437	10	2	0	11

15-Minute Interval Summary

4:00 PM to 6:00 PM

Interval			bound				bound				ound			Westb						strians	
Start		Ten E	yck Rd			Ten E	yck Rd			Hw	y 26			Hwy	/ 26		Interval		Cros	swalk	
Time	L	Т	R	Bikes	L	T	R	Bikes	L	T	R	Bikes	L	T	R	Bikes	Total	North	South	East	Γ
4:00 PM	19	9	1	0	6	5	27	0	41	206	36	0	1	192	7	0	550	4	0	0	Г
4:15 PM	21	5	1	0	10	2	18	0	28	200	31	0	0	234	3	0	553	3	0	0	Γ
4:30 PM	31	6	0	0	5	10	20	0	41	220	35	0	1	222	4	0	595	0	0	0	Γ
4:45 PM	33	8	4	0	9	4	25	0	38	192	44	0	3	213	8	0	581	0	1	0	Γ
5:00 PM	26	7	1	0	14	6	30	0	39	231	27	0	1	223	4	0	609	0	0	0	Γ
5:15 PM	23	4	1	0	14	2	23	0	37	208	24	0	2	210	1	0	549	1	0	0	Γ
5:30 PM	17	3	1	0	6	4	25	0	48	214	24	0	0	146	2	0	490	1	1	0	Γ
5:45 PM	23	6	2	0	8	1	24	0	42	200	28	0	0	173	3	0	510	1	0	0	Γ
Total Survey	193	48	11	0	72	34	192	0	314	1,671	249	0	8	1,613	32	0	4,437	10	2	0	

Peak Hour Summary

4:20 PM to 5:20 PM

By			oound ck Rd				bound yck Rd				ound / 26				oound y 26		Total
Approach	In	Out	Total	Bikes	In	Out	Total	Bikes	In	Out	Total	Bikes	In	Out	Total	Bikes	
Volume	147	162	309	0	161	161 191 352 0				1,096	2,235	0	901	899	1,800	0	2,348
%HV		2.0)%			0.6	5%			2.5	5%			5.	1%		3.4%
PHF		0.	75			0.	76			0.	89			0.	91		0.94

	Pedes	trians	
	Cross	swalk	
North	South	East	West
2	1	0	- 5

By Movement		North Ten E	bound yck Rd				bound yck Rd				ound y 26			Westl: Hwy			Total
wovernent	L	Т	R	Total	L	T	R	Total	L	Т	R	Total	L	Т	R	Total	
Volume	116	24	7	147	38	23	100	161	151	854	134	1,139	5	880	16	901	2,348
%HV	0.9%	8.3%	0.0%	2.0%	0.0%	0.0%	1.0%	0.6%	0.7%	3.3%	0.0%	2.5%	0.0%	5.2%	0.0%	5.1%	3.4%
PHF	0.83	0.55	0.44	0.75	0.63	0.58	0.76	0.76	0.76	0.86	0.76	0.89	0.42	0.91	0.50	0.91	0.94

Rolling Hour Summary 4:00 PM to 6:00 PM

Interval		North	bound			South	bound			Easth	ound			Westk	oound				Pedes	trians	
Start		Ten E	yck Rd			Ten E	yck Rd			Hw	y 26			Hwy	y 26		Interval		Cross	swalk	
Time	L	Т	R	Bikes	L	T	R	Bikes	L	T	R	Bikes	L	Т	R	Bikes	Total	North	South	East	West
4:00 PM	104	28	6	0	30	21	90	0	148	818	146	0	5	861	22	0	2,279	7	1	0	7
4:15 PM	111	26	6	0	38	22	93	0	146	843	137	0	5	892	19	0	2,338	3	1	0	5
4:30 PM	113	25	6	0	42	22	98	0	155	851	130	0	7	868	17	0	2,334	1	1	0	5
4:45 PM	99	22	7	0	43	16	103	0	162	845	119	0	6	792	15	0	2,229	2	2	0	5
5:00 PM	80	20	- 5	0	12	12	102	0	166	952	102	0	2	752	10	0	2.159	2	- 1	0	4

Heavy Vehicle Summary



Clay Carney (503) 833-2740

Ten Eyck Rd & Hwy 26

Wednesday, February 19, 2020 4:00 PM to 6:00 PM

In 29 Peak Hour Summary 4:20 PM to 5:20 PM

Heavy Vehicle 5-Minute Interval Summary 4:00 PM to 6:00 PM

Interval		North	bound			South	bound			Easth	ound			West	oound		
Start		Ten E	yck Rd			Ten E	yck Rd			Hw	y 26			Hw	y 26		Interval
Time	L	T	R	Total	L	T	R	Total	L	T	R	Total	L	T	R	Total	Total
4:00 PM	1	0	0	1	0	0	2	2	0	3	0	3	0	4	1	5	11
4:05 PM	0	0	0	0	0	0	0	0	0	8	0	8	0	4	0	4	12
4:10 PM	0	0	0	0	0	0	0	0	0	2	0	2	0	5	0	5	7
4:15 PM	0	0	0	0	0	0	0	0	0	3	0	3	0	6	1	7	10
4:20 PM	0	0	0	0	0	0	0	0	0	8	0	8	0	6	0	6	14
4:25 PM	0	0	0	0	0	0	0	0	0	4	0	4	0	5	0	5	9
4:30 PM	1	2	0	3	0	0	0	0	0	2	0	2	0	3	0	3	8
4:35 PM	0	0	0	0	0	0	0	0	0	2	0	2	0	4	0	4	6
4:40 PM	0	0	0	0	0	0	0	0	0	2	0	2	0	3	0	3	5
4:45 PM	0	0	0	0	0	0	0	0	0	2	0	2	0	4	0	4	6
4:50 PM	0	0	0	0	0	0	0	0	0	1	0	1	0	3	0	3	4
4:55 PM	0	0	0	0	0	0	0	0	0	1	0	1	0	7	0	7	8
5:00 PM	0	0	0	0	0	0	0	0	0	4	0	4	0	3	0	3	7
5:05 PM	0	0	0	0	0	0	0	0	1	0	0	1	0	5	0	5	6
5:10 PM	0	0	0	0	0	0	1	1	0	2	0	2	0	2	0	2	5
5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	1
5:20 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	3	0	3	3
5:25 PM	0	0	0	0	0	0	0	0	0	1	0	1	0	2	0	2	3
5:30 PM	0	0	0	0	0	0	0	0	0	2	0	2	0	2	0	2	4
5:35 PM	0	0	0	0	0	0	0	0	0	1	0	1	0	2	0	2	3
5:40 PM	1	0	0	1	0	0	0	0	0	1	0	1	0	0	0	0	2
5:45 PM	0	0	0	0	0	0	0	0	1	1	0	2	0	2	0	2	4
5:50 PM	0	0	0	0	0	0	0	0	0	4	0	4	0	3	0	3	7
5:55 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0	2	2
Total Survey	3	2	0	5	0	0	3	3	2	54	0	56	0	81	2	83	147

Heavy Vehicle 15-Minute Interval Summary 4:00 PM to 6:00 PM

Interval		North	bound			South	bound			Easth	ound			Westk	oound		
Start		Ten E	yck Rd			Ten E	yck Rd			Hw	y 26			Hwy	y 26		Interval
Time	L	T	R	Total	L	T	R	Total	L	T	R	Total	L	T	R	Total	Total
4:00 PM	1	0	0	1	0	0	2	2	0	13	0	13	0	13	1	14	30
4:15 PM	0	0	0	0	0	0	0	0	0	15	0	15	0	17	1	18	33
4:30 PM	1	2	0	3	0	0	0	0	0	6	0	6	0	10	0	10	19
4:45 PM	0	0	0	0	0	0	0	0	0	4	0	4	0	14	0	14	18
5:00 PM	0	0	0	0	0	0	1	1	1	6	0	7	0	10	0	10	18
5:15 PM	0	0	0	0	0	0	0	0	0	1	0	1	0	6	0	6	7
5:30 PM	1	0	0	1	0	0	0	0	0	4	0	4	0	4	0	4	9
5:45 PM	0	0	0	0	0	0	0	0	1	5	0	6	0	7	0	7	13
Total Survey	3	2	0	5	0	0	3	3	2	54	0	56	0	81	2	83	147

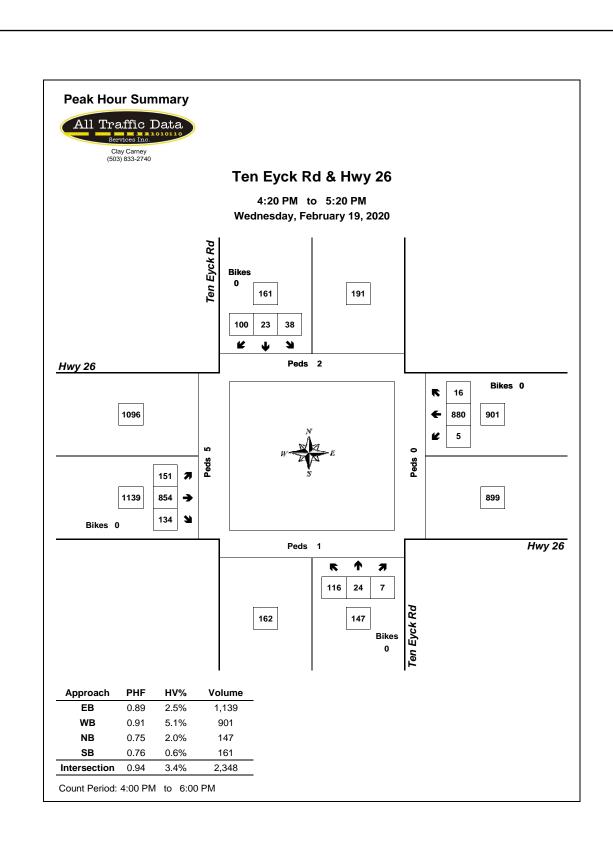
Heavy Vehicle Peak Hour Summary 4:20 PM to 5:20 PM

Bv		North	bound		South	bound		Easth	ound		West	bound	
,		Ten E	yck Rd		Ten E	yck Rd		Hw	y 26		Hw	y 26	Total
Approach	In				Out	Total	In	Out	Total	In	Out	Total	
Volume	3	3 0 3			3	4	29	48	77	46	28	74	79
PHF	0.25			0.25			0.52			0.77			0.64

By			bound yck Rd				bound yck Rd				ound / 26			Westl	oound / 26		Total
Movement	L	Т	R	Total	L	Т	R	Total	L	Т	R	Total	L	Т	R	Total	
Volume	1	2	0	3	0	0	1	1	1	28	0	29	0	46	0	46	79
PHF	0.25	0.25	0.00	0.25	0.00	0.00	0.25	0.25	0.25	0.50	0.00	0.52	0.00	0.77	0.00	0.77	0.64

Heavy Vehicle Rolling Hour Summary 4:00 PM to 6:00 PM

Interval Start			bound yck Rd				bound yck Rd				oound v 26			West			Interval
Time	L	T	R	Total	L	T	R	Total	L	T	R	Total	L	Т	R	Total	Total
4:00 PM	2	2	0	4	0	0	2	2	0	38	0	38	0	54	2	56	100
4:15 PM	1	2	0	3	0	0	1	1	1	31	0	32	0	51	1	52	88
4:30 PM	1	2	0	3	0	0	1	1	1	17	0	18	0	40	0	40	62
4:45 PM	1	0	0	1	0	0	1	1	1	15	0	16	0	34	0	34	52
E-00 DM	1	0	0	1	0	0	1	- 1	2	16	0	10	0	27	0	27	47



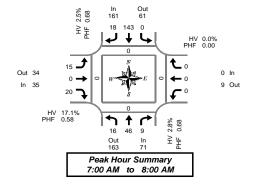
Total Vehicle Summary



Ten Eyck Rd & Pleasant St

Wednesday, February 19, 2020 7:00 AM to 9:00 AM

5-Minute Interval Summary 7:00 AM to 9:00 AM



Interval Start			bound yck Rd			South Ten E					oound ant St			Westi Pleas	bound ant St		Interval		Cros	strians swalk	
Time	L	T	R	Bikes	L	Т	R	Bikes	L	T	R	Bikes	L	Т	R	Bikes	Total	North	South	East	West
7:00 AM	1	5	2	0	0	11	0	0	0	0	1	0	0	0	0	0	20	0	0	0	0
7:05 AM	1	2	0	0	0	15	1	0	0	0	1	0	0	0	0	0	20	0	0	0	0
7:10 AM	1	0	0	0	0	18	1	0	0	0	1	0	0	0	0	0	21	0	0	0	0
7:15 AM	2	3	1	0	0	19	4	0	0	0	2	0	0	0	0	0	31	0	0	0	0
7:20 AM	3	1	1	0	0	16	1	0	0	0	1	0	0	0	0	0	23	0	0	0	0
7:25 AM	3	4	0	0	0	11	11	0	4	0	2	0	0	0	0	0	25	0	0	0	0
7:30 AM	1	6	2	0	0	9	3	0	0	0	2	0	0	0	0	0	23	0	0	0	0
7:35 AM	1	4	0	0	0	9	2	0	4	0	3	0	0	0	0	0	23	0	0	0	0
7:40 AM	0	11	1	0	0	10	0	0	2	0	1	0	0	0	0	0	25	0	0	0	0
7:45 AM	1	3	0	0	0	13	2	0	3	0	1	0	0	0	0	0	23	0	0	0	0
7:50 AM	1	4	1	0	0	4	1	0	1	0	5	0	0	0	0	0	17	0	0	0	0
7:55 AM	1	3	1	0	0	8	2	0	1	0	0	0	0	0	0	0	16	0	0	0	0
8:00 AM	0	1	2	0	0	6	11	0	3	0	1	0	0	0	0	0	14	0	0	0	0
8:05 AM	0	2	1	0	0	6	1	0	0	0	1	0	1	0	0	0	12	0	0	0	0
8:10 AM	0	3	0	0	0	13	0	0	1	0	0	0	0	0	0	0	17	0	0	0	0
8:15 AM	0	5	1	0	0	12	2	0	1	0	2	0	0	1	0	0	24	0	0	0	0
8:20 AM	0	6	0	0	0	8	1	0	0	0	1	0	0	0	0	0	16	0	0	0	0
8:25 AM	2	3	1	0	0	13	1	0	1	0	0	0	0	0	0	0	21	0	0	0	0
8:30 AM	0	5	2	0	0	13	0	0	2	0	3	0	0	0	0	0	25	0	0	0	0
8:35 AM	0	4	0	0	0	8	0	0	0	0	0	0	1	0	0	0	13	0	0	0	0
8:40 AM	1	4	1	0	1	15	2	0	0	1	4	0	0	0	1	0	30	0	0	0	0
8:45 AM	0	6	1	0	0	11	2	0	0	0	3	0	0	0	0	0	23	0	0	0	0
8:50 AM	0	6	1	0	0	10	0	0	0	1	2	0	2	0	0	0	22	0	0	0	0
8:55 AM	0	3	0	0	0	10	0	0	0	1	1	0	0	0	0	0	15	0	0	0	0
Total Survey	19	94	19	0	1	268	28	0	23	3	38	0	4	1	1	0	499	0	0	0	0

15-Minute Interval Summary 7:00 AM to 9:00 AM

Interval		North	bound			South	bound			Easth	ound			West	oound				Pedes	trians	
Start		Ten E	yck Rd			Ten E	yck Rd			Pleas	ant St			Pleas	ant St		Interval		Cross	swalk	
Time	L	T	R	Bikes	L	T	R	Bikes	L	T	R	Bikes	L	T	R	Bikes	Total	North	South	East	Wes
7:00 AM	3	7	2	0	0	44	2	0	0	0	3	0	0	0	0	0	61	0	0	0	0
7:15 AM	8	8	2	0	0	46	6	0	4	0	5	0	0	0	0	0	79	0	0	0	0
7:30 AM	2	21	3	0	0	28	5	0	6	0	6	0	0	0	0	0	71	0	0	0	0
7:45 AM	3	10	2	0	0	25	5	0	5	0	6	0	0	0	0	0	56	0	0	0	0
8:00 AM	0	6	3	0	0	25	2	0	4	0	2	0	1	0	0	0	43	0	0	0	0
8:15 AM	2	14	2	0	0	33	4	0	2	0	3	0	0	1	0	0	61	0	0	0	0
8:30 AM	1	13	3	0	1	36	2	0	2	1	7	0	1	0	1	0	68	0	0	0	0
8:45 AM	0	15	2	0	0	31	2	0	0	2	6	0	2	0	0	0	60	0	0	0	0
Total Survey	19	94	19	0	1	268	28	0	23	3	38	0	4	1	1	0	499	0	0	0	0

Peak Hour Summary 7:00 AM to 8:00 AM

By			bound yck Rd				bound yck Rd				oound ant St				oound ant St		Total
Approach	In	Out	Total	Bikes	In	Out	Total	Bikes	In	Out	Total	Bikes	In	Out	Total	Bikes	
Volume	71	163	234	0	161	61	222	0	35	34	69	0	0	9	9	0	267
%HV		2.8	3%			2.5%				17	.1%			0.0	0%		4.5%
PHF		0.	68			0.	68			0.	58			0.	00		0.84

	Ш		Pedes	trians	
ı			Cross	swalk	
	П	North	South	East	West
	I	0	0	0	0

By Movement			bound yck Rd				bound yck Rd				ound ant St			Westl			Total
wovernerit	L	Т	R	Total	L	T	R	Total	L	T	R	Total	L	Т	R	Total	
Volume	16	46	9	71	0	143	18	161	15	0	20	35	0	0	0	0	267
%HV	6.3%	2.2%	0.0%	2.8%	0.0%	1.4%	11.1%	2.5%	6.7%	0.0%	25.0%	17.1%	0.0%	0.0%	0.0%	0.0%	4.5%
PHF	0.50	0.55	0.75	0.68	0.00	0.67	0.75	0.68	0.42	0.00	0.71	0.58	0.00	0.00	0.00	0.00	0.84

Rolling Hour Summary 7:00 AM to 9:00 AM

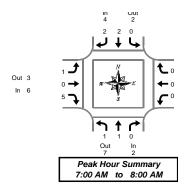
Interval		North	bound			South	bound			Easth	ound			West	oound				Pedes	trians	
Start		Ten E	yck Rd			Ten Ey	yck Rd			Pleas	ant St			Pleas	ant St		Interval		Cross	swalk	
Time	L	T	R	Bikes	L	T	R	Bikes	L	T	R	Bikes	L	T	R	Bikes	Total	North	South	East	West
7:00 AM	16	46	9	0	0	143	18	0	15	0	20	0	0	0	0	0	267	0	0	0	0
7:15 AM	13	45	10	0	0	124	18	0	19	0	19	0	1	0	0	0	249	0	0	0	0
7:30 AM	7	51	10	0	0	111	16	0	17	0	17	0	1	1	0	0	231	0	0	0	0
7:45 AM	6	43	10	0	1	119	13	0	13	1	18	0	2	1	1	0	228	0	0	0	0
8:00 AM	3	48	10	1 0	1	125	10	0	. 8	3	18	0	4	1	1	0	232	0	0	0	0

Heavy Vehicle Summary



Ten Eyck Rd & Pleasant St

Wednesday, February 19, 2020 7:00 AM to 9:00 AM



Heavy Vehicle 5-Minute Interval Summary 7:00 AM to 9:00 AM

		9.00 A															
Interval		North					bound				ound				bound		
Start			yck Rd			Ten E	yck Rd			Pleas	ant St				ant St		Interval
Time	L	T	R	Total	L	T	R	Total	L	T	R	Total	L	T	R	Total	Total
7:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:05 AM	0	0	0	0	0	0	0	0	0	0	1	1	0	0	0	0	1
7:10 AM	1	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
7:15 AM	0	0	0	0	0	0	0	0	0	0	1	1	0	0	0	0	1
7:20 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:25 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:30 AM	0	1	0	1	0	0	1	1	0	0	0	0	0	0	0	0	2
7:35 AM	0	0	0	0	0	1	1	2	1	0	1	2	0	0	0	0	4
7:40 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:45 AM	0	0	0	0	0	1	0	1	0	0	1	1	0	0	0	0	2
7:50 AM	0	0	0	0	0	0	0	0	0	0	1	1	0	0	0	0	1
7:55 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:00 AM	0	0	0	0	0	11	0	1	1	0	0	11	0	0	0	0	2
8:05 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:10 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:15 AM	0	0	0	0	0	0	2	2	1	0	0	1	0	0	0	0	3
8:20 AM	0	1	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
8:25 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:30 AM	0	1	0	1	0	1	0	1	0	0	1	1	0	0	0	0	3
8:35 AM	0	1	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
8:40 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:50 AM	0	0	0	0	0	2	0	2	0	0	0	0	0	0	0	0	2
8:55 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total Survev	1	4	0	5	0	6	4	10	3	0	6	9	0	0	0	0	24

Heavy Vehicle 15-Minute Interval Summary 7:00 AM to 9:00 AM

Interval		North	bound			South	bound			Easth	ound			Westi	oound		
Start		Ten E	yck Rd			Ten E	yck Rd			Pleas	ant St			Pleas	ant St		Interval
Time	L	T	R	Total	L	Т	R	Total	L	T	R	Total	L	T	R	Total	Total
7:00 AM	1	0	0	1	0	0	0	0	0	0	1	1	0	0	0	0	2
7:15 AM	0	0	0	0	0	0	0	0	0	0	1	1	0	0	0	0	1
7:30 AM	0	1	0	1	0	1	2	3	1	0	1	2	0	0	0	0	6
7:45 AM	0	0	0	0	0	1	0	1	0	0	2	2	0	0	0	0	3
8:00 AM	0	0	0	0	0	1	0	1	1	0	0	1	0	0	0	0	2
8:15 AM	0	1	0	1	0	0	2	2	1	0	0	1	0	0	0	0	4
8:30 AM	0	2	0	2	0	1	0	1	0	0	1	1	0	0	0	0	4
8:45 AM	0	0	0	0	0	2	0	2	0	0	0	0	0	0	0	0	2
Total Survey	1	4	0	5	0	6	4	10	3	0	6	9	0	0	0	0	24

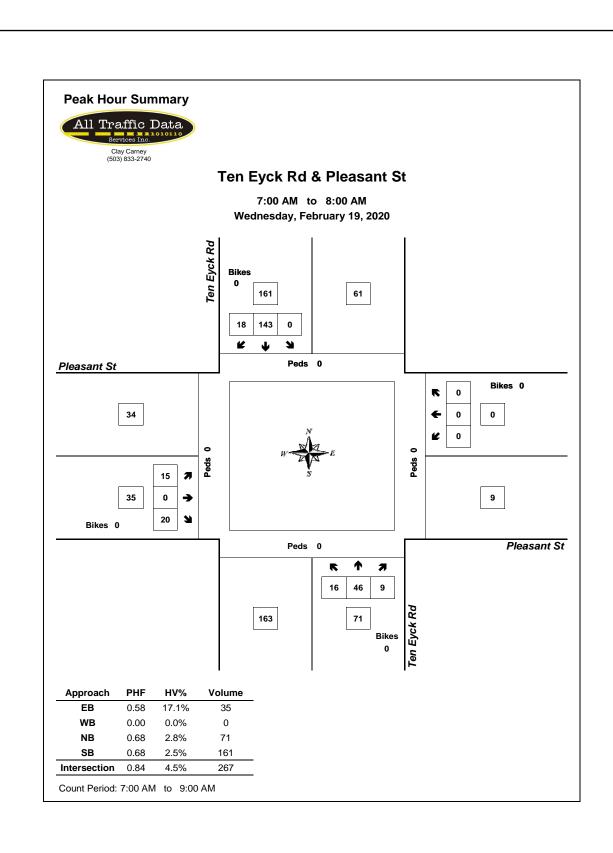
Heavy Vehicle Peak Hour Summary 7:00 AM to 8:00 AM

Bv		North	bound		South	bound		Easth	oound		West	bound	
,		Ten E	yck Rd		Ten E	yck Rd		Pleas	ant St		Pleas	ant St	Total
Approach	In	Out	Total										
Volume	2	7	9	4	2	6	6	3	9	0	0	0	12
PHF	0.50			0.33			0.50			0.00			0.50

By Movement		North Ten E	bound yck Rd				bound yck Rd				ound ant St			Westl			Total
Movement	L	T	R	Total	L	T	R	Total	L	Т	R	Total	L	Т	R	Total	
Volume	1	1	0	2	0	2	2	4	1	0	5	6	0	0	0	0	12
PHF	0.25	0.25	0.00	0.50	0.00	0.25	0.25	0.33	0.25	0.00	0.63	0.50	0.00	0.00	0.00	0.00	0.50

Heavy Vehicle Rolling Hour Summary 7:00 AM to 9:00 AM

Interval Start			bound yck Rd				bound yck Rd				ound ant St			Westl			Interval
Time	L	T	R	Total	L	T	R	Total	L	T	R	Total	L	Т	R	Total	Total
7:00 AM	1	1	0	2	0	2	2	4	1	0	5	6	0	0	0	0	12
7:15 AM	0	1	0	1	0	3	2	5	2	0	4	6	0	0	0	0	12
7:30 AM	0	2	0	2	0	3	4	7	3	0	3	6	0	0	0	0	15
7:45 AM	0	3	0	3	0	3	2	5	2	0	3	5	0	0	0	0	13
8.UU AM	0	3	n	3	0	4	2	6	2	0	1	3	0	0	0	0	12



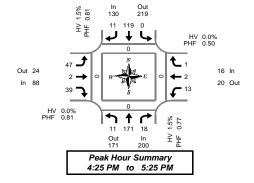
Total Vehicle Summary



Ten Eyck Rd & Pleasant St

Wednesday, February 19, 2020 4:00 PM to 6:00 PM

5-Minute Interval Summary 4:00 PM to 6:00 PM



Interval Start		North Ten E	bound yck Rd				bound yck Rd				oound ant St			Westi Pleas			Interval		Cros	strians swalk	
Time	L	T	R	Bikes	L	T	R	Bikes	L	T	R	Bikes	L	Т	R	Bikes	Total	North	South	East	West
4:00 PM	0	9	1	0	0	9	1	0	2	0	4	0	2	0	0	0	28	0	0	0	0
4:05 PM	1	20	2	0	0	5	2	0	1	0	2	0	0	2	2	0	37	0	0	0	0
4:10 PM	1	10	2	0	0	7	2	0	5	0	3	0	11	0	1	0	32	0	0	0	0
4:15 PM	2	11	1	0	0	8	0	0	4	1	1	0	0	2	0	0	30	0	0	0	0
4:20 PM	1	9	2	0	0	7	0	0	2	1	1	0	0	1	0	0	24	0	0	0	0
4:25 PM	0	11	0	0	0	8	0	0	7	0	4	0	11	0	0	0	31	0	0	0	0
4:30 PM	1	14	1	0	0	7	0	0	5	0	3	0	0	0	0	0	31	0	0	0	0
4:35 PM	1	16	3	0	0	12	0	0	4	0	1	0	0	0	0	0	37	0	0	0	0
4:40 PM	0	22	1	0	0	6	4	0	1	0	2	0	11	0	0	0	37	0	0	0	0
4:45 PM	4	17	1	0	0	9	0	0	5	0	1	0	2	0	1	0	40	0	0	0	0
4:50 PM	1	12	2	0	0	18	0	0	3	0	6	0	2	2	0	0	46	0	0	0	0
4:55 PM	0	11	0	0	0	10	2	0	3	0	5	0	0	0	0	0	31	0	0	0	0
5:00 PM	0	19	1	0	0	10	0	0	4	1	5	0	11	0	0	0	41	0	0	0	0
5:05 PM	1	12	4	0	0	14	2	0	5	0	4	0	0	0	0	0	42	0	0	0	0
5:10 PM	0	14	2	0	0	10	3	0	3	0	3	0	3	0	0	0	38	0	0	0	0
5:15 PM	2	9	1	0	0	6	0	0	3	1	3	0	0	0	0	0	25	0	0	0	0
5:20 PM	1	14	2	0	0	9	0	0	4	0	2	0	3	0	0	0	35	0	0	0	0
5:25 PM	0	14	0	0	0	7	1	0	1	0	2	0	0	0	0	0	25	0	0	0	0
5:30 PM	0	17	1	0	0	12	0	0	2	0	1	0	0	0	0	0	33	0	0	0	0
5:35 PM	0	14	2	0	0	4	0	0	3	0	2	0	2	0	0	0	27	0	0	0	0
5:40 PM	1	16	2	0	1	10	0	0	1	0	4	0	1	0	0	0	36	0	0	0	0
5:45 PM	0	12	1	0	0	8	1	0	2	0	1	0	0	0	0	0	25	0	0	0	0
5:50 PM	0	12	2	0	0	8	1	0	1	0	4	0	11	1	1	0	31	0	0	0	0
5:55 PM	0	13	2	0	0	7	1	0	3	0	4	0	1	0	0	0	31	0	0	0	0
Total Survev	17	328	36	0	1	211	20	0	74	4	68	0	21	8	5	0	793	0	0	0	0

15-Minute Interval Summary

4:00 PM to 6:00 PM

Interval		North	bound			South	bound			Easth	ound			West	oound				Pedes	strians	
Start		Ten E	yck Rd			Ten E	yck Rd			Pleas	ant St			Pleas	ant St		Interval		Cross	swalk	
Time	L	T	R	Bikes	L	T	R	Bikes	L	T	R	Bikes	L	T	R	Bikes	Total	North	South	East	West
4:00 PM	2	39	5	0	0	21	5	0	8	0	9	0	3	2	3	0	97	0	0	0	0
4:15 PM	3	31	3	0	0	23	0	0	13	2	6	0	1	3	0	0	85	0	0	0	0
4:30 PM	2	52	5	0	0	25	4	0	10	0	6	0	1	0	0	0	105	0	0	0	0
4:45 PM	5	40	3	0	0	37	2	0	11	0	12	0	4	2	1	0	117	0	0	0	0
5:00 PM	1	45	7	0	0	34	5	0	12	1	12	0	4	0	0	0	121	0	0	0	0
5:15 PM	3	37	3	0	0	22	1	0	8	1	7	0	3	0	0	0	85	0	0	0	0
5:30 PM	1	47	5	0	1	26	0	0	6	0	7	0	3	0	0	0	96	0	0	0	0
5:45 PM	0	37	5	0	0	23	3	0	6	0	9	0	2	1	1	0	87	0	0	0	0
Total Survey	17	328	36	0	1	211	20	0	74	4	68	0	21	8	5	0	793	0	0	0	0

Peak Hour Summary 4:25 PM to 5:25 PM

By			bound yck Rd				bound yck Rd				oound ant St				bound ant St		Total
Approach	In	Out	Total	Bikes	In	Out	Total	Bikes	In	Out	Total	Bikes	In	Out	Total	Bikes	
Volume	200	171	371	0	130	219	349	0	88	24	112	0	16	20	36	0	434
%HV		1.5	5%			1.5	5%			0.0	0%			0.0	0%		1.2%
PHF		0.	77			0.	81			0.	81			0.	50		0.88

		ı		Pedes	trians	
	Total			Cross	swalk	
Bikes			North	South	East	West
0	434	1	0	0	0	0
	1 20/	1				

By Movement			bound yck Rd		Southbound Ten Eyck Rd						ound ant St			Westl	ound ant St		Total
Movement	L	Т	R	Total	L	T	R	Total	L	Т	R	Total	L	Т	R	Total	
Volume	11	171	18	200	0	119	11	130	47	2	39	88	13	2	1	16	434
%HV	0.0%	1.8%	0.0%	1.5%	0.0%	0.8%	9.1%	1.5%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	1.2%
PHF	0.55	0.78	0.64	0.77	0.00	0.78	0.55	0.81	0.73	0.50	0.61	0.81	0.54	0.25	0.25	0.50	0.88

Rolling Hour Summary 4:00 PM to 6:00 PM

Interval		North	bound			Southbound				Easth	ound			West	bound				Pedes	trians	
Start		Ten E	yck Rd			Ten Ey	yck Rd			Pleas	ant St			Pleas	ant St		Interval		Cross	swalk	
Time	L	T	R	Bikes	L	T	R	Bikes	L	T	R	Bikes	L	T	R	Bikes	Total	North	South	East	West
4:00 PM	12	162	16	0	0	106	11	0	42	2	33	0	9	7	4	0	404	0	0	0	0
4:15 PM	11	168	18	0	0	119	11	0	46	3	36	0	10	5	1	0	428	0	0	0	0
4:30 PM	11	174	18	0	0	118	12	0	41	2	37	0	12	2	1	0	428	0	0	0	0
4:45 PM	10	169	18	0	1	119	8	0	37	2	38	0	14	2	1	0	419	0	0	0	0
5:00 PM	5	166	20	0	1	105	a	0	32	2	35	0	12	1	1	0	380	0	0	0	0

Heavy Vehicle Summary



Clay Carney (503) 833-2740

Ten Eyck Rd & Pleasant St

Wednesday, February 19, 2020 4:00 PM to 6:00 PM

Out Peak Hour Summary 4:25 PM to 5:25 PM

Out 1

In 0

Heavy Vehicle 5-Minute Interval Summary 4:00 PM to 6:00 PM

4:00 PIVI	ίΟ (
Interval		North	bound			South	bound			Easth	ound			Westi	oound		
Start		Ten E	yck Rd			Ten E	yck Rd			Pleas	ant St			Pleas	ant St		Interval
Time	L	T	R	Total	L	Т	R	Total	L	Т	R	Total	L	T	R	Total	Total
4:00 PM	0	1	0	1	0	3	0	3	0	0	0	0	0	0	0	0	4
4:05 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:10 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:15 PM	0	0	0	0	0	1	0	1	0	0	0	0	0	0	0	0	1
4:20 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:25 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:30 PM	0	2	0	2	0	0	0	0	0	0	0	0	0	0	0	0	2
4:35 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:40 PM	0	0	0	0	0	0	1	1	0	0	0	0	0	0	0	0	11
4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:50 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:55 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:00 PM	0	1	0	1	0	0	0	0	0	0	0	0	0	. 0	0	0	11
5:05 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	. 0	0	0	0
5:10 PM	0	0	0	0	0	1	0	1	0	0	0	0	0	. 0	0	0	1
5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:20 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:25 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:35 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:40 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:45 PM	0	1	0	1	0	0	0	0	0	0	0	0	0	. 0	0	0	11
5:50 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:55 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total Survey	0	5	0	5	0	5	1	6	0	0	0	0	0	0	0	0	11

Heavy Vehicle 15-Minute Interval Summary 4:00 PM to 6:00 PM

Interval		North	bound			South	bound			Easth	ound			Westk	oound		
Start		Ten E	yck Rd			Ten E	yck Rd			Pleas	ant St			Pleas	ant St		Interval
Time	L	T	R	Total	L	Т	R	Total	L	Т	R	Total	L	Т	R	Total	Total
4:00 PM	0	1	0	1	0	3	0	3	0	0	0	0	0	0	0	0	4
4:15 PM	0	0	0	0	0	1	0	1	0	0	0	0	0	0	0	0	1
4:30 PM	0	2	0	2	0	0	1	1	0	0	0	0	0	0	0	0	3
4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:00 PM	0	1	0	1	0	1	0	1	0	0	0	0	0	0	0	0	2
5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:45 PM	0	1	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
Total Survey	0	5	0	5	0	5	1	6	0	0	0	0	0	0	0	0	11

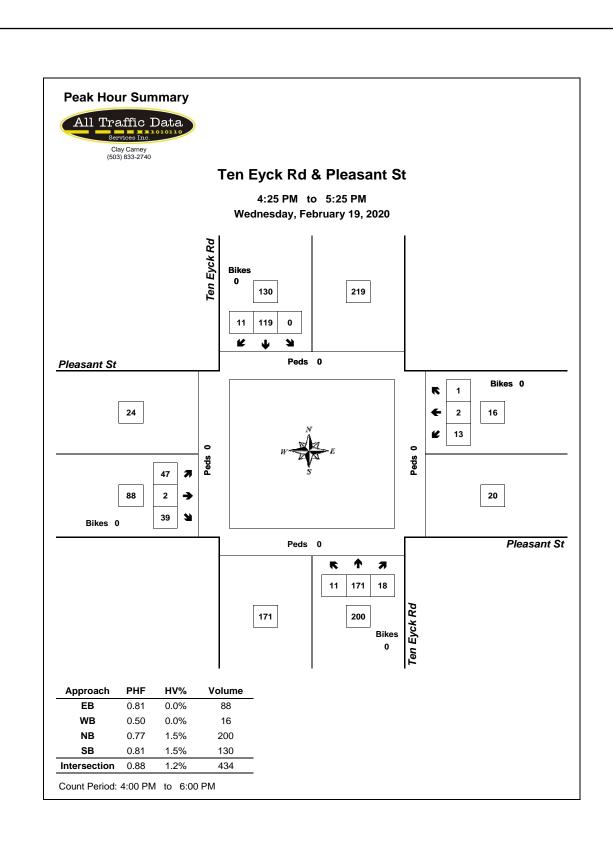
Heavy Vehicle Peak Hour Summary 4:25 PM to 5:25 PM

Bv		North	bound		South	bound		Easth	ound		West	bound	
,		Ten E	yck Rd		Ten E	yck Rd		Pleas	ant St		Pleas	sant St	Total
Approach	In	Out	Total	1									
Volume	3	1	4	2	3	5	0	1	1	0	0	0	5
PHF	0.38			0.50			0.00			0.00			0.42

By		Northbound Ten Eyck Rd					bound yck Rd			Eastb Pleas	ound ant St			Westl			Total
Movement	L	Т	R	Total	L	Т	R	Total	L	Т	R	Total	L	Т	R	Total	
Volume	0	3	0	3	0	1	1	2	0	0	0	0	0	0	0	0	5
PHF	0.00	0.38	0.00	0.38	0.00	0.25	0.25	0.50	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.42

Heavy Vehicle Rolling Hour Summary 4:00 PM to 6:00 PM

Interval Start			bound yck Rd			Southbound Ten Eyck Rd					ound ant St			Westl	oound ant St		Interval
Time	L	T	R	Total	L	T	R	Total	L	T	R	Total	L	Т	R	Total	Total
4:00 PM	0	3	0	3	0	4	1	5	0	0	0	0	0	0	0	0	8
4:15 PM	0	3	0	3	0	2	1	3	0	0	0	0	0	0	0	0	6
4:30 PM	0	3	0	3	0	1	1	2	0	0	0	0	0	0	0	0	5
4:45 PM	0	1	0	1	0	1	0	1	0	0	0	0	0	0	0	0	2
5:00 PM	0	2	0	2	0	1	0	1	0	0	0	0	0	0	0	0	3



Mt. Hoc	od Highway No. 26 (US 26)				ODOT TV	T Data		
MP	Location		2013	2014	2015	2016	2017	2018
24.59	EB, just west of Ten Eyck Rd		12,500	12,600	11,200	11,500	11,400	12,500
24.61	WB, just west of Ten Eyck Rd		11,600	11,700	11,700	12,100	12,000	12,600
25.10	EB+WB, just west of Langensand	d Rd	16,900	17,100	18,000	18,500	18,400	20,700
		Totals	41,000	41,400	40,900	42,100	41,800	45,800
	Annual Growth from 2013	r	n/a	1.0%	-0.1%	0.9%	0.5%	2.3%

SEASONAL TREND TABLE (Upo	dated: 6/26/	19)		Consens! Trend
TREND	1-Feb	15-Feb	1-Mar	Seasonal Trend Peak Period Factor
COMMUTER	1.08	1.06	1.04	0.94
RECREATIONAL SUMMER WINTER	1.02	1.04	1.05	0.70
SUMMER	1.24	1.21	1.15	0.83

^{*}Seasonal Trend Table factors are based on previous year ATR data. The table is updated yearly. *Grey shading indicates months were seasonal factor is greater than 30%.

Commuter, Recreational Summer-Winter, & Summer	1.11 1.34	0.83	<< Over 30%, so too high to be used
Commuter & Summer	1.14 1.28	0.89	<< This is an approved blend of trends, so this one is used.
Recreational Summer-Winter only	1.04 1.49	0.70	<< Over 30%, so too high to be used
Commuter & Recreational Summer-Winter	1.05 1.28	0.82	<< This is not an approved blend of trends, but it validates the number used above.

Appendix B Trip Generation Calculations
Appendix b
Trin Generation Calculations
Trip deficiation calculations

Detailed Land Use Data

For 9.6 1000 Sq. Ft. GFA of CLINIC 1 (630) Clinic

Project: Sandy Medical Clinic

Open Date: 3/3/2020 Analysis Date: 3/3/2020

Day / Period	Total Trips	Pass-By Trips	Avg Rate	Min Rate	Max Rate	Std Dev	Avg Size	% Enter	% _Exit_	Use Eq.	Equation	R2
Weekday Average Daily Trips Source: Trip Generation Manual 10th Edition	366	0	38.16	25.25	86.21	30.18	21	50	50	False		
Weekday AM Peak Hour of Adjacent Street Traffic Source: Trip Generation Manual 10th Edition	35	0	3.69	2.27	9.36	2.82	21	78	22	False		
Weekday PM Peak Hour of Adjacent Street Traffic	31	0	3.28	1.93	7	1.84	18	29	71	False	Ln(T) = 0.72 Ln(X) + 1.97	0.7

Source: Trip Generation Manual 10th Edition

Detailed Land Use Data

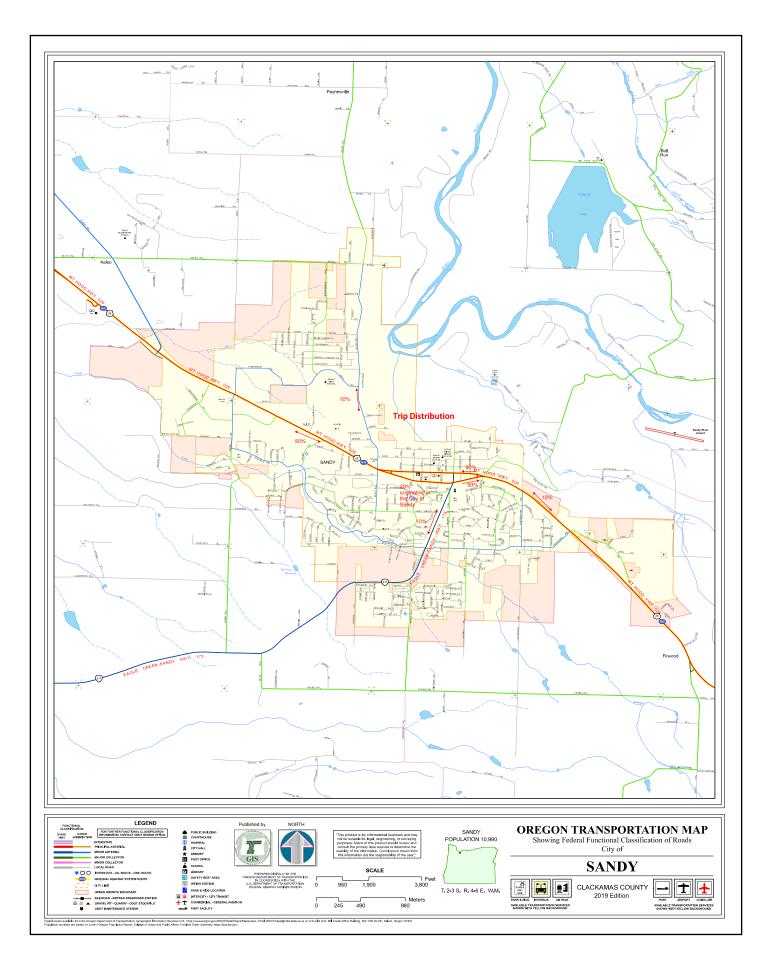
For 6.3 1000 Sq. Ft. GFA of WAREHOUSE 1 (150) Warehousing

Project: Sandy Medical Clinic

Open Date: 3/3/2020 Analysis Date: 3/3/2020

Day / Period	Total Trips	Pass-By Trips	Avg Rate	Min Rate	Max Rate	Std Dev	Avg Size	% Enter	% Exit	Use Eq.	Equation	
Weekday Average Daily Trips Source: Trip Generation Manual 10th Edition	11	0	1.74	0.15	16.93	1.55	285	50	50	False	T = 1.58(X) + 45.54	0.93
Weekday AM Peak Hour of Adjacent Street Traffic Source: Trip Generation Manual 10th Edition	1	0	0.17	0.02	1.93	0.2	451	77	23	False	T = 0.12(X) + 25.32	0.69
Weekday PM Peak Hour of Adjacent Street Traffic	1	0	0.19	0.01	1.8	0.18	400	27	73	False	T = 0.12(X) + 27.82	0.65

Source: Trip Generation Manual 10th Edition



John A. Manix

From:Wilson, James < jwilson2@clackamas.us>Sent:Tuesday, February 18, 2020 3:18 PM

To: John A. Manix

Cc: Cockrell, Deborah; Kelly, Steve

Subject: RE: PBS Engineers - Traffic Study Question(s)

Hi John

Here are estimates for staff members:

25% trips originate from within the City limits of Sandy

50% trips originate from the west of Sandy 8% trips originate from the north of Sandy 8% trips originate from the east of Sandy 9% trips originate from the south of Sandy

James

From: John A. Manix < Manix@pbsusa.com>
Sent: Tuesday, February 18, 2020 10:52 AM
To: Wilson, James < jwilson2@clackamas.us>

Cc: Cockrell, Deborah < DCockrell@clackamas.us>; Kelly, Steve < SteveKel@clackamas.us>

Subject: RE: PBS Engineers - Traffic Study Question(s)

James: This is good. Thank you

I will assume that the origin of staff trips will be about the same. If you have anything similar for staff trips that will be helpful.

John Manix, PE | Senior Traffic Engineer | PBS Vancouver | 360.607.1854 (cell)

From: Wilson, James < <u>jwilson2@clackamas.us</u>>
Sent: Tuesday, February 18, 2020 10:36 AM
To: John A. Manix < <u>Manix@pbsusa.com</u>>

Cc: Cockrell, Deborah < DCockrell@clackamas.us>; Kelly, Steve < SteveKel@clackamas.us>

Subject: RE: PBS Engineers - Traffic Study Question(s)

Hi John

Here are estimates of where patients and clients of the new clinic will originate.

These are based on current patient demographics, information from the Health Resources and Services Administration (HRSA), and information from Medicaid / Oregon Health Plan insurance networks.

If you need anything more, please let me know.

45% trips originate from within the City limits of Sandy 8% trips originate from the west of Sandy (Kelso, Boring)

5% trips originate from the north of Sandy

12% trips originate from the east of Sandy (Mt Hood Village, and Mountain Communities)

30% trips originate from the south of Sandy (Estacada / Eagle Creek)

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Have a great morning James

JAMES WILSON CHIEF OPERATIONS OFFICER CLACKAMAS HEALTH CENTERS 503-655-8697

EXCEPTIONAL CARE FOR THE WHOLE PERSON DELIVERED WITH DIGNITY AND RESPECT FOR ALL.

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From: Cockrell, Deborah < DCockrell@clackamas.us > Sent: Tuesday, February 18, 2020 9:30 AM

To: Wilson, James < iwilson2@clackamas.us > Subjects FMX DDS Facility of Tariffic Charles Overhical (Control of Charles of Char

Subject: FW: PBS Engineers - Traffic Study Question(s)

FYI data needed

From: John A. Manix [mailto:Manix@pbsusa.com]
Sent: Tuesday, February 18, 2020 8:39 AM
To: Cockrell, Deborah < DCockrell@clackamas.us>
Subject: RE: PBS Engineers - Traffic Study Question(s)

Deborah: I understand the data will not be easy to produce. If you or the staff can give me a rough estimate of the origin of patrons and/or staff, I would appreciate the effort. It would be best to revise my estimate below in the February 13, 2020 email.

John Manix, PE | Senior Traffic Engineer | PBS Vancouver | 360.607.1854 (cell)

From: Cockrell, Deborah < DCockrell@clackamas.us Sent: Monday, February 17, 2020 12:42 PM

To: John A. Manix < Manix@pbsusa.com>

Cc: Kelly, Steve < SteveKel@clackamas.us>; Wilson, James < jwilson2@clackamas.us>

Subject: RE: PBS Engineers - Traffic Study Question(s)

Unfortunately this data will not be easy to produce. We will have to survey staff. JAMES: Can you pull this data for us? Also, dental is the only new traffic. BH and PC already have a presence in Sandy

From: John A. Mani]

Sent: Thursday, February 13, 2020 8:59 AM

To: Cockrell, Deborah < DCockrell@clackamas.us

Cc: Kelly, Steve < Steve Kel@clackamas.us>

Subject: RE: PBS Engineers - Traffic Study Question(s)

Deborah: Thank you. This is good information but more important is the were in the community do the trips to your clinic come from. Please edit my estimate below based on your experience.

For example do most patients and staff live in the Sandy or do they live north, east, west or south of town. This is not an exact process but based on residential density and my experience with the community, here is my estimate:

50% trips originate from within the City limits of Sandy,

10% trips originate from the west of Sandy

10% trips originate from the north of Sandy

20% trips originate from the east of Sandy

10% trips originate from the south of Sandy

For the proposed clinic at Hwy 26 and Ten Eyke Road, I need to know which direction the staff and patience approach the clinic. Feel free to call if you have questions.

Thanks

John Manix, PE

Senior Traffic Engineer

PB5

415 W 6th St., Suite 601, Vancouver, WA 98660 office: 360.695.3488 | direct: 360.567.2117 | cell: 360.607.1854 john.manix@pbsusa.com



From: Kelly, Steve < SteveKel@clackamas.us > Sent: Wednesday, February 12, 2020 1:20 PM
To: John A. Manix < Manix@pbsusa.com >

Subject: FW: PBS Engineers - Traffic Study Question(s)

John

Here is what I got from the Director of all of County Health Clinics. See below, sir.

Steve Kelly, Project Coordinator Clackamas County Community Development 2051 Kaen Road, Suite 245 Oregon City, OR 97045

503 . 650 . 5665 stevekel@clackamas.us

From: Cockrell, Deborah < DCockrell@clackamas.us Sent: Tuesday, February 11, 2020 1:08 PM To: Kelly, Steve < SteveKel@clackamas.us>

Subject: RE: PBS Engineers - Traffic Study Question(s)

MD will see 18 people per day, 4 days a week. DMD will see 14 people per day, 4 days a week. Probably 40 people per day for the therapists.

From: Kelly, Steve

Sent: Tuesday, February 11, 2020 11:33 AM

To: Cockrell, Deborah < DCockrell@clackamas.us Subject: PBS Engineers - Traffic Study Question(s)

Importance: High

John Manix of PBS.

Wants to know if we have information of any kind that determines visitors (patients) that are driven to the sites in Sandy. He would need them if they are available for counts for vehicular usage to both clinic sites presently (high school and other location). am assuming someone else has these numbers or data. He is getting me a letter with a price today he said. I will provide him our Professional Services Contract and use his letter as an Exhibit A. Thanks. Steve Kelly, Project Coordinator Clackamas County Community Development 2051 Kaen Road, Suite 245 Oregon City, OR 97045 503 . 650 . 5665 stevekel@clackamas.us NOTE: This message was trained as non-spam. If this is wrong, please correct the training as soon as possible.

Appendix C Level of Service Calculations
Level of Service Calculations

Critical Intersection Volume-to-Capacity Ratio Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26) 2020 Existing Conditions Weekday AM Peak Hour

	EB	WB		NB	SB
Lane Group	EBL EB	ST WBL	WBTR	NBLTR	SBLTR
Adj Flow Rate (veh/h)	59	904	0 103	4 152	69
Sat Flow (veh/h)	1589	2988	1667 326	9 1469	1618
Critical Flow Ratios	0.04	0.30	0.00 0.3	2 0.10	0.04
	critical		critical	critical	

Sum of Critical Flow Ratios 0.46

Cycle Length 64.4 seconds
Lost Time per Phase 4 seconds
Total Lost Time 12 seconds

Critical Intersection V/C Ratio: 0.56

Method follows ODOT Analysis Procedures Manual, Version 2, Chapter 13.4.4.

Critical Intersection Volume-to-Capacity Ratio						
Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26)						
2022 Without Project	Weekday AM Peak Hour					

	EB		WB		NB	SB
Lane Group	EBL	EBT	WBL	WBTR	NBLTR	SBLTR
Adj Flow Rate (veh/h)	6	1 940) 0	1076	157	70
Sat Flow (veh/h)	1589	9 2988	1641	3269	1444	1593
Critical Flow Ratios	0.04	4 0.31	0.00	0.33	0.11	0.04
	critical			critical	critical	

Sum of Critical Flow Ratios 0.48

Cycle Length 69.3 seconds
Lost Time per Phase 4 seconds
Total Lost Time 12 seconds

Critical Intersection V/C Ratio: 0.58

Critical Intersection Volume-to-Capacity Ratio Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26) 2022 With Project Trips Weekday AM Peak Hour

	EB	WB			NB	SB
Lane Group	EBL EBT	WBL	WBT	R	NBLTR	SBLTR
Adj Flow Rate (veh/h)	87	940	0	1080	157	73
Sat Flow (veh/h)	1589	2988	1641	3267	1447	1595
Critical Flow Ratios	0.05	0.31	0.00	0.33	0.11	0.05
	critical		critic	al	critical	

Sum of Critical Flow Ratios 0.49

Cycle Length 71.7 seconds
Lost Time per Phase 4 seconds
Total Lost Time 12 seconds

Critical Intersection V/C Ratio: 0.59

Method follows ODOT Analysis Procedures Manual, Version 2, Chapter 13.4.4.

Critical Intersection Volume-to-Capacity Ratio							
Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26)							
2029 Without Project	Weekday AM Peak Hour						

	EB		WB			NB	SB
Lane Group	EBL	EBT	WBL	WBT	₹	NBLTR	SBLTR
Adj Flow Rate (veh/h)	7	0 10	067	0	1222	179	75
Sat Flow (veh/h)	158	9 29	988	1641	3268	1438	1604
Critical Flow Ratios	0.0	4 0).36	0.00	0.37	0.12	0.05
	critical			critico	al	critical	

Sum of Critical Flow Ratios 0.54

Cycle Length 76.8 seconds
Lost Time per Phase 4 seconds
Total Lost Time 12 seconds

Critical Intersection V/C Ratio: 0.64

Critical Intersection Volume-to-Capacity Ratio					
Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26)					
2029 With Project Trips	Weekday AM Peak Hour				

	EB	WB		NI	В	SB
Lane Group	EBL EB	BT WBL	WBT	R NI	BLTR	SBLTR
Adj Flow Rate (veh/h)	96	1067	0	1225	179	76
Sat Flow (veh/h)	1589	2988	1641	3266	1439	1606
Critical Flow Ratios	0.06	0.36	0.00	0.38	0.12	0.05
	critical		critic	al cri	itical	

Cycle Length 78.3 seconds
Lost Time per Phase 4 seconds
Total Lost Time 12 seconds

Critical Intersection V/C Ratio: 0.66

Method follows ODOT Analysis Procedures Manual, Version 2, Chapter 13.4.4.

Critical Intersection Volume-to-Capacity Ratio							
Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26)							
2029 With Project Trips with WB Right-Turn Lane	Weekday AM Peak Hour						

	EB	WB			NB	SB
Lane Group	EBL EBT	WBL	WBT		NBLTR	SBLTR
Adj Flow Rate (veh/h)	96	1067	0	1202	179	78
Sat Flow (veh/h)	1589	2988	1641	3195	1440	1604
Critical Flow Ratios	0.06	0.36	0.00	0.38	0.12	0.05
	critical		critic	al	critical	

Sum of Critical Flow Ratios 0.56

Cycle Length 77.3 seconds
Lost Time per Phase 4 seconds
Total Lost Time 12 seconds

Critical Intersection V/C Ratio: 0.66

Critical Intersection Volume-to-Capacity Ratio				
Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26)				
2020 Existing Conditions	Weekday PM Peak Hour			

	EB		WB		NB	SB
Lane Group	EBL	EBT	WBL	WBTR	NBLTR	SBLTR
Adj Flow Rate (veh/h)	161	1165	5	1216	154	114
Sat Flow (veh/h)	1654	3247	1667	3272	1219	1587
Critical Flow Ratios	0.10	0.36	0.00	0.37	0.13	0.07
	critical			critical	critical	

Cycle Length 110 seconds
Lost Time per Phase 4 seconds
Total Lost Time 12 seconds

Critical Intersection V/C Ratio: 0.67

Method follows ODOT Analysis Procedures Manual, Version 2, Chapter 13.4.4.

Critical Intersection Volume-to-Capacity Ratio				
Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26)				
2022 Without Project	Weekday PM Peak Hour			
,	,			

	EB		WB		NB	SB
Lane Group	EBL	EBT	WBL	WBTR	NBLTR	SBLTR
Adj Flow Rate (veh/h)	167	1212	. 5	1265	161	127
Sat Flow (veh/h)	1641	3247	1641	3271	1174	1575
Critical Flow Ratios	0.10	0.37	0.00	0.39	0.14	0.08
	critical			critical	critical	

Sum of Critical Flow Ratios 0.63

Cycle Length 110 seconds
Lost Time per Phase 4 seconds
Total Lost Time 12 seconds

Critical Intersection V/C Ratio: 0.70

Critical Intersection Volume-to-Capacity Ratio				
Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26)				
2022 With Project Trips	Weekday PM Peak Hour			

	EB	WB			NB	SB
Lane Group	EBL EBT	WBL	WI	BTR	NBLTR	SBLTR
Adj Flow Rate (veh/h)	176	1212	5	1266	161	143
Sat Flow (veh/h)	1641	3247	1641	3270	1121	1577
Critical Flow Ratios	0.11	0.37	0.00	0.39	0.14	0.09
	critical		cri	tical	critical	

Cycle Length 110 seconds
Lost Time per Phase 4 seconds
Total Lost Time 12 seconds

Critical Intersection V/C Ratio: 0.72

Method follows ODOT Analysis Procedures Manual, Version 2, Chapter 13.4.4.

Critical Intersection Volume-to-Capacity Ratio					
Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26)					
2029 Without Project	Weekday PM Peak Hour				

	EB		WB		NB	SB
Lane Group	EBL	EBT	WBL	WBTR	NBLTR	SBLTR
Adj Flow Rate (veh/h)	189	1374	6	1435	183	149
Sat Flow (veh/h)	1641	3247	1641	3271	1119	1578
Critical Flow Ratios	0.12	0.42	0.00	0.44	0.16	0.09
	critical			critical	critical	

Sum of Critical Flow Ratios 0.72

Cycle Length 110 seconds
Lost Time per Phase 4 seconds
Total Lost Time 12 seconds

Critical Intersection V/C Ratio: 0.81

Critical Intersection Volume-to-Capacity Ratio				
Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26)				
2029 With Project Trips	Weekday PM Peak Hour			

	EB	WB			NB	SB
Lane Group	EBL EB	T WBL	WBTI	₹	NBLTR	SBLTR
Adj Flow Rate (veh/h)	198	1374	6	1436	183	163
Sat Flow (veh/h)	1630	3228	1630	3160	1065	1587
Critical Flow Ratios	0.12	0.43	0.00	0.45	0.17	0.10
	critical		critico	al (critical	

Cycle Length 110 seconds
Lost Time per Phase 4 seconds
Total Lost Time 12 seconds

Critical Intersection V/C Ratio: 0.84

Method follows ODOT Analysis Procedures Manual, Version 2, Chapter 13.4.4.

Critical Intersection Volume-to-Capacity Ratio				
Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26)				
2029 With Project Trips with WB Right-Turn Lane	Weekday PM Peak Hour			

	EB		WB		NB	SB
Lane Group	EBL	EBT	WBL	WBT	NBLTR	SBLTR
Adj Flow Rate (veh/h)	198	1374	6	1416	183	163
Sat Flow (veh/h)	1641	3247	1641	3195	1065	1587
Critical Flow Ratios	0.12	0.42	0.00	0.44	0.17	0.10
	critical			critical	critical	

Sum of Critical Flow Ratios 0.74

Cycle Length 110 seconds
Lost Time per Phase 4 seconds
Total Lost Time 12 seconds

Critical Intersection V/C Ratio: 0.83

Lanes, Volumes, Timings

03/06/2020

	ၨ	→	•	•	←	•	•	†	<i>></i>	/	ļ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	^	7	ሻ	∱ }			4			4	
Traffic Volume (vph)	54	832	33	0	937	16	136	3	2	21	7	135
Future Volume (vph)	54	832	33	0	937	16	136	3	2	21	7	135
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Storage Length (ft)	115		100	105		0	0		0	0		0
Storage Lanes	1		1	1		0	0		0	0		0
Taper Length (ft)	100		•	100		-	100		-	100		-
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	0.95	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor			0.98		1.00			1.00			0.99	
Frt			0.850		0.998			0.998			0.889	
Flt Protected	0.950		0.000		0.000			0.954			0.994	
Satd. Flow (prot)	1568	2942	1403	1750	3162	0	0	1604	0	0	1458	0
Flt Permitted	0.950	2012	1400	1700	0102	U	U	0.592	•	U	0.953	J
Satd. Flow (perm)	1568	2942	1372	1750	3162	0	0	994	0	0	1398	0
Right Turn on Red	1000	2572	Yes	1700	0102	Yes	U	337	Yes	U	1000	Yes
Satd. Flow (RTOR)			77		2	103		1	103		147	103
Link Speed (mph)		25	- 11		25			25			25	
Link Distance (ft)		468			302			365			210	
Travel Time (s)		12.8			8.2			10.0			5.7	
Confl. Peds. (#/hr)		12.0	1	1	0.2		2	10.0			5.1	2
Confl. Bikes (#/hr)			- 1			1	2					2
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	6%	13%	6%	0.92	5%	0.92	4%	0.92	0.92	24%	0.92	2%
	59	904	36	0 /0	1018	17	148	3	2	24 /0	8	147
Adj. Flow (vph) Shared Lane Traffic (%)	39	904	30	U	1010	17	140	3	2	23	0	147
. ,	59	904	36	0	1025	0	0	150	0	0	170	٥
Lane Group Flow (vph)	No No		No.		1035 No		No	153	No		178	0
Enter Blocked Intersection		No R NA	R NA	No	R NA	No		No		No	No	No
Lane Alignment	L NA	12	RINA	L NA	12	R NA	L NA	R NA	R NA	L NA	R NA	R NA
Median Width(ft)		-12			0			0			0	
Link Offset(ft)								0				
Crosswalk Width(ft)		72			42			32			30	
Two way Left Turn Lane	4 4 4	1.11	1.11	1.11	1 11	1.11	1.11	1.11	1.11	1.11	1.11	1.11
Headway Factor	1.11		1.11		1.11	1.11			1.11			1.11
Number of Detectors	2	1	•	2	1		2	2		2	2	
Detector Template	Left	Det25	Right	Left	Det25		Left	Side St		Left	Side St	
Leading Detector (ft)	78	153	153	78	153		78	78		78	78	
Trailing Detector (ft)	2	137	137	2	137		2	2		2	2	
Detector 1 Position(ft)	2	137	137	2	137		2	2		2	2	
Detector 1 Size(ft)	16	16	16	16	16		16	16		16	16	
Detector 1 Type	CI+Ex	CI+Ex	CI+Ex	Cl+Ex	CI+Ex		CI+Ex	CI+Ex		CI+Ex	CI+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Detector 2 Position(ft)	72			72			72	72		72	72	
Detector 2 Size(ft)	6			6			6	6		6	6	
Detector 2 Type	CI+Ex			Cl+Ex			CI+Ex	CI+Ex		CI+Ex	CI+Ex	
Detector 2 Channel												
Detector 2 Extend (s)	0.0			0.0			0.0	0.0		0.0	0.0	

Sandy Health Clinic - PBS Project 71524.000 2020 Existing Conditions - Weekday PM Peak Hour

Synchro 10 Report - by PBS Engineering and Environmental Page 1

Lanes, Volumes, Timings 3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26) 03/06/2020 ၨ t **EBL** EBT **EBR** WBL WBT WBR NBL **NBT** NBR SBL Lane Group **SBT** Turn Type Prot NA Perm Prot NA Perm NA Perm NA **Protected Phases** 5 2 6 8 4 **Permitted Phases** 8 5 2 2 6 4 **Detector Phase** 1 8 8 4 Switch Phase Minimum Initial (s) 4.0 10.0 10.0 4.0 10.0 6.0 6.0 6.0 6.0 Minimum Split (s) 8.5 26.0 26.0 8.5 30.0 11.5 11.5 23.5 23.5 29.5 54.0 54.0 44.0 25.5 25.5 25.5 Total Split (s) 19.5 25.5 Total Split (%) 29.8% 54.5% 54.5% 19.7% 44.4% 25.8% 25.8% 25.8% 25.8% Maximum Green (s) 25.0 50.0 50.0 15.0 40.0 20.0 20.0 20.0 20.0 4.0 Yellow Time (s) 3.5 3.5 3.5 3.5 3.5 4.0 4.0 4.0 All-Red Time (s) 1.0 0.5 0.5 1.0 0.5 1.5 1.5 1.5 1.5 Lost Time Adjust (s) -0.5 0.0 0.0 -0.5 0.0 -1.5 -1.5 Total Lost Time (s) 4.0 4.0 4.0 4.0 4.0 4.0 4.0 Lead/Lag Lag Lag Lag Lead Lead Lead-Lag Optimize? Vehicle Extension (s) 2.3 5.8 5.8 2.3 5.8 2.5 2.5 2.5 2.5 2.0 0.5 0.5 2.0 2.0 2.0 Minimum Gap (s) 3.8 3.8 3.8 Time Before Reduce (s) 8.0 5.0 5.0 5.0 10.0 10.0 8.0 10.0 5.0 Time To Reduce (s) 3.0 10.0 10.0 3.0 10.0 5.0 5.0 5.0 5.0 Recall Mode Min Min None None None Min None None None Walk Time (s) 7.0 7.0 7.0 7.0 7.0 Flash Dont Walk (s) 15.0 15.0 19.0 11.0 11.0 Pedestrian Calls (#/hr) 0 0 0 0 35.2 8.3 35.2 28.3 20.5 20.5 Act Effct Green (s) 0.13 Actuated g/C Ratio 0.55 0.55 0.44 0.32 0.32 v/c Ratio 0.29 0.05 0.74 0.48 0.33 0.56 Control Delay 34.6 10.3 19.9 29.4 0.4 8.8 Queue Delay 0.0 0.0 0.0 0.0 0.0 0.0 **Total Delay** 34.6 10.3 19.9 29.4 8.8 0.4 LOS C В В С Α Approach Delay 11.4 19.9 29.4 8.8 С Approach LOS В В Α Intersection Summary Other Area Type: Cycle Length: 99 Actuated Cycle Length: 64.4) Natural Cycle: 65 Control Type: Actuated-Uncoordinated Maximum v/c Ratio: 0.74 Intersection Signal Delay: 16.1 Intersection LOS: B Intersection Capacity Utilization 65.0% ICU Level of Service C Analysis Period (min) 15 Splits and Phases: 3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26) Ø1 Ø4 <u>≯_{ø5}</u> ¶ ø8 2020 Existing Conditions - Weekday PM Peak Hour Page 2

Queues

3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26)

03/06/2020

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Lane Group	EBL	EBT	EBR	WBT	NBT	SBT
Lane Group Flow (vph)	59	904	36	1035	153	178
v/c Ratio	0.29	0.56	0.05	0.74	0.48	0.33
Control Delay	34.6	10.3	0.4	19.9	29.4	8.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	34.6	10.3	0.4	19.9	29.4	8.8
Queue Length 50th (ft)	24	112	0	200	54	10
Queue Length 95th (ft)	65	151	2	290	#142	64
Internal Link Dist (ft)		388		222	285	130
Turn Bay Length (ft)	115		100			
Base Capacity (vph)	675	2332	1103	2100	361	601
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.09	0.39	0.03	0.49	0.42	0.30
Intersection Summary						

^{# 95}th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis 3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26)

03/06/2020

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	^	7	¥	ħβ			4			4	
Traffic Volume (vph)	54	832	33	0	937	16	136	3	2	21	7	135
Future Volume (vph)	54	832	33	0	937	16	136	3	2	21	7	135
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Total Lost time (s)	4.0	4.0	4.0		4.0			4.0			4.0	
Lane Util. Factor	1.00	0.95	1.00		0.95			1.00			1.00	
Frpb, ped/bikes	1.00	1.00	0.98		1.00			1.00			0.99	
Flpb, ped/bikes	1.00	1.00	1.00		1.00			1.00			1.00	
Frt	1.00	1.00	0.85		1.00			1.00			0.89	
Flt Protected	0.95	1.00	1.00		1.00			0.95			0.99	
Satd. Flow (prot)	1568	2942	1373		3160			1602			1458	
Flt Permitted	0.95	1.00	1.00		1.00			0.59			0.95	
Satd. Flow (perm)	1568	2942	1373		3160			994			1398	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	59	904	36	0	1018	17	148	3	2	23	8	147
RTOR Reduction (vph)	0	0	16	0	1	0	0	1	0	0	101	0
Lane Group Flow (vph)	59	904	20	0	1034	0	0	152	0	0	77	0
Confl. Peds. (#/hr)			1	1			2					2
Confl. Bikes (#/hr)						1						
Heavy Vehicles (%)	6%	13%	6%	0%	5%	0%	4%	0%	0%	24%	0%	2%
Turn Type	Prot	NA	Perm	Prot	NA		Perm	NA		Perm	NA	
Protected Phases	5	2		1	6			8			4	
Permitted Phases			2				8			4		
Actuated Green, G (s)	4.6	37.4	37.4		28.3			18.9			18.9	
Effective Green, g (s)	5.1	37.4	37.4		28.3			20.4			20.4	
Actuated g/C Ratio	0.08	0.57	0.57		0.43			0.31			0.31	
Clearance Time (s)	4.5	4.0	4.0		4.0			5.5			5.5	
Vehicle Extension (s)	2.3	5.8	5.8		5.8			2.5			2.5	
Lane Grp Cap (vph)	121	1672	780		1359			308			433	
v/s Ratio Prot	0.04	c0.31			c0.33							
v/s Ratio Perm			0.01					c0.15			0.05	
v/c Ratio	0.49	0.54	0.03		0.76			0.49			0.18	
Uniform Delay, d1	29.1	8.8	6.2		15.9			18.5			16.6	
Progression Factor	1.00	1.00	1.00		1.00			1.00			1.00	
Incremental Delay, d2	1.8	0.8	0.0		3.3			0.9			0.1	
Delay (s)	30.9	9.6	6.3		19.1			19.4			16.7	
Level of Service	С	Α	Α		В			В			В	
Approach Delay (s)		10.8			19.1			19.4			16.7	
Approach LOS		В			В			В			В	
Intersection Summary												
HCM 2000 Control Delay			15.4	H	CM 2000	Level of S	Service		В			
HCM 2000 Volume to Capac	city ratio		0.66									
Actuated Cycle Length (s)	•		65.8	Sı	um of lost	time (s)			12.0			
Intersection Capacity Utilizat	tion		65.0%		U Level	()			С			
Analysis Period (min)			15									
c Critical Lane Group												

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HCM 6th Signalized Intersection Summary

3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26)

03/06/2020

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	75	^	7	7	↑ ↑			4			4	
Traffic Volume (veh/h)	54	832	33	0	937	16	136	3	2	21	7	135
Future Volume (veh/h)	54	832	33	0	937	16	136	3	2	21	7	135
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		0.98	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1668	1573	1668	1750	1682	1682	1750	1750	1750	1750	1750	1750
Adj Flow Rate, veh/h	59	904	0	0	1018	16	148	3	1	23	8	38
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	6	13	6	0	5	5	0	0	0	0	0	0
Cap, veh/h	86	2069	979	3	1853	29	350	5	2	140	59	139
Arrive On Green	0.05	0.69	0.00	0.00	0.58	0.58	0.13	0.16	0.13	0.13	0.16	0.13
Sat Flow, veh/h	1589	2988	1414	1667	3218	51	1430	29	10	349	378	891
Grp Volume(v), veh/h	59	904	0	0	505	529	152	0	0	69	0	0
Grp Sat Flow(s), veh/h/ln	1589	1494	1414	1667	1598	1671	1469	0	0	1618	0	0
Q Serve(g_s), s	2.0	7.5	0.0	0.0	11.0	11.0	3.3	0.0	0.0	0.0	0.0	0.0
Cycle Q Clear(g_c), s	2.0	7.5	0.0	0.0	11.0	11.0	5.5	0.0	0.0	2.2	0.0	0.0
Prop In Lane	1.00	1.5	1.00	1.00	11.0	0.03	0.97	0.0	0.01	0.33	0.0	0.55
Lane Grp Cap(c), veh/h	86	2069	979	3	920	963	316	0	0.01	294	0	0.55
V/C Ratio(X)	0.69	0.44	0.00	0.00	0.55	0.55	0.48	0.00	0.00	0.23	0.00	0.00
Avail Cap(c a), veh/h	725	2673	1265	462	1144	1196	633	0.00	0.00	635	0.00	0.00
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	0.00	0.00	1.00	1.00	1.00	0.00	0.00	1.00	0.00	0.00
					7.4					21.4		
Uniform Delay (d), s/veh	26.0	3.8	0.0	0.0	1.7	7.4	22.8	0.0	0.0		0.0	0.0
Incr Delay (d2), s/veh	5.9	0.5	0.0	0.0		1.6	0.8	0.0	0.0	0.3	0.0	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.9	1.5	0.0	0.0	3.3	3.5	2.0	0.0	0.0	0.8	0.0	0.0
Unsig. Movement Delay, s/veh		4.0		0.0			00 =	0.0		04 =	0.0	0.0
LnGrp Delay(d),s/veh	31.9	4.3	0.0	0.0	9.0	8.9	23.7	0.0	0.0	21.7	0.0	0.0
LnGrp LOS	С	Α	A	Α	Α	A	С	A	A	С	A	A
Approach Vol, veh/h		963			1034			152			69	
Approach Delay, s/veh		6.0			9.0			23.7			21.7	
Approach LOS		Α			Α			С			С	
Timer - Assigned Phs	1	2		4	5	6		8				
Phs Duration (G+Y+Rc), s	0.0	43.2		12.7	7.0	36.2		12.7				
Change Period (Y+Rc), s	4.5	* 4.5		5.5	4.5	4.0		5.5				
Max Green Setting (Gmax), s	15.0	* 50		20.0	25.0	40.0		20.0				
Max Q Clear Time (g_c+l1), s	0.0	9.5		4.2	4.0	13.0		7.5				
Green Ext Time (p_c), s	0.0	23.0		0.2	0.1	19.2		0.4				
Intersection Summary												
HCM 6th Ctrl Delay			9.1									
HCM 6th LOS			A									
Notos												

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

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3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (U	S 26))
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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	^	7	7	↑ Ъ			4			4	
Traffic Volume (vph)	151	1095	134	5	1128	16	116	24	7	38	23	100
Future Volume (vph)	151	1095	134	5	1128	16	116	24	7	38	23	100
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Storage Length (ft)	115		100	105		0	0		0	0		0
Storage Lanes	1		1	1		0	0		0	0		0
Taper Length (ft)	100			100			100			100		
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	0.95	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor	1.00		0.98	1.00	1.00			1.00			0.99	
Frt			0.850		0.998			0.994			0.916	
Flt Protected	0.950			0.950				0.962			0.988	
Satd. Flow (prot)	1646	3228	1488	1662	3161	0	0	1639	0	0	1553	0
Flt Permitted	0.950	0220		0.950	0.0.		•	0.548	•	Ū	0.910	J
Satd. Flow (perm)	1645	3228	1454	1662	3161	0	0	930	0	0	1431	0
Right Turn on Red			Yes			Yes			Yes	-		Yes
Satd. Flow (RTOR)			95		2			2			67	
Link Speed (mph)		25			25			25			25	
Link Distance (ft)		468			302			365			210	
Travel Time (s)		12.8			8.2			10.0			5.7	
Confl. Peds. (#/hr)	2		1	1		2	5				<u> </u>	5
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Heavy Vehicles (%)	1%	3%	0%	0%	5%	0%	1%	8%	0%	0%	0%	1%
Adj. Flow (vph)	161	1165	143	5	1200	17	123	26	7	40	24	106
Shared Lane Traffic (%)		.,							-			
Lane Group Flow (vph)	161	1165	143	5	1217	0	0	156	0	0	170	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	L NA	R NA	R NA	L NA	R NA	R NA	L NA	R NA	R NA	L NA	R NA	R NA
Median Width(ft)		12			12			0			0	
Link Offset(ft)		-12			0			0			4	
Crosswalk Width(ft)		72			42			32			30	
Two way Left Turn Lane												
Headway Factor	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11
Number of Detectors	2	1	1	2	1		2	2		2	2	
Detector Template	Left	Det25	Right	Left	Det25		Left	Side St		Left	Side St	
Leading Detector (ft)	78	153	153	78	153		78	78		78	78	
Trailing Detector (ft)	2	137	137	2	137		2	2		2	2	
Detector 1 Position(ft)	2	137	137	2	137		2	2		2	2	
Detector 1 Size(ft)	16	16	16	16	16		16	16		16	16	
Detector 1 Type	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex		CI+Ex	CI+Ex		CI+Ex	CI+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Detector 2 Position(ft)	72			72			72	72		72	72	
Detector 2 Size(ft)	6			6			6	6		6	6	
Detector 2 Type	CI+Ex			CI+Ex			CI+Ex	CI+Ex		CI+Ex	CI+Ex	
Detector 2 Channel												
Detector 2 Extend (s)	0.0			0.0			0.0	0.0		0.0	0.0	
Turn Type	Prot	NA	Perm	Prot	NA		Perm	NA		Perm	NA	

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Synchro 10 Report - by PBS Engineering and Environmental Page 1

Lanes, Volumes, Timings 3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26) 03/06/2020 ٠ t **EBL NBT NBR** SBL Lane Group **EBT EBR Protected Phases** 2 Permitted Phases 2 8 **Detector Phase** 5 2 2 8 8 4 4 Switch Phase 10.0 10.0 10.0 6.0 Minimum Initial (s) 4.0 4.0 6.0 6.0 6.0 Minimum Split (s) 8.5 26.0 26.0 8.5 30.0 11.5 11.5 23.5 23.5 Total Split (s) 20.0 70.0 70.0 15.0 65.0 25.0 25.0 25.0 25.0 18.2% 63.6% 59.1% 22.7% 22.7% Total Split (%) 63.6% 13.6% 22.7% 22.7% Maximum Green (s) 15.5 66.0 66.0 10.5 61.0 19.5 19.5 19.5 19.5 Yellow Time (s) 3.5 3.5 3.5 3.5 3.5 4.0 4.0 4.0 4.0 0.5 All-Red Time (s) 1.0 0.5 1.0 0.5 1.5 1.5 1.5 1.5 Lost Time Adjust (s) -0.5 0.0 0.0 -0.5 0.0 -1.5 -1.5 4.0 4.0 4.0 4.0 4.0 4.0 4.0 Total Lost Time (s) Lead/Lag Lag Lead Lag Lag Lead Lead-Lag Optimize? Vehicle Extension (s) 2.3 5.8 5.8 2.3 5.8 2.5 2.5 2.5 2.5 Minimum Gap (s) 0.5 3.8 3.8 0.5 3.8 2.0 2.0 2.0 2.0 Time Before Reduce (s) 5.0 5.0 5.0 8.0 10.0 10.0 8.0 10.0 5.0 Time To Reduce (s) 3.0 5.0 5.0 5.0 5.0 10.0 10.0 3.0 10.0 Recall Mode None C-Min C-Min None C-Min None None None None Walk Time (s) 7.0 7.0 7.0 7.0 7.0 Flash Dont Walk (s) 15.0 15.0 19.0 11.0 11.0 Pedestrian Calls (#/hr) 0 0 0 0 Act Effct Green (s) 15.0 76.5 76.5 5.8 59.7 23.3 23.3 Actuated g/C Ratio 0.14 0.70 0.70 0.05 0.54 0.21 0.21 v/c Ratio 0.72 0.78 0.52 0.14 0.06 0.71 0.48 68.1 Control Delay 63.6 9.5 2.8 50.6 22.2 27.7 0.0 0.0 0.0 0.0 Queue Delay 0.0 0.0 0.0 **Total Delay** 63.6 9.5 2.8 50.6 22.2 68.1 27.7 LOS Ε Α Α D C Ε C Approach Delay 14.8 22.3 68.1 27.7 Approach LOS В С Ε С Intersection Summary Area Type: Other Cycle Length: 110 Actuated Cycle Length: 110 Offset: 0 (0%), Referenced to phase 2:EBT and 6:WBT, Start of Green, Master Intersection Natural Cycle: 70 Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.78 Intersection Signal Delay: 21.3 Intersection LOS: C Intersection Capacity Utilization 76.9% ICU Level of Service D Analysis Period (min) 15 Splits and Phases: 3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26) ÿ1 Ø4 ₩ Ø2 (R) <u>≯_{Ø5}</u> ¶ Ø8 Ø6 (R) 2020 Existing Conditions - Weekday PM Peak Hour Page 2

3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26)

03/06/2020

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Lane Group	EBL	EBT	EBR	WBL	WBT	NBT	SBT
Lane Group Flow (vph)	161	1165	143	5	1217	156	170
v/c Ratio	0.72	0.52	0.14	0.06	0.71	0.78	0.48
Control Delay	63.6	9.5	2.8	50.6	22.2	68.1	27.7
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	63.6	9.5	2.8	50.6	22.2	68.1	27.7
Queue Length 50th (ft)	109	205	11	3	366	97	58
Queue Length 95th (ft)	#182	280	35	16	396	#233	136
Internal Link Dist (ft)		388			222	285	130
Turn Bay Length (ft)	115		100	105			
Base Capacity (vph)	248	2246	1040	166	1799	202	361
Starvation Cap Reductn	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	0.65	0.52	0.14	0.03	0.68	0.77	0.47
Intersection Summary							

^{# 95}th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis 3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26)

03/06/2020

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	^	7	7	∱ î≽			4			44	
Traffic Volume (vph)	151	1095	134	5	1128	16	116	24	7	38	23	100
Future Volume (vph)	151	1095	134	5	1128	16	116	24	7	38	23	100
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0			4.0			4.0	
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95			1.00			1.00	
Frpb, ped/bikes	1.00	1.00	0.98	1.00	1.00			1.00			0.99	
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00			1.00			1.00	
Frt	1.00	1.00	0.85	1.00	1.00			0.99			0.92	
Flt Protected	0.95	1.00	1.00	0.95	1.00			0.96			0.99	
Satd. Flow (prot)	1646	3228	1454	1662	3161			1632			1554	
Flt Permitted	0.95	1.00	1.00	0.95	1.00			0.55			0.91	
Satd. Flow (perm)	1646	3228	1454	1662	3161			930			1430	
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Adj. Flow (vph)	161	1165	143	5	1200	17	123	26	7	40	24	106
RTOR Reduction (vph)	0	0	32	0	1	0	0	2	0	0	53	0
Lane Group Flow (vph)	161	1165	111	5	1216	0	0	154	0	0	117	0
Confl. Peds. (#/hr)	2		1	1		2	5					5
Heavy Vehicles (%)	1%	3%	0%	0%	5%	0%	1%	8%	0%	0%	0%	1%
Turn Type	Prot	NA	Perm	Prot	NA		Perm	NA		Perm	NA	
Protected Phases	5	2		1	6			8			4	
Permitted Phases			2				8			4		
Actuated Green, G (s)	18.0	73.0	73.0	1.2	56.2			21.8			21.8	
Effective Green, g (s)	18.5	73.0	73.0	1.7	56.2			23.3			23.3	
Actuated g/C Ratio	0.17	0.66	0.66	0.02	0.51			0.21			0.21	
Clearance Time (s)	4.5	4.0	4.0	4.5	4.0			5.5			5.5	
Vehicle Extension (s)	2.3	5.8	5.8	2.3	5.8			2.5			2.5	
Lane Grp Cap (vph)	276	2142	964	25	1614			196			302	
v/s Ratio Prot	0.10	c0.36		0.00	c0.38							
v/s Ratio Perm			0.08					c0.17			0.08	
v/c Ratio	0.58	0.54	0.12	0.20	0.75			0.79			0.39	
Uniform Delay, d1	42.2	9.7	6.7	53.5	21.4			41.0			37.2	
Progression Factor	1.00	1.00	1.00	1.00	1.00			1.00			1.00	
Incremental Delay, d2	2.4	1.0	0.2	2.3	3.3			18.0			0.6	
Delay (s)	44.6	10.7	7.0	55.8	24.7			59.0			37.8	
Level of Service	D	В	Α	Е	С			Е			D	
Approach Delay (s)		14.1			24.8			59.0			37.8	
Approach LOS		В			С			Е			D	
Intersection Summary												
HCM 2000 Control Delay			22.1	Н	CM 2000	Level of S	Service		С			
HCM 2000 Volume to Capac	itv ratio		0.73									
Actuated Cycle Length (s)	,		110.0	S	um of lost	time (s)			12.0			
Intersection Capacity Utilizati	ion		76.9%		CU Level	()			D			
Analysis Period (min)			15						_			
c Critical Lane Group												

Sandy Health Clinic - PBS Project 71524.000 2020 Existing Conditions - Weekday PM Peak Hour

HCM 6th Signalized Intersection Summary

3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26)

03/06/2020

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	75	^	7	ሻ	∱ Љ			4			4	
Traffic Volume (veh/h)	151	1095	134	5	1128	16	116	24	7	38	23	100
Future Volume (veh/h)	151	1095	134	5	1128	16	116	24	7	38	23	100
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	0.99		0.99	0.99		0.99
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1736	1709	1750	1750	1682	1682	1641	1641	1641	1750	1750	1750
Adj Flow Rate, veh/h	161	1165	112	5	1200	16	123	26	5	40	24	50
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Percent Heavy Veh, %	1	3	0	0	5	5	8	8	8	0	0	0
Cap, veh/h	375	2335	1065	16	1636	22	210	35	6	112	73	110
Arrive On Green	0.23	0.72	0.72	0.01	0.51	0.51	0.14	0.16	0.14	0.14	0.16	0.14
Sat Flow, veh/h	1654	3247	1481	1667	3229	43	958	221	40	432	464	700
Grp Volume(v), veh/h	161	1165	112	5	594	622	154	0	0	114	0	0
Grp Sat Flow(s),veh/h/ln	1654	1624	1481	1667	1598	1674	1219	0	0	1596	0	0
Q Serve(g_s), s	9.2	17.3	2.5	0.3	32.1	32.1	6.6	0.0	0.0	0.0	0.0	0.0
Cycle Q Clear(g_c), s	9.2	17.3	2.5	0.3	32.1	32.1	13.8	0.0	0.0	7.2	0.0	0.0
Prop In Lane	1.00		1.00	1.00		0.03	0.80		0.03	0.35		0.44
Lane Grp Cap(c), veh/h	375	2335	1065	16	810	848	234	0	0	274	0	0
V/C Ratio(X)	0.43	0.50	0.11	0.31	0.73	0.73	0.66	0.00	0.00	0.42	0.00	0.00
Avail Cap(c_a), veh/h	375	2335	1065	167	886	928	278	0	0	324	0	0
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	0.00	1.00	0.00	0.00
Uniform Delay (d), s/veh	36.5	6.8	4.7	54.1	21.3	21.3	45.7	0.0	0.0	42.6	0.0	0.0
Incr Delay (d2), s/veh	0.5	0.8	0.2	6.4	5.8	5.6	3.6	0.0	0.0	0.7	0.0	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	3.8	5.6	0.8	0.2	13.0	13.6	4.4	0.0	0.0	3.0	0.0	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	36.9	7.5	4.9	60.5	27.1	26.9	49.3	0.0	0.0	43.3	0.0	0.0
LnGrp LOS	D	Α	Α	Е	С	С	D	Α	Α	D	Α	Α
Approach Vol. veh/h		1438			1221			154			114	
Approach Delay, s/veh		10.6			27.1			49.3			43.3	
Approach LOS		В			С			D			D	
Timer - Assigned Phs	1	2		4	5	6		8				
Phs Duration (G+Y+Rc), s	5.1	83.6		21.3	28.9	59.8		21.3				
Change Period (Y+Rc), s	4.5	* 4.5		5.5	4.5	4.0		5.5				
Max Green Setting (Gmax), s	10.5	* 66		19.5	15.5	61.0		19.5				
Max Q Clear Time (g_c+l1), s	2.3	19.3		9.2	11.2	34.1		15.8				
Green Ext Time (p_c), s	0.0	34.8		0.3	0.2	21.7		0.2				
Intersection Summary	0.0	U-T.U		0.0	0.2	£1.1		U.Z				
			20.8									
HCM 6th LOS												
HCM 6th LOS			С									
Notes												

^{*} HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Sandy Health Clinic - PBS Project 71524.000 2020 Existing Conditions - Weekday PM Peak Hour

Lanes, Volumes, Timings

3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26)

03/10/2020

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	^	7	ሻ	∱ }			4			4	
Traffic Volume (vph)	56	865	34	0	974	17	141	3	2	22	7	140
Future Volume (vph)	56	865	34	0	974	17	141	3	2	22	7	140
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Storage Length (ft)	115		100	105		0	0		0	0		0
Storage Lanes	1		1	1		0	0		0	0		0
Taper Length (ft)	100			100			100			100		
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	0.95	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor			0.98		1.00			1.00			0.99	
Frt			0.850		0.997			0.998			0.888	
Flt Protected	0.950							0.954			0.994	
Satd. Flow (prot)	1568	2942	1403	1716	3158	0	0	1603	0	0	1454	0
Flt Permitted	0.950							0.572			0.951	
Satd. Flow (perm)	1568	2942	1372	1716	3158	0	0	959	0	0	1391	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			77		2			1			152	
Link Speed (mph)		25			25			25			25	
Link Distance (ft)		468			302			365			210	
Travel Time (s)		12.8			8.2			10.0			5.7	
Confl. Peds. (#/hr)			1	1			2					2
Confl. Bikes (#/hr)						1						_
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	6%	13%	6%	2%	5%	2%	4%	2%	2%	24%	2%	2%
Adj. Flow (vph)	61	940	37	0	1059	18	153	3	2	24	8	152
Shared Lane Traffic (%)												-
Lane Group Flow (vph)	61	940	37	0	1077	0	0	158	0	0	184	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	L NA	R NA	R NA	L NA	R NA	R NA	L NA	R NA	R NA	L NA	R NA	R NA
Median Width(ft)		12			12			0			0	
Link Offset(ft)		-12			0			0			4	
Crosswalk Width(ft)		72			42			32			30	
Two way Left Turn Lane												
Headway Factor	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11
Number of Detectors	2	1	1	2	1		2	2		2	2	
Detector Template	Left	Det25	Right	Left	Det25		Left	Side St		Left	Side St	
Leading Detector (ft)	78	153	153	78	153		78	78		78	78	
Trailing Detector (ft)	2	137	137	2	137		2	2		2	2	
Detector 1 Position(ft)	2	137	137	2	137		2	2		2	2	
Detector 1 Size(ft)	16	16	16	16	16		16	16		16	16	
Detector 1 Type	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex		CI+Ex	CI+Ex		CI+Ex	CI+Ex	
Detector 1 Channel		· ·	· ·	· ·	• •			•. =		· ·	· ·	
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Detector 2 Position(ft)	72	0.0	0.0	72	0.0		72	72		72	72	
Detector 2 Size(ft)	6			6			6	6		6	6	
Detector 2 Type	CI+Ex			Cl+Ex			CI+Ex	CI+Ex		CI+Ex	CI+Ex	
Detector 2 Channel	51. LX			O. P. E.A			5. · LX	O LX		5 LX	OI. EX	
Detector 2 Extend (s)	0.0			0.0			0.0	0.0		0.0	0.0	
DOLOGIO Z EXIONA (5)	0.0			0.0			0.0	0.0		0.0	0.0	

Sandy Health Clinic - PBS Project 71524.000 2022 Without Project Conditions - Weekday AM Peak Hour

Lanes, Volumes, Timings 3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26) 03/10/2020 ၨ t **EBL** EBT **EBR** WBL WBT WBR NBL **NBT** NBR SBL Lane Group **SBT** Turn Type Prot NA Perm Prot NA Perm NA Perm NA **Protected Phases** 5 2 6 8 4 Permitted Phases 8 5 2 2 6 4 **Detector Phase** 1 8 8 4 Switch Phase Minimum Initial (s) 4.0 10.0 10.0 4.0 10.0 6.0 6.0 6.0 6.0 Minimum Split (s) 8.5 26.0 26.0 8.5 30.0 11.5 11.5 23.5 23.5 29.5 54.0 54.0 44.0 25.5 25.5 25.5 Total Split (s) 19.5 25.5 Total Split (%) 29.8% 54.5% 54.5% 19.7% 44.4% 25.8% 25.8% 25.8% 25.8% Maximum Green (s) 25.0 50.0 50.0 15.0 40.0 20.0 20.0 20.0 20.0 4.0 4.0 Yellow Time (s) 3.5 3.5 3.5 3.5 3.5 4.0 4.0 All-Red Time (s) 1.0 0.5 0.5 1.0 0.5 1.5 1.5 1.5 1.5 Lost Time Adjust (s) -0.5 0.0 0.0 -0.5 0.0 -1.5 -1.5 Total Lost Time (s) 4.0 4.0 4.0 4.0 4.0 4.0 4.0 Lead/Lag Lag Lag Lag Lead Lead Lead-Lag Optimize? Vehicle Extension (s) 2.3 5.8 5.8 2.3 5.8 2.5 2.5 2.5 2.5 2.0 2.0 0.5 0.5 2.0 2.0 Minimum Gap (s) 3.8 3.8 3.8 Time Before Reduce (s) 8.0 5.0 5.0 5.0 10.0 10.0 8.0 10.0 5.0 Time To Reduce (s) 3.0 10.0 10.0 3.0 10.0 5.0 5.0 5.0 5.0 Recall Mode Min Min None None None Min None None None Walk Time (s) 7.0 7.0 7.0 7.0 7.0 Flash Dont Walk (s) 15.0 15.0 19.0 11.0 11.0 Pedestrian Calls (#/hr) 0 0 0 0 0 39.6 39.6 30.0 8.3 21.2 21.2 Act Effct Green (s) 0.12 0.43 0.31 Actuated g/C Ratio 0.57 0.57 0.31 v/c Ratio 0.32 0.56 0.05 0.79 0.54 0.35 Control Delay 36.8 22.3 33.3 10.1 0.4 9.1 Queue Delay 0.0 0.0 0.0 0.0 0.0 0.0 **Total Delay** 36.8 10.1 0.4 22.3 33.3 9.1 LOS В С С Α Approach Delay 11.3 22.3 33.3 9.1 С Approach LOS В С Α Intersection Summary Other Area Type: Cycle Length: 99 Actuated Cycle Length: 69.3 Natural Cycle: 65 Control Type: Actuated-Uncoordinated Maximum v/c Ratio: 0.79 Intersection Signal Delay: 17.4 Intersection LOS: B Intersection Capacity Utilization 66.8% ICU Level of Service C Analysis Period (min) 15 Splits and Phases: 3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26) 01 Ø4 ₩ Ø2 **≯** Ø5 T Ø8 Ø6 2022 Without Project Conditions - Weekday AM Peak Hour Page 2

3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26)

03/10/2020

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Lane Group	EBL	EBT	EBR	WBT	NBT	SBT
Lane Group Flow (vph)	61	940	37	1077	158	184
v/c Ratio	0.32	0.56	0.05	0.79	0.54	0.35
Control Delay	36.8	10.1	0.4	22.3	33.3	9.1
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	36.8	10.1	0.4	22.3	33.3	9.1
Queue Length 50th (ft)	26	118	0	213	59	10
Queue Length 95th (ft)	67	160	2	309	#160	65
Internal Link Dist (ft)		388		222	285	130
Turn Bay Length (ft)	115		100			
Base Capacity (vph)	613	2189	1040	1940	317	560
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.10	0.43	0.04	0.56	0.50	0.33
Intersection Summary						

^{# 95}th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis

3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26)

03/10/2020

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	^	7	ሻ	∱ β			44			44	
Traffic Volume (vph)	56	865	34	0	974	17	141	3	2	22	7	140
Future Volume (vph)	56	865	34	0	974	17	141	3	2	22	7	140
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Total Lost time (s)	4.0	4.0	4.0		4.0			4.0			4.0	
Lane Util. Factor	1.00	0.95	1.00		0.95			1.00			1.00	
Frpb, ped/bikes	1.00	1.00	0.98		1.00			1.00			0.99	
Flpb, ped/bikes	1.00	1.00	1.00		1.00			1.00			1.00	
Frt	1.00	1.00	0.85		1.00			1.00			0.89	
Flt Protected	0.95	1.00	1.00		1.00			0.95			0.99	
Satd. Flow (prot)	1568	2942	1373		3159			1601			1456	
Flt Permitted	0.95	1.00	1.00		1.00			0.57			0.95	
Satd. Flow (perm)	1568	2942	1373		3159			960			1394	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	61	940	37	0	1059	18	153	3	2	24	8	152
RTOR Reduction (vph)	0	0	15	0	1	0	0	1	0	0	106	0
Lane Group Flow (vph)	61	940	22	0	1076	0	0	157	0	0	78	0
Confl. Peds. (#/hr)			1	1			2					2
Confl. Bikes (#/hr)						1						
Heavy Vehicles (%)	6%	13%	6%	2%	5%	2%	4%	2%	2%	24%	2%	2%
Turn Type	Prot	NA	Perm	Prot	NA		Perm	NA		Perm	NA	
Protected Phases	5	2		1	6			8			4	
Permitted Phases			2				8			4		
Actuated Green, G (s)	6.2	40.7	40.7		30.0			19.6			19.6	
Effective Green, g (s)	6.7	40.7	40.7		30.0			21.1			21.1	
Actuated g/C Ratio	0.10	0.58	0.58		0.43			0.30			0.30	
Clearance Time (s)	4.5	4.0	4.0		4.0			5.5			5.5	
Vehicle Extension (s)	2.3	5.8	5.8		5.8			2.5			2.5	
Lane Grp Cap (vph)	150	1715	800		1357			290			421	
v/s Ratio Prot	0.04	c0.32			c0.34							
v/s Ratio Perm			0.02					c0.16			0.06	
v/c Ratio	0.41	0.55	0.03		0.79			0.54			0.19	
Uniform Delay, d1	29.7	8.9	6.2		17.2			20.3			18.0	
Progression Factor	1.00	1.00	1.00		1.00			1.00			1.00	
Incremental Delay, d2	1.0	0.8	0.0		3.9			1.6			0.2	
Delay (s)	30.7	9.7	6.2		21.1			22.0			18.2	
Level of Service	С	Α	Α		С			С			В	
Approach Delay (s)		10.8			21.1			22.0			18.2	
Approach LOS		В			С			С			В	
Intersection Summary												
HCM 2000 Control Delay			16.6	Н	CM 2000	Level of S	Service		В			
HCM 2000 Volume to Capa	city ratio		0.70									
Actuated Cycle Length (s)	-		69.8	Sı	um of lost	time (s)			12.0			
Intersection Capacity Utiliza	ition		66.8%		U Level	()			С			
Analysis Period (min)			15									
c Critical Lane Group												

Sandy Health Clinic - PBS Project 71524.000 2022 Without Project Conditions - Weekday AM Peak Hour

HCM 6th Signalized Intersection Summary

3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26)

03/10/2020

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	75	^	7	ሻ	↑ ↑			4			4	
Traffic Volume (veh/h)	56	865	34	0	974	17	141	3	2	22	7	140
Future Volume (veh/h)	56	865	34	0	974	17	141	3	2	22	7	140
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		0.98	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1668	1573	1668	1723	1682	1682	1723	1723	1723	1723	1723	1723
Adj Flow Rate, veh/h	61	940	1	0	1059	17	153	3	1	24	8	38
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	6	13	6	2	5	5	2	2	2	2	2	2
Cap, veh/h	88	2069	978	3	1854	30	349	4	1	140	61	139
Arrive On Green	0.06	0.69	0.69	0.00	0.58	0.58	0.13	0.16	0.13	0.13	0.16	0.13
Sat Flow, veh/h	1589	2988	1413	1641	3217	52	1407	28	9	350	378	865
Grp Volume(v), veh/h	61	940	1	0	526	550	157	0	0	70	0	0
Grp Sat Flow(s), veh/h/ln	1589	1494	1413	1641	1598	1671	1444	0	0	1594	0	0
Q Serve(g_s), s	2.2	8.1	0.0	0.0	12.0	12.0	3.6	0.0	0.0	0.0	0.0	0.0
Cycle Q Clear(g_c), s	2.2	8.1	0.0	0.0	12.0	12.0	6.0	0.0	0.0	2.3	0.0	0.0
		0.1			12.0	0.03		0.0			0.0	0.54
Prop In Lane	1.00	2060	1.00	1.00	004		0.97	٥	0.01	0.34	٥	
Lane Grp Cap(c), veh/h	88	2069	978	3	921	963	317	0	0	298	0	0
V/C Ratio(X)	0.69	0.45	0.00	0.00	0.57	0.57	0.50	0.00	0.00	0.24	0.00	0.00
Avail Cap(c_a), veh/h	702	2590	1225	441	1108	1159	606	0	0	609	0	0
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	0.00	1.00	1.00	1.00	0.00	0.00	1.00	0.00	0.00
Uniform Delay (d), s/veh	26.8	4.0	2.7	0.0	7.7	7.7	23.4	0.0	0.0	21.9	0.0	0.0
Incr Delay (d2), s/veh	5.8	0.5	0.0	0.0	1.8	1.7	0.9	0.0	0.0	0.3	0.0	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.9	1.7	0.0	0.0	3.7	3.9	2.1	0.0	0.0	0.9	0.0	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	32.6	4.5	2.7	0.0	9.5	9.4	24.3	0.0	0.0	22.2	0.0	0.0
LnGrp LOS	С	Α	Α	Α	Α	Α	С	Α	Α	С	Α	A
Approach Vol, veh/h		1002			1076			157			70	
Approach Delay, s/veh		6.2			9.5			24.3			22.2	
Approach LOS		Α			Α			С			С	
Timer - Assigned Phs	1	2		4	5	6		8				
Phs Duration (G+Y+Rc), s	0.0	44.4		13.2	7.2	37.2		13.2				
Change Period (Y+Rc), s	4.5	* 4.5		5.5	4.5	4.0		5.5				
Max Green Setting (Gmax), s	15.0	* 50		20.0	25.0	40.0		20.0				
Max Q Clear Time (g_c+l1), s	0.0	10.1		4.3	4.2	14.0		8.0				
Green Ext Time (p_c), s	0.0	23.7		0.2	0.2	19.3		0.4				
Intersection Summary	- 0.0											
HCM 6th Ctrl Delay			9.5									
•												
HCM 6th LOS			A									
Notes												

^{*} HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Sandy Health Clinic - PBS Project 71524.000 2022 Without Project Conditions - Weekday AM Peak Hour

Lanes, Volumes, Timings

3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26)

03/10/2020

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	^	7	ሻ	∱ }			4			4	
Traffic Volume (vph)	157	1139	139	5	1173	17	121	25	7	40	24	104
Future Volume (vph)	157	1139	139	5	1173	17	121	25	7	40	24	104
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Storage Length (ft)	115		100	105		0	0		0	0		0
Storage Lanes	1		1	1		0	0		0	0		0
Taper Length (ft)	100		•	100			100		•	100		J
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	0.95	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor	1.00	0.00	0.98	1.00	1.00	0.00	1.00	1.00	1.00	1.00	0.99	1.00
Frt	1.00		0.850	1.00	0.998			0.994			0.917	
Flt Protected	0.950		0.000	0.950	0.000			0.962			0.988	
Satd. Flow (prot)	1630	3228	1458	1630	3161	0	0	1625	0	0	1534	0
Flt Permitted	0.950	0220	1400	0.950	0101	· ·	· ·	0.549	•	U	0.906	U
Satd. Flow (perm)	1629	3228	1426	1629	3161	0	0	924	0	0	1407	0
Right Turn on Red	1020	0220	Yes	1020	0101	Yes	J	024	Yes	U	1407	Yes
Satd. Flow (RTOR)			95		2	100		2	100		65	100
Link Speed (mph)		25	00		25			25			25	
Link Distance (ft)		468			302			365			210	
Travel Time (s)		12.8			8.2			10.0			5.7	
Confl. Peds. (#/hr)	2	12.0	1	1	0.2	2	5	10.0			0.1	5
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Heavy Vehicles (%)	2%	3%	2%	2%	5%	2%	2%	8%	2%	2%	2%	2%
Adj. Flow (vph)	167	1212	148	5	1248	18	129	27	7	43	26	111
Shared Lane Traffic (%)	107	1212	170		1240	10	123	21		70	20	- '''
Lane Group Flow (vph)	167	1212	148	5	1266	0	0	163	0	0	180	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	L NA	R NA	R NA	L NA	R NA	R NA	L NA	R NA	R NA	L NA	R NA	R NA
Median Width(ft)		12	11171		12	1 (10)		0			0	1010
Link Offset(ft)		-12			0			0			4	
Crosswalk Width(ft)		72			42			32			30	
Two way Left Turn Lane		· -			· -							
Headway Factor	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11
Number of Detectors	2	1	1	2	1		2	2		2	2	
Detector Template	Left	Det25	Right	Left	Det25		Left	Side St		Left	Side St	
Leading Detector (ft)	78	153	153	78	153		78	78		78	78	
Trailing Detector (ft)	2	137	137	2	137		2	2		2	2	
Detector 1 Position(ft)	2	137	137	2	137		2	2		2	2	
Detector 1 Size(ft)	16	16	16	16	16		16	16		16	16	
Detector 1 Type	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex		CI+Ex	CI+Ex		CI+Ex	CI+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Detector 2 Position(ft)	72			72			72	72		72	72	
Detector 2 Size(ft)	6			6			6	6		6	6	
Detector 2 Type	CI+Ex			CI+Ex			CI+Ex	CI+Ex		CI+Ex	CI+Ex	
Detector 2 Channel												
Detector 2 Extend (s)	0.0			0.0			0.0	0.0		0.0	0.0	
Turn Type	Prot	NA	Perm	Prot	NA		Perm	NA		Perm	NA	
Turn Type	Prot	NA	Perm	Prot	NA		Perm	NA		Perm	NA	

Sandy Health Clinic - PBS Project 71524.000 2022 Without Project Conditions - Weekday PM Peak Hour

Lanes, Volumes, Timings 3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26) 03/10/2020 ٠ t **NBT NBR** SBL Lane Group **EBL EBT EBR** SBT Protected Phases 2 Permitted Phases 2 8 **Detector Phase** 5 2 2 8 8 4 4 Switch Phase 10.0 10.0 10.0 6.0 Minimum Initial (s) 4.0 4.0 6.0 6.0 6.0 Minimum Split (s) 8.5 26.0 26.0 8.5 30.0 11.5 11.5 23.5 23.5 Total Split (s) 20.0 70.0 70.0 15.0 65.0 25.0 25.0 25.0 25.0 18.2% 63.6% 59.1% 22.7% 22.7% Total Split (%) 63.6% 13.6% 22.7% 22.7% Maximum Green (s) 15.5 66.0 66.0 10.5 61.0 19.5 19.5 19.5 19.5 Yellow Time (s) 3.5 3.5 3.5 3.5 3.5 4.0 4.0 4.0 4.0 All-Red Time (s) 1.0 0.5 0.5 1.0 0.5 1.5 1.5 1.5 1.5 Lost Time Adjust (s) -0.5 0.0 0.0 -0.5 0.0 -1.5 -1.5 4.0 4.0 4.0 4.0 4.0 4.0 4.0 Total Lost Time (s) Lead/Lag Lag Lead Lag Lag Lead Lead-Lag Optimize? Vehicle Extension (s) 2.3 5.8 5.8 2.3 5.8 2.5 2.5 2.5 2.5 Minimum Gap (s) 0.5 3.8 3.8 0.5 3.8 2.0 2.0 2.0 2.0 Time Before Reduce (s) 5.0 5.0 5.0 8.0 10.0 10.0 8.0 10.0 5.0 Time To Reduce (s) 3.0 5.0 5.0 5.0 5.0 10.0 10.0 3.0 10.0 Recall Mode None C-Min C-Min None C-Min None None None None Walk Time (s) 7.0 7.0 7.0 7.0 7.0 Flash Dont Walk (s) 15.0 15.0 19.0 11.0 11.0 Pedestrian Calls (#/hr) 0 0 0 0 Act Effct Green (s) 15.2 74.6 74.6 5.8 57.5 25.3 25.3 Actuated g/C Ratio 0.14 0.68 0.68 0.05 0.52 0.23 0.23 v/c Ratio 0.74 0.76 0.55 0.15 0.06 0.77 0.48 65.3 64.2 28.9 Control Delay 10.5 2.9 50.6 24.8 0.0 0.0 0.0 0.0 Queue Delay 0.0 0.0 0.0 **Total Delay** 65.3 10.5 2.9 50.6 24.8 64.2 28.9 LOS Ε В Α D C Ε C Approach Delay 15.8 24.9 64.2 28.9 Approach LOS В С Ε С Intersection Summary Area Type: Other Cycle Length: 110 Actuated Cycle Length: 110 Offset: 0 (0%), Referenced to phase 2:EBT and 6:WBT, Start of Green, Master Intersection Natural Cycle: 75 Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.77 Intersection Signal Delay: 22.7 Intersection LOS: C Intersection Capacity Utilization 79.4% ICU Level of Service D Analysis Period (min) 15 Splits and Phases: 3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26) ÿ1 Ø4 ₩ Ø2 (R) <u>≯_{Ø5}</u> ¶ Ø8 Ø6 (R) 2022 Without Project Conditions - Weekday PM Peak Hour Page 2

3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26)

03/10/2020

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Lane Group	EBL	EBT	EBR	WBL	WBT	NBT	SBT
Lane Group Flow (vph)	167	1212	148	5	1266	163	180
v/c Ratio	0.74	0.55	0.15	0.06	0.77	0.76	0.48
Control Delay	65.3	10.5	2.9	50.6	24.8	64.2	28.9
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	65.3	10.5	2.9	50.6	24.8	64.2	28.9
Queue Length 50th (ft)	113	218	12	3	397	102	66
Queue Length 95th (ft)	#207	297	37	16	416	#245	147
Internal Link Dist (ft)		388			222	285	130
Turn Bay Length (ft)	115		100	105			
Base Capacity (vph)	247	2189	997	163	1758	214	373
Starvation Cap Reductn	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	0.68	0.55	0.15	0.03	0.72	0.76	0.48
Intersection Summary							

^{# 95}th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis 3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26)

03/10/2020

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	^	7	7	ተኈ			- €			↔	
Traffic Volume (vph)	157	1139	139	5	1173	17	121	25	7	40	24	104
Future Volume (vph)	157	1139	139	5	1173	17	121	25	7	40	24	104
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0			4.0			4.0	
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95			1.00			1.00	
Frpb, ped/bikes	1.00	1.00	0.98	1.00	1.00			1.00			0.99	
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00			1.00			1.00	
Frt	1.00	1.00	0.85	1.00	1.00			0.99			0.92	
Flt Protected	0.95	1.00	1.00	0.95	1.00			0.96			0.99	
Satd. Flow (prot)	1630	3228	1426	1630	3160			1618			1534	
Flt Permitted	0.95	1.00	1.00	0.95	1.00			0.55			0.91	
Satd. Flow (perm)	1630	3228	1426	1630	3160			924			1406	
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Adj. Flow (vph)	167	1212	148	5	1248	18	129	27	7	43	26	111
RTOR Reduction (vph)	0	0	34	0	1	0	0	2	0	0	50	0
Lane Group Flow (vph)	167	1212	114	5	1265	0	0	161	0	0	130	0
Confl. Peds. (#/hr)	2		1	1		2	5					5
Heavy Vehicles (%)	2%	3%	2%	2%	5%	2%	2%	8%	2%	2%	2%	2%
Turn Type	Prot	NA	Perm	Prot	NA		Perm	NA		Perm	NA	
Protected Phases	5	2		1	6			8			4	
Permitted Phases			2				8			4		
Actuated Green, G (s)	18.3	71.0	71.0	1.2	53.9			23.8			23.8	
Effective Green, g (s)	18.8	71.0	71.0	1.7	53.9			25.3			25.3	
Actuated g/C Ratio	0.17	0.65	0.65	0.02	0.49			0.23			0.23	
Clearance Time (s)	4.5	4.0	4.0	4.5	4.0			5.5			5.5	
Vehicle Extension (s)	2.3	5.8	5.8	2.3	5.8			2.5			2.5	
Lane Grp Cap (vph)	278	2083	920	25	1548			212			323	
v/s Ratio Prot	0.10	c0.38	020	0.00	c0.40						020	
v/s Ratio Perm	0.10	00.00	0.08	0.00	00.10			c0.17			0.09	
v/c Ratio	0.60	0.58	0.12	0.20	0.82			0.76			0.40	
Uniform Delay, d1	42.1	11.1	7.5	53.5	23.9			39.5			35.9	
Progression Factor	1.00	1.00	1.00	1.00	1.00			1.00			1.00	
Incremental Delay, d2	2.8	1.2	0.3	2.3	4.9			14.3			0.6	
Delay (s)	45.0	12.3	7.8	55.8	28.8			53.8			36.5	
Level of Service	D	В	A	E	C			D			D	
Approach Delay (s)		15.4	7.	_	28.9			53.8			36.5	
Approach LOS		В			C			D			D	
Intersection Summary												
HCM 2000 Control Delay			24.1	Н	CM 2000	Level of S	Service		С			
HCM 2000 Volume to Capac	city ratio		0.77									
Actuated Cycle Length (s)			110.0	S	um of lost	time (s)			12.0			
Intersection Capacity Utilizat	tion		79.4%	IC	CU Level o	of Service			D			
Analysis Period (min)			15									
c Critical Lane Group												

Sandy Health Clinic - PBS Project 71524.000 2022 Without Project Conditions - Weekday PM Peak Hour

HCM 6th Signalized Intersection Summary

3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26)

03/10/2020

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	^	7	ሻ	∱ ∱			4			44	
Traffic Volume (veh/h)	157	1139	139	5	1173	17	121	25	7	40	24	104
Future Volume (veh/h)	157	1139	139	5	1173	17	121	25	7	40	24	104
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	0.99		0.99	0.99		0.99
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1723	1709	1723	1723	1682	1682	1641	1641	1641	1723	1723	1723
Adj Flow Rate, veh/h	167	1212	116	5	1248	17	129	27	5	43	26	58
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Percent Heavy Veh, %	2	3	2	2	5	5	8	8	8	2	2	2
Cap, veh/h	339	2299	1032	16	1665	23	215	36	6	114	74	121
Arrive On Green	0.21	0.71	0.71	0.01	0.52	0.52	0.15	0.17	0.15	0.15	0.17	0.15
Sat Flow, veh/h	1641	3247	1458	1641	3227	44	924	214	36	414	442	719
Grp Volume(v), veh/h	167	1212	116	5	618	647	161	0	0	127	0	0
Grp Sat Flow(s), veh/h/ln	1641	1624	1458	1641	1598	1674	1174	0	0	1574	0	0
Q Serve(q s), s	9.9	19.1	2.8	0.3	33.6	33.6	7.0	0.0	0.0	0.0	0.0	0.0
Cycle Q Clear(g_c), s	9.9	19.1	2.8	0.3	33.6	33.6	15.0	0.0	0.0	8.1	0.0	0.0
Prop In Lane	1.00	13.1	1.00	1.00	33.0	0.03	0.80	0.0	0.03	0.34	0.0	0.46
Lane Grp Cap(c), veh/h	339	2299	1032	1.00	824	864	241	0	0.03	288	0	0.40
V/C Ratio(X)	0.49	0.53	0.11	0.31	0.75	0.75	0.67	0.00	0.00	0.44	0.00	0.00
Avail Cap(c a), veh/h	339	2299	1032	164	886	928	270	0.00	0.00	320	0.00	0.00
HCM Platoon Ratio	1.00		1.00		1.00	1.00		1.00		1.00		1.00
		1.00		1.00	1.00		1.00		1.00		1.00	0.00
Upstream Filter(I)	1.00	1.00	1.00	1.00		1.00	1.00	0.00	0.00	1.00	0.00	
Uniform Delay (d), s/veh	38.6	7.5	5.1	54.1	21.0	21.0	45.3	0.0	0.0	41.9	0.0	0.0
Incr Delay (d2), s/veh	0.7	0.9	0.2	6.7	6.2	5.9	4.7	0.0	0.0	8.0	0.0	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	4.1	6.3	0.9	0.2	13.6	14.2	4.7	0.0	0.0	3.3	0.0	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	39.2	8.3	5.3	60.8	27.2	26.9	50.0	0.0	0.0	42.7	0.0	0.0
LnGrp LOS	D	A	A	E	С	С	D	A	A	D	A	A
Approach Vol, veh/h		1495			1270			161			127	
Approach Delay, s/veh		11.6			27.2			50.0			42.7	
Approach LOS		В			С			D			D	
Timer - Assigned Phs	1	2		4	5	6		8				
Phs Duration (G+Y+Rc), s	5.1	82.4		22.5	26.7	60.8		22.5				
Change Period (Y+Rc), s	4.5	* 4.5		5.5	4.5	4.0		5.5				
Max Green Setting (Gmax), s	10.5	* 66		19.5	15.5	61.0		19.5				
Max Q Clear Time (g_c+l1), s	2.3	21.1		10.1	11.9	35.6		17.0				
Green Ext Time (p_c), s	0.0	34.9		0.3	0.2	21.2		0.1				
Intersection Summary												
HCM 6th Ctrl Delay			21.4									
HCM 6th LOS												
			C									

^{*} HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Sandy Health Clinic - PBS Project 71524.000 2022 Without Project Conditions - Weekday PM Peak Hour

Lanes, Volumes, Timings

03/10/2020

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	^	7	ሻ	∱ }			4			4	
Traffic Volume (vph)	80	865	34	0	974	20	141	3	2	23	7	147
Future Volume (vph)	80	865	34	0	974	20	141	3	2	23	7	147
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Storage Length (ft)	115		100	105		0	0		0	0		0
Storage Lanes	1		1	1		0	0		0	0		0
Taper Length (ft)	100			100			100			100		
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	0.95	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor			0.98		1.00			1.00			0.99	
Frt			0.850		0.997			0.998			0.888	
Flt Protected	0.950							0.954			0.994	
Satd. Flow (prot)	1568	2942	1403	1716	3158	0	0	1603	0	0	1455	0
Flt Permitted	0.950							0.555			0.951	
Satd. Flow (perm)	1568	2942	1372	1716	3158	0	0	931	0	0	1392	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			77		2			1			160	
Link Speed (mph)		25			25			25			25	
Link Distance (ft)		468			302			365			210	
Travel Time (s)		12.8			8.2			10.0			5.7	
Confl. Peds. (#/hr)			1	1			2					2
Confl. Bikes (#/hr)						1						
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	6%	13%	6%	2%	5%	2%	4%	2%	2%	24%	2%	2%
Adj. Flow (vph)	87	940	37	0	1059	22	153	3	2	25	8	160
Shared Lane Traffic (%)												
Lane Group Flow (vph)	87	940	37	0	1081	0	0	158	0	0	193	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	L NA	R NA	R NA	L NA	R NA	R NA	L NA	R NA	R NA	L NA	R NA	R NA
Median Width(ft)		12			12			0			0	
Link Offset(ft)		-12			0			0			4	
Crosswalk Width(ft)		72			42			32			30	
Two way Left Turn Lane												
Headway Factor	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11
Number of Detectors	2	1	1	2	1		2	2		2	2	
Detector Template	Left	Det25	Right	Left	Det25		Left	Side St		Left	Side St	
Leading Detector (ft)	78	153	153	78	153		78	78		78	78	
Trailing Detector (ft)	2	137	137	2	137		2	2		2	2	
Detector 1 Position(ft)	2	137	137	2	137		2	2		2	2	
Detector 1 Size(ft)	16	16	16	16	16		16	16		16	16	
Detector 1 Type	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex		CI+Ex	CI+Ex		CI+Ex	CI+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Detector 2 Position(ft)	72			72			72	72		72	72	
Detector 2 Size(ft)	6			6			6	6		6	6	
Detector 2 Type	CI+Ex			CI+Ex			CI+Ex	CI+Ex		CI+Ex	CI+Ex	
Detector 2 Channel	J.: LA			J L.			J. : LA	J L.K		J LA	J L.	
Detector 2 Extend (s)	0.0			0.0			0.0	0.0		0.0	0.0	
= 5.00tor = Extorio (0)	0.0			0.0			0.0	0.0		0.0	0.0	

Sandy Health Clinic - PBS Project 71524.000 2022 With Project Conditions - Weekday AM Peak Hour

Lanes, Volumes, Timings 3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26) 03/10/2020 ၨ t **EBL** EBT **EBR** WBL WBT WBR NBL **NBT** NBR SBL Lane Group **SBT** Turn Type Prot NA Perm Prot NA Perm NA Perm NA **Protected Phases** 5 2 6 8 4 **Permitted Phases** 8 5 2 2 6 4 **Detector Phase** 1 8 8 4 Switch Phase Minimum Initial (s) 4.0 10.0 10.0 4.0 10.0 6.0 6.0 6.0 6.0 Minimum Split (s) 8.5 26.0 26.0 8.5 30.0 11.5 11.5 23.5 23.5 29.5 54.0 54.0 44.0 25.5 25.5 25.5 Total Split (s) 19.5 25.5 Total Split (%) 29.8% 54.5% 54.5% 19.7% 44.4% 25.8% 25.8% 25.8% 25.8% Maximum Green (s) 25.0 50.0 50.0 15.0 40.0 20.0 20.0 20.0 20.0 4.0 Yellow Time (s) 3.5 3.5 3.5 3.5 3.5 4.0 4.0 4.0 All-Red Time (s) 1.0 0.5 0.5 1.0 0.5 1.5 1.5 1.5 1.5 Lost Time Adjust (s) -0.5 0.0 0.0 -0.5 0.0 -1.5 -1.5 Total Lost Time (s) 4.0 4.0 4.0 4.0 4.0 4.0 4.0 Lead/Lag Lag Lag Lag Lead Lead Lead-Lag Optimize? Vehicle Extension (s) 2.3 5.8 5.8 2.3 5.8 2.5 2.5 2.5 2.5 2.0 0.5 0.5 2.0 2.0 2.0 Minimum Gap (s) 3.8 3.8 3.8 Time Before Reduce (s) 8.0 5.0 5.0 5.0 10.0 10.0 8.0 10.0 5.0 Time To Reduce (s) 3.0 10.0 10.0 3.0 10.0 5.0 5.0 5.0 5.0 Recall Mode Min Min None None None Min None None None Walk Time (s) 7.0 7.0 7.0 7.0 7.0 Flash Dont Walk (s) 15.0 15.0 19.0 11.0 11.0 Pedestrian Calls (#/hr) 0 0 0 0 0 41.4 41.4 30.6 9.6 21.9 21.9 Act Effct Green (s) 0.13 0.43 Actuated g/C Ratio 0.58 0.58 0.31 0.31 v/c Ratio 0.42 0.55 0.04 0.80 0.55 0.36 Control Delay 38.3 23.7 35.6 10.0 0.4 9.2 Queue Delay 0.0 0.0 0.0 0.0 0.0 0.0 **Total Delay** 38.3 10.0 0.4 23.7 35.6 9.2 LOS D Α C D Α Approach Delay 11.9 23.7 35.6 9.2 Approach LOS В С D Α Intersection Summary Other Area Type: Cycle Length: 99 Actuated Cycle Length: 71.7) Natural Cycle: 65 Control Type: Actuated-Uncoordinated Maximum v/c Ratio: 0.80 Intersection Signal Delay: 18.3 Intersection LOS: B Intersection Capacity Utilization 68.9% ICU Level of Service C Analysis Period (min) 15 Splits and Phases: 3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26) Ø1 Ø4 ₩ Ø2 <u>≯_{ø5}</u> **↑**08 Ø6 2022 With Project Conditions - Weekday AM Peak Hour Page 2

3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26)

03/10/2020

	•	→	•	•	†	ļ
Lane Group	EBL	EBT	EBR	WBT	NBT	SBT
Lane Group Flow (vph)	87	940	37	1081	158	193
v/c Ratio	0.42	0.55	0.04	0.80	0.55	0.36
Control Delay	38.3	10.0	0.4	23.7	35.6	9.2
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	38.3	10.0	0.4	23.7	35.6	9.2
Queue Length 50th (ft)	38	118	0	223	63	11
Queue Length 95th (ft)	88	160	2	327	#171	69
Internal Link Dist (ft)		388		222	285	130
Turn Bay Length (ft)	115		100			
Base Capacity (vph)	587	2179	1036	1858	295	549
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.15	0.43	0.04	0.58	0.54	0.35
Intersection Summary						

^{# 95}th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis 3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26)

03/10/2020

Movement		۶	→	•	•	←	•	1	†	/	/	+	4
Traffic Volume (vph)	Movement		EBT		WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Future Volume (vph)	Lane Configurations	ሻ	^	7	Ť	∱ }			- ↔			- ↔	
Ideal Flow (yphp) 1750 1	Traffic Volume (vph)	80	865	34	0	974	20	141	3	2	23	7	147
Total Lost time (s)	Future Volume (vph)	80	865	34	0	974	20	141	3	2	23	7	147
Lane Util. Factor 1.00 0.95 1.00 0.95 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.0	Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Frpb, ped/bikes	Total Lost time (s)	4.0	4.0	4.0		4.0			4.0			4.0	
Fipb, ped/bikes	Lane Util. Factor		0.95										
Fit 1.00 1.00 0.85 1.00 1.00 0.85 0.99 Satd. Flow (prot) 1568 2942 1373 3157 1601 1455 Flit Permitted 0.95 1.00 1.00 1.00 0.55 0.99 Satd. Flow (prot) 1568 2942 1373 3157 1601 1455 Flit Permitted 0.95 1.00 1.00 1.00 0.55 Satd. Flow (perm) 1568 2942 1373 3157 931 1333 Satd. Flow (perm) 1568 2942 1373 3157 931 1333 Satd. Flow (perm) 1568 2942 1373 3157 931 1333 Satd. Flow (perm) 1568 2942 1373 3157 931 1333 Satd. Flow (perm) 1568 2942 1373 3157 931 1333 Satd. Flow (perm) 1568 2942 1373 3157 931 1333 Reak-hour factor, PHF 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92	_ 1 - 2 1												
File Protected 0.95 1.00 1.00 1.00 0.95 0.99 Satd. Flow (prot) 1568 2942 1373 3157 1601 1455 File Permitted 0.95 1.00 1.00 1.00 0.555 0.95 Satd. Flow (perm) 1568 2942 1373 3157 931 1393 Satd. Flow (perm) 1568 2942 1373 3157 931 1393 Satd. Flow (perm) 1568 2942 1373 3157 931 1393 Satd. Flow (perm) 1568 2942 1373 3157 931 1393 Satd. Flow (perm) 1568 2942 1373 3157 931 1393 Satd. Flow (perm) 1568 2942 1373 3157 931 1393 Satd. Flow (perm) 1568 2942 1373 3157 931 1393 Satd. Flow (perm) 1568 2942 1373 3157 931 Satd. Flow (perm) 1568 2942 1373 3157 931 Satd. Flow (perm) 1568 2942 1373 3157 931 Satd. Flow (perm) 1568 2942 153 3 2 25 8 160 Satd. Flow (perm) 1568 2942 153 3 2 25 8 160 Satd. Flow (perm) 1568 2942 153 3 2 25 8 160 Satd. Flow (perm) 1568 294 294 294 294 294 294 294 294 294 294	1 1												
Satd. Flow (prot)													
Fit Permitted 0.95 1.00 1.00 1.00 0.55 0.95	Flt Protected		1.00	1.00		1.00						0.99	
Satd. Flow (perm) 1568 2942 1373 3157 931 1393 Peak-hour factor, PHF 0.92 105 0 <td>Satd. Flow (prot)</td> <td></td> <td>2942</td> <td>1373</td> <td></td> <td>3157</td> <td></td> <td></td> <td>1601</td> <td></td> <td></td> <td>1455</td> <td></td>	Satd. Flow (prot)		2942	1373		3157			1601			1455	
Peak-hour factor, PHF	Flt Permitted		1.00			1.00							
Adj. Flow (vph)	Satd. Flow (perm)	1568	2942	1373		3157			931			1393	
RTOR Reduction (vph)	Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Lane Group Flow (vph) 87 940 22 0 1080 0 0 157 0 0 81 0 Confl. Peds. (#/hr) 1 1 1 2 2 5 2 2 2 2 2 2 2 2 2 2 2 2 2 2	Adj. Flow (vph)	87	940	37	0	1059	22	153	3		25	8	160
Confi. Peds. (#/hr)	RTOR Reduction (vph)	0	0	15	0	1	0	0	1	0	0		0
Confl. Bikes (#/hr) Heavy Vehicles (%) 6% 13% 6% 2% 5% 2% 4% 2% 2% 24% 2% 2% 2% Turn Type Prot NA Perm Prot NA Perm NA Perm NA Perm NA Protected Phases 5 2 1 6 8 8 4 Permitted Phases 2 8 8 4 Actuated Green, G (s) 7.4 42.5 42.5 30.6 20.3 20.3 20.3 Effective Green, g (s) 7.9 42.5 42.5 30.6 21.8 21.8 21.8 Actuated g/C Ratio 0.11 0.59 0.59 0.42 0.30 0.30 Clearance Time (s) 4.5 4.0 4.0 4.0 5.5 5 5.5 Vehicle Extension (s) 2.3 5.8 5.8 5.8 2.5 2.5 2.5 Lane Grp Cap (vph) 171 1729 807 1336 280 420 Vs Ratio Prot 0.06 c0.32 c0.34 c0.34 20.3 0.03 0.03 0.03 0.03 0.03 0.03 0.0	Lane Group Flow (vph)	87	940		0	1080	0		157	0	0	81	
Heavy Vehicles (%)	Confl. Peds. (#/hr)			1	1			2					2
Turn Type Prot NA Perm Prot NA Perm NA Perm NA Protected Phases 5 2 1 6 8 4 Permitted Phases 2 8 4 Actuated Green, G (s) 7.4 42.5 42.5 30.6 20.3 20.3 Effective Green, g (s) 7.9 42.5 42.5 30.6 21.8 21.8 Actuated g/C Ratio 0.11 0.59 0.59 0.42 0.30 0.30 Clearance Time (s) 4.5 4.0 4.0 4.0 5.5 5.5 Vehicle Extension (s) 2.3 5.8 5.8 5.8 2.5 2.5 Lane Grp Cap (vph) 171 1729 807 1336 280 420 v/s Ratio Prot 0.06 c0.32 c0.34 v/s Ratio Perm 0.02 c0.34 Uniform Delay, d1 30.4 9.0 6.2 18.3 21.2 18.7 <							1						
Protected Phases 5	Heavy Vehicles (%)	6%		6%	2%		2%	4%		2%	24%		2%
Permitted Phases	Turn Type	Prot	NA	Perm	Prot	NA		Perm	NA		Perm	NA	
Actuated Green, G (s) 7.4 42.5 42.5 30.6 20.3 20.3 Effective Green, g (s) 7.9 42.5 42.5 30.6 21.8 21.8 Actuated g/C Ratio 0.11 0.59 0.59 0.42 0.30 0.30 Clearance Time (s) 4.5 4.0 4.0 4.0 5.5 5.5 Vehicle Extension (s) 2.3 5.8 5.8 5.8 2.5 2.5 Lane Grp Cap (vph) 171 1729 807 1336 280 420 v/s Ratio Prot 0.06 c0.32 60.34 v/s Ratio Perm 0.02 60.17 0.06 v/s Ratio Perm 0.05 0.51 0.54 0.03 0.81 0.56 0.19 Uniform Delay, d1 30.4 9.0 6.2 18.3 21.2 18.7 Progression Factor 1.00 1.00 1.00 1.00 1.00 1.00 Incremental Delay, d2 1.4 0.8 0.0 4.4 2.1 0.2 Delay (s) 31.8 9.8 6.3 22.7 23.3 18.9 Level of Service C A A A C C C B Approach Delay (s) 11.5 22.7 23.3 18.9 Approach Delay (s) 11.5 22.7 23.3 18.9 Intersection Summary HCM 2000 Control Delay 17.6 HCM 2000 Level of Service B HCM 2000 Volume to Capacity ratio Actuated Cycle Length (s) 72.3 Sum of lost time (s) 12.0 Intersection Capacity Utilization 68.9% ICU Level of Service C Analysis Period (min) 15	Protected Phases	5	2		1	6			8			4	
Effective Green, g (s) 7.9 42.5 42.5 30.6 21.8 21.8 Actuated g/C Ratio 0.11 0.59 0.59 0.42 0.30 0.30 Clearance Time (s) 4.5 4.0 4.0 4.0 5.5 5.5 Vehicle Extension (s) 2.3 5.8 5.8 5.8 2.5 2.5 Lane Grp Cap (vph) 171 1729 807 1336 280 420 v/s Ratio Prot 0.06 c0.32 c0.34 20.0 420 v/s Ratio Perm 0.02 c0.34 0.06 0.09 v/s Ratio 0.51 0.54 0.03 0.81 0.56 0.19 Uniform Delay, d1 30.4 9.0 6.2 18.3 21.2 18.7 Progression Factor 1.00 1.00 1.00 1.00 1.00 1.00 Incremental Delay, d2 1.4 0.8 0.0 4.4 2.1 0.2 Delay (s) 31.8 9.8 6.3 22.7 23.3 18.9 Approach Delay (s) 11.5 <td>Permitted Phases</td> <td></td> <td></td> <td>2</td> <td></td> <td></td> <td></td> <td>8</td> <td></td> <td></td> <td>4</td> <td></td> <td></td>	Permitted Phases			2				8			4		
Actuated g/C Ratio 0.11 0.59 0.59 0.42 0.30 0.30 Clearance Time (s) 4.5 4.0 4.0 4.0 5.5 5.5 Vehicle Extension (s) 2.3 5.8 5.8 5.8 2.5 2.5 Lane Grp Cap (vph) 171 1729 807 1336 280 420 v/s Ratio Prot 0.06 c0.32 c0.34 c0.34 c0.17 0.06 v/s Ratio Perm 0.02 c0.34 c0.17 0.06 c0.17 0.06 v/s Ratio 0.51 0.54 0.03 0.81 0.56 0.19 Uniform Delay, d1 30.4 9.0 6.2 18.3 21.2 18.7 Progression Factor 1.00 1.00 1.00 1.00 1.00 1.00 Incremental Delay, d2 1.4 0.8 0.0 4.4 2.1 0.2 Delay (s) 31.8 9.8 6.3 22.7 23.3 18.9 Approach LOS B C C C B Intersec	Actuated Green, G (s)	7.4	42.5	42.5		30.6			20.3			20.3	
Clearance Time (s) 4.5 4.0 4.0 4.0 5.5 5.5 Vehicle Extension (s) 2.3 5.8 5.8 5.8 2.5 2.5 Lane Grp Cap (vph) 171 1729 807 1336 280 420 v/s Ratio Prot 0.06 c0.32 c0.34 v/s Ratio Perm 0.02 c0.34 Uniform Delay, d1 30.4 9.0 6.2 18.3 21.2 18.7 Progression Factor 1.00 1.00 1.00 1.00 1.00 1.00 1.00 Incremental Delay, d2 1.4 0.8 0.0 4.4 2.1 0.2 Delay (s) 31.8 9.8 6.3 22.7 23.3 18.9 Approach Delay (s) 11.	Effective Green, g (s)	7.9	42.5	42.5		30.6			21.8			21.8	
Vehicle Extension (s) 2.3 5.8 5.8 5.8 2.5 2.5 Lane Grp Cap (vph) 171 1729 807 1336 280 420 v/s Ratio Prot 0.06 c0.32 c0.34 c0.17 0.06 v/s Ratio Perm 0.02 c0.17 0.06 v/c Ratio 0.51 0.54 0.03 0.81 0.56 0.19 Uniform Delay, d1 30.4 9.0 6.2 18.3 21.2 18.7 Progression Factor 1.00 1.00 1.00 1.00 1.00 1.00 Incremental Delay, d2 1.4 0.8 0.0 4.4 2.1 0.2 Delay (s) 31.8 9.8 6.3 22.7 23.3 18.9 Level of Service C A A C C B Approach LOS B C C C B Intersection Summary T.6 HCM 2000 Level of Service B HCM 2000 Volum	Actuated g/C Ratio	0.11	0.59	0.59		0.42			0.30			0.30	
Lane Grp Cap (vph) 171 1729 807 1336 280 420 v/s Ratio Prot 0.06 c0.32	Clearance Time (s)	4.5	4.0	4.0		4.0			5.5			5.5	
v/s Ratio Prot 0.06 c0.32 c0.34 v/s Ratio Perm 0.02 c0.17 0.06 v/c Ratio 0.51 0.54 0.03 0.81 0.56 0.19 Uniform Delay, d1 30.4 9.0 6.2 18.3 21.2 18.7 Progression Factor 1.00 1.00 1.00 1.00 1.00 1.00 Incremental Delay, d2 1.4 0.8 0.0 4.4 2.1 0.2 Delay (s) 31.8 9.8 6.3 22.7 23.3 18.9 Level of Service C A A C C B Approach Delay (s) 11.5 22.7 23.3 18.9 Approach LOS B C C B Intersection Summary HCM 2000 Control Delay 17.6 HCM 2000 Level of Service B HCM 2000 Volume to Capacity ratio 0.71 Actuated Cycle Length (s) 72.3 Sum of lost time (s) 12.0 Intersection	Vehicle Extension (s)	2.3	5.8	5.8		5.8			2.5			2.5	
v/s Ratio Perm 0.02 c0.17 0.06 v/c Ratio 0.51 0.54 0.03 0.81 0.56 0.19 Uniform Delay, d1 30.4 9.0 6.2 18.3 21.2 18.7 Progression Factor 1.00 1.00 1.00 1.00 1.00 1.00 Incremental Delay, d2 1.4 0.8 0.0 4.4 2.1 0.2 Delay (s) 31.8 9.8 6.3 22.7 23.3 18.9 Level of Service C A A C C B Approach Delay (s) 11.5 22.7 23.3 18.9 Approach LOS B C C B Intersection Summary B C C B HCM 2000 Control Delay 17.6 HCM 2000 Level of Service B HCM 2000 Volume to Capacity ratio 0.71 3 3 3 3 4 4 4 4 4 4 4 4 </td <td>Lane Grp Cap (vph)</td> <td>171</td> <td>1729</td> <td>807</td> <td></td> <td>1336</td> <td></td> <td></td> <td>280</td> <td></td> <td></td> <td>420</td> <td></td>	Lane Grp Cap (vph)	171	1729	807		1336			280			420	
v/c Ratio 0.51 0.54 0.03 0.81 0.56 0.19 Uniform Delay, d1 30.4 9.0 6.2 18.3 21.2 18.7 Progression Factor 1.00 1.00 1.00 1.00 1.00 Incremental Delay, d2 1.4 0.8 0.0 4.4 2.1 0.2 Delay (s) 31.8 9.8 6.3 22.7 23.3 18.9 Level of Service C A A C C B Approach Delay (s) 11.5 22.7 23.3 18.9 Approach LOS B C C B Intersection Summary HCM 2000 Control Delay 17.6 HCM 2000 Level of Service B HCM 2000 Volume to Capacity ratio 0.71 Actuated Cycle Length (s) 72.3 Sum of lost time (s) 12.0 Intersection Capacity Utilization 68.9% ICU Level of Service C C	v/s Ratio Prot	0.06	c0.32			c0.34							
Uniform Delay, d1 30.4 9.0 6.2 18.3 21.2 18.7 Progression Factor 1.00 1.00 1.00 1.00 1.00 Incremental Delay, d2 1.4 0.8 0.0 4.4 2.1 0.2 Delay (s) 31.8 9.8 6.3 22.7 23.3 18.9 Level of Service C A A C C B Approach Delay (s) 11.5 22.7 23.3 18.9 Approach LOS B C C B Intersection Summary HCM 2000 Control Delay 17.6 HCM 2000 Level of Service B HCM 2000 Volume to Capacity ratio 0.71 Actuated Cycle Length (s) 72.3 Sum of lost time (s) 12.0 Intersection Capacity Utilization 68.9% ICU Level of Service C Analysis Period (min) 15	v/s Ratio Perm			0.02					c0.17			0.06	
Progression Factor 1.00 18.9 <td>v/c Ratio</td> <td>0.51</td> <td>0.54</td> <td>0.03</td> <td></td> <td>0.81</td> <td></td> <td></td> <td>0.56</td> <td></td> <td></td> <td>0.19</td> <td></td>	v/c Ratio	0.51	0.54	0.03		0.81			0.56			0.19	
Incremental Delay, d2	Uniform Delay, d1	30.4	9.0	6.2		18.3			21.2			18.7	
Delay (s) 31.8 9.8 6.3 22.7 23.3 18.9 Level of Service C A A C C B Approach Delay (s) 11.5 22.7 23.3 18.9 Approach LOS B C C B Intersection Summary HCM 2000 Control Delay HCM 2000 Control Delay 17.6 HCM 2000 Level of Service B HCM 2000 Volume to Capacity ratio 0.71 Actuated Cycle Length (s) 72.3 Sum of lost time (s) 12.0 Intersection Capacity Utilization 68.9% ICU Level of Service C Analysis Period (min) 15	Progression Factor	1.00	1.00	1.00		1.00			1.00			1.00	
Level of Service C A A C C B Approach Delay (s) 11.5 22.7 23.3 18.9 Approach LOS B C C B Intersection Summary HCM 2000 Control Delay 17.6 HCM 2000 Level of Service B HCM 2000 Volume to Capacity ratio 0.71 Actuated Cycle Length (s) 72.3 Sum of lost time (s) 12.0 Intersection Capacity Utilization 68.9% ICU Level of Service C Analysis Period (min) 15	Incremental Delay, d2	1.4	0.8	0.0		4.4			2.1			0.2	
Approach Delay (s) 11.5 22.7 23.3 18.9 Approach LOS B C C B Intersection Summary HCM 2000 Control Delay 17.6 HCM 2000 Level of Service B HCM 2000 Volume to Capacity ratio 0.71 Actuated Cycle Length (s) 72.3 Sum of lost time (s) 12.0 Intersection Capacity Utilization 68.9% ICU Level of Service C Analysis Period (min) 15	Delay (s)	31.8	9.8	6.3		22.7			23.3			18.9	
Approach LOS B C C B Intersection Summary HCM 2000 Control Delay 17.6 HCM 2000 Level of Service B HCM 2000 Volume to Capacity ratio 0.71 Actuated Cycle Length (s) 72.3 Sum of lost time (s) 12.0 Intersection Capacity Utilization 68.9% ICU Level of Service C Analysis Period (min) 15	Level of Service	С	Α	Α		С			С			В	
Intersection Summary HCM 2000 Control Delay 17.6 HCM 2000 Level of Service B HCM 2000 Volume to Capacity ratio 0.71 Actuated Cycle Length (s) 72.3 Sum of lost time (s) 12.0 Intersection Capacity Utilization 68.9% ICU Level of Service C Analysis Period (min) 15	Approach Delay (s)		11.5			22.7			23.3			18.9	
HCM 2000 Control Delay 17.6 HCM 2000 Level of Service B HCM 2000 Volume to Capacity ratio 0.71 Actuated Cycle Length (s) 72.3 Sum of lost time (s) 12.0 Intersection Capacity Utilization 68.9% ICU Level of Service C Analysis Period (min) 15	Approach LOS		В			С			С			В	
HCM 2000 Volume to Capacity ratio Actuated Cycle Length (s) 72.3 Sum of lost time (s) 12.0 Intersection Capacity Utilization 68.9% ICU Level of Service C Analysis Period (min) 15	Intersection Summary												
Actuated Cycle Length (s) 72.3 Sum of lost time (s) 12.0 Intersection Capacity Utilization 68.9% ICU Level of Service C Analysis Period (min) 15	HCM 2000 Control Delay			17.6	H	CM 2000	Level of S	Service		В			,
Actuated Cycle Length (s) 72.3 Sum of lost time (s) 12.0 Intersection Capacity Utilization 68.9% ICU Level of Service C Analysis Period (min) 15	HCM 2000 Volume to Capac	city ratio		0.71									
Intersection Capacity Utilization 68.9% ICU Level of Service C Analysis Period (min) 15	•				Sı	um of lost	time (s)			12.0			
Analysis Period (min) 15		tion					()						
													_

Sandy Health Clinic - PBS Project 71524.000 2022 With Project Conditions - Weekday AM Peak Hour

HCM 6th Signalized Intersection Summary

3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26)

03/10/2020

	•	→	•	•	←	4	4	†	1	-		1
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ř	^	7	ň	∱ ∱			4			4	
Traffic Volume (veh/h)	80	865	34	0	974	20	141	3	2	23	7	147
Future Volume (veh/h)	80	865	34	0	974	20	141	3	2	23	7	147
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		0.98	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1668	1573	1668	1723	1682	1682	1723	1723	1723	1723	1723	1723
Adj Flow Rate, veh/h	87	940	2	0	1059	21	153	3	1	25	8	40
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	6	13	6	2	5	5	2	2	2	2	2	2
Cap, veh/h	124	2090	988	3	1803	36	342	4	1	137	58	139
Arrive On Green	0.08	0.70	0.70	0.00	0.56	0.56	0.13	0.16	0.13	0.13	0.16	0.13
Sat Flow, veh/h	1589	2988	1413	1641	3203	64	1410	28	9	354	367	874
Grp Volume(v), veh/h	87	940	2	0	528	552	157	0	0	73	0	0
Grp Sat Flow(s),veh/h/ln	1589	1494	1413	1641	1598	1669	1447	0	0	1594	0	0
Q Serve(g_s), s	3.2	8.3	0.0	0.0	12.9	12.9	3.7	0.0	0.0	0.0	0.0	0.0
Cycle Q Clear(g_c), s	3.2	8.3	0.0	0.0	12.9	12.9	6.2	0.0	0.0	2.5	0.0	0.0
Prop In Lane	1.00		1.00	1.00		0.04	0.97		0.01	0.34		0.55
Lane Grp Cap(c), veh/h	124	2090	988	3	900	939	312	0	0	294	0	0
V/C Ratio(X)	0.70	0.45	0.00	0.00	0.59	0.59	0.50	0.00	0.00	0.25	0.00	0.00
Avail Cap(c_a), veh/h	677	2497	1181	425	1068	1116	585	0	0	587	0	0
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	0.00	1.00	1.00	1.00	0.00	0.00	1.00	0.00	0.00
Uniform Delay (d), s/veh	26.9	3.9	2.7	0.0	8.5	8.5	24.3	0.0	0.0	22.8	0.0	0.0
Incr Delay (d2), s/veh	4.4	0.5	0.0	0.0	2.0	1.9	0.9	0.0	0.0	0.3	0.0	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	1.3	1.7	0.0	0.0	4.1	4.3	2.2	0.0	0.0	0.9	0.0	0.0
Unsig. Movement Delay, s/veh	1											
LnGrp Delay(d),s/veh	31.3	4.4	2.7	0.0	10.5	10.4	25.3	0.0	0.0	23.2	0.0	0.0
LnGrp LOS	С	Α	Α	Α	В	В	С	Α	Α	С	Α	Α
Approach Vol, veh/h		1029			1080			157			73	
Approach Delay, s/veh		6.7			10.5			25.3			23.2	
Approach LOS		Α.			В			C			C	
•	,			•		•					-	
Timer - Assigned Phs	1	2		4	5	6		8				
Phs Duration (G+Y+Rc), s	0.0	46.3		13.5	8.7	37.7		13.5				
Change Period (Y+Rc), s	4.5	* 4.5		5.5	4.5	4.0		5.5				
Max Green Setting (Gmax), s	15.0	* 50		20.0	25.0	40.0		20.0				
Max Q Clear Time (g_c+I1), s	0.0	10.3		4.5	5.2	14.9		8.2				
Green Ext Time (p_c), s	0.0	23.7		0.2	0.2	18.8		0.4				
Intersection Summary												
HCM 6th Ctrl Delay			10.2									
HCM 6th LOS			В									
Notes												

^{*} HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Sandy Health Clinic - PBS Project 71524.000 2022 With Project Conditions - Weekday AM Peak Hour

Lanes, Volumes, Timings

3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26)

03/10/2020

Lane Group
Traffic Volume (vph) 165 1139 139 5 1173 18 121 25 7 42 24 124 Future Volume (vph) 165 1139 139 5 1173 18 121 25 7 42 24 124 Ideal Flow (vphpl) 1750
Traffic Volume (vph) 165 1139 139 5 1173 18 121 25 7 42 24 124 Future Volume (vph) 165 1139 139 5 1173 18 121 25 7 42 24 124 Ideal Flow (vphpl) 1750
Future Volume (vph)
Ideal Flow (vphpl)
Storage Length (ft) 115 100 105 0 0 0 0 0 Storage Lanes 1 1 1 1 0 0 0 0 0 Taper Length (ft) 100 100 100 100 100 100 Lane Util. Factor 1.00 0.95 1.00 1.00 0.95 1.00
Storage Lanes 1 1 1 1 0 0 0 0 0 Taper Length (ft) 100 100 100 100 100 100 100 100 100 100 100 1.00 0.999 1.00 1.00 0.999 1.00 1.00 0.912 1.00 1.00 0.912 1.00 1.00 0.912 1.00 1.00 0.912 1.00 0.912 0.912 1.00 0.942 0.942 0.948 0.912 1.00 1.00 0.948 0.912 0.948 0.948 0.94 0.94 0.94 0.94 0.94 0.94 0.94 0.94 0.94 0.
Taper Length (ff) 100 100 100 100 100 100 100 100 100 1.00 0.99 1.00 1.00 0.99 1.00 0.99 0.99 0.91 0.912 1.00
Ped Bike Factor 1.00 0.98 1.00 1.00 1.00 0.99 Frt 0.850 0.998 0.994 0.912 Flt Protected 0.950 0.950 0.962 0.989 Satd. Flow (prot) 1630 3228 1458 1630 3161 0 0 1625 0 0 1526 0 Flt Permitted 0.950 0.950 0.950 0.529 0.910 Satd. Flow (perm) 1629 3228 1426 1629 3161 0 0 890 0 0 1404 0 Right Turn on Red Yes Yes Yes Yes Yes Yes Satd. Flow (RTOR) 95 2 2 75 25 Link Speed (mph) 25 25 25 25 25 Link Distance (ft) 468 302 365 210 Travel Time (s) 12.8 8.2 10.0 5.7 Confl. Peds. (#/hr)
Frt 0.850 0.998 0.994 0.912 Fit Protected 0.950 0.950 0.962 0.989 Satd. Flow (prot) 1630 3228 1458 1630 3161 0 0 1625 0 0 1526 0 Fit Permitted 0.950 0.950 0.950 0.529 0.910 Satd. Flow (perm) 1629 3228 1426 1629 3161 0 0 890 0 0 1404 0 Right Turn on Red Yes Yes Yes Yes Yes Yes Satd. Flow (RTOR) 95 2 2 75 1 1 1 2 25 25 25 25 1 2 1 1 2 25 25 25 25 25 25 210 1 1 2 5 2 2 5 7 5 7 2 1 1 2 5 5
Fit Protected 0.950 0.950 0.962 0.989 Satd. Flow (prot) 1630 3228 1458 1630 3161 0 0 1625 0 0 1526 0 Flt Permitted 0.950 0.950 0.529 0.910 0.910 Satd. Flow (perm) 1629 3228 1426 1629 3161 0 0 890 0 0 1404 0 Right Turn on Red Yes Yes Yes Yes Yes Yes Yes Satd. Flow (RTOR) 95 2 2 2 75 1 1 1 2 25 25 25 1 2 25 25 25 25 25 25 25 25 25 25 25 210 1 1 2 5 2 5 20 1 1 2 5 5 2 5 5 1 5 7 5 2
Satd. Flow (prot) 1630 3228 1458 1630 3161 0 0 1625 0 0 1526 0 Flt Permitted 0.950 0.950 0.950 0.529 0.910 Satd. Flow (perm) 1629 3228 1426 1629 3161 0 0 890 0 0 1404 0 Right Turn on Red Yes Yes Yes Yes Yes Yes Satd. Flow (RTOR) 95 2 2 2 75 15 Link Speed (mph) 25 25 25 25 25 25 Link Distance (ft) 468 302 365 210 11 2 10.0 5.7 Confl. Peds. (#/hr) 2 1 1 2 5 5 5 Peak Hour Factor 0.94 0.94 0.94 0.94 0.94 0.94 0.94 0.94 0.94 0.94 0.94 0.94 0.94 <td< td=""></td<>
Satd. Flow (prot) 1630 3228 1458 1630 3161 0 0 1625 0 0 1526 0 Flt Permitted 0.950 0.950 0.950 0.529 0.910 Satd. Flow (perm) 1629 3228 1426 1629 3161 0 0 890 0 0 1404 0 Right Turn on Red Yes Yes Yes Yes Yes Yes Satd. Flow (RTOR) 95 2 2 2 75 15 Link Speed (mph) 25 25 25 25 25 25 Link Distance (ft) 468 302 365 210 11 2 10.0 5.7 Confl. Peds. (#/hr) 2 1 1 2 5 5 5 Peak Hour Factor 0.94 0.94 0.94 0.94 0.94 0.94 0.94 0.94 0.94 0.94 0.94 0.94 0.94 <td< td=""></td<>
Fit Permitted 0.950 0.950 0.529 0.910 Satd. Flow (perm) 1629 3228 1426 1629 3161 0 0 890 0 0 1404 0 Right Turn on Red Yes Yes Yes Yes Yes Yes Satd. Flow (RTOR) 95 2 2 75 10
Right Turn on Red Yes
Satd. Flow (RTOR) 95 2 2 75 Link Speed (mph) 25 25 25 25 Link Distance (ft) 468 302 365 210 Travel Time (s) 12.8 8.2 10.0 5.7 Confl. Peds. (#/hr) 2 1 1 2 5 5 Peak Hour Factor 0.94 0
Satd. Flow (RTOR) 95 2 2 75 Link Speed (mph) 25 25 25 25 Link Distance (ft) 468 302 365 210 Travel Time (s) 12.8 8.2 10.0 5.7 Confl. Peds. (#/hr) 2 1 1 2 5 5 Peak Hour Factor 0.94 0
Link Speed (mph) 25 25 25 25 25 Link Distance (ft) 468 302 365 210 Travel Time (s) 12.8 8.2 10.0 5.7 Confl. Peds. (#/hr) 2 1 1 2 5 5 Peak Hour Factor 0.94 0.9
Link Distance (ft) 468 302 365 210 Travel Time (s) 12.8 8.2 10.0 5.7 Confl. Peds. (#/hr) 2 1 1 2 5 5 Peak Hour Factor 0.94
Travel Time (s) 12.8 8.2 10.0 5.7 Confl. Peds. (#/hr) 2 1 1 2 5 Peak Hour Factor 0.94
Confl. Peds. (#/hr) 2 1 1 2 5 5 Peak Hour Factor 0.94
Peak Hour Factor 0.94 0.9
Heavy Vehicles (%) 2% 3% 2% 2% 5% 2% 2% 8% 2% 2% 2% 2% Adj. Flow (vph) 176 1212 148 5 1248 19 129 27 7 45 26 132 Shared Lane Traffic (%)
Adj. Flow (vph) 176 1212 148 5 1248 19 129 27 7 45 26 132 Shared Lane Traffic (%)
Shared Lane Traffic (%)
Lane Group Flow (vph) 176 1212 148 5 1267 0 0 163 0 0 203 0
Enter Blocked Intersection No
Lane Alignment L NA R NA R NA L NA R NA L NA R NA L NA R NA L NA R NA R
Median Width(ft) 12 12 0 0
Link Offset(ft) -12 0 0 4
Crosswalk Width(ft) 72 42 32 30
Two way Left Turn Lane
Headway Factor 1.11 1.11 1.11 1.11 1.11 1.11 1.11 1.
Number of Detectors 2 1 1 2 1 2 2 2 2
Detector Template Left Det25 Right Left Det25 Left Side St Left Side St
Leading Detector (ft) 78 153 153 78 153 78 78 78
Trailing Detector (ft) 2 137 137 2 137 2 2 2 2
Detector 1 Position(ft) 2 137 137 2 137 2 2 2 2
Detector 1 Size(ft) 16 16 16 16 16 16 16 16 16
Detector 1 Type CI+Ex CI+Ex CI+Ex CI+Ex CI+Ex CI+Ex CI+Ex CI+Ex
Detector 1 Channel
Detector 1 Extend (s) 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
Detector 1 Queue (s) 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
Detector 1 Delay (s) 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0
Detector 2 Position(ft) 72 72 72 72 72 72
Detector 2 Size(ft) 6 6 6 6
Detector 2 Type CI+Ex CI+Ex CI+Ex CI+Ex CI+Ex
Detector 2 Channel
Detector 2 Extend (s) 0.0 0.0 0.0 0.0 0.0 0.0
Turn Type Prot NA Perm Prot NA Perm NA Perm NA

Sandy Health Clinic - PBS Project 71524.000 2022 With Project Trips Conditions - Weekday PM Peak Hour

Lanes, Volumes, Timings 3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26) 03/10/2020 ٠ t **EBL NBT NBR** SBL Lane Group **EBT EBR** SBT Protected Phases 2 Permitted Phases 2 8 **Detector Phase** 5 2 2 8 8 4 4 Switch Phase 10.0 10.0 10.0 6.0 Minimum Initial (s) 4.0 4.0 6.0 6.0 6.0 Minimum Split (s) 8.5 26.0 26.0 8.5 30.0 11.5 11.5 23.5 23.5 Total Split (s) 20.0 70.0 70.0 15.0 65.0 25.0 25.0 25.0 25.0 18.2% 63.6% 59.1% 22.7% 22.7% Total Split (%) 63.6% 13.6% 22.7% 22.7% Maximum Green (s) 15.5 66.0 66.0 10.5 61.0 19.5 19.5 19.5 19.5 Yellow Time (s) 3.5 3.5 3.5 3.5 3.5 4.0 4.0 4.0 4.0 1.5 1.5 All-Red Time (s) 1.0 0.5 0.5 1.0 0.5 1.5 1.5 Lost Time Adjust (s) -0.5 0.0 0.0 -0.5 0.0 -1.5 -1.5 4.0 4.0 4.0 4.0 4.0 4.0 4.0 Total Lost Time (s) Lead/Lag Lag Lead Lag Lag Lead Lead-Lag Optimize? Vehicle Extension (s) 2.3 5.8 5.8 2.3 5.8 2.5 2.5 2.5 2.5 Minimum Gap (s) 0.5 3.8 3.8 0.5 3.8 2.0 2.0 2.0 2.0 Time Before Reduce (s) 5.0 5.0 5.0 8.0 10.0 10.0 8.0 10.0 5.0 Time To Reduce (s) 3.0 5.0 5.0 5.0 5.0 10.0 10.0 3.0 10.0 Recall Mode None C-Min C-Min None C-Min None None None None Walk Time (s) 7.0 7.0 7.0 7.0 7.0 Flash Dont Walk (s) 15.0 15.0 19.0 11.0 11.0 Pedestrian Calls (#/hr) 0 0 0 0 Act Effct Green (s) 15.7 72.6 72.6 5.8 55.0 27.3 27.3 Actuated g/C Ratio 0.14 0.66 0.66 0.05 0.50 0.25 0.25 v/c Ratio 0.76 0.73 0.57 0.15 0.06 0.80 0.50 61.0 Control Delay 66.2 11.4 2.9 50.6 27.1 28.8 0.0 0.0 0.0 0.0 Queue Delay 0.0 0.0 0.0 **Total Delay** 66.2 11.4 2.9 50.6 27.1 61.0 28.8 LOS Ε В Α D C Ε C Approach Delay 16.9 27.2 61.0 28.8 Approach LOS В С Ε С Intersection Summary Area Type: Other Cycle Length: 110 Actuated Cycle Length: 110 Offset: 0 (0%), Referenced to phase 2:EBT and 6:WBT, Start of Green, Master Intersection Natural Cycle: 80 Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.80 Intersection Signal Delay: 24.0 Intersection LOS: C Intersection Capacity Utilization 81.2% ICU Level of Service D Analysis Period (min) 15 Splits and Phases: 3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26) Ø1 04 ₩ Ø2 (R) T Ø8 Ø6 (R)

2022 With Project Trips Conditions - Weekday PM Peak Hour

Page 2

3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26)

03/10/2020

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Lane Group	EBL	EBT	EBR	WBL	WBT	NBT	SBT
Lane Group Flow (vph)	176	1212	148	5	1267	163	203
v/c Ratio	0.76	0.57	0.15	0.06	0.80	0.73	0.50
Control Delay	66.2	11.4	2.9	50.6	27.1	61.0	28.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	66.2	11.4	2.9	50.6	27.1	61.0	28.8
Queue Length 50th (ft)	118	195	11	3	378	107	78
Queue Length 95th (ft)	#224	297	37	16	417	#250	164
Internal Link Dist (ft)		388			222	285	130
Turn Bay Length (ft)	115		100	105			
Base Capacity (vph)	250	2130	973	163	1753	222	404
Starvation Cap Reductn	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	0.70	0.57	0.15	0.03	0.72	0.73	0.50
Intersection Summary							

^{# 95}th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis
3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26)

03/10/2020

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ች	^	7	7	∱ β			4			4	
Traffic Volume (vph)	165	1139	139	5	1173	18	121	25	7	42	24	124
Future Volume (vph)	165	1139	139	5	1173	18	121	25	7	42	24	124
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0			4.0			4.0	
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95			1.00			1.00	
Frpb, ped/bikes	1.00	1.00	0.98	1.00	1.00			1.00			0.99	
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00			1.00			1.00	
Frt	1.00	1.00	0.85	1.00	1.00			0.99			0.91	
Flt Protected	0.95	1.00	1.00	0.95	1.00			0.96			0.99	
Satd. Flow (prot)	1630	3228	1426	1630	3160			1619			1527	
Flt Permitted	0.95	1.00	1.00	0.95	1.00			0.53			0.91	
Satd. Flow (perm)	1630	3228	1426	1630	3160			890			1405	
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Adj. Flow (vph)	176	1212	148	5	1248	19	129	27	7	45	26	132
RTOR Reduction (vph)	0	0	35	0	1	0	0	2	0	0	56	0
Lane Group Flow (vph)	176	1212	113	5	1266	0	0	161	0	0	147	0
Confl. Peds. (#/hr)	2		1	1		2	5					5
Heavy Vehicles (%)	2%	3%	2%	2%	5%	2%	2%	8%	2%	2%	2%	2%
Turn Type	Prot	NA	Perm	Prot	NA		Perm	NA		Perm	NA	
Protected Phases	5	2		1	6			8			4	
Permitted Phases			2				8			4		
Actuated Green, G (s)	18.8	69.0	69.0	1.2	51.4			25.8			25.8	
Effective Green, g (s)	19.3	69.0	69.0	1.7	51.4			27.3			27.3	
Actuated g/C Ratio	0.18	0.63	0.63	0.02	0.47			0.25			0.25	
Clearance Time (s)	4.5	4.0	4.0	4.5	4.0			5.5			5.5	
Vehicle Extension (s)	2.3	5.8	5.8	2.3	5.8			2.5			2.5	
Lane Grp Cap (vph)	285	2024	894	25	1476			220			348	
v/s Ratio Prot	0.11	c0.38		0.00	c0.40							
v/s Ratio Perm			0.08					c0.18			0.10	
v/c Ratio	0.62	0.60	0.13	0.20	0.86			0.73			0.42	
Uniform Delay, d1	41.9	12.2	8.3	53.5	26.0			38.0			34.7	
Progression Factor	1.00	1.00	1.00	1.00	1.00			1.00			1.00	
Incremental Delay, d2	3.1	1.3	0.3	2.3	6.7			11.3			0.6	
Delay (s)	45.1	13.6	8.6	55.8	32.7			49.3			35.3	
Level of Service	D	В	Α	Е	С			D			D	
Approach Delay (s)		16.7			32.8			49.3			35.3	
Approach LOS		В			С			D			D	
Intersection Summary												
HCM 2000 Control Delay			26.0	Н	CM 2000	Level of S	Service		С			
HCM 2000 Volume to Capaci	ty ratio		0.79									
Actuated Cycle Length (s)			110.0	S	um of lost	time (s)			12.0			
Intersection Capacity Utilization	on		81.2%		CU Level o	٠,			D			
Analysis Period (min)			15									
c Critical Lane Group												

Sandy Health Clinic - PBS Project 71524.000 2022 With Project Trips Conditions - Weekday PM Peak Hour

HCM 6th Signalized Intersection Summary

3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26)

03/10/2020

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	75	^	7	7	∱ ∱			- 4			4	
Traffic Volume (veh/h)	165	1139	139	5	1173	18	121	25	7	42	24	124
Future Volume (veh/h)	165	1139	139	5	1173	18	121	25	7	42	24	124
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	0.99		0.99	1.00		0.99
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1723	1709	1723	1723	1682	1682	1641	1641	1641	1723	1723	1723
Adj Flow Rate, veh/h	176	1212	114	5	1248	18	129	27	5	45	26	72
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Percent Heavy Veh, %	2	3	2	2	5	5	8	8	8	2	2	2
Cap, veh/h	328	2278	1023	16	1664	24	212	37	6	109	71	139
Arrive On Green	0.20	0.70	0.70	0.01	0.52	0.52	0.16	0.18	0.16	0.16	0.18	0.16
Sat Flow, veh/h	1641	3247	1458	1641	3224	46	875	211	35	379	404	794
Grp Volume(v), veh/h	176	1212	114	5	618	648	161	0	0	143	0	0
Grp Sat Flow(s),veh/h/ln	1641	1624	1458	1641	1598	1673	1121	0	0	1577	0	0
Q Serve(g_s), s	10.6	19.6	2.8	0.3	33.6	33.6	6.7	0.0	0.0	0.0	0.0	0.0
Cycle Q Clear(g_c), s	10.6	19.6	2.8	0.3	33.6	33.6	15.8	0.0	0.0	9.1	0.0	0.0
Prop In Lane	1.00		1.00	1.00		0.03	0.80		0.03	0.31		0.50
Lane Grp Cap(c), veh/h	328	2278	1023	16	825	864	240	0	0	298	0	0
V/C Ratio(X)	0.54	0.53	0.11	0.31	0.75	0.75	0.67	0.00	0.00	0.48	0.00	0.00
Avail Cap(c_a), veh/h	328	2278	1023	164	886	928	260	0	0	321	0	0
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	0.00	1.00	0.00	0.00
Uniform Delay (d), s/veh	39.5	7.8	5.3	54.1	21.0	21.0	45.0	0.0	0.0	41.8	0.0	0.0
Incr Delay (d2), s/veh	1.2	0.9	0.2	6.7	6.2	5.9	5.3	0.0	0.0	0.9	0.0	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	4.4	6.5	0.9	0.2	13.6	14.3	4.8	0.0	0.0	3.7	0.0	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	40.7	8.7	5.5	60.8	27.2	27.0	50.3	0.0	0.0	42.7	0.0	0.0
LnGrp LOS	D	Α	Α	Е	С	С	D	Α	Α	D	Α	Α
Approach Vol, veh/h		1502			1271			161			143	
Approach Delay, s/veh		12.2			27.2			50.3			42.7	
Approach LOS		В			С			D			D	
Timer - Assigned Phs	1	2		4	5	6		8				
Phs Duration (G+Y+Rc), s	5.1	81.7		23.3	26.0	60.8		23.3				
Change Period (Y+Rc), s	4.5	* 4.5		5.5	4.5	4.0		5.5				
Max Green Setting (Gmax), s	10.5	* 66		19.5	15.5	61.0		19.5				
Max Q Clear Time (g_c+l1), s	2.3	21.6		11.1	12.6	35.6		17.8				
Green Ext Time (p_c), s	0.0	34.6		0.3	0.2	21.2		0.1				
Intersection Summary												
HCM 6th Ctrl Delay			21.8									
HCM 6th LOS			C C									
N (

^{*} HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Sandy Health Clinic - PBS Project 71524.000 2022 With Project Trips Conditions - Weekday PM Peak Hour

Lanes, Volumes, Timings

3: Wolf Drive/Ten E	yck Road 8	& Mt. Hood Hi	ghway ((US 26)	
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03/10/2020

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	^	7	*	∱ }			4			4	
Traffic Volume (vph)	64	982	39	0	1106	19	160	4	2	25	8	159
Future Volume (vph)	64	982	39	0	1106	19	160	4	2	25	8	159
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Storage Length (ft)	115		100	105		0	0		0	0		0
Storage Lanes	1		1	1		0	0		0	0		0
Taper Length (ft)	100			100			100			100		
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	0.95	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor			0.98		1.00			1.00			0.99	
Frt			0.850		0.997			0.998			0.888	
Flt Protected	0.950							0.954			0.994	
Satd. Flow (prot)	1568	2942	1403	1716	3158	0	0	1603	0	0	1455	0
Flt Permitted	0.950							0.515			0.947	
Satd. Flow (perm)	1568	2942	1372	1716	3158	0	0	864	0	0	1386	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			77		2			1			173	
Link Speed (mph)		25			25			25			25	
Link Distance (ft)		468			302			365			210	
Travel Time (s)		12.8			8.2			10.0			5.7	
Confl. Peds. (#/hr)			1	1	V. <u>_</u>		2					2
Confl. Bikes (#/hr)			•	•		1	=					_
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Heavy Vehicles (%)	6%	13%	6%	2%	5%	2%	4%	2%	2%	24%	2%	2%
Adj. Flow (vph)	70	1067	42	0	1202	21	174	4	2	27	9	173
Shared Lane Traffic (%)	10	1001		•	1202			•	_		•	110
Lane Group Flow (vph)	70	1067	42	0	1223	0	0	180	0	0	209	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	L NA	R NA	R NA	L NA	R NA	R NA	L NA	R NA	R NA	L NA	R NA	R NA
Median Width(ft)		12			12			0			0	
Link Offset(ft)		-12			0			0			4	
Crosswalk Width(ft)		72			42			32			30	
Two way Left Turn Lane								02				
Headway Factor	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11
Number of Detectors	2	1	1	2	1		2	2		2	2	
Detector Template	Left	Det25	Right	Left	Det25		Left	Side St		Left	Side St	
Leading Detector (ft)	78	153	153	78	153		78	78		78	78	
Trailing Detector (ft)	2	137	137	2	137		2	2		2	2	
Detector 1 Position(ft)	2	137	137	2	137		2	2		2	2	
Detector 1 Size(ft)	16	16	16	16	16		16	16		16	16	
Detector 1 Type	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex		CI+Ex	CI+Ex		CI+Ex	CI+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Detector 2 Position(ft)	72			72			72	72		72	72	
Detector 2 Size(ft)	6			6			6	6		6	6	
Detector 2 Type	CI+Ex			CI+Ex			CI+Ex	CI+Ex		CI+Ex	CI+Ex	
Detector 2 Channel	J.: LA			J. L.			J LA	J L.		J.: LA	J L.	
Detector 2 Extend (s)	0.0			0.0			0.0	0.0		0.0	0.0	
= ====== (0)	0.0						0.0	0.0		0.0	0.0	

Sandy Health Clinic - PBS Project 71524.000 2029 Without Project Conditions - Weekday AM Peak Hour

Lanes, Volumes, Timings 3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26) 03/10/2020 ၨ t **EBL** EBT **EBR** WBL WBT WBR NBL **NBT** NBR SBL Lane Group **SBT** Turn Type Prot NA Perm Prot NA Perm NA Perm NA **Protected Phases** 5 2 6 8 4 Permitted Phases 8 5 2 2 6 4 **Detector Phase** 1 8 8 4 Switch Phase Minimum Initial (s) 4.0 10.0 10.0 4.0 10.0 6.0 6.0 6.0 6.0 Minimum Split (s) 8.5 26.0 26.0 8.5 30.0 11.5 11.5 23.5 23.5 29.5 54.0 54.0 44.0 25.5 25.5 25.5 Total Split (s) 19.5 25.5 Total Split (%) 29.8% 54.5% 54.5% 19.7% 44.4% 25.8% 25.8% 25.8% 25.8% Maximum Green (s) 25.0 50.0 50.0 15.0 40.0 20.0 20.0 20.0 20.0 4.0 4.0 Yellow Time (s) 3.5 3.5 3.5 3.5 3.5 4.0 4.0 All-Red Time (s) 1.0 0.5 0.5 1.0 0.5 1.5 1.5 1.5 1.5 Lost Time Adjust (s) -0.5 0.0 0.0 -0.5 0.0 -1.5 -1.5 Total Lost Time (s) 4.0 4.0 4.0 4.0 4.0 4.0 4.0 Lead/Lag Lag Lag Lag Lead Lead Lead-Lag Optimize? Vehicle Extension (s) 2.3 5.8 5.8 2.3 5.8 2.5 2.5 2.5 2.5 2.0 2.0 0.5 0.5 2.0 2.0 Minimum Gap (s) 3.8 3.8 3.8 Time Before Reduce (s) 8.0 5.0 5.0 5.0 10.0 10.0 8.0 10.0 5.0 Time To Reduce (s) 3.0 10.0 10.0 3.0 10.0 5.0 5.0 5.0 5.0 Recall Mode Min Min None None None Min None None None Walk Time (s) 7.0 7.0 7.0 7.0 7.0 Flash Dont Walk (s) 15.0 15.0 19.0 11.0 11.0 Pedestrian Calls (#/hr) 0 0 0 0 0 46.5 46.5 36.2 8.8 22.1 22.1 Act Effct Green (s) 0.47 0.29 Actuated g/C Ratio 0.11 0.61 0.61 0.29 v/c Ratio 0.39 0.60 0.05 0.82 0.72 0.40 Control Delay 40.5 10.4 23.8 47.8 0.5 9.5 Queue Delay 0.0 0.0 0.0 0.0 0.0 0.0 **Total Delay** 40.5 10.4 0.5 23.8 47.8 9.5 LOS D В С D Α Approach Delay 11.9 23.8 47.8 9.5 Approach LOS В С D Α Intersection Summary Other Area Type: Cycle Length: 99 Actuated Cycle Length: 76.8) Natural Cycle: 65 Control Type: Actuated-Uncoordinated Maximum v/c Ratio: 0.82 Intersection Signal Delay: 19.2 Intersection LOS: B Intersection Capacity Utilization 73.9% ICU Level of Service D Analysis Period (min) 15 Splits and Phases: 3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26) 01 Ø4 ₩ Ø2 **≯** Ø5 T Ø8 Ø6 2029 Without Project Conditions - Weekday AM Peak Hour Page 2

3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26)

03/10/2020

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Lane Group	EBL	EBT	EBR	WBT	NBT	SBT
Lane Group Flow (vph)	70	1067	42	1223	180	209
v/c Ratio	0.39	0.60	0.05	0.82	0.72	0.40
Control Delay	40.5	10.4	0.5	23.8	47.8	9.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	40.5	10.4	0.5	23.8	47.8	9.5
Queue Length 50th (ft)	34	143	0	264	86	14
Queue Length 95th (ft)	74	193	4	385	#210	73
Internal Link Dist (ft)		388		222	285	130
Turn Bay Length (ft)	115		100			
Base Capacity (vph)	535	2063	985	1692	249	522
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.13	0.52	0.04	0.72	0.72	0.40
Intersection Summary						

^{# 95}th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis 3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26)

03/10/2020

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	^	7	7	∱ Љ			4			4	
Traffic Volume (vph)	64	982	39	0	1106	19	160	4	2	25	8	159
Future Volume (vph)	64	982	39	0	1106	19	160	4	2	25	8	159
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Total Lost time (s)	4.0	4.0	4.0		4.0			4.0			4.0	
Lane Util. Factor	1.00	0.95	1.00		0.95			1.00			1.00	
Frpb, ped/bikes	1.00	1.00	0.98		1.00			1.00			0.99	
Flpb, ped/bikes	1.00	1.00	1.00		1.00			1.00			1.00	
Frt	1.00	1.00	0.85		1.00			1.00			0.89	
Flt Protected	0.95	1.00	1.00		1.00			0.95			0.99	
Satd. Flow (prot)	1568	2942	1373		3159			1601			1455	
Flt Permitted	0.95	1.00	1.00		1.00			0.52			0.95	
Satd. Flow (perm)	1568	2942	1373		3159			865			1387	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	70	1067	42	0	1202	21	174	4	2	27	9	173
RTOR Reduction (vph)	0	0	16	0	1	0	0	1	0	0	124	0
Lane Group Flow (vph)	70	1067	26	0	1222	0	0	179	0	0	85	0
Confl. Peds. (#/hr)			1	1			2					2
Confl. Bikes (#/hr)						1						
Heavy Vehicles (%)	6%	13%	6%	2%	5%	2%	4%	2%	2%	24%	2%	2%
Turn Type	Prot	NA	Perm	Prot	NA		Perm	NA		Perm	NA	
Protected Phases	5	2		1	6			8			4	
Permitted Phases			2				8			4		
Actuated Green, G (s)	6.9	47.6	47.6		36.2			20.6			20.6	
Effective Green, g (s)	7.4	47.6	47.6		36.2			22.1			22.1	
Actuated g/C Ratio	0.10	0.61	0.61		0.47			0.28			0.28	
Clearance Time (s)	4.5	4.0	4.0		4.0			5.5			5.5	
Vehicle Extension (s)	2.3	5.8	5.8		5.8			2.5			2.5	
Lane Grp Cap (vph)	149	1802	841		1471			246			394	
v/s Ratio Prot	0.04	c0.36			c0.39							
v/s Ratio Perm			0.02					c0.21			0.06	
v/c Ratio	0.47	0.59	0.03		0.83			0.73			0.22	
Uniform Delay, d1	33.3	9.1	5.9		18.1			25.1			21.2	
Progression Factor	1.00	1.00	1.00		1.00			1.00			1.00	
Incremental Delay, d2	1.4	0.9	0.0		4.8			9.7			0.2	
Delay (s)	34.7	10.1	6.0		22.9			34.8			21.4	
Level of Service	С	В	Α		С			С			С	
Approach Delay (s)		11.4			22.9			34.8			21.4	
Approach LOS		В			С			С			С	
Intersection Summary												
HCM 2000 Control Delay			18.7	H	CM 2000	Level of S	Service		В			,
HCM 2000 Volume to Capacit	y ratio		0.79									
Actuated Cycle Length (s)			77.7	Sı	um of lost	time (s)			12.0			
Intersection Capacity Utilization	n		73.9%		U Level c	٠,			D			
Analysis Period (min)			15									
c Critical Lane Group												

Sandy Health Clinic - PBS Project 71524.000 2029 Without Project Conditions - Weekday AM Peak Hour

HCM 6th Signalized Intersection Summary

3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26)

03/10/2020

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	75	^	7	ሻ	↑ ↑			4			4	
Traffic Volume (veh/h)	64	982	39	0	1106	19	160	4	2	25	8	159
Future Volume (veh/h)	64	982	39	0	1106	19	160	4	2	25	8	159
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		0.98	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1668	1573	1668	1723	1682	1682	1723	1723	1723	1723	1723	1723
Adj Flow Rate, veh/h	70	1067	9	0	1202	20	174	4	1	27	9	39
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	6	13	6	2	5	5	2	2	2	2	2	2
Cap, veh/h	100	2065	976	3	1842	31	356	6	1	146	66	146
Arrive On Green	0.06	0.69	0.69	0.00	0.57	0.57	0.15	0.17	0.15	0.15	0.17	0.15
Sat Flow, veh/h	1589	2988	1413	1641	3215	53	1398	32	8	392	378	834
Grp Volume(v), veh/h	70	1067	9	0	597	625	179	0	0	75	0	0
Grp Sat Flow(s), veh/h/ln	1589	1494	1413	1641	1598	1671	1438	0	0	1603	0	0
Q Serve(g_s), s	2.7	10.9	0.1	0.0	16.2	16.2	4.8	0.0	0.0	0.0	0.0	0.0
Cycle Q Clear(g_c), s	2.7	10.9	0.1	0.0	16.2	16.2	7.5	0.0	0.0	2.7	0.0	0.0
Prop In Lane	1.00	10.5	1.00	1.00	10.2	0.03	0.97	0.0	0.01	0.36	0.0	0.52
Lane Grp Cap(c), veh/h	100	2065	976	3	916	958	329	0	0.01	319	0	0.52
V/C Ratio(X)	0.70	0.52	0.01	0.00	0.65	0.65	0.54	0.00	0.00	0.23	0.00	0.00
Avail Cap(c a), veh/h	639	2357	1114	401	1008	1054	553	0.00	0.00	560	0.00	0.00
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	0.00	1.00	1.00	1.00	0.00	0.00	1.00	0.00	0.00
Uniform Delay (d), s/veh	29.1	4.7	3.0	0.00	9.2	9.2	25.2	0.00	0.00	23.3	0.00	0.00
Incr Delay (d2), s/veh	5.4	0.7	0.0	0.0	2.7	2.6	1.0	0.0	0.0	0.3	0.0	0.0
Initial Q Delay(d3),s/veh	0.0		0.0	0.0	0.0	0.0	0.0			0.0	0.0	0.0
		0.0						0.0	0.0			
%ile BackOfQ(50%),veh/ln	1.2	2.5	0.0	0.0	5.3	5.5	2.7	0.0	0.0	1.0	0.0	0.0
Unsig. Movement Delay, s/veh		F 4	2.4	0.0	40.0	44.0	00.0	0.0	0.0	00.5	0.0	0.0
LnGrp Delay(d),s/veh	34.5	5.4	3.1	0.0	12.0	11.9	26.3	0.0	0.0	23.5	0.0	0.0
LnGrp LOS	С	A	A	A	В	В	C	A	A	C	A	A
Approach Vol, veh/h		1146			1222			179			75	
Approach Delay, s/veh		7.1			11.9			26.3			23.5	
Approach LOS		Α			В			С			С	
Timer - Assigned Phs	1	2		4	5	6		8				
Phs Duration (G+Y+Rc), s	0.0	48.3		15.1	8.0	40.3		15.1				
Change Period (Y+Rc), s	4.5	* 4.5		5.5	4.5	4.0		5.5				
Max Green Setting (Gmax), s	15.0	* 50		20.0	25.0	40.0		20.0				
Max Q Clear Time (g_c+l1), s	0.0	12.9		4.7	4.7	18.2		9.5				
Green Ext Time (p_c), s	0.0	25.7		0.2	0.2	18.2		0.5				
Intersection Summary												
HCM 6th Ctrl Delay			11.1									
HCM 6th LOS			В									
Notos			<u> </u>									

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Sandy Health Clinic - PBS Project 71524.000 2029 Without Project Conditions - Weekday AM Peak Hour

Lanes, Volumes, Timings

3: Wolf Drive/Ten E	yck Road 8	& Mt. Hood Hi	ghway ((US 26)	
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03/10/2020

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	† †	7	, J	∱ }			4			4	
Traffic Volume (vph)	178	1292	158	6	1331	19	137	28	8	45	27	118
Future Volume (vph)	178	1292	158	6	1331	19	137	28	8	45	27	118
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Storage Length (ft)	115		100	105		0	0		0	0		0
Storage Lanes	1		1	1		0	0		0	0		0
Taper Length (ft)	100			100			100			100		-
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	0.95	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor	1.00		0.98	1.00	1.00			1.00			0.99	
Frt			0.850		0.998			0.993			0.916	
Flt Protected	0.950		0.000	0.950	0.000			0.962			0.988	
Satd. Flow (prot)	1630	3228	1458	1630	3161	0	0	1623	0	0	1532	0
Flt Permitted	0.950	0220	1100	0.950	0101	v		0.512	•	•	0.905	J
Satd. Flow (perm)	1629	3228	1426	1629	3161	0	0	861	0	0	1404	0
Right Turn on Red	1023	0220	Yes	1023	0101	Yes	U	001	Yes	U	1707	Yes
Satd. Flow (RTOR)			95		2	103		2	103		66	103
Link Speed (mph)		25	00		25			25			25	
Link Distance (ft)		468			302			365			210	
Travel Time (s)		12.8			8.2			10.0			5.7	
Confl. Peds. (#/hr)	2	12.0	1	1	0.2	2	5	10.0			5.7	5
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Heavy Vehicles (%)	2%	3%	2%	2%	5%	2%	2%	8%	2%	2%	2%	2%
Adj. Flow (vph)	189	1374	168	6	1416	20	146	30	9	48	29	126
Shared Lane Traffic (%)	109	1374	100	U	1410	20	140	30	9	40	23	120
Lane Group Flow (vph)	189	1374	168	6	1436	0	0	185	0	0	203	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
	L NA	R NA	R NA	L NA	R NA	R NA	L NA	R NA	R NA	L NA	R NA	R NA
Lane Alignment	LINA	12	KINA	LINA	12	KINA	LINA	0	KINA	LINA	R NA	KINA
Median Width(ft)		-12			0			0			4	
Link Offset(ft)		72			42			32			30	
Crosswalk Width(ft)		12			42			32			30	
Two way Left Turn Lane	1.11	1 11	1.11	1.11	1.11	1.11	1.11	1.11	1 11	1.11	1.11	1.11
Headway Factor	1.11	1.11	1.11	2	1.11	1.11	2	2	1.11	2	2	1.11
Number of Detectors					Det25							
Detector Template	Left	Det25	Right	Left			Left 78	Side St		Left	Side St	
Leading Detector (ft)	78 2	153	153	78	153 137		2	78		78 2	78	
Trailing Detector (ft)	2	137 137	137 137	2	137		2	2		2	2	
Detector 1 Position(ft)								_				
Detector 1 Size(ft)	16	16	16 CI+Ex	16	16		16	16		16	16	
Detector 1 Type	CI+Ex	CI+Ex	CI+EX	CI+Ex	CI+Ex		CI+Ex	CI+Ex		CI+Ex	Cl+Ex	
Detector 1 Channel	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Detector 2 Position(ft)	72			72			72	72		72	72	
Detector 2 Size(ft)	6			6			6	6		6	6	
Detector 2 Type	CI+Ex			CI+Ex			CI+Ex	CI+Ex		CI+Ex	CI+Ex	
Detector 2 Channel												
Detector 2 Extend (s)	0.0		_	0.0			0.0	0.0		0.0	0.0	
Turn Type	Prot	NA	Perm	Prot	NA		Perm	NA		Perm	NA	

Sandy Health Clinic - PBS Project 71524.000 2029 Without Project Conditions - Weekday PM Peak Hour

Lanes, Volumes, Timings 3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26) 03/10/2020 ٠ t **NBT NBR** SBL Lane Group **EBL EBT EBR** SBT Protected Phases 2 Permitted Phases 2 8 **Detector Phase** 5 2 2 8 8 4 4 Switch Phase 10.0 10.0 10.0 6.0 Minimum Initial (s) 4.0 4.0 6.0 6.0 6.0 Minimum Split (s) 8.5 26.0 26.0 8.5 30.0 11.5 11.5 23.5 23.5 Total Split (s) 20.0 70.0 70.0 15.0 65.0 25.0 25.0 25.0 25.0 18.2% 63.6% 59.1% 22.7% 22.7% Total Split (%) 63.6% 13.6% 22.7% 22.7% Maximum Green (s) 15.5 66.0 66.0 10.5 61.0 19.5 19.5 19.5 19.5 Yellow Time (s) 3.5 3.5 3.5 3.5 3.5 4.0 4.0 4.0 4.0 All-Red Time (s) 1.0 0.5 0.5 1.0 0.5 1.5 1.5 1.5 1.5 Lost Time Adjust (s) -0.5 0.0 0.0 -0.5 0.0 -1.5 -1.5 4.0 4.0 4.0 4.0 4.0 4.0 4.0 Total Lost Time (s) Lead/Lag Lag Lead Lag Lag Lead Lead-Lag Optimize? Vehicle Extension (s) 2.3 5.8 5.8 2.3 5.8 2.5 2.5 2.5 2.5 Minimum Gap (s) 0.5 3.8 3.8 0.5 3.8 2.0 2.0 2.0 2.0 Time Before Reduce (s) 5.0 5.0 5.0 8.0 10.0 10.0 8.0 10.0 5.0 Time To Reduce (s) 3.0 5.0 5.0 5.0 5.0 10.0 10.0 3.0 10.0 Recall Mode None C-Min C-Min None C-Min None None None None Walk Time (s) 7.0 7.0 7.0 7.0 7.0 Flash Dont Walk (s) 15.0 15.0 19.0 11.0 11.0 Pedestrian Calls (#/hr) 0 0 0 0 Act Effct Green (s) 15.5 75.3 75.3 5.8 58.0 24.5 24.5 Actuated g/C Ratio 0.14 0.68 0.68 0.05 0.53 0.22 0.22 v/c Ratio 0.83 0.96 0.62 0.17 0.07 0.86 0.56 11.2 100.2 Control Delay 73.8 50.8 28.6 33.4 3.1 0.0 0.0 0.0 0.0 0.0 Queue Delay 0.0 0.0 **Total Delay** 73.8 11.2 3.1 50.8 28.6 100.2 33.4 LOS Ε В Α D C F C Approach Delay 17.3 28.7 100.2 33.4 Approach LOS В С F С Intersection Summary Area Type: Other Cycle Length: 110 Actuated Cycle Length: 110 Offset: 0 (0%), Referenced to phase 2:EBT and 6:WBT, Start of Green, Master Intersection Natural Cycle: 90 Control Type: Actuated-Coordinated Maximum v/c Ratio: 0.96 Intersection Signal Delay: 27.1 Intersection LOS: C Intersection Capacity Utilization 87.9% ICU Level of Service E Analysis Period (min) 15 Splits and Phases: 3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26) ÿ1 Ø4 ₩ Ø2 (R) <u>≯_{Ø5}</u> ¶ Ø8 Ø6 (R) 2029 Without Project Conditions - Weekday PM Peak Hour Page 2

3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26)

03/10/2020

	•	→	•	•	←	†	↓	
Lane Group	EBL	EBT	EBR	WBL	WBT	NBT	SBT	
Lane Group Flow (vph)	189	1374	168	6	1436	185	203	
v/c Ratio	0.83	0.62	0.17	0.07	0.86	0.96	0.56	
Control Delay	73.8	11.2	3.1	50.8	28.6	100.2	33.4	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	73.8	11.2	3.1	50.8	28.6	100.2	33.4	
Queue Length 50th (ft)	129	201	12	4	426	~150	89	
Queue Length 95th (ft)	#247	369	44	18	517	#295	171	
Internal Link Dist (ft)		388			222	285	130	
Turn Bay Length (ft)	115		100	105				
Base Capacity (vph)	241	2210	1006	163	1753	193	364	
Starvation Cap Reductn	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.78	0.62	0.17	0.04	0.82	0.96	0.56	

Intersection Summary

Volume exceeds capacity, queue is theoretically infinite.
 Queue shown is maximum after two cycles.

^{# 95}th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis 3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26)

03/10/2020

	•	-	•	•	←	•	•	†	/	>	ļ	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	^	7	7	ħβ			4			4	
Traffic Volume (vph)	178	1292	158	6	1331	19	137	28	8	45	27	118
Future Volume (vph)	178	1292	158	6	1331	19	137	28	8	45	27	118
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0			4.0			4.0	
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95			1.00			1.00	
Frpb, ped/bikes	1.00	1.00	0.98	1.00	1.00			1.00			0.99	
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00			1.00			1.00	
Frt	1.00	1.00	0.85	1.00	1.00			0.99			0.92	
Flt Protected	0.95	1.00	1.00	0.95	1.00			0.96			0.99	
Satd. Flow (prot)	1630	3228	1426	1630	3160			1618			1533	
Flt Permitted	0.95	1.00	1.00	0.95	1.00			0.51			0.90	
Satd. Flow (perm)	1630	3228	1426	1630	3160			861			1403	
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Adj. Flow (vph)	189	1374	168	6	1416	20	146	30	9	48	29	126
RTOR Reduction (vph)	0	0	33	0	1	0	0	2	0	0	51	0
Lane Group Flow (vph)	189	1374	135	6	1435	0	0	183	0	0	152	0
Confl. Peds. (#/hr)	2		1	1		2	5					5
Heavy Vehicles (%)	2%	3%	2%	2%	5%	2%	2%	8%	2%	2%	2%	2%
Turn Type	Prot	NA	Perm	Prot	NA		Perm	NA		Perm	NA	
Protected Phases	5	2		1	6			8			4	
Permitted Phases			2				8			4		
Actuated Green, G (s)	18.6	71.7	71.7	1.3	54.4			23.0			23.0	
Effective Green, g (s)	19.1	71.7	71.7	1.8	54.4			24.5			24.5	
Actuated g/C Ratio	0.17	0.65	0.65	0.02	0.49			0.22			0.22	
Clearance Time (s)	4.5	4.0	4.0	4.5	4.0			5.5			5.5	
Vehicle Extension (s)	2.3	5.8	5.8	2.3	5.8			2.5			2.5	
Lane Grp Cap (vph)	283	2104	929	26	1562			191			312	
v/s Ratio Prot	0.12	c0.43	020	0.00	c0.45						0.12	
v/s Ratio Perm	0.12	00.10	0.09	0.00	0 0.10			c0.21			0.11	
v/c Ratio	0.67	0.65	0.15	0.23	0.92			0.96			0.49	
Uniform Delay, d1	42.5	11.6	7.4	53.4	25.8			42.3			37.3	
Progression Factor	1.00	1.00	1.00	1.00	1.00			1.00			1.00	
Incremental Delay, d2	5.0	1.6	0.3	2.6	10.2			53.5			0.9	
Delay (s)	47.5	13.2	7.7	56.1	35.9			95.8			38.1	
Level of Service	T/ .0	В	Α.	E	D			F			D	
Approach Delay (s)		16.4	7.	_	36.0			95.8			38.1	
Approach LOS		В			D			F			D	
Intersection Summary												
HCM 2000 Control Delay			29.7	Н	CM 2000	Level of	Service		С			
HCM 2000 Volume to Capa	city ratio		0.89									
Actuated Cycle Length (s)			110.0	S	um of lost	time (s)			12.0			
Intersection Capacity Utiliza	tion		87.9%	IC	CU Level o	of Service			Е			
Analysis Period (min)			15									
c Critical Lane Group												

Sandy Health Clinic - PBS Project 71524.000 2029 Without Project Conditions - Weekday PM Peak Hour

HCM 6th Signalized Intersection Summary

3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26)

03/10/2020

Movement		۶	→	•	•	←	•	•	†	~	/	ļ	4
Traffic Volume (velvhi)	Movement						WBR	NBL		NBR	SBL		SBR
Future Volume (veh/h)			^	7	7	∱ î≽			4			4	
Initial Q (Qb), veh		178			6		19	137			45		118
Ped-Bike Adj(A_pbT)													
Parking Bus. Adj			0			0			0			0	
Work Zone On Ápproach													
Adj Star Flow, vehrhin 1723 1709 1723 1723 1682 1682 1681 1641 1641 1641 1723 1723 1723 1723 1723 1723 1723 172		1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Adj Flow Rate, veh/h Peak Hour Factor O.94 O.94 O.94 O.94 O.94 O.94 O.94 O.94													
Peak Hour Factor 0.94 0.94 0.94 0.94 0.94 0.94 0.94 0.94													
Percent Heavy Veh, % 2 3 2 2 5 5 8 8 8 2 2 2 2 Cap, veh/h 264 2224 998 17 1740 23 227 37 8 120 79 146 Arrive On Green 0.16 0.68 0.68 0.68 0.01 0.54 0.54 0.18 0.19 0.18 0.18 0.19 0.18 Sat Flow, veh/h 1641 3247 1458 1641 3228 43 882 194 43 401 414 763 Grp Volume(v), veh/h 189 1374 136 6 700 735 183 0 0 149 0 0 0 Grp Sat Flow(s), veh/h/ln 1641 1624 1458 1641 1598 1674 1118 0 0 1579 0 0 0 Q Serve(g.s), s 12.0 25.4 3.6 0.4 39.6 39.7 8.6 0.0 0.0 0.0 0.0 0.0 0.0 Cycle Q Clear(g.c), s 12.0 25.4 3.6 0.4 39.6 39.7 8.6 0.0 0.0 0.0 0.0 0.0 0.0 Cycle Q Clear(g.c), s 12.0 25.4 3.6 0.4 39.6 39.7 18.0 0.0 0.0 9.4 0.0 0.0 V/C Ratio(X) 0 72 0.62 0.14 0.34 0.81 0.91 0.92 257 0 0 323 0 0 V/C Ratio(X) 0 72 0.62 0.14 0.34 0.81 0.91 0.91 0.90 0.00 0.00 0.00 0.00 0.0													
Cap, veh/h OR Green O													
Arrive On Green													
Sat Flow, veh/h 1641 3247 1458 1641 3228 43 882 194 43 401 414 763 Grp Volume(v), veh/h 189 1374 136 6 700 735 183 0 0 149 0 0 Grp Sat Flow(s), veh/h/n 1641 1624 1458 1641 1598 1674 1118 0 0 1579 0 0 Q Serve(g.s), s 12.0 25.4 3.6 0.4 39.6 39.7 18.0 0.0													
Grp Volume(v), veh/h Grp Sat Flow(s), veh/h/h/ln Grp Sat Flow(s), veh/h/h/ln Grp Sat Flow(s), veh/h/ln Grp Sat Flow(s), veh/h Grp Sat Flow(s), veh/h Grp Sat Flow(s), veh/h/ln Grp Sat Flow(s), veh/h Grp Los Flow(s), veh/h Grp Los Flow(s), veh/h Grp Sat Flow(s), veh/h Grp Los Flow(Arrive On Green		0.68							0.18			0.18
Grp Sat Flow(s), veh/h/ln 1641 1624 1458 1641 1598 1674 1118 0 0 1579 0 0 0 Serve(g. s), s 12.0 25.4 3.6 0.4 39.6 39.7 8.6 0.0 0.0 0.0 0.0 0.0 0.0 0.0 Cycle Q Clear(g. c), s 12.0 25.4 3.6 0.4 39.6 39.7 18.0 0.0 0.0 0.9 4 0.0 0.0 Prop In Lane 1.00 1.00 1.00 0.03 0.80 0.04 0.32 0.48 Lane Grp Cap(c), veh/h 264 2224 998 17 861 902 257 0 0 323 0 0 0 V/C Ratio(X) 0.72 0.62 0.14 0.34 0.81 0.81 0.71 0.00 0.00 0.46 0.00 0.00 Avail Cap(c_a), veh/h 264 2224 998 164 886 928 257 0 0 323 0 0 0 HCM Platoon Ratio 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.0			3247		1641	3228		882	194	43	401	414	763
Q Serve(g_s), s 12.0 25.4 3.6 0.4 39.6 39.7 8.6 0.0 0.0 0.0 0.0 0.0 0.0 Cycle Q Clear(g_c), s 12.0 25.4 3.6 0.4 39.6 39.7 18.0 0.0 0.0 9.4 0.0 0.0 Cycle Q Clear(g_c), s 12.0 25.4 3.6 0.4 39.6 39.7 18.0 0.0 0.0 9.4 0.0 0.0 0.0 Prop In Lane 1.00 1.00 1.00 1.00 0.03 0.80 0.04 0.32 0.48 Lane Grp Cap(c), veh/h 264 2224 998 17 861 902 257 0 0 323 0 0 V/C Ratio(X) 0.72 0.62 0.14 0.34 0.81 0.81 0.71 0.00 0.00 0.46 0.00 0.00 Avail Cap(c_a), veh/h 264 2224 998 164 886 928 257 0 0 323 0 0 0 HCM Platoon Ratio 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.0		189	1374	136	6	700	735	183	0	0		0	
Cycle Q Clear(g_c), s					1641		1674	1118		0		0	
Prop In Lane 1.00 1.00 1.00 1.00 0.03 0.80 0.04 0.32 0.48 Lane Grp Cap(c), veh/h 264 2224 998 17 861 902 257 0 0 323 0 0 V/C Ratio(X) 0.72 0.62 0.14 0.34 0.81 0.81 0.71 0.00 0.00 0.46 0.00 0.00 Avail Cap(c_a), veh/h 264 2224 998 164 886 928 257 0 0 323 0 0 HCM Platoon Ratio 1.00 <td>Q Serve(g_s), s</td> <td></td> <td>25.4</td> <td></td> <td>0.4</td> <td></td> <td>39.7</td> <td></td> <td>0.0</td> <td>0.0</td> <td>0.0</td> <td>0.0</td> <td></td>	Q Serve(g_s), s		25.4		0.4		39.7		0.0	0.0	0.0	0.0	
Lane Grp Cap(c), veh/h 264 2224 998 17 861 902 257 0 0 323 0 0 0 V/C Ratio(X) 0.72 0.62 0.14 0.34 0.81 0.81 0.71 0.00 0.00 0.00 0.46 0.00 0.00 0.00 0.40 0.00 0.0	Cycle Q Clear(g_c), s		25.4		0.4	39.6		18.0	0.0	0.0		0.0	
V/C Ratio(X) 0.72 0.62 0.14 0.34 0.81 0.71 0.00 0.00 0.46 0.00 0.00 Avail Cap(c_a), veh/h 264 2224 998 164 886 928 257 0 0 323 0 0 HCM Platoon Ratio 1.00 <td>Prop In Lane</td> <td></td> <td></td> <td>1.00</td> <td>1.00</td> <td></td> <td>0.03</td> <td></td> <td></td> <td>0.04</td> <td></td> <td></td> <td>0.48</td>	Prop In Lane			1.00	1.00		0.03			0.04			0.48
Avail Cap(c_a), veh/h	Lane Grp Cap(c), veh/h	264	2224	998	17	861	902	257	0	0	323	0	0
HCM Platoon Ratio	V/C Ratio(X)	0.72	0.62	0.14	0.34	0.81	0.81	0.71	0.00	0.00	0.46	0.00	0.00
Upstream Filter(I) 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.0		264	2224	998	164	886	928		0	0		0	0
Uniform Delay (d), s/veh	HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Incr Delay (d2), s/veh 8.1 1.3 0.3 7.0 8.3 8.0 8.4 0.0 0.0 0.8 0.0 0.0 Initial Q Delay(d3),s/veh 0.0	Upstream Filter(I)		1.00	1.00	1.00	1.00	1.00	1.00	0.00	0.00	1.00	0.00	0.00
Initial Q Delay(d3),s/veh 0.0 <td>Uniform Delay (d), s/veh</td> <td>43.7</td> <td>9.5</td> <td>6.0</td> <td>54.0</td> <td>20.8</td> <td>20.8</td> <td>44.6</td> <td>0.0</td> <td>0.0</td> <td>40.3</td> <td>0.0</td> <td>0.0</td>	Uniform Delay (d), s/veh	43.7	9.5	6.0	54.0	20.8	20.8	44.6	0.0	0.0	40.3	0.0	0.0
%ile BackOQ(50%),veh/ln 5.5 8.7 1.1 0.2 16.2 17.0 5.6 0.0 0.0 3.8 0.0 0.0 Unsig. Movement Delay, s/veh 51.9 10.8 6.3 61.0 29.1 28.8 53.0 0.0 0.0 41.1 0.0 0.0 LnGrp LOS D B A E C C D A A D A A Approach Vol, veh/h 1699 1441 183 149 Approach LOS B C D D D D Timer - Assigned Phs 1 2 4 5 6 8 8	Incr Delay (d2), s/veh	8.1	1.3	0.3	7.0	8.3	8.0	8.4	0.0	0.0	0.8	0.0	0.0
Unsig. Movement Delay, s/veh LnGrp Delay(d),s/veh 51.9 10.8 6.3 61.0 29.1 28.8 53.0 0.0 0.0 41.1 0.0 0.0 LnGrp LOS D B A E C C D A A D A A Approach Vol, veh/h 1699 1441 183 149 Approach Delay, s/veh 15.0 29.1 53.0 41.1 Approach LOS B C D D Timer - Assigned Phs 1 2 4 5 6 8 Phs Duration (G+Y+Rc), s 5.2 79.8 25.0 21.7 63.3 25.0 Change Period (Y+Rc), s 4.5 *4.5 5.5 4.5 4.0 5.5 Max Green Setting (Gmax), s 10.5 *66 19.5 15.5 61.0 19.5 Max Q Clear Time (g_c+11), s 2.4 27.4 11.4 14.0 41.7 20.0 Green Ext Time (p_c), s 0.0 33.5 0.3 0.1 17.6 0.0 Intersection Summary HCM 6th Ctrl Delay 24.0 HCM 6th LOS C	Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
LnGrp Delay(d),s/veh 51.9 10.8 6.3 61.0 29.1 28.8 53.0 0.0 0.0 41.1 0.0 0.0 LnGrp LOS D B A E C C D A A D A A Approach Vol, veh/h 1699 1441 183 149 Approach Delay, s/veh 15.0 29.1 53.0 41.1 Approach LOS B C D D D Timer - Assigned Phs 1 2 4 5 6 8 Phs Duration (G+Y+Rc), s 5.2 79.8 25.0 21.7 63.3 25.0 Change Period (Y+Rc), s 4.5 *4.5 5.5 4.5 4.0 5.5 Max Green Setting (Gmax), s 10.5 *66 19.5 15.5 61.0 19.5 Max Q Clear Time (g_c+I1), s 2.4 27.4 11.4 14.0 41.7 20.0 Green Ext Time (p_c), s 0.0 <t< td=""><td></td><td></td><td>8.7</td><td>1.1</td><td>0.2</td><td>16.2</td><td>17.0</td><td>5.6</td><td>0.0</td><td>0.0</td><td>3.8</td><td>0.0</td><td>0.0</td></t<>			8.7	1.1	0.2	16.2	17.0	5.6	0.0	0.0	3.8	0.0	0.0
LnGrp LOS D B A E C C D A A D A A Approach Vol, veh/h 1699 1441 183 149 Approach Delay, s/veh 15.0 29.1 53.0 41.1 Approach LOS B C D D D Timer - Assigned Phs 1 2 4 5 6 8 Phs Duration (G+Y+Rc), s 5.2 79.8 25.0 21.7 63.3 25.0 Change Period (Y+Rc), s 4.5 *4.5 5.5 4.5 4.0 5.5 Max Green Setting (Gmax), s 10.5 *66 19.5 15.5 61.0 19.5 Max Q Clear Time (g_c+I1), s 2.4 27.4 11.4 14.0 41.7 20.0 Green Ext Time (p_c), s 0.0 33.5 0.3 0.1 17.6 0.0 Intersection Summary HCM 6th LOS C C D A A A D	Unsig. Movement Delay, s/veh												
Approach Vol, veh/h 1699 1441 183 149 Approach Delay, s/veh 15.0 29.1 53.0 41.1 Approach LOS B C D D Timer - Assigned Phs 1 2 4 5 6 8 Phs Duration (G+Y+Rc), s 5.2 79.8 25.0 21.7 63.3 25.0 Change Period (Y+Rc), s 4.5 *4.5 5.5 4.5 4.0 5.5 Max Green Setting (Gmax), s 10.5 *66 19.5 15.5 61.0 19.5 Max Q Clear Time (g_c+l1), s 2.4 27.4 11.4 14.0 41.7 20.0 Green Ext Time (p_c), s 0.0 33.5 0.3 0.1 17.6 0.0 Intersection Summary HCM 6th Ctrl Delay 24.0 HCM 6th LOS			10.8	6.3	61.0	29.1	28.8	53.0	0.0	0.0	41.1	0.0	0.0
Approach Vol, veh/h 1699 1441 183 149 Approach Delay, s/veh 15.0 29.1 53.0 41.1 Approach LOS B C D D Timer - Assigned Phs 1 2 4 5 6 8 Phs Duration (G+Y+Rc), s 5.2 79.8 25.0 21.7 63.3 25.0 Change Period (Y+Rc), s 4.5 *4.5 5.5 4.5 4.0 5.5 Max Green Setting (Gmax), s 10.5 *66 19.5 15.5 61.0 19.5 Max Q Clear Time (g_c+l1), s 2.4 27.4 11.4 14.0 41.7 20.0 Green Ext Time (p_c), s 0.0 33.5 0.3 0.1 17.6 0.0 Intersection Summary HCM 6th Ctrl Delay 24.0 HCM 6th LOS	LnGrp LOS	D	В	Α	Е	С	С	D	Α	Α	D	Α	Α
Approach Delay, s/veh 15.0 29.1 53.0 41.1 Approach LOS B C D D Timer - Assigned Phs 1 2 4 5 6 8 Phs Duration (G+Y+Rc), s 5.2 79.8 25.0 21.7 63.3 25.0 Change Period (Y+Rc), s 4.5 *4.5 5.5 4.5 4.0 5.5 Max Green Setting (Gmax), s 10.5 *66 19.5 15.5 61.0 19.5 Max Q Clear Time (g_c+I1), s 2.4 27.4 11.4 14.0 41.7 20.0 Green Ext Time (p_c), s 0.0 33.5 0.3 0.1 17.6 0.0 Intersection Summary HCM 6th Ctrl Delay 24.0 HCM 6th LOS C			1699			1441			183			149	
Approach LOS B C D D Timer - Assigned Phs 1 2 4 5 6 8 Phs Duration (G+Y+Rc), s 5.2 79.8 25.0 21.7 63.3 25.0 Change Period (Y+Rc), s 4.5 *4.5 5.5 4.5 4.0 5.5 Max Green Setting (Gmax), s 10.5 *66 19.5 15.5 61.0 19.5 Max Q Clear Time (g_c+I1), s 2.4 27.4 11.4 14.0 41.7 20.0 Green Ext Time (p_c), s 0.0 33.5 0.3 0.1 17.6 0.0 Intersection Summary HCM 6th Ctrl Delay 24.0 HCM 6th LOS C	Approach Delay, s/veh					29.1						41.1	
Phs Duration (G+Y+Rc), s 5.2 79.8 25.0 21.7 63.3 25.0 Change Period (Y+Rc), s 4.5 *4.5 5.5 4.5 4.0 5.5 Max Green Setting (Gmax), s 10.5 *66 19.5 15.5 61.0 19.5 Max Q Clear Time (g_c+l1), s 2.4 27.4 11.4 14.0 41.7 20.0 Green Ext Time (p_c), s 0.0 33.5 0.3 0.1 17.6 0.0 Intersection Summary HCM 6th Ctrl Delay 24.0 HCM 6th LOS C													
Phs Duration (G+Y+Rc), s 5.2 79.8 25.0 21.7 63.3 25.0 Change Period (Y+Rc), s 4.5 *4.5 5.5 4.5 4.0 5.5 Max Green Setting (Gmax), s 10.5 *66 19.5 15.5 61.0 19.5 Max Q Clear Time (g_c+l1), s 2.4 27.4 11.4 14.0 41.7 20.0 Green Ext Time (p_c), s 0.0 33.5 0.3 0.1 17.6 0.0 Intersection Summary HCM 6th Ctrl Delay 24.0 HCM 6th LOS C	Timer - Assigned Phs	1	2		4	5	6		8				
Change Period (Y+Rc), s 4.5 *4.5 5.5 4.5 4.0 5.5 Max Green Setting (Gmax), s 10.5 *66 19.5 15.5 61.0 19.5 Max Q Clear Time (g_c+I1), s 2.4 27.4 11.4 14.0 41.7 20.0 Green Ext Time (p_c), s 0.0 33.5 0.3 0.1 17.6 0.0 Intersection Summary HCM 6th Ctrl Delay 24.0 HCM 6th LOS C													
Max Green Setting (Gmax), s 10.5 * 66 19.5 15.5 61.0 19.5 Max Q Clear Time (g_c+l1), s 2.4 27.4 11.4 14.0 41.7 20.0 Green Ext Time (p_c), s 0.0 33.5 0.3 0.1 17.6 0.0 Intersection Summary HCM 6th Ctrl Delay 24.0 HCM 6th LOS C													
Max Q Clear Time (g_c+l1), s 2.4 27.4 11.4 14.0 41.7 20.0 Green Ext Time (p_c), s 0.0 33.5 0.3 0.1 17.6 0.0 Intersection Summary HCM 6th Ctrl Delay 24.0 HCM 6th LOS C	. ,												
Green Ext Time (p_c), s 0.0 33.5 0.3 0.1 17.6 0.0 Intersection Summary HCM 6th Ctrl Delay 24.0 HCM 6th LOS C	3 ().												
Intersection Summary HCM 6th Ctrl Delay 24.0 HCM 6th LOS C	(5-):												
HCM 6th Ctrl Delay 24.0 HCM 6th LOS C	u = 7:	0.0	00.0		0.0	V. 1	17.5		0.0				
HCM 6th LOS C				24.0									
	•												
	Notes			U									

^{*} HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Sandy Health Clinic - PBS Project 71524.000 2029 Without Project Conditions - Weekday PM Peak Hour

Lanes, Volumes, Timings

3: Wolf Drive/Ten E	yck Road 8	& Mt. Hood Hi	ghway ((US 26)	
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03/10/2020

Lane Group EBL EBT EBR WBL WBT WBR NBL NBT NBR SBL SBT Lane Configurations 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 0 <t< th=""><th>166 166 1750 0 0</th></t<>	166 166 1750 0 0
Traffic Volume (vph) 88 982 39 0 1106 22 160 4 2 26 8 Future Volume (vph) 88 982 39 0 1106 22 160 4 2 26 8 Ideal Flow (vphpl) 1750	166 1750 0
Traffic Volume (vph) 88 982 39 0 1106 22 160 4 2 26 8 Future Volume (vph) 88 982 39 0 1106 22 160 4 2 26 8 Ideal Flow (vphpl) 1750	166 1750 0
Future Volume (vph) 88 982 39 0 1106 22 160 4 2 26 8 Ideal Flow (vphpl) 1750	1750 0 0
Ideal Flow (vphpl) 1750 <td>0</td>	0
Storage Length (ft) 115 100 105 0 0 0 0 Storage Lanes 1 1 1 0 0 0 0	0
Storage Lanes 1 1 1 0 0 0	
- Y	
Taper Length (ft) 100 100 100 100	1.00
Lane Util. Factor 1.00 0.95 1.00 1.00 0.95 0.95 1.00 1.00 1.00 1.00	
Ped Bike Factor 0.98 1.00 1.00 0.99	
Frt 0.850 0.997 0.998 0.888	
Flt Protected 0.950 0.954 0.994	
Satd. Flow (prot) 1568 2942 1403 1716 3158 0 0 1603 0 0 1455	0
Flt Permitted 0.950 0.499 0.946	
Satd. Flow (perm) 1568 2942 1372 1716 3158 0 0 837 0 0 1385	0
Right Turn on Red Yes Yes Yes	Yes
Satd. Flow (RTOR) 77 2 1 180)	
Link Speed (mph) 25 25 25 25	
Link Distance (ft) 468 302 365 210	
Travel Time (s) 12.8 8.2 10.0 5.7	
Confl. Peds. (#/hr) 1 1 2	2
Confl. Bikes (#/hr)	_
Peak Hour Factor 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92	0.92
Heavy Vehicles (%) 6% 13% 6% 2% 5% 2% 4% 2% 2% 24% 2%	2%
Adj. Flow (vph) 96 1067 42 0 1202 24 174 4 2 28 9	180
Shared Lane Traffic (%)	
Lane Group Flow (vph) 96 1067 42 0 1226 0 0 180 0 0 217	0
Enter Blocked Intersection No	No
Lane Alignment L NA R NA R NA L NA R NA L NA R NA L NA R NA L NA R NA	R NA
Median Width(ft) 12 12 0 0	
Link Offset(ft) -12 0 0 4	
Crosswalk Width(ft) 72 42 32 30	
Two way Left Turn Lane	
Headway Factor 1.11 1.11 1.11 1.11 1.11 1.11 1.11 1.	1.11
Number of Detectors 2 1 1 2 1 2 2 2 2	
Detector Template Left Det25 Right Left Det25 Left Side St Left Side St	
Leading Detector (ft) 78 153 153 78 153 78 78 78 78	
Trailing Detector (ft) 2 137 137 2 137 2 2 2 2	
Detector 1 Position(ft) 2 137 137 2 137 2 2 2 2	
Detector 1 Size(ft) 16 16 16 16 16 16 16 16 16 16	
Detector 1 Type CI+Ex CI+Ex CI+Ex CI+Ex CI+Ex CI+Ex CI+Ex CI+Ex	
Detector 1 Channel	
Detector 1 Extend (s) 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.	
Detector 1 Queue (s) 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.	
Detector 1 Delay (s) 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0	
Detector 2 Position(ft) 72 72 72 72 72 72	
Detector 2 Size(ft) 6 6 6 6	
Detector 2 Type CI+Ex CI+Ex CI+Ex CI+Ex CI+Ex	
Detector 2 Channel	
Detector 2 Extend (s) 0.0 0.0 0.0 0.0 0.0 0.0	

Sandy Health Clinic - PBS Project 71524.000 2029 With Project Conditions - Weekday AM Peak Hour

Lanes, Volumes, Timings 3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26) 03/10/2020 ၨ t **EBL** EBT **EBR** WBL WBT WBR NBL **NBT** NBR SBL Lane Group **SBT** Turn Type Prot NA Perm Prot NA Perm NA Perm NA **Protected Phases** 5 2 6 8 4 **Permitted Phases** 8 2 2 6 4 **Detector Phase** 5 1 8 8 4 Switch Phase Minimum Initial (s) 4.0 10.0 10.0 4.0 10.0 6.0 6.0 6.0 6.0 Minimum Split (s) 8.5 26.0 26.0 8.5 30.0 11.5 11.5 23.5 23.5 29.5 54.0 54.0 44.0 25.5 25.5 25.5 Total Split (s) 19.5 25.5 Total Split (%) 29.8% 54.5% 54.5% 19.7% 44.4% 25.8% 25.8% 25.8% 25.8% Maximum Green (s) 25.0 50.0 50.0 15.0 40.0 20.0 20.0 20.0 20.0 4.0 Yellow Time (s) 3.5 3.5 3.5 3.5 3.5 4.0 4.0 4.0 All-Red Time (s) 1.0 0.5 0.5 1.0 0.5 1.5 1.5 1.5 1.5 Lost Time Adjust (s) -0.5 0.0 0.0 -0.5 0.0 -1.5 -1.5 Total Lost Time (s) 4.0 4.0 4.0 4.0 4.0 4.0 4.0 Lead/Lag Lag Lag Lag Lead Lead Lead-Lag Optimize? 2.3 Vehicle Extension (s) 2.3 5.8 5.8 5.8 2.5 2.5 2.5 2.5 2.0 0.5 0.5 2.0 2.0 2.0 Minimum Gap (s) 3.8 3.8 3.8 Time Before Reduce (s) 8.0 5.0 5.0 5.0 10.0 10.0 8.0 10.0 5.0 Time To Reduce (s) 3.0 10.0 10.0 3.0 10.0 5.0 5.0 5.0 5.0 Recall Mode Min Min None None None Min None None None Walk Time (s) 7.0 7.0 7.0 7.0 7.0 Flash Dont Walk (s) 15.0 15.0 19.0 11.0 11.0 Pedestrian Calls (#/hr) 0 0 0 0 0 47.9 47.9 36.5 10.2 22.2 22.2 Act Effct Green (s) 0.47 0.28 Actuated g/C Ratio 0.13 0.61 0.61 0.28 v/c Ratio 0.47 0.59 0.05 0.83 0.76 0.42 Control Delay 41.9 25.2 53.0 10.2 0.5 9.8 Queue Delay 0.0 0.0 0.0 0.0 0.0 0.0 **Total Delay** 41.9 10.2 25.2 53.0 9.8 0.5 LOS D В C D Α Approach Delay 12.4 25.2 53.0 9.8 Approach LOS В С D Α Intersection Summary Other Area Type: Cycle Length: 99 Actuated Cycle Length: 78.3) Natural Cycle: 65 Control Type: Actuated-Uncoordinated Maximum v/c Ratio: 0.83 Intersection Signal Delay: 20.3 Intersection LOS: C Intersection Capacity Utilization 76.0% ICU Level of Service D Analysis Period (min) 15 Splits and Phases: 3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26) Ø1 Ø4 <u>≯_{ø5}</u> ¶ ø8 Ø6 2029 With Project Conditions - Weekday AM Peak Hour Page 2

3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26)

03/10/2020

	•	→	•	•	†	ţ
Lane Group	EBL	EBT	EBR	WBT	NBT	SBT
Lane Group Flow (vph)	96	1067	42	1226	180	217
v/c Ratio	0.47	0.59	0.05	0.83	0.76	0.42
Control Delay	41.9	10.2	0.5	25.2	53.0	9.8
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	41.9	10.2	0.5	25.2	53.0	9.8
Queue Length 50th (ft)	48	143	0	275	89	15
Queue Length 95th (ft)	95	191	4	#406	#220	76
Internal Link Dist (ft)		388		222	285	130
Turn Bay Length (ft)	115		100			
Base Capacity (vph)	526	2064	985	1663	237	520
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.18	0.52	0.04	0.74	0.76	0.42
Intersection Summary						

^{# 95}th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis 3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26)

03/10/2020

	۶	→	•	•	←	•	1	†	/	/		1
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	^	7	, N	∱ }			4			4	
Traffic Volume (vph)	88	982	39	0	1106	22	160	4	2	26	8	166
Future Volume (vph)	88	982	39	0	1106	22	160	4	2	26	8	166
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Total Lost time (s)	4.0	4.0	4.0		4.0			4.0			4.0	
Lane Util. Factor	1.00	0.95	1.00		0.95			1.00			1.00	
Frpb, ped/bikes	1.00	1.00	0.98		1.00			1.00			0.99	
Flpb, ped/bikes	1.00	1.00	1.00		1.00			1.00			1.00	
Frt	1.00	1.00	0.85		1.00			1.00			0.89	
Flt Protected	0.95	1.00	1.00		1.00			0.95			0.99	
Satd. Flow (prot)	1568	2942	1373		3158			1601			1455	
Flt Permitted	0.95	1.00	1.00		1.00			0.50			0.95	
Satd. Flow (perm)	1568	2942	1373		3158			838			1385	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	96	1067	42	0	1202	24	174	4	2	28	9	180
RTOR Reduction (vph)	0	0	16	0	1	0	0	1	0	0	130	0
Lane Group Flow (vph)	96	1067	26	0	1225	0	0	179	0	0	87	0
Confl. Peds. (#/hr)			1	1			2					2
Confl. Bikes (#/hr)						1						
Heavy Vehicles (%)	6%	13%	6%	2%	5%	2%	4%	2%	2%	24%	2%	2%
Turn Type	Prot	NA	Perm	Prot	NA		Perm	NA		Perm	NA	
Protected Phases	5	2		1	6			8			4	
Permitted Phases			2				8			4		
Actuated Green, G (s)	8.0	49.0	49.0		36.5			20.6			20.6	
Effective Green, g (s)	8.5	49.0	49.0		36.5			22.1			22.1	
Actuated g/C Ratio	0.11	0.62	0.62		0.46			0.28			0.28	
Clearance Time (s)	4.5	4.0	4.0		4.0			5.5			5.5	
Vehicle Extension (s)	2.3	5.8	5.8		5.8			2.5			2.5	
Lane Grp Cap (vph)	168	1822	850		1457			234			386	
v/s Ratio Prot	0.06	c0.36			c0.39							
v/s Ratio Perm			0.02					c0.21			0.06	
v/c Ratio	0.57	0.59	0.03		0.84			0.77			0.23	
Uniform Delay, d1	33.6	9.0	5.8		18.7			26.1			21.9	
Progression Factor	1.00	1.00	1.00		1.00			1.00			1.00	
Incremental Delay, d2	3.4	0.9	0.0		5.2			13.3			0.2	
Delay (s)	37.0	9.9	5.9		24.0			39.5			22.1	
Level of Service	D	Α	Α		С			D			С	
Approach Delay (s)		11.9			24.0			39.5			22.1	
Approach LOS		В			С			D			С	
Intersection Summary												
HCM 2000 Control Delay			19.7	H	CM 2000	Level of S	Service		В			
HCM 2000 Volume to Capacit	y ratio		0.81									
Actuated Cycle Length (s)			79.1	Sı	um of lost	time (s)			12.0			
Intersection Capacity Utilization	n		76.0%		U Level c	٠,			D			
Analysis Period (min)			15									
c Critical Lane Group												

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HCM 6th Signalized Intersection Summary

3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26)

03/10/2020

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	75	^	7	ሻ	↑ 1>			4			4	
Traffic Volume (veh/h)	88	982	39	0	1106	22	160	4	2	26	8	166
Future Volume (veh/h)	88	982	39	0	1106	22	160	4	2	26	8	166
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		0.98	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1668	1573	1668	1723	1682	1682	1723	1723	1723	1723	1723	1723
Adj Flow Rate, veh/h	96	1067	9	0	1202	23	174	4	1	28	9	39
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	6	13	6	2	5	5	2	2	2	2	2	2
Cap, veh/h	135	2083	985	3	1792	34	351	6	1	146	64	143
Arrive On Green	0.08	0.70	0.70	0.00	0.56	0.56	0.15	0.17	0.15	0.15	0.17	0.15
Sat Flow, veh/h	1589	2988	1413	1641	3205	61	1399	32	8	411	371	824
Grp Volume(v), veh/h	96	1067	9	0	599	626	179	0	0	76	0	0
Grp Sat Flow(s), veh/h/ln	1589	1494	1413	1641	1598	1669	1440	0	0	1606	0	0
Q Serve(g_s), s	3.9	11.0	0.1	0.0	17.3	17.4	5.0	0.0	0.0	0.0	0.0	0.0
Cycle Q Clear(g_c), s	3.9	11.0	0.1	0.0	17.3	17.4	7.8	0.0	0.0	2.8	0.0	0.0
Prop In Lane	1.00	11.0	1.00	1.00	17.0	0.04	0.97	0.0	0.01	0.37	0.0	0.51
Lane Grp Cap(c), veh/h	135	2083	985	3	893	933	325	0	0.01	317	0	0.51
V/C Ratio(X)	0.71	0.51	0.01	0.00	0.67	0.67	0.55	0.00	0.00	0.24	0.00	0.00
Avail Cap(c a), veh/h	617	2277	1077	388	974	1018	535	0.00	0.00	543	0.00	0.00
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	0.00	1.00	1.00	1.00	0.00	0.00	1.00	0.00	0.00
Uniform Delay (d), s/veh	29.2	4.7	3.0	0.00	10.2	10.2	26.2	0.00	0.00	24.2	0.00	0.00
Incr Delay (d2), s/veh	4.2	0.6	0.0	0.0	3.1	3.0	1.1	0.0	0.0	0.3	0.0	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0			0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	1.6	2.6	0.0	0.0	5.9	6.2	2.8	0.0	0.0	1.1		0.0
		2.0	0.0	0.0	5.9	0.2	2.0	0.0	0.0	1.1	0.0	0.0
Unsig. Movement Delay, s/veh		F 2	2.0	0.0	40.0	42.0	07.0	0.0	0.0	04.5	0.0	0.0
LnGrp Delay(d),s/veh	33.5	5.3	3.0	0.0	13.3	13.2	27.3	0.0	0.0	24.5	0.0	0.0
LnGrp LOS	С	A	A	A	B	В	С	A	A	С	A	A
Approach Vol, veh/h		1172			1225			179			76	
Approach Delay, s/veh		7.6			13.2			27.3			24.5	
Approach LOS		Α			В			С			С	
Timer - Assigned Phs	1	2		4	5	6		8				
Phs Duration (G+Y+Rc), s	0.0	50.2		15.4	9.6	40.7		15.4				
Change Period (Y+Rc), s	4.5	* 4.5		5.5	4.5	4.0		5.5				
Max Green Setting (Gmax), s	15.0	* 50		20.0	25.0	40.0		20.0				
Max Q Clear Time (g_c+l1), s	0.0	13.0		4.8	5.9	19.4		9.8				
Green Ext Time (p_c), s	0.0	25.6		0.2	0.3	17.3		0.5				
Intersection Summary												
HCM 6th Ctrl Delay			12.0									
HCM 6th LOS			12.0 B									
Notos			<u> </u>									

^{*} HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Sandy Health Clinic - PBS Project 71524.000 2029 With Project Conditions - Weekday AM Peak Hour Synchro 10 Report - by PBS Engineering and Environmental Page 5 Lanes, Volumes, Timings

3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26)

03/09/2020

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	† †	7	7	∱ }			4			4	
Traffic Volume (vph)	186	1292	158	6	1331	20	137	28	8	47	27	138
Future Volume (vph)	186	1292	158	6	1331	20	137	28	8	47	27	138
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Storage Length (ft)	115		100	105		0	0		0	0		0
Storage Lanes	1		1	1		0	0		0	0		0
Taper Length (ft)	100			100			100			100		
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	0.95	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor	1.00		0.98	1.00	1.00			1.00			0.99	
Frt			0.850		0.998			0.993			0.912	
Flt Protected	0.950			0.950				0.962			0.989	
Satd. Flow (prot)	1630	3228	1458	1630	3161	0	0	1623	0	0	1526	0
Flt Permitted	0.950	0220	1100	0.950	0101	Ū	•	0.473		•	0.910	
Satd. Flow (perm)	1629	3228	1426	1629	3161	0	0	796	0	0	1404	0
Right Turn on Red	1020	0220	Yes	1020	0101	Yes	· ·	700	Yes	•	1101	Yes
Satd. Flow (RTOR)			95		2			2			75	
Link Speed (mph)		25			25			25			25	
Link Distance (ft)		468			302			365			210	
Travel Time (s)		12.8			8.2			10.0			5.7	
Confl. Peds. (#/hr)	2	12.0	1	1	0.2	2	5	10.0			0.1	5
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Heavy Vehicles (%)	2%	3%	2%	2%	5%	2%	2%	8%	2%	2%	2%	2%
Adj. Flow (vph)	198	1374	168	6	1416	21	146	30	9	50	29	147
Shared Lane Traffic (%)	130	1017	100		1710	۷,	170	00	J	30	25	177
Lane Group Flow (vph)	198	1374	168	6	1437	0	0	185	0	0	226	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	L NA	R NA	R NA	L NA	R NA	R NA	L NA	R NA	R NA	L NA	R NA	R NA
Median Width(ft)		12	11171		12	1 (1 () (_ I W \	0	11171		0	1 (1 (7 (
Link Offset(ft)		-12			0			0			4	
Crosswalk Width(ft)		72			42			32			30	
Two way Left Turn Lane		12			72			02			00	
Headway Factor	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11
Number of Detectors	2	1.11	1.11	2	1.11	1.11	2	2	1.11	2	2	1.11
Detector Template	Left	Det25	Right	Left	Det25		Left	Side St		Left	Side St	
Leading Detector (ft)	78	153	153	78	153		78	78		78	78	
Trailing Detector (ft)	2	137	137	2	137		2	2		2	2	
Detector 1 Position(ft)	2	137	137	2	137		2	2		2	2	
Detector 1 Size(ft)	16	16	16	16	16		16	16		16	16	
Detector 1 Type	Cl+Ex	CI+Ex	Cl+Ex	Cl+Ex	CI+Ex		CI+Ex	CI+Ex		Cl+Ex	CI+Ex	
Detector 1 Channel	OIILX	OITEX	OITEX	OIILX	OITEX		OITEX	OITEX		OITEX	OITEX	
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Detector 2 Position(ft)	72	0.0	0.0	72	0.0		72	72		72	72	
Detector 2 Size(ft)	6			6			6	6		6	6	
Detector 2 Type	CI+Ex			CI+Ex			CI+Ex	CI+Ex		CI+Ex	CI+Ex	
Detector 2 Type Detector 2 Channel	OI+EX			OI+EX			OI+EX	OI+EX		OI+EX	UI+EX	
Detector 2 Extend (s)	0.0			0.0			0.0	0.0		0.0	0.0	
Turn Type	Prot	NA	Perm	Prot	NA		Perm	NA		Perm	NA	
Tuili Type	רוטנ	INA	r ellil	17101	INA		r emi	INA		r C IIII	INA	

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Lanes, Volumes, Timings 3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26) 03/09/2020 ٠ t **NBT NBR** SBL Lane Group **EBL EBT EBR** Protected Phases 2 5 Permitted Phases 2 8 **Detector Phase** 5 2 2 8 8 4 4 Switch Phase 10.0 10.0 10.0 6.0 Minimum Initial (s) 4.0 4.0 6.0 6.0 6.0 Minimum Split (s) 8.5 26.0 26.0 8.5 30.0 11.5 11.5 23.5 23.5 Total Split (s) 20.0 70.0 70.0 15.0 65.0 25.0 25.0 25.0 25.0 18.2% 63.6% 59.1% 22.7% 22.7% Total Split (%) 63.6% 13.6% 22.7% 22.7% Maximum Green (s) 15.5 66.0 66.0 10.5 61.0 19.5 19.5 19.5 19.5 Yellow Time (s) 3.5 3.5 3.5 3.5 3.5 4.0 4.0 4.0 4.0 1.5 1.5 All-Red Time (s) 1.0 0.5 0.5 1.0 0.5 1.5 1.5 Lost Time Adjust (s) -0.5 0.0 0.0 -0.5 0.0 -1.5 -1.5 4.0 4.0 4.0 4.0 4.0 4.0 4.0 Total Lost Time (s) Lead/Lag Lead Lag Lag Lag Lead Lead-Lag Optimize? Vehicle Extension (s) 2.3 5.8 5.8 2.3 5.8 2.5 2.5 2.5 2.5 Minimum Gap (s) 0.5 3.8 3.8 0.5 3.8 2.0 2.0 2.0 2.0 Time Before Reduce (s) 5.0 5.0 5.0 8.0 10.0 10.0 8.0 10.0 5.0 Time To Reduce (s) 3.0 5.0 5.0 5.0 5.0 10.0 10.0 3.0 10.0 Recall Mode None C-Min C-Min None C-Min None None None None Walk Time (s) 7.0 7.0 7.0 7.0 7.0 Flash Dont Walk (s) 15.0 15.0 19.0 11.0 11.0 Pedestrian Calls (#/hr) 0 0 0 Act Effct Green (s) 15.9 75.7 75.7 5.8 58.0 24.1 24.1 Actuated g/C Ratio 0.14 0.69 0.69 0.05 0.53 0.22 0.22 v/c Ratio 0.84 1.05 0.62 0.17 0.07 0.86 0.62 11.0 126.5 35.2 Control Delay 75.7 50.8 28.6 3.1 0.0 0.0 0.0 Queue Delay 0.0 0.0 0.0 0.0 **Total Delay** 75.7 11.0 3.1 50.8 28.6 126.5 35.2 LOS Ε В Α D C F D Approach Delay 17.6 28.7 126.5 35.2 Approach LOS В С F D Intersection Summary Area Type: Other Cycle Length: 110 Actuated Cycle Length: 110 Offset: 0 (0%), Referenced to phase 2:EBT and 6:WBT, Start of Green, Master Intersection Natural Cycle: 90 Control Type: Actuated-Coordinated Maximum v/c Ratio: 1.05 Intersection Signal Delay: 28.8 Intersection LOS: C Intersection Capacity Utilization 89.6% ICU Level of Service E Analysis Period (min) 15 Splits and Phases: 3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26) ÿ1 Ø4 ₩ Ø2 (R) <u>≯_{Ø5}</u> ¶ Ø8 Ø6 (R)

2029 With Project Trips Conditions - Weekday PM Peak Hour

Queues

3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26)

03/09/2020

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Lane Group	EBL	EBT	EBR	WBL	WBT	NBT	SBT
Lane Group Flow (vph)	198	1374	168	6	1437	185	226
v/c Ratio	0.84	0.62	0.17	0.07	0.86	1.05	0.62
Control Delay	75.7	11.0	3.1	50.8	28.6	126.5	35.2
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	75.7	11.0	3.1	50.8	28.6	126.5	35.2
Queue Length 50th (ft)	136	201	12	4	426	~159	100
Queue Length 95th (ft)	#263	369	44	18	518	#305	#189
Internal Link Dist (ft)		388			222	285	130
Turn Bay Length (ft)	115		100	105			
Base Capacity (vph)	244	2221	1011	163	1753	176	366
Starvation Cap Reductn	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	0.81	0.62	0.17	0.04	0.82	1.05	0.62

Intersection Summary

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Volume exceeds capacity, queue is theoretically infinite.
 Queue shown is maximum after two cycles.

^{# 95}th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis 3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26)

03/09/2020

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	^	7	7	∱ Љ			4			44	
Traffic Volume (vph)	186	1292	158	6	1331	20	137	28	8	47	27	138
Future Volume (vph)	186	1292	158	6	1331	20	137	28	8	47	27	138
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0			4.0			4.0	
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95			1.00			1.00	
Frpb, ped/bikes	1.00	1.00	0.98	1.00	1.00			1.00			0.99	
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00			1.00			1.00	
Frt	1.00	1.00	0.85	1.00	1.00			0.99			0.91	
Flt Protected	0.95	1.00	1.00	0.95	1.00			0.96			0.99	
Satd. Flow (prot)	1630	3228	1426	1630	3160			1619			1527	
Flt Permitted	0.95	1.00	1.00	0.95	1.00			0.47			0.91	
Satd. Flow (perm)	1630	3228	1426	1630	3160			796			1405	
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Adj. Flow (vph)	198	1374	168	6	1416	21	146	30	9	50	29	147
RTOR Reduction (vph)	0	0	33	0	1	0	0	2	0	0	59	0
Lane Group Flow (vph)	198	1374	135	6	1436	0	0	183	0	0	167	0
Confl. Peds. (#/hr)	2		1	1		2	5					5
Heavy Vehicles (%)	2%	3%	2%	2%	5%	2%	2%	8%	2%	2%	2%	2%
Turn Type	Prot	NA	Perm	Prot	NA		Perm	NA		Perm	NA	
Protected Phases	5	2		1	6			8			4	
Permitted Phases			2				8			4		
Actuated Green, G (s)	19.0	72.1	72.1	1.3	54.4			22.6			22.6	
Effective Green, g (s)	19.5	72.1	72.1	1.8	54.4			24.1			24.1	
Actuated g/C Ratio	0.18	0.66	0.66	0.02	0.49			0.22			0.22	
Clearance Time (s)	4.5	4.0	4.0	4.5	4.0			5.5			5.5	
Vehicle Extension (s)	2.3	5.8	5.8	2.3	5.8			2.5			2.5	
Lane Grp Cap (vph)	288	2115	934	26	1562			174			307	
v/s Ratio Prot	0.12	c0.43		0.00	c0.45							
v/s Ratio Perm			0.09					c0.23			0.12	
v/c Ratio	0.69	0.65	0.14	0.23	0.92			1.05			0.55	
Uniform Delay, d1	42.4	11.4	7.2	53.4	25.8			42.9			38.1	
Progression Factor	1.00	1.00	1.00	1.00	1.00			1.00			1.00	
Incremental Delay, d2	5.8	1.6	0.3	2.6	10.2			83.3			1.6	
Delay (s)	48.2	12.9	7.5	56.1	36.0			126.3			39.7	
Level of Service	D	В	Α	Е	D			F			D	
Approach Delay (s)		16.4			36.1			126.3			39.7	
Approach LOS		В			D			F			D	
Intersection Summary												
HCM 2000 Control Delay			31.4	Н	CM 2000	Level of S	Service		С			
HCM 2000 Volume to Capac	ity ratio		0.91									
Actuated Cycle Length (s)	,		110.0	S	um of los	time (s)			12.0			
Intersection Capacity Utilizati	on		89.6%			of Service			E			
Analysis Period (min)			15						_			
c Critical Lane Group												

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HCM 6th Signalized Intersection Summary

3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26)

03/09/2020

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	75	^	7	ሻ	∱ ∱			4			4	
Traffic Volume (veh/h)	186	1292	158	6	1331	20	137	28	8	47	27	138
Future Volume (veh/h)	186	1292	158	6	1331	20	137	28	8	47	27	138
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		0.99	1.00		0.99
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1723	1709	1723	1723	1682	1682	1641	1641	1641	1723	1723	1723
Adj Flow Rate, veh/h	198	1374	137	6	1416	20	146	30	7	50	29	84
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Percent Heavy Veh, %	2	3	2	2	5	5	8	8	8	2	2	2
Cap, veh/h	264	2224	998	17	1739	25	219	35	8	115	74	156
Arrive On Green	0.16	0.68	0.68	0.01	0.54	0.54	0.18	0.19	0.18	0.18	0.19	0.18
Sat Flow, veh/h	1641	3247	1458	1641	3226	46	839	185	41	379	390	818
Grp Volume(v), veh/h	198	1374	137	6	701	735	183	0	0	163	0	0
Grp Sat Flow(s),veh/h/ln	1641	1624	1458	1641	1598	1673	1064	0	0	1587	0	0
Q Serve(g_s), s	12.7	25.4	3.6	0.4	39.6	39.7	8.6	0.0	0.0	0.0	0.0	0.0
Cycle Q Clear(g_c), s	12.7	25.4	3.6	0.4	39.6	39.7	18.9	0.0	0.0	10.4	0.0	0.0
Prop In Lane	1.00		1.00	1.00		0.03	0.80		0.04	0.31		0.52
Lane Grp Cap(c), veh/h	264	2224	998	17	861	902	247	0	0	324	0	0
V/C Ratio(X)	0.75	0.62	0.14	0.34	0.81	0.81	0.74	0.00	0.00	0.50	0.00	0.00
Avail Cap(c_a), veh/h	264	2224	998	164	886	928	247	0	0	324	0	0
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	0.00	1.00	0.00	0.00
Uniform Delay (d), s/veh	44.0	9.5	6.0	54.0	20.8	20.8	45.1	0.0	0.0	40.8	0.0	0.0
Incr Delay (d2), s/veh	10.6	1.3	0.3	7.0	8.3	8.0	10.6	0.0	0.0	0.9	0.0	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	5.9	8.7	1.1	0.2	16.3	17.0	5.8	0.0	0.0	4.2	0.0	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	54.6	10.8	6.3	61.0	29.1	28.9	55.8	0.0	0.0	41.7	0.0	0.0
LnGrp LOS	D	В	Α	Е	С	С	Е	Α	Α	D	Α	Α
Approach Vol, veh/h		1709			1442			183			163	
Approach Delay, s/veh		15.5			29.1			55.8			41.7	
Approach LOS		В			С			Е			D	
Timer - Assigned Phs	1	2		4	5	6		8				
Phs Duration (G+Y+Rc), s	5.2	79.8		25.0	21.7	63.3		25.0				
Change Period (Y+Rc), s	4.5	* 4.5		5.5	4.5	4.0		5.5				
Max Green Setting (Gmax), s	10.5	* 66		19.5	15.5	61.0		19.5				
Max Q Clear Time (g_c+l1), s	2.4	27.4		12.4	14.7	41.7		20.9				
Green Ext Time (p_c), s	0.0	33.5		0.3	0.1	17.6		0.0				
Intersection Summary												
HCM 6th Ctrl Delay			24.4									
HCM 6th LOS			C									
Notes												

^{*} HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Sandy Health Clinic - PBS Project 71524.000 2029 With Project Trips Conditions - Weekday PM Peak Hour Synchro 10 Report - by PBS Engineering and Environmental Page 5 Lanes, Volumes, Timings

3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26)

03/10/2020

Bear Group		۶	→	•	€	+	•	1	†	<i>></i>	/	+	4
Traffic Volume (vph)	Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Traffic Volume (vph)	Lane Configurations	*	44	7	*	^	7		4			43-	
Future Volume (vorh) [Mag and Several Content of Se								160		2	26		166
Ideal Flow (phph)					0				4			8	
Storage Length (ft)													
Storage Lanes	,												
Taper Length (ff)													
Lane Util. Factor				•			•			•			J
Ped Bike Factor			0.95	1 00		0.95	1 00		1 00	1 00		1 00	1 00
Fith		1.00	0.00		1.00	0.00		1.00		1.00	1.00		1.00
Fit Protected 0.950 Satol. Flow (prot) 1568 2942 1403 1716 3167 1458 0 1603 0 0 1455 0 0.946 Satol. Flow (prot) 1568 2942 1372 1716 3167 1428 0 846 0 0 1385 0 0.00													
Satd. Flow (proft) 1568 2942 1403 1716 3167 1458 0 1603 0 0 1455 0 Fit Permitted 0.950 0.950 0.904 0.946 Satd. Flow (perm) 1568 2942 1372 1716 3167 1428 0 846 0 0 0 1385 0 Right Turn on Red Yes Yes Yes Yes Yes Yes Yes Link Speed (mph) 25 25 25 25 25 Link Distance (ft) 468 302 365 210 Travel Time (s) 128 82 302 365 210 Travel Time (s) 128 302 365 210 Travel Time (s) 128 302 365 210 Travel Time (s) 128 302 392 0.92 0.92 0.92 0.92 0.92 Peak Hour Factor 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 Heavy Vehicles (%) 6% 13% 6% 2% 5% 2% 4% 2% 2% 24% 2% 2%		0.950		0.000			0.000						
Fit Permitted			2942	1403	1716	3167	1458	٥		0	0		0
Satd. Flow (perm) 1568 2942 1372 1716 3167 1428 0 846 0 0 0 1385 0	,		2572	1700	17 10	0107	1400	U		U	U		U
Page			20/12	1372	1716	3167	1/128	٥		Λ	0		0
Satd. Flow (RTOR)	" ,	1300	2342		17 10	3107		U	0+0		U	1000	
Link Speed (mph)									- 1	163		190	163
Link Distance (ft)			25	11		25	- 11						
Travel Time (s)													
Confi. Peds. (#/hr)													
Confl. Bikes (#/hr) Peak Hour Factor 0.92 0			12.0	1	1	0.2		2	10.0			5.7	2
Peak Hour Factor				ı	ı		4	2					Z
Heavy Vehicles (%)		0.00	0.00	0.00	0.00	0.00		0.00	0.00	0.00	0.00	0.00	0.00
Adj. Flow (vph) 96 1067 42 0 1202 24 174 4 2 28 9 180													
Shared Lane Traffic (%) Lane Group Flow (vph) 96 1067 42 0 1202 24 0 180 0 0 217 0													
Lane Group Flow (vph) 96 1067 42 0 1202 24 0 180 0 0 217 0		96	1067	42	U	1202	24	1/4	4	2	28	9	180
Enter Blocked Intersection	` ,	00	4007	40	•	4000	0.4	•	400	•	•	0.47	0
Lane Alignment													
Median Width(ft) 12 12 0 0 Link Offset(ft) -12 0 0 4 Crosswalk Width(ft) 72 42 32 30 Two way Left Turn Lane 1.11 1.													
Link Offset(ft) -12 0 0 4 Crosswalk Width(ft) 72 42 32 30 Two way Left Turn Lane Headway Factor 1.11 1	9	L NA		RNA	L NA		RNA	L NA		RNA	L NA		RNA
Crosswalk Width(ft) 72 42 32 30 Two way Left Turn Lane Headway Factor 1.11													
Two way Left Turn Lane Headway Factor 1.11 1.11 1.11 1.11 1.11 1.11 1.11 1.													
Headway Factor			72			42			32			30	
Number of Detectors 2 1 1 2 1 1 2													
Detector Template										1.11			1.11
Leading Detector (ft) 78 153 153 78 153 153 78													
Trailing Detector (ft) 2 137 137 2 137 137 2 2 2 2 2 2 Detector 1 Position(ft) 2 137 137 2 137 137 2 2 2 2 2 2 Detector 1 Size(ft) 16 16 16 16 16 16 16 16 16 16 16 16 Detector 1 Type CI+Ex													
Detector 1 Position(ft) 2 137 137 2 137 137 2 <t< td=""><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></t<>													
Detector 1 Size(ft) 16 12 <td></td>													
Detector 1 Type CI+Ex													
Detector 1 Channel Detector 1 Extend (s) 0.0 <td>Detector 1 Size(ft)</td> <td>16</td> <td>16</td> <td></td> <td></td> <td>16</td> <td></td> <td></td> <td></td> <td></td> <td>16</td> <td></td> <td></td>	Detector 1 Size(ft)	16	16			16					16		
Detector 1 Extend (s) 0.0	Detector 1 Type	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex		CI+Ex	CI+Ex	
Detector 1 Queue (s) 0.0	Detector 1 Channel												
Detector 1 Delay (s) 0.0	Detector 1 Extend (s)												
Detector 2 Position(ft) 72	Detector 1 Queue (s)		0.0	0.0		0.0	0.0	0.0			0.0		
Detector 2 Size(ft) 6 6 6 6 6 6 6 Detector 2 Type CI+Ex CI+Ex CI+Ex CI+Ex CI+Ex CI+Ex Detector 2 Channel CI+Ex CI+Ex CI+Ex CI+Ex CI+Ex	Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 2 Size(ft) 6 6 6 6 6 6 6 Detector 2 Type CI+Ex CI+Ex </td <td>Detector 2 Position(ft)</td> <td>72</td> <td></td> <td></td> <td>72</td> <td></td> <td></td> <td>72</td> <td>72</td> <td></td> <td>72</td> <td>72</td> <td></td>	Detector 2 Position(ft)	72			72			72	72		72	72	
Detector 2 Type CI+Ex CI+Ex CI+Ex CI+Ex CI+Ex CI+Ex Detector 2 Channel	Detector 2 Size(ft)	6			6			6	6		6	6	
Detector 2 Channel													
	Detector 2 Extend (s)	0.0			0.0			0.0	0.0		0.0	0.0	

Sandy Health Clinic - PBS Project 71524.000

Synchro 10 Report - by PBS Engineering and Environmental

2029 With Project Conditions - Weekday AM Peak Hour + With WB Right-Turn Lane

Lanes, Volumes, Timings 3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26) 03/10/2020 ၨ t **EBL** EBT WBL WBT WBR **NBT** NBR SBL Lane Group **EBR** NBL **SBT** Turn Type Prot NA Perm Prot NA Perm Perm NA NA Perm **Protected Phases** 5 2 6 8 4 **Permitted Phases** 8 2 4 **Detector Phase** 5 2 1 6 6 8 8 4 Switch Phase Minimum Initial (s) 4.0 10.0 10.0 4.0 10.0 10.0 6.0 6.0 6.0 6.0 Minimum Split (s) 8.5 26.0 26.0 8.5 30.0 30.0 11.5 11.5 23.5 23.5 29.5 54.0 44.0 25.5 25.5 Total Split (s) 54.0 19.5 44.0 25.5 25.5 Total Split (%) 29.8% 54.5% 54.5% 19.7% 44.4% 44.4% 25.8% 25.8% 25.8% 25.8% Maximum Green (s) 25.0 50.0 50.0 15.0 40.0 40.0 20.0 20.0 20.0 20.0 4.0 Yellow Time (s) 3.5 3.5 3.5 3.5 3.5 3.5 4.0 4.0 4.0 All-Red Time (s) 1.0 0.5 0.5 1.0 0.5 0.5 1.5 1.5 1.5 1.5 Lost Time Adjust (s) -0.5 0.0 0.0 -0.5 0.0 0.0 -1.5 -1.5 Total Lost Time (s) 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 Lead/Lag Lag Lag Lag Lead Lead Lead Lead-Lag Optimize? Vehicle Extension (s) 2.3 5.8 5.8 2.3 5.8 5.8 2.5 2.5 2.5 2.5 2.0 0.5 0.5 2.0 2.0 2.0 Minimum Gap (s) 3.8 3.8 3.8 3.8 5.0 5.0 Time Before Reduce (s) 8.0 10.0 10.0 8.0 10.0 10.0 5.0 5.0 Time To Reduce (s) 3.0 10.0 10.0 3.0 10.0 10.0 5.0 5.0 5.0 5.0 Recall Mode Min Min Min None None Min None None None None Walk Time (s) 7.0 7.0 7.0 7.0 7.0 7.0 Flash Dont Walk (s) 15.0 19.0 19.0 11.0 15.0 11.0 Pedestrian Calls (#/hr) 0 0 0 0 0 35.4 35.4 10.1 46.7 46.7 22.3 22.3 Act Effct Green (s) Actuated g/C Ratio 0.13 0.60 0.60 0.46 0.46 0.29 0.29 0.47 0.60 0.05 0.83 0.03 0.74 0.41 v/c Ratio Control Delay 25.1 50.4 41.5 10.4 0.5 0.1 9.7 Queue Delay 0.0 0.0 0.0 0.0 0.0 0.0 0.0 **Total Delay** 41.5 10.4 25.1 0.1 50.4 9.7 0.5 LOS D В С D Α Approach Delay 12.5 24.6 50.4 9.7 В С D Approach LOS Α Intersection Summary Other Area Type: Cycle Length: 99 Actuated Cycle Length: 77.3) Natural Cycle: 65 Control Type: Actuated-Uncoordinated Maximum v/c Ratio: 0.83 Intersection Signal Delay: 20.0 Intersection LOS: B ICU Level of Service D Intersection Capacity Utilization 75.2% Analysis Period (min) 15 Splits and Phases: 3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26) Ø1 Ø4 **≯**_{Ø5} ¶ ø8 Ø6 2029 With Project Conditions - Weekday AM Peak Hour + With WB Right-Turn Lane Page 2

Queues

3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26)

03/10/2020

	•	\rightarrow	•	•	•	†	↓
Lane Group	EBL	EBT	EBR	WBT	WBR	NBT	SBT
Lane Group Flow (vph)	96	1067	42	1202	24	180	217
v/c Ratio	0.47	0.60	0.05	0.83	0.03	0.74	0.41
Control Delay	41.5	10.4	0.5	25.1	0.1	50.4	9.7
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	41.5	10.4	0.5	25.1	0.1	50.4	9.7
Queue Length 50th (ft)	48	143	0	267	0	89	15
Queue Length 95th (ft)	95	191	4	391	0	#219	76
Internal Link Dist (ft)		388		222		285	130
Turn Bay Length (ft)	115		100				
Base Capacity (vph)	536	2101	1001	1698	801	244	527
Starvation Cap Reductn	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0
Reduced v/c Ratio	0.18	0.51	0.04	0.71	0.03	0.74	0.41
Intersection Summary							

^{# 95}th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis 3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26)

021	10	Inc	200
03/	ΙU	IΖU	ZU

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	^	7	ሻ	^	7		4			4	
Traffic Volume (vph)	88	982	39	0	1106	22	160	4	2	26	8	166
Future Volume (vph)	88	982	39	0	1106	22	160	4	2	26	8	166
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Total Lost time (s)	4.0	4.0	4.0		4.0	4.0		4.0			4.0	
Lane Util. Factor	1.00	0.95	1.00		0.95	1.00		1.00			1.00	
Frpb, ped/bikes	1.00	1.00	0.98		1.00	0.98		1.00			0.99	
Flpb, ped/bikes	1.00	1.00	1.00		1.00	1.00		1.00			1.00	
Frt	1.00	1.00	0.85		1.00	0.85		1.00			0.89	
Flt Protected	0.95	1.00	1.00		1.00	1.00		0.95			0.99	
Satd. Flow (prot)	1568	2942	1373		3167	1428		1601			1455	
Flt Permitted	0.95	1.00	1.00		1.00	1.00		0.50			0.95	
Satd. Flow (perm)	1568	2942	1373		3167	1428		846			1386	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	96	1067	42	0	1202	24	174	4	2	28	9	180
RTOR Reduction (vph)	0	0	16	0	0	13	0	1	0	0	129	0
Lane Group Flow (vph)	96	1067	26	0	1202	11	0	179	0	0	88	0
Confl. Peds. (#/hr)			1	1			2					2
Confl. Bikes (#/hr)			•	•		1	_					_
Heavy Vehicles (%)	6%	13%	6%	2%	5%	2%	4%	2%	2%	24%	2%	2%
Turn Type	Prot	NA	Perm	Prot	NA	Perm	Perm	NA		Perm	NA	
Protected Phases	5	2	1 01111	1	6	1 01111	1 01111	8		1 01111	4	
Permitted Phases	•	_	2	•	· ·	6	8	•		4	•	
Actuated Green, G (s)	8.0	47.9	47.9		35.4	35.4	Ū	20.7			20.7	
Effective Green, g (s)	8.5	47.9	47.9		35.4	35.4		22.2			22.2	
Actuated g/C Ratio	0.11	0.61	0.61		0.45	0.45		0.28			0.28	
Clearance Time (s)	4.5	4.0	4.0		4.0	4.0		5.5			5.5	
Vehicle Extension (s)	2.3	5.8	5.8		5.8	5.8		2.5			2.5	
Lane Grp Cap (vph)	170	1804	842		1435	647		240			393	
v/s Ratio Prot	0.06	c0.36	042		c0.38	041		240			000	
v/s Ratio Perm	0.00	60.50	0.02		0 0.00	0.01		c0.21			0.06	
v/c Ratio	0.56	0.59	0.02		0.84	0.02		0.75			0.00	
Uniform Delay, d1	33.0	9.2	6.0		18.8	11.8		25.4			21.4	
Progression Factor	1.00	1.00	1.00		1.00	1.00		1.00			1.00	
Incremental Delay, d2	3.1	0.9	0.0		5.1	0.0		11.4			0.2	
Delay (s)	36.1	10.1	6.0		23.9	11.8		36.8			21.6	
Level of Service	D	В	Α		23.3 C	11.0 B		50.0 D			21.0 C	
Approach Delay (s)		12.0			23.7			36.8			21.6	
Approach LOS		В			C			D			C	
Intersection Summary												
HCM 2000 Control Delay			19.4	H	CM 2000	Level of	Service		В			
HCM 2000 Volume to Capac	city ratio		0.80									
Actuated Cycle Length (s)			78.1	Sı	um of lost	time (s)			12.0			
Intersection Capacity Utiliza	tion		75.2%	IC	U Level	of Service	1		D			
Analysis Period (min)			15									
c Critical Lane Group												

Sandy Health Clinic - PBS Project 71524.000

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2029 With Project Conditions - Weekday AM Peak Hour + With WB Right-Turn Lane

HCM 6th Signalized Intersection Summary

3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26)

03/10/2020

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	^	7	Ť	^	7		4			4	
Traffic Volume (veh/h)	88	982	39	0	1106	22	160	4	2	26	8	166
Future Volume (veh/h)	88	982	39	0	1106	22	160	4	2	26	8	166
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1668	1573	1668	1723	1682	1723	1723	1723	1723	1723	1723	1723
Adj Flow Rate, veh/h	96	1067	8	0	1202	0	174	4	1	28	9	41
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	6	13	6	2	5	2	2	2	2	2	2	2
Cap, veh/h	135	2083	985	3	1786	816	351	6	1	143	63	146
Arrive On Green	0.08	0.70	0.70	0.00	0.56	0.00	0.15	0.17	0.15	0.15	0.17	0.15
Sat Flow, veh/h	1589	2988	1413	1641	3195	1460	1400	32	8	394	367	843
Grp Volume(v), veh/h	96	1067	8	0	1202	0	179	0	0	78	0	0
Grp Sat Flow(s),veh/h/ln	1589	1494	1413	1641	1598	1460	1440	0	0	1604	0	0
Q Serve(g_s), s	3.9	11.0	0.1	0.0	17.4	0.0	4.9	0.0	0.0	0.0	0.0	0.0
Cycle Q Clear(g_c), s	3.9	11.0	0.1	0.0	17.4	0.0	7.8	0.0	0.0	2.9	0.0	0.0
Prop In Lane	1.00	11.0	1.00	1.00		1.00	0.97	0.0	0.01	0.36	0.0	0.53
Lane Grp Cap(c), veh/h	135	2083	985	3	1786	816	325	0	0.01	316	0	0.00
V/C Ratio(X)	0.71	0.51	0.01	0.00	0.67	0.00	0.55	0.00	0.00	0.25	0.00	0.00
Avail Cap(c_a), veh/h	618	2280	1078	388	1951	891	535	0.00	0.00	542	0.00	0.00
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	0.00	1.00	0.00	1.00	0.00	0.00	1.00	0.00	0.00
Uniform Delay (d), s/veh	29.2	4.7	3.0	0.0	10.2	0.0	26.1	0.0	0.0	24.2	0.0	0.0
Incr Delay (d2), s/veh	4.2	0.6	0.0	0.0	1.6	0.0	1.1	0.0	0.0	0.3	0.0	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	1.6	2.6	0.0	0.0	5.6	0.0	2.8	0.0	0.0	1.1	0.0	0.0
Unsig. Movement Delay, s/veh		2.0	0.0	0.0	5.0	0.0	2.0	0.0	0.0	1.1	0.0	0.0
LnGrp Delay(d),s/veh	33.4	5.3	3.0	0.0	11.8	0.0	27.2	0.0	0.0	24.5	0.0	0.0
LnGrp LOS	33.4 C	5.5 A	3.0 A	Α	11.0 B	Α	21.2 C	Α	0.0 A	24.5 C	Α	
		1171	^		1202			179			78	A
Approach Vol, veh/h												
Approach Delay, s/veh		7.6			11.8			27.2			24.5	
Approach LOS		Α			В			С			С	
Timer - Assigned Phs	1	2		4	5	6		8				
Phs Duration (G+Y+Rc), s	0.0	50.2		15.3	9.6	40.6		15.3				
Change Period (Y+Rc), s	4.5	* 4.5		5.5	4.5	4.0		5.5				
Max Green Setting (Gmax), s	15.0	* 50		20.0	25.0	40.0		20.0				
Max Q Clear Time (g_c+l1), s	0.0	13.0		4.9	5.9	19.4		9.8				
Green Ext Time (p_c), s	0.0	25.6		0.2	0.3	17.2		0.5				
Intersection Summary												
HCM 6th Ctrl Delay			11.4									
HCM 6th LOS			B									
			٥									
Notes												

^{*} HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Sandy Health Clinic - PBS Project 71524.000

Synchro 10 Report - by PBS Engineering and Environmental

2029 With Project Conditions - Weekday AM Peak Hour + With WB Right-Turn Lane

Lanes, Volumes, Timings

3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26)

03/10/2020

	•	→	*	€	+	•	•	†	<i>></i>	/	+	-√
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	^	7	ሻ	^	7		4			4	
Traffic Volume (vph)	186	1292	158	6	1331	20	137	28	8	47	27	138
Future Volume (vph)	186	1292	158	6	1331	20	137	28	8	47	27	138
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Storage Length (ft)	115		100	105		100	0		0	0		0
Storage Lanes	1		1	1		0	0		0	0		0
Taper Length (ft)	100		•	100		•	100			100		
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor	1.00		0.98	1.00		0.98		1.00			0.99	
Frt			0.850			0.850		0.993			0.912	
Flt Protected	0.950		0.000	0.950		0.000		0.962			0.989	
Satd. Flow (prot)	1630	3228	1458	1630	3167	1458	0	1623	0	0	1526	0
Flt Permitted	0.950	0220	1100	0.950	0101	1100		0.477	•	Ū	0.909	J
Satd. Flow (perm)	1629	3228	1426	1629	3167	1423	0	802	0	0	1403	0
Right Turn on Red	1020	0220	Yes	1020	0101	Yes	· ·	002	Yes	Ū	1100	Yes
Satd. Flow (RTOR)			95			69		2			75	
Link Speed (mph)		25			25			25			25	
Link Distance (ft)		468			302			365			210	
Travel Time (s)		12.8			8.2			10.0			5.7	
Confl. Peds. (#/hr)	2		1	1	V. <u>–</u>	2	5					5
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Heavy Vehicles (%)	2%	3%	2%	2%	5%	2%	2%	8%	2%	2%	2%	2%
Adj. Flow (vph)	198	1374	168	6	1416	21	146	30	9	50	29	147
Shared Lane Traffic (%)												, , ,
Lane Group Flow (vph)	198	1374	168	6	1416	21	0	185	0	0	226	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	L NA	R NA	R NA	L NA	R NA	R NA	L NA	R NA	R NA	L NA	R NA	R NA
Median Width(ft)		12			12			0			0	
Link Offset(ft)		-12			0			0			4	
Crosswalk Width(ft)		72			42			32			30	
Two way Left Turn Lane												
Headway Factor	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11	1.11
Number of Detectors	2	1	1	2	1	1	2	2		2	2	
Detector Template	Left	Det25	Right	Left	Det25	Right	Left	Side St		Left	Side St	
Leading Detector (ft)	78	153	153	78	153	153	78	78		78	78	
Trailing Detector (ft)	2	137	137	2	137	137	2	2		2	2	
Detector 1 Position(ft)	2	137	137	2	137	137	2	2		2	2	
Detector 1 Size(ft)	16	16	16	16	16	16	16	16		16	16	
Detector 1 Type	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex	CI+Ex		CI+Ex	CI+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 2 Position(ft)	72			72			72	72		72	72	
Detector 2 Size(ft)	6			6			6	6		6	6	
Detector 2 Type	CI+Ex			CI+Ex			CI+Ex	CI+Ex		CI+Ex	CI+Ex	
Detector 2 Channel												
Detector 2 Extend (s)	0.0			0.0			0.0	0.0		0.0	0.0	
Turn Type	Prot	NA	Perm	Prot	NA	Perm	Perm	NA		Perm	NA	

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Synchro 10 Report - by PBS Engineering and Environmental

2029 With Project Trips Conditions - Weekday PM Peak Hour + With Right-Turn Lane

Lanes, Volumes, Timings 3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26) 03/10/2020 ٠ t WBR **NBT NBR** SBL **EBL EBT EBR WBT** Lane Group SBT Protected Phases 2 5 Permitted Phases 2 6 8 **Detector Phase** 5 2 2 6 8 8 4 4 Switch Phase 10.0 10.0 10.0 10.0 6.0 Minimum Initial (s) 4.0 4.0 6.0 6.0 6.0 Minimum Split (s) 8.5 26.0 26.0 8.5 30.0 30.0 11.5 11.5 23.5 23.5 Total Split (s) 20.0 70.0 70.0 15.0 65.0 65.0 25.0 25.0 25.0 25.0 18.2% 63.6% 59.1% 59.1% 22.7% 22.7% Total Split (%) 63.6% 13.6% 22.7% 22.7% Maximum Green (s) 15.5 66.0 66.0 10.5 61.0 61.0 19.5 19.5 19.5 19.5 Yellow Time (s) 3.5 3.5 3.5 3.5 3.5 3.5 4.0 4.0 4.0 4.0 1.5 All-Red Time (s) 1.0 0.5 0.5 1.0 0.5 0.5 1.5 1.5 1.5 Lost Time Adjust (s) -0.5 0.0 0.0 -0.5 0.0 0.0 -1.5 -1.5 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 Total Lost Time (s) Lead/Lag Lead Lead Lead Lag Lag Lag Lead-Lag Optimize? Vehicle Extension (s) 2.3 5.8 5.8 2.3 5.8 5.8 2.5 2.5 2.5 2.5 Minimum Gap (s) 0.5 3.8 3.8 0.5 3.8 3.8 2.0 2.0 2.0 2.0 Time Before Reduce (s) 5.0 8.0 10.0 10.0 8.0 10.0 10.0 5.0 5.0 5.0 Time To Reduce (s) 3.0 5.0 5.0 5.0 5.0 10.0 10.0 3.0 10.0 10.0 Recall Mode None C-Min C-Min None C-Min C-Min None None None None Walk Time (s) 7.0 7.0 7.0 7.0 7.0 7.0 Flash Dont Walk (s) 15.0 15.0 19.0 19.0 11.0 11.0 Pedestrian Calls (#/hr) 0 0 0 0 0 Act Effct Green (s) 16.0 75.4 75.4 5.8 57.5 57.5 24.5 24.5 Actuated g/C Ratio 0.15 0.69 0.69 0.05 0.52 0.52 0.22 0.22 0.84 0.03 v/c Ratio 0.62 0.17 0.07 0.86 1.03 0.61 74.8 120.1 Control Delay 11.2 50.8 28.5 0.1 34.8 3.1 0.0 0.0 0.0 0.0 0.0 0.0 Queue Delay 0.0 0.0 Total Delay 74.8 11.2 3.1 50.8 28.5 0.1 120.1 34.8 LOS Ε В Α D C Α F C Approach Delay 17.6 28.1 120.1 34.8 Approach LOS В С F С Intersection Summary Area Type: Other Cycle Length: 110 Actuated Cycle Length: 110 Offset: 0 (0%), Referenced to phase 2:EBT and 6:WBT, Start of Green, Master Intersection Natural Cycle: 90 Control Type: Actuated-Coordinated Maximum v/c Ratio: 1.03 Intersection Signal Delay: 28.2 Intersection LOS: C Intersection Capacity Utilization 88.9% ICU Level of Service E Analysis Period (min) 15 3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26) Splits and Phases: Ø1 04 ₩ Ø2 (R) **≯** ø5 T Ø8 Ø6 (R) 2029 With Project Trips Conditions - Weekday PM Peak Hour + With Right-Turn Lane Page 2

Queues

3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26)

03/10/2020

	•	-	*	•	←	•	†	ļ	
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBT	SBT	
Lane Group Flow (vph)	198	1374	168	6	1416	21	185	226	
v/c Ratio	0.84	0.62	0.17	0.07	0.86	0.03	1.03	0.61	
Control Delay	74.8	11.2	3.1	50.8	28.5	0.1	120.1	34.8	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	74.8	11.2	3.1	50.8	28.5	0.1	120.1	34.8	
Queue Length 50th (ft)	135	201	12	4	418	0	~158	100	
Queue Length 95th (ft)	#263	369	44	18	504	0	#304	#190	
Internal Link Dist (ft)		388			222		285	130	
Turn Bay Length (ft)	115		100	105		100			
Base Capacity (vph)	245	2211	1007	163	1756	819	179	370	
Starvation Cap Reductn	0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.81	0.62	0.17	0.04	0.81	0.03	1.03	0.61	

Intersection Summary

Queue shown is maximum after two cycles.

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Synchro 10 Report - by PBS Engineering and Environmental rurn Lane Page 3

2029 With Project Trips Conditions - Weekday PM Peak Hour + With Right-Turn Lane

Volume exceeds capacity, queue is theoretically infinite.
 Queue shown is maximum after two cycles.

^{# 95}th percentile volume exceeds capacity, queue may be longer.

HCM Signalized Intersection Capacity Analysis 3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26)

03/10/2020

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ች	^	7	7	^	7		4			4	
Traffic Volume (vph)	186	1292	158	6	1331	20	137	28	8	47	27	138
Future Volume (vph)	186	1292	158	6	1331	20	137	28	8	47	27	138
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0		4.0			4.0	
Lane Util. Factor	1.00	0.95	1.00	1.00	0.95	1.00		1.00			1.00	
Frpb, ped/bikes	1.00	1.00	0.98	1.00	1.00	0.98		1.00			0.99	
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00		1.00			1.00	
Frt	1.00	1.00	0.85	1.00	1.00	0.85		0.99			0.91	
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00		0.96			0.99	
Satd. Flow (prot)	1630	3228	1426	1630	3167	1423		1619			1527	
FIt Permitted	0.95	1.00	1.00	0.95	1.00	1.00		0.48			0.91	
Satd. Flow (perm)	1630	3228	1426	1630	3167	1423		803			1404	
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Adj. Flow (vph)	198	1374	168	6	1416	21	146	30	9	50	29	147
RTOR Reduction (vph)	0	0	33	0	0	11	0	2	0	0	58	0
Lane Group Flow (vph)	198	1374	135	6	1416	10	0	183	0	0	168	0
Confl. Peds. (#/hr)	2		1	1		2	5					5
Heavy Vehicles (%)	2%	3%	2%	2%	5%	2%	2%	8%	2%	2%	2%	2%
Turn Type	Prot	NA	Perm	Prot	NA	Perm	Perm	NA		Perm	NA	
Protected Phases	5	2		1	6	-		8			4	
Permitted Phases			2			6	8			4		
Actuated Green, G (s)	19.2	71.7	71.7	1.3	53.8	53.8		23.0			23.0	
Effective Green, g (s)	19.7	71.7	71.7	1.8	53.8	53.8		24.5			24.5	
Actuated g/C Ratio	0.18	0.65	0.65	0.02	0.49	0.49		0.22			0.22	
Clearance Time (s)	4.5	4.0	4.0	4.5	4.0	4.0		5.5			5.5	
Vehicle Extension (s)	2.3	5.8	5.8	2.3	5.8	5.8		2.5			2.5	
Lane Grp Cap (vph)	291	2104	929	26	1548	695		178			312	
v/s Ratio Prot	0.12	c0.43		0.00	c0.45							
v/s Ratio Perm			0.09			0.01		c0.23			0.12	
v/c Ratio	0.68	0.65	0.15	0.23	0.91	0.01		1.03			0.54	
Uniform Delay, d1	42.2	11.6	7.4	53.4	26.0	14.5		42.8			37.7	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00		1.00			1.00	
Incremental Delay, d2	5.6	1.6	0.3	2.6	9.9	0.0		75.7			1.4	
Delay (s)	47.8	13.2	7.7	56.1	35.9	14.5		118.5			39.1	
Level of Service	D	В	Α	Е	D	В		F			D	
Approach Delay (s)		16.6			35.7			118.5			39.1	
Approach LOS		В			D			F			D	
Intersection Summary												
HCM 2000 Control Delay			30.9	Н	CM 2000	Level of	Service		С			
HCM 2000 Volume to Capacit	ty ratio		0.91									
Actuated Cycle Length (s)	•		110.0	S	um of lost	time (s)			12.0			
Intersection Capacity Utilization	on		88.9%	IC	CU Level	of Service			E			
Analysis Period (min)			15									
c Critical Lane Group												

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2029 With Project Trips Conditions - Weekday PM Peak Hour + With Right-Turn Lane

HCM 6th Signalized Intersection Summary

3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26)

03/10/2020

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Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	75	^	7	7	^	7		4			4	
Traffic Volume (veh/h)	186	1292	158	6	1331	20	137	28	8	47	27	138
Future Volume (veh/h)	186	1292	158	6	1331	20	137	28	8	47	27	138
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		0.99	1.00		0.99
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1723	1709	1723	1723	1682	1723	1641	1641	1641	1723	1723	1723
Adj Flow Rate, veh/h	198	1374	137	6	1416	0	146	30	7	50	29	84
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Percent Heavy Veh, %	2	3	2	2	5	2	8	8	8	2	2	2
Cap, veh/h	264	2224	998	17	1722	787	219	35	8	115	74	156
Arrive On Green	0.16	0.68	0.68	0.01	0.54	0.00	0.18	0.19	0.18	0.18	0.19	0.18
Sat Flow, veh/h	1641	3247	1458	1641	3195	1460	839	185	41	379	390	818
Grp Volume(v), veh/h	198	1374	137	6	1416	0	183	0	0	163	0	0
Grp Sat Flow(s), veh/h/ln	1641	1624	1458	1641	1598	1460	1064	0	0	1587	0	0
Q Serve(g_s), s	12.7	25.4	3.6	0.4	40.4	0.0	8.6	0.0	0.0	0.0	0.0	0.0
Cycle Q Clear(g_c), s	12.7	25.4	3.6	0.4	40.4	0.0	18.9	0.0	0.0	10.4	0.0	0.0
Prop In Lane	1.00	20.4	1.00	1.00	70.7	1.00	0.80	0.0	0.04	0.31	0.0	0.52
Lane Grp Cap(c), veh/h	264	2224	998	1.00	1722	787	247	0	0.04	324	0	0.52
V/C Ratio(X)	0.75	0.62	0.14	0.34	0.82	0.00	0.74	0.00	0.00	0.50	0.00	0.00
Avail Cap(c a), veh/h	264	2224	998	164	1772	810	247	0.00	0.00	324	0.00	0.00
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	0.00	1.00	0.00	0.00	1.00	0.00	0.00
Uniform Delay (d), s/veh	44.0	9.5	6.0	54.0	21.0	0.00	45.1	0.00	0.00	40.8	0.00	0.00
Incr Delay (d2), s/veh	10.6	1.3	0.0	7.0	4.6	0.0	10.6	0.0	0.0	0.9	0.0	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	0.0
						0.0			0.0			
%ile BackOfQ(50%),veh/ln	5.9	8.7	1.1	0.2	15.6	0.0	5.8	0.0	0.0	4.2	0.0	0.0
Unsig. Movement Delay, s/veh		40.0	C 2	C4 0	05.0	0.0	FF 0	0.0	0.0	44.7	0.0	0.0
LnGrp Delay(d),s/veh	54.6	10.8	6.3	61.0	25.6	0.0	55.8	0.0	0.0	41.7	0.0	0.0
LnGrp LOS	D	В	A	E	С	A	E	Α	A	D	A	A
Approach Vol, veh/h		1709			1422			183			163	
Approach Delay, s/veh		15.5			25.7			55.8			41.7	
Approach LOS		В			С			Е			D	
Timer - Assigned Phs	1	2		4	5	6		8				
Phs Duration (G+Y+Rc), s	5.2	79.8		25.0	21.7	63.3		25.0				
Change Period (Y+Rc), s	4.5	* 4.5		5.5	4.5	4.0		5.5				
Max Green Setting (Gmax), s	10.5	* 66		19.5	15.5	61.0		19.5				
Max Q Clear Time (g_c+l1), s	2.4	27.4		12.4	14.7	42.4		20.9				
Green Ext Time (p_c), s	0.0	33.5		0.3	0.1	16.9		0.0				
Intersection Summary												
HCM 6th Ctrl Delay			23.0									
HCM 6th LOS			C									
Notes												

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Sandy Health Clinic - PBS Project 71524.000

Synchro 10 Report - by PBS Engineering and Environmental

2029 With Project Trips Conditions - Weekday PM Peak Hour + With Right-Turn Lane

1: Ten Eyck Road & Pleasant Street (W) Performance by movement

Movement	EBL	EBR	NBL	NBT	SBT	SBR	All
Vehicles Entered	17	25	18	56	163	24	303
Vehicles Exited	17	25	18	56	163	24	303
Hourly Exit Rate	17	25	18	56	163	24	303
Input Volume	18	24	19	54	169	21	306
% of Volume	94	105	94	103	97	113	99

2: Ten Eyck Road & Pleasant Street (E) Performance by movement

Movement	NBT	NBR	SBT	All
Vehicles Entered	78	10	192	280
Vehicles Exited	78	10	192	280
Hourly Exit Rate	78	10	192	280
Input Volume	78	11	196	284
% of Volume	101	93	98	99

3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26) Performance by movement

Movement	EBL	EBT	EBR	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	All
Vehicles Entered	63	958	46	1086	20	162	4	2	23	12	157	2533
Vehicles Exited	63	959	46	1087	20	162	4	2	23	12	157	2535
Hourly Exit Rate	63	959	46	1087	20	162	4	2	23	12	157	2535
Input Volume	64	982	39	1106	19	160	4	2	25	13	159	2572
% of Volume	98	98	118	98	107	101	100	100	93	91	99	99

Total Network Performance

Vehicles Entered	2579
Vehicles Exited	2582
Hourly Exit Rate	2582
Input Volume	7900
% of Volume	33

Sandy Health Clinic - PBS Project 71524.000 2029 Without Project Conditions - Weekday AM Peak Hour SimTraffic 10 Report - by PBS Page 1

Intersection: 1: Ten Eyck Road & Pleasant Street (W)

Movement	EB	NB	SB
Directions Served	LR	LT	TR
Maximum Queue (ft)	70	63	40
Average Queue (ft)	25	7	1
95th Queue (ft)	52)	35	19
Link Distance (ft)	624	59	201
Upstream Blk Time (%)		0	
Queuing Penalty (veh)		0	
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 2: Ten Eyck Road & Pleasant Street (E)

Movement	NB	SB
Directions Served	TR	LT
Maximum Queue (ft)	3	71
Average Queue (ft)	0	8
95th Queue (ft)	3	41
Link Distance (ft)	105	59
Upstream Blk Time (%)		1
Queuing Penalty (veh)		2
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

Intersection: 3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26)

Movement	EB	EB	EB	EB	WB	WB	B8	B8	NB	SB	
Directions Served	L	Т	Т	R	Т	TR	Т	Т	LTR	LTR	
Maximum Queue (ft)	152	291	233	106	283	267	108	68	183	143	
Average Queue (ft)	33	142	94	15	174	162	9	3	84	83	
95th Queue (ft)	94	249	189	59	267	252	(55	30	151	140	
Link Distance (ft)		349	349		199	199	1221	1221	285	105	
Upstream Blk Time (%)		0			4	3				6	
Queuing Penalty (veh)		0			0	0				12	
Storage Bay Dist (ft)	115			100							
Storage Blk Time (%)	0	7	3		19						
Queuing Penalty (veh)	1	5	1		0						

Queuing and Blocking Report 2029 Without Project Conditions - Weekday AM Peak Hour

03/11/2020

Intersection: 8: Bend

Movement	EB
Directions Served	Т
Maximum Queue (ft)	6
Average Queue (ft)	0
95th Queue (ft)	6
Link Distance (ft)	199
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Network Summary

Network wide Queuing Penalty: 21

1: Ten Eyck Road & Pleasant Street (W) Performance by movement

Movement	EBL	EBR	NBL	NBT	SBT	SBR	All
Vehicles Entered	52	48	14	194	140	15	463
Vehicles Exited	52	48	14	194	141	15	464
Hourly Exit Rate	52	48	14	194	141	15	464
Input Volume	53	48	17	198	138	13	466
% of Volume	98	101	82	98	102	118	100

2: Ten Eyck Road & Pleasant Street (E) Performance by movement

Marramant	WDI	WDD	NDT	NDD	CDI	CDT	ΛII
Movement	WBL	WBR	NBT	NBR	SBL	SBT	All
Vehicles Entered	12	6	204	20	4	187	433
Vehicles Exited	12	6	205	21	4	187	435
Hourly Exit Rate	12	6	205	21	4	187	435
Input Volume	12	5	211	21	4	184	437
% of Volume	102	114	97	100	94	102	100

3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26) Performance by movement

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Vehicles Entered	167	1289	160	6	1333	19	137	31	8	44	34	120
Vehicles Exited	165	1289	161	6	1338	19	139	32	8	44	34	120
Hourly Exit Rate	165	1289	161	6	1338	19	139	32	8	44	34	120
Input Volume	178	1292	158	6	1331	19	137	28	8	45	33	118
% of Volume	93	100	102	100	101	99	101	115	97	98	103	102

3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26) Performance by movement

Movement	All
Vehicles Entered	3348
Vehicles Exited	3355
Hourly Exit Rate	3355
Input Volume	3354
% of Volume	100

Total Network Performance

Vehicles Entered	3433
Vehicles Exited	3436
Hourly Exit Rate	3436
Input Volume	10386
% of Volume	33

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Intersection: 1: Ten Eyck Road & Pleasant Street (W)

Movement	EB	NB	SB
Directions Served	LR	LT	TR
Maximum Queue (ft)	100	58	75
Average Queue (ft)	39	3	5
95th Queue (ft)	72)	26	36
Link Distance (ft)	624	59	201
Upstream Blk Time (%)		0	
Queuing Penalty (veh)		0	
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 2: Ten Eyck Road & Pleasant Street (E)

Movement	WB	NB	SB
Directions Served	LR	TR	LT
Maximum Queue (ft)	44	9	91
Average Queue (ft)	15	0	25
95th Queue (ft)	41)	7	78
Link Distance (ft)	599	106	59
Upstream Blk Time (%)			7
Queuing Penalty (veh)			12
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26)

Movement	EB	EB	EB	EB	WB	WB	WB	B8	B8	NB	SB	
Directions Served	L	Т	Т	R	L	T	TR	Т	Т	LTR	LTR	
Maximum Queue (ft)	214	437	402	200	67	292	289	365	342	297	142	
Average Queue (ft)	127	215	186	52	7	257	247	111	84	153	109	
95th Queue (ft)	224	368	340	159	39	308	314	280	246)	268	157	
Link Distance (ft)		700	700			200	200	1221	1221	882	106	
Upstream Blk Time (%)					0	24	20				25	
Queuing Penalty (veh)					0	0	0				49	
Storage Bay Dist (ft)	115			100	105							
Storage Blk Time (%)	15	14	13	0		36						
Queuing Penalty (veh)	98	25	20	0		2						

Network Summary

Network wide Queuing Penalty: 208

1: Ten Eyck Road & Pleasant Street (W) Performance by movement

Movement	EBL	EBR	NBL	NBT	SBT	SBR	All
Vehicles Entered	16	24	17	58	166	24	305
Vehicles Exited	16	24	17	58	167	25	307
Hourly Exit Rate	16	24	17	58	167	25	307
Input Volume	18	24	19	54	169	21	306
% of Volume	89	101	88	106	99	118	100

2: Ten Eyck Road & Pleasant Street (E) Performance by movement

Movement	WBL	NBT	NBR	SBT	All
Vehicles Entered	7	79	38	195	319
Vehicles Exited	7	78	38	195	318
Hourly Exit Rate	7	78	38	195	318
Input Volume	8	78	38	196	320
% of Volume	90	100	99	100	100

3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26) Performance by movement

Movement	EBL	EBT	EBR	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	All
Vehicles Entered	87	974	40	1102	23	160	4	2	25	14	163	2594
Vehicles Exited	87	977	40	1103	23	161	4	2	25	15	163	2600
Hourly Exit Rate	87	977	40	1103	23	161	4	2	25	15	163	2600
Input Volume	88	982	39	1106	22	160	4	2	26	14	166	2608
% of Volume	99	99	103	100	106	101	100	100	97	105	98	100

Total Network Performance

Vehicles Entered	2639
Vehicles Exited	2650
Hourly Exit Rate	2650
Input Volume	8011
% of Volume	33

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Intersection: 1: Ten Eyck Road & Pleasant Street (W)

Movement	EB	NB	SB
Directions Served	LR	LT	TR
Maximum Queue (ft)	55	52	12
Average Queue (ft)	23	5	1
95th Queue (ft)	49	28	8
Link Distance (ft)	624	59	201
Upstream Blk Time (%)		0	
Queuing Penalty (veh)		0	
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 2: Ten Eyck Road & Pleasant Street (E)

Movement	WB	NB	SB
Directions Served	LR	TR	LT
Maximum Queue (ft)	33	9	75
Average Queue (ft)	7	0	8
95th Queue (ft)	28)	6	42
Link Distance (ft)	599	105	59
Upstream Blk Time (%)			1
Queuing Penalty (veh)			2
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26)

Movement	EB	EB	EB	EB	WB	WB	B8	B8	NB	SB	
Directions Served	L	T	Т	R	Т	TR	Т	Т	LTR	LTR	
Maximum Queue (ft)	149	310	233	55	272	269	98	75	204	141	
Average Queue (ft)	44	143	96	12	190	173	11	5	91	85	
95th Queue (ft)	109	251	193	40	283	267	(56	37)	164	143	
Link Distance (ft)		349	349		199	199	1221	1221	285	105	
Upstream Blk Time (%)		0			6	4			0	7	
Queuing Penalty (veh)		0			0	0			0	13	
Storage Bay Dist (ft)	115			100							
Storage Blk Time (%)	0	7	3	0	22						
Queuing Penalty (veh)	2	7	1	0	0						

Queuing and Blocking Report 2029 With Project Conditions - Weekday AM Peak Hour

03/11/2020

Intersection: 8: Bend

Movement	EB
Directions Served	Т
Maximum Queue (ft)	6
Average Queue (ft)	0
95th Queue (ft)	6
Link Distance (ft)	199
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Network Summary

Network wide Queuing Penalty: 25

1: Ten Eyck Road & Pleasant Street (W) Performance by movement

Movement	EBL	EBR	NBL	NBT	SBT	SBR	All
Vehicles Entered	56	46	18	196	142	12	470
Vehicles Exited	56	46	18	196	143	12	471
Hourly Exit Rate	56	46	18	196	143	12	471
Input Volume	53	48	17	198	138	13	466
% of Volume	106	96	106	99	103	94	101

2: Ten Eyck Road & Pleasant Street (E) Performance by movement

Mayamant	WDI	WBR	NDT	NDD	CDI	CDT	ΛII
Movement	WBL	WBR	NBT	NBR	SBL	SBT	All
Vehicles Entered	33	7	208	27	4	186	465
Vehicles Exited	34	6	210	27	4	186	467
Hourly Exit Rate	34	6	210	27	4	186	467
Input Volume	34	5	212	30	4	184	468
% of Volume	100	114	99	91	94	101	100

3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26) Performance by movement

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Vehicles Entered	180	1297	162	5	1350	20	136	29	8	45	35	141
Vehicles Exited	179	1298	163	5	1354	20	137	29	8	45	36	141
Hourly Exit Rate	179	1298	163	5	1354	20	137	29	8	45	36	141
Input Volume	186	1292	158	6	1331	20	137	28	8	47	34	138
% of Volume	96	100	103	83	102	99	100	105	97	96	107	102

3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26) Performance by movement

Movement	All
Vehicles Entered	3408
Vehicles Exited	3415
Hourly Exit Rate	3415
Input Volume	3386
% of Volume	101

Total Network Performance

Vehicles Entered	3490
Vehicles Exited	3494
Hourly Exit Rate	3494
Input Volume	10484
% of Volume	33

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Intersection: 1: Ten Eyck Road & Pleasant Street (W)

Movement	EB	NB	SB
Directions Served	LR	LT	TR
Maximum Queue (ft)	131	46	96
Average Queue (ft)	42	4	9
95th Queue (ft)	97)	28	(56)
Link Distance (ft)	624	59	201
Upstream Blk Time (%)		0	0
Queuing Penalty (veh)		0	0
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 2: Ten Eyck Road & Pleasant Street (E)

Movement	WB	NB	SB
Directions Served	LR	TR	LT
Maximum Queue (ft)	128	24	89
Average Queue (ft)	39	1	32
95th Queue (ft)	102	16	88
Link Distance (ft)	599	106	59
Upstream Blk Time (%)		0	10
Queuing Penalty (veh)		0	20
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26)

Movement	EB	EB	EB	EB	WB	WB	WB	B8	B8	NB	SB	
Directions Served	L	T	Т	R	L	T	TR	Т	Т	LTR	LTR	
Maximum Queue (ft)	215	556	500	200	126	304	307	371	336	284	145	
Average Queue (ft)	147	254	213	58	8	259	249	116	90	145	115	
95th Queue (ft)	243	470	419	170	54	316	314	284	251	248	158	
Link Distance (ft)		700	700			200	200	1221	1221	882	106	
Upstream Blk Time (%)		0	0		0	24	20				34	
Queuing Penalty (veh)		0	0		0	0	0				76	
Storage Bay Dist (ft)	115			100	105							
Storage Blk Time (%)	23	15	14	0	0	35						
Queuing Penalty (veh)	148	28	22	0	0	2						

Queuing and Blocking Report 2029 With Project Trips Conditions - Weekday PM Peak Hour

03/11/2020

Intersection: 8: Bend

Movement	EB	EB
Directions Served	Т	T
Maximum Queue (ft)	6	11
Average Queue (ft)	0	0
95th Queue (ft)	6	8
Link Distance (ft)	200	200
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

Network Summary

Network wide Queuing Penalty: 297

1: Ten Eyck Road & Pleasant Street (W) Performance by movement

Movement	EBL	EBR	NBL	NBT	SBT	SBR	All
Vehicles Entered	18	26	17	56	172	24	313
Vehicles Exited	18	26	17	56	172	24	313
Hourly Exit Rate	18	26	17	56	172	24	313
Input Volume	18	24	19	54	169	21	306
% of Volume	100	109	88	103	102	113	102

2: Ten Eyck Road & Pleasant Street (E) Performance by movement

Movement	WBL	NBT	NBR	SBT	All
Vehicles Entered	7	78	36	201	322
Vehicles Exited	7	78	37	201	323
Hourly Exit Rate	7	78	37	201	323
Input Volume	8	78	38	196	320
% of Volume	90	100	97	103	101

3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26) Performance by movement

Movement	EBL	EBT	EBR	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	All
Vehicles Entered	88	993	39	1119	21	165	4	4	24	13	173	2643
Vehicles Exited	87	993	39	1123	21	165	4	4	24	13	173	2646
Hourly Exit Rate	87	993	39	1123	21	165	4	4	24	13	173	2646
Input Volume	88	982	39	1106	22	160	4	2	26	14	166	2608
% of Volume	99	101	100	102	97	103	100	200	93	91	104	101

Total Network Performance

Vehicles Entered	2691
Vehicles Exited	2693
Hourly Exit Rate	2693
Input Volume	8011
% of Volume	34

Sandy Health Clinic - PBS Project 71524.000 2029 With Project Conditions - Weekday AM Peak Hour + SBLT Lane

Intersection: 1: Ten Eyck Road & Pleasant Street (W)

Movement	EB	NB	SB
Directions Served	LR	LT	TR
Maximum Queue (ft)	58	46	19
Average Queue (ft)	24	5	1
95th Queue (ft)	49)	27	17
Link Distance (ft)	624	59	201
Upstream Blk Time (%)		0	
Queuing Penalty (veh)		0	
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 2: Ten Eyck Road & Pleasant Street (E)

Movement	WB	SB
Directions Served	LR	LT
Maximum Queue (ft)	34	41
Average Queue (ft)	7	2
95th Queue (ft)	28)	20
Link Distance (ft)	601	59
Upstream Blk Time (%)		0
Queuing Penalty (veh)		1
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

Intersection: 3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26)

Movement	EB	EB	EB	EB	WB	WB	B8	B8	NB	SB	SB	
Directions Served	L	Т	Т	R	T	TR	Т	Т	LTR	L	TR	
Maximum Queue (ft)	193	301	242	80	275	262	143	113	202	81	130	
Average Queue (ft)	47	139	92	12	190	176	16	10	90	22	67	
95th Queue (ft)	119	252	195	49	285	268	(84	65)	158	60	118	
Link Distance (ft)		341	341		192	192	1221	1221	285	104	104	
Upstream Blk Time (%)		0	0		7	6				0	3	
Queuing Penalty (veh)		0	0		0	0				0	3	
Storage Bay Dist (ft)	115			100								
Storage Blk Time (%)	0	7	3	0	22							
Queuing Penalty (veh)	2	6	1	0	0							

Network Summary

Network wide Queuing Penalty: 14

1: Ten Eyck Road & Pleasant Street (W) Performance by movement

Movement	EBL	EBR	NBL	NBT	SBT	SBR	All
Vehicles Entered	50	48	17	198	144	14	471
Vehicles Exited	51	48	17	198	145	14	473
Hourly Exit Rate	51	48	17	198	145	14	473
Input Volume	53	48	17	198	138	13	466
% of Volume	96	101	100	100	105	110	101

2: Ten Eyck Road & Pleasant Street (E) Performance by movement

Movement	WBL	WBR	NBT	NBR	SBL	SBT	All
Vehicles Entered	31	5	212	29	4	191	472
Vehicles Exited	32	5	213	29	4	191	474
Hourly Exit Rate	32	5	213	29	4	191	474
Input Volume	34	5	212	30	4	184	468
% of Volume	94	95	101	97	94	104	101

3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26) Performance by movement

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Vehicles Entered	190	1277	152	6	1336	20	136	23	8	45	38	141
Vehicles Exited	190	1279	153	5	1340	20	139	24	8	44	38	140
Hourly Exit Rate	190	1279	153	5	1340	20	139	24	8	44	38	140
Input Volume	186	1292	158	6	1331	20	137	28	8	47	34	138
% of Volume	102	99	97	83	101	99	101	86	97	94	113	101

3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26) Performance by movement

Movement	All
Vehicles Entered	3372
Vehicles Exited	3380
Hourly Exit Rate	3380
Input Volume	3386
% of Volume	100

Total Network Performance

Vehicles Entered	3449
Vehicles Exited	3456
Hourly Exit Rate	3456
Input Volume	10484
% of Volume	33

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Intersection: 1: Ten Eyck Road & Pleasant Street (W)

Movement	EB	NB	SB
Directions Served	LR	LT	TR
Maximum Queue (ft)	95	56	56
Average Queue (ft)	38	5	4
95th Queue (ft)	70)	33	32
Link Distance (ft)	624	59	201
Upstream Blk Time (%)		0	
Queuing Penalty (veh)		0	
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 2: Ten Eyck Road & Pleasant Street (E)

Movement	WB	NB	SB
Directions Served	LR	TR	LT
Maximum Queue (ft)	96	40	86
Average Queue (ft)	28	2	14
95th Queue (ft)	68)	21	59
Link Distance (ft)	601	104	59
Upstream Blk Time (%)		0	3
Queuing Penalty (veh)		0	6
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 3: Wolf Drive/Ten Eyck Road & Mt. Hood Highway (US 26)

Movement	EB	EB	EB	EB	WB	WB	WB	B8	B8	NB	SB	SB
Directions Served	L	Т	Т	R	L	Т	TR	Т	Т	LTR	L	TR
Maximum Queue (ft)	215	550	490	200	128	302	289	376	353	280	102	144
Average Queue (ft)	145	240	197	50	10	253	242	121	94	147	36	86
95th Queue (ft)	239	440	385	153	64)	306	302	303	281)	247	85	147
Link Distance (ft)		692	692			193	193	1221	1221	881	104	104
Upstream Blk Time (%)		0			0	25	20				1	13
Queuing Penalty (veh)		0			0	0	0				1	15
Storage Bay Dist (ft)	115			100	105							
Storage Blk Time (%)	24	14	13	0		35						
Queuing Penalty (veh)	153	26	20	0		2						

Network Summary

Network wide Queuing Penalty: 223

Appendix D Crash History

TRANSPORTATION DATA SECTION - CRASH ANAYLYSIS AND REPORTING UNIT

CONTINUOUS SYSTEM CRASH LISTING

026: MT. HOOD Highway 026 ALL ROAD TYPES, MP 24.61 to 24.64 01/01/2014 to 12/31/2018, Both Add and Non-Add mileage

1 - 4 of 11 Crash records shown.

	S D M																			
SER#	P RJS	W DATE	COUNTY	RD# FC CONN#	RD CHAR	INT-TYPE	3				SPCL USE									
INVEST	E A U I C	O DAY	CITY	COMPNT FIRST STREET	DIRECT	(MEDIAN)	INT-REL	OFFRD	WTHR	CRASH	TRLR QTY	MOVE			A S	S				
RD DPT	ELGNH	R TIME	URBAN AREA	MLG TYP SECOND STREET	LOCTN	LEGS	TRAF-	RNDBT	SURF	COLL	OWNER	FROM	PRTC	INJ	G 1	E LICNS	PED			
UNLOC?	D C S V L	K LAT	LONG	MILEPNT LRS		(#LANES)	CONTL	DRVWY	LIGHT	SVRTY	V# TYPE	TO	P# TYPE	SVRTY	E	X RES	LOC	ERROR	ACT EVENT	CAUSE
03162	N N N N	08/16/2014	CLACKAMAS	1 14	INTER	5-LEG	N	N	CLR	S-1STOP	01 NONE 0	STRGHT								29
NO RPT		SA	SANDY	MN 0 PIONEER BLVD	E		TRF SIGNAL	N	DRY	REAR	UNKN	E -W							000	00
N		4P	SANDY UA	24.61 SE TEN EYCK RD	06	0		N	DAY	INJ	MTRCYCLE		01 DRVR	NONE	00 M	TIMK		026	000	29
14		TI.	DANDI OA	24.01 SE IEN EICK KD	00	0		IN	DAI	1110	MIRCICEE		OI DRVR	NONE	00 14	OIVIC		020	000	23
N		45 23 49.25	-122 15 19.74	002600100S00												UNK				
											02 NONE 0	STOP								
											PRVTE	E -W							011	00
											PSNGR CAR		01 DRVR	NONE	72 M	OR-Y OR<25		000	000	00
											02 NONE 0	STOP				UR<25				
											PRVTE	E -W							011	00
											PSNGR CAR		02 PSNG	INJC	64 F			000	000	00
03787	NNNNN	N 09/15/2015	CLACKAMAS	1 14	INTER	5-LEG	N	N	CLR	S-1STOP	01 NONE 0	STRGHT								29
NONE		TU	SANDY	MN 0 PIONEER BLVD	E		TRF SIGNAL	N	DRY	REAR	PRVTE	E -W							000	00
27		1P	CANDY IIA	24 C1 OF BOX BYOK DD	06	0		N	DAN	TNT	DOMOD CAD		01 DRVR	MONTE	71 14	OD 17		026	000	29
N		IP	SANDY UA	24.61 SE TEN EYCK RD	06	U		IN	DAY	INJ	PSNGR CAR		UI DRVR	NONE	/ 1 M	OR-Y		026	000	29
N		45 23 49.24	-122 15 19.74	002600100S00												OR<25				
											02 NONE 0	STOP								
											PRVTE	E -W							011	00
										PSNGR CAR		01 DRVR	INJC	38 F			000	000	00	
																N-RES				
	N N N N	06/24/2014	CLACKAMAS	1 14	INTER	5-LEG	N	N	CLR	S-1STOP	01 NONE 0	STRGHT							222	29
NONE		TU	SANDY	MN 0 PIONEER BLVD	SE		TRF SIGNAL	N	DRY	REAR	PRVTE	SE-NW							000	00
N		8A	SANDY UA	24.61 SE TEN EYCK RD	06	0		N	DAY	INJ	PSNGR CAR		01 DRVR	NONE	55 F	OR-Y		026	000	29
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N		45 23 49.25	-122 15 19.74	002600100S00							02 NONE 0	STOP				OR<25				
											PRVTE	SE-NW							011	00
											PSNGR CAR	52 1111	01 DRVR	INJC	51 F	OR-Y		000	000	00
																OR<25				
04899	NNNNN	N 12/04/2018	CLACKAMAS	1 14	STRGHT		Y	N	CLR	S-1STOP	01 NONE 0	STRGHT								10
CITY		TU	SANDY	MN 0 PIONEER BLVD	E	(NONE)	TRF SIGNAL	N	DRY	SS-O	PRVTE	E -W							000	00
		0.7	G117011 111	04 63 - 201 - 20	0.4						DOMEST CAR		01 DDID	T11TG	40	arran		000	006	1.0
N		8A	SANDY UA	24.63 WOLF DR	04			N	DAY	INJ	PSNGR CAR		01 DRVR	INJC	48 F	SUSP		080	026	10
N	N 45 23 49.07 -122 15	-122 15 18.32	002600100S00		(04)										OR<25					
											02 NONE 0	STOP								
											PUBLC	E -W							011	00
											SCHL BUS		01 DRVR	NONE	44 F			000	000	00
																OR<25				
00248	N N N N	01/20/2018	CLACKAMAS	1 14	STRGHT		N	N	CLD	S-1STOP	01 NONE 0	STRGHT							003	07,29
CITY		SA	SANDY	MN 0 PROCTOR BLVD	SE	(NONE)	NONE	N	DRY	REAR	PRVTE	SE-NW							000	00
N		3P	SANDY UA	24.64 WOLF DR	05			N	DAY	INJ	PSNGR CAR		01 DRVR	NONE	22 F	OR-Y		043,026	038 003	07,29
		45 00 40 00	100 15 15 65	000600100000		(04)										on of				
N		45 23 48.99	-122 15 17.63	002600100S00		(04)										OR>25				

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03/10/2020

TRANSPORTATION DATA SECTION - CRASH ANAYLYSIS AND REPORTING UNIT

CONTINUOUS SYSTEM CRASH LISTING

026: MT. HOOD Highway 026 ALL ROAD TYPES, MP 24.61 to 24.64 01/01/2014 to 12/31/2018, Both Add and Non-Add mileage

5 - 9 of 11 Crash records shown.

c	S D M																	
	PRJSW DATE	COUNTY	RD# FC CONN#	RD CHAR	INT-TYPE					SPCL USE								
	E A U I C O DAY	CITY	COMPNT FIRST STREET	DIRECT	(MEDIAN)		OFFRD	WTHR	CRASH	TRLR QTY	MOVE			A	S			
	E L G N H R TIME	URBAN AREA	MLG TYP SECOND STREET	LOCTN	LEGS	TRAF-	RNDBT	SURF	COLL	OWNER	FROM	PRTC	INJ		E LICNS PE	D		
UNLOC? D	O C S V L K LAT	LONG	MILEPNT LRS		(#LANES)	CONTL	DRVWY	LIGHT	SVRTY	V# TYPE	TO	P# TYPE	SVRTY				ACT EVENT	CAUSE
				,						02 NONE 0	STOP							
										PRVTE	SE-NW	0.1 DDIM	TNIC	42 14	OD W	000	011 000	00
										PSNGR CAR		01 DRVR	INUC	45 M	OR-1 OR<25	000	000	00
										02 NONE 0	STOP				010 123			
										PRVTE	SE-NW						011	00
										PSNGR CAR		02 PSNG	INJC	35 F		000	000	00
										0.2 MONTH 0	GEOD.							
										02 NONE 0 PRVTE	STOP SE-NW						011	00
										PSNGR CAR	DE IVI	03 PSNG	NONE	04 M		000	000	00
04077 N	N N N N 10/14/2014	CLACKAMAS	1 14	INTER	6-LEG	N	N	RAIN	S-1STOP	01 NONE 0	STRGHT							29
NO RPT	TU	SANDY	CP 0 PIONEER BLVD	W		TRF SIGNAL	N	WET	REAR	UNKN	W -E						000	00
N	6A	SANDY UA	24.61 WOLF DR	06	0		N	DLIT	INJ	PSNGR CAR		01 DRVR	NONE	00 M	OR-Y	026	000	29
N 45 23 49.25	-122 15 19.74	002600100S00							0.2 MONTH 0	GEOD.				UNK				
										02 NONE 0 PRVTE	STOP W -E						011	00
										PSNGR CAR	" =	01 DRVR	INJC	70 M	OR-Y	000	000	00
															OR<25			
03023 N	N N N N N 08/06/2014	CLACKAMAS	1 14	INTER	5-LEG	N	N	CLR	ANGL-OTH	01 NONE 0	STRGHT						082	04
CITY	WE	SANDY	CP 0 PIONEER BLVD	CN		TRF SIGNAL	N	DRY	ANGL	PRVTE	W -E						000	00
N	1P	SANDY UA	24.61 WOLF DR	03	0		N	DAY	PDO	PSNGR CAR		01 DRVR	NONE	19 F	OR-Y	000	000	00
N	45 23 49.25	-122 15 19.74	002600100S00							0.2 MONTH 0	GMD GLIM				OR<25			
										02 NONE 0 PRVTE	STRGHT N -S						000	00
										PSNGR CAR	1. 5	01 DRVR	NONE	59 M	OR-Y	020	000 082	04
															OR<25			
01741 N	N N N N N 05/09/2015	CLACKAMAS	1 14	INTER	5-LEG	N	N	CLR	ANGL-OTH	01 NONE 0	STRGHT							04
NONE	SA	SANDY	CP 0 PIONEER BLVD	CN		TRF SIGNAL	N	DRY	ANGL	PRVTE	S -N						000	00
N	6A	SANDY UA	24.61 WOLF DR	04	0		N	DAY	PDO	PSNGR CAR		01 DRVR	NONE	25 M	OTH-Y	020	026	04
	45 00 40 05	100 15 10 54	000500100												05			
N	45 23 49.25	-122 15 19.74	002600100S00							02 NONE 0	STRGHT				OR<25			
										PRVTE	W -E						000	00
										PSNGR CAR		01 DRVR	NONE	51 F	OR-Y	000	000	00
															OR<25			
00512 N	N N N N N N 02/07/2017	CLACKAMAS	1 14	INTER	5-LEG	N	N	RAIN	ANGL-OTH	01 NONE 0	TURN-L							04
CITY	TU	SANDY	CP 0 PIONEER BLVD	CN		TRF SIGNAL	N	WET	TURN	PRVTE	S -W						000	00
N	4P	SANDY UA	24.61 WOLF DR	04	0		N	DUSK	INJ	PSNGR CAR		01 DRVR	INJC	55 F	OR-Y	000	000	00
	45 22 40 25																	
N	45 23 49.25	-122 15 19.74	002600100S00							02 NONE 0	STRGHT				OR<25			
										PRVTE	W -E						000	00
										PSNGR CAR		01 DRVR	NONE	63 M	OR-Y	020	000	04
															OR>25			

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TRANSPORTATION DATA SECTION - CRASH ANAYLYSIS AND REPORTING UNIT

CONTINUOUS SYSTEM CRASH LISTING

026: MT. HOOD Highway 026 ALL ROAD TYPES, MP 24.61 to 24.64 01/01/2014 to 12/31/2018, Both Add and Non-Add mileage

10 - 11 of 11 Crash records shown.

S D M																			
SER# P R J S	W DATE	COUNTY	RD# FC CONN#	RD CHAR	INT-TYPE					SPCL USE									
INVEST E A U I C	O DAY	CITY	COMPNT FIRST STREET	DIRECT	(MEDIAN)	INT-REL	OFFRD	WTHR	CRASH	TRLR QTY	MOVE			A	S				
RD DPT E L G N H	R TIME	URBAN AREA	MLG TYP SECOND STREET	LOCTN	LEGS	TRAF-	RNDBT	SURF	COLL	OWNER	FROM	PRTC	INJ	G	E LICNS	PED			
UNLOC? D C S V L	K LAT	LONG	MILEPNT LRS		(#LANES)	CONTL	DRVWY	LIGHT	SVRTY	V# TYPE	TO	P# TYPE	SVRTY	E	X RES	LOC	ERROR	ACT EVENT	CAUSE
03089 N N N N	09/03/2018	CLACKAMAS	2 14	INTER	5-LEG	N	N	CLR	S-1STOP	01 NONE 0	STRGHT								29
NONE	MO	SANDY	MN 0 PROCTOR BLVD	SE		TRF SIGNAL	N	DRY	REAR	UNKN	SE-NW							000	00
N	3P	SANDY UA	24.63 SE TEN EYCK RD	06	0		N	DAY	INJ	PSNGR CAR		01 DRVR	NONE	00	F UNK		026	000	29
N	45 23 49.25	-122 15 19.75	002600200800												UNK				
										02 NONE 0	STOP								
										PRVTE	SE-NW							011	00
										PSNGR CAR		01 DRVR	INJC	25	F OR-Y OR>25		000	000	00
05173 N N N N	11/08/2016	CLACKAMAS	2 14	INTER	5-LEG	N	N	CLR	ANGL-OTH	01 NONE 9	U-TURN								06
NONE	TU	SANDY	CP 0 PROCTOR BLVD	W		TRF SIGNAL	N	DRY	TURN	N/A	M - M							000	00
N	5P	SANDY UA	24.63 SE TEN EYCK RD	05	0		N	DUSK	PDO	PSNGR CAR		01 DRVR	NONE	00	Unk UNK		000	000	00
N	45 23 49.25	-122 15 19.74	002600200800		(02)										UNK				
										02 NONE 9	TURN-R								
										N/A	NE-W							000	00
										PSNGR CAR		01 DRVR	NONE	00	Unk UNK		000	000	00

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03/10/2020

General & Site Information					
Analyst:	John Manix PE				
Agency/Company:	PBS Engineering and Env.				
Date:	3/10/2020				
Project Name:	Sandy Health Clinic				

	Intersection Crash Data						
	Intersection			Year			
Intersection	Type	2014	2015	2016	2017	2018	Total
Highway 26 and Ten Eyck	Urban 4SG	4	2	1	1	3	11
							0
							0
							0
							0
							0
							0
							0
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							0
							0
	Total	4	2	1	1	3	11

Oregon Dept of Transportation

Transportation Planning Analysis Unit

Intersection	n Population Typ	e Crash Rate								
Average Cr	Average Crash Rate per intersection type									
Intersection Pop. Type	Sum of Crashes	Sum of 5- year MEV	Avg Crash Rate for Ref Pop.	INT in Pop						
Rural 3SG	0	0								
Rural 3ST	0	0								
Rural 4SG	0	0								
Rural 4ST	0	0								
Urban 3ST	0	0								
Urban 3SG	0	0								
Urban 4ST	0	0								
Urban 4SG	11	43	0.2567	1						

	Critical Rate Calculation							
				Intersection		Reference		
	AADT Entering			Population	Intersection	Population Crash	Critical	Over
Intersection	Intersection	5-year MEV	Crash Total	Туре	Crash Rate	Rate	Rate	Critical
Highway 26 and Ten Eyck	23,480	42.9	11	Urban 4SG	0.26	0.26	0.40	Under

Oregon Dept of Transportation

Transportation Planning Analysis Unit



EXHIBIT I Deborah Cockrell Clackamas Health Centers

Administration Public Service Building 2051 Kaen Road, Suite 367 Oregon City, OR 97045-4035 503-742-5300

Police Chief Ernie Roberts Sandy Police Department 39850 Pleasant Street

Sandy, Oregon 97055

February 10, 2020

Beavercreek Health Center 110 Beavercreek Road Oregon City, OR 97045-4023 503-655-8471 Primary Care, Dental Services and Behavioral Health

RE: SIX SHARED PARKING SPACES ALIGNED ON THE PROPERTY LINE BETWEEN OUR PROPERTIES

Sunnyside Health Center 503-655-8471 Primary Care, Dental Services

9775 SE Sunnyside Road Suite 200 Clackamas, OR 97015-5721 and Behavioral Health

Gladstone Health Clinic 18911 Portland Avenue Gladstone, OR 97027-1630 503-850-4472 Primary Care Pediatrics Dental Services and Behavioral Health

Sandy Health Clinic 37400 SE Bell Street Sandy, OR 97055-7868 503-668-3493 Primary Care and Behavioral Health

Hilltop Behavioral Health Clinic 998 Library Court Oregon City, OR 97045-4041 503-655-8401

Stewart Behavioral Health Clinic 1002 Library Court Oregon City, OR 97045-4066 503-655-8264

> Sandy Behavioral Health Clinic 38872 Proctor Boulevard PO Box 1390 Sandy, OR 97055-8035 503-722-6950

School Based Health Centers Oregon City High School 19761 S Beavercreek Road Oregon City, OR 97045-9557 503-785-8770

> Rex Putnam High School 4950 SE Roethe Rd. Milwaukie, OR 97267-5746 503-722-6858

> > Sandy High School 37400 Bell Street Sandy, OR 97055-7868 503-668-3483

Dear Chief Roberts:

This letter is to communicate openly with your office regarding the six shared parking spaces, along your west property line. At the time of our interest in the vacant building (i.e. property) adjacent to your precinct, we were informed by the seller(s) of the existing six shared parking spaces agreement that was established some time ago. The City of Sandy Design Review Process requires the County to maintain the use of the existing six shared parking spaces.

Our architectural firm, Ankrom Moisan Architects, Inc. on our behalf is working with the City of Sandy Planning Office to meet their requirements for our new building design. At this point, we are unaware of an exact start of our construction schedule for the New Sandy Health Clinic. However, the construction project will occur sometime in 2020-2021. As soon as our project is released to the public via the bidding process to contractors, we will alert your office of our planned steps and keep you apprised of an established construction schedule, once we receive bids. Once a general contractor is hired, we will need to use the parking spaces between our property lines for staging for the project. We are hopeful this is an allowable request.

In regards to the shared parking spaces, no employee will park there in the future, once the new building opens to the public. Only patients will be parking in our new lot and periodically using the shared parking spaces. Attached please find the proposed building site plan, which includes the shared parking spaces.

It is the County's full intention to be good neighbors of the Sandy Police Department. If you have questions, I can reach me at my office phone number 503-742-5495. The assigned Project Coordinator is Steve Kelly and his phone number is 503-650-5665. Thank you for your valuable time.

Sincerely,

Deborah Cockrell, FQHC Health Centers, Director

Cc: Steve Kelly, Project Coordinator

Ankrom Moisan Architects, Inc.

Healthy Families. Strong Communities.

2051 Kaen Road, Oregon City, OR 97045 • www.clackamas.us/healthcenters Phone (503) 742-5300 • Fax (503) 742-5979

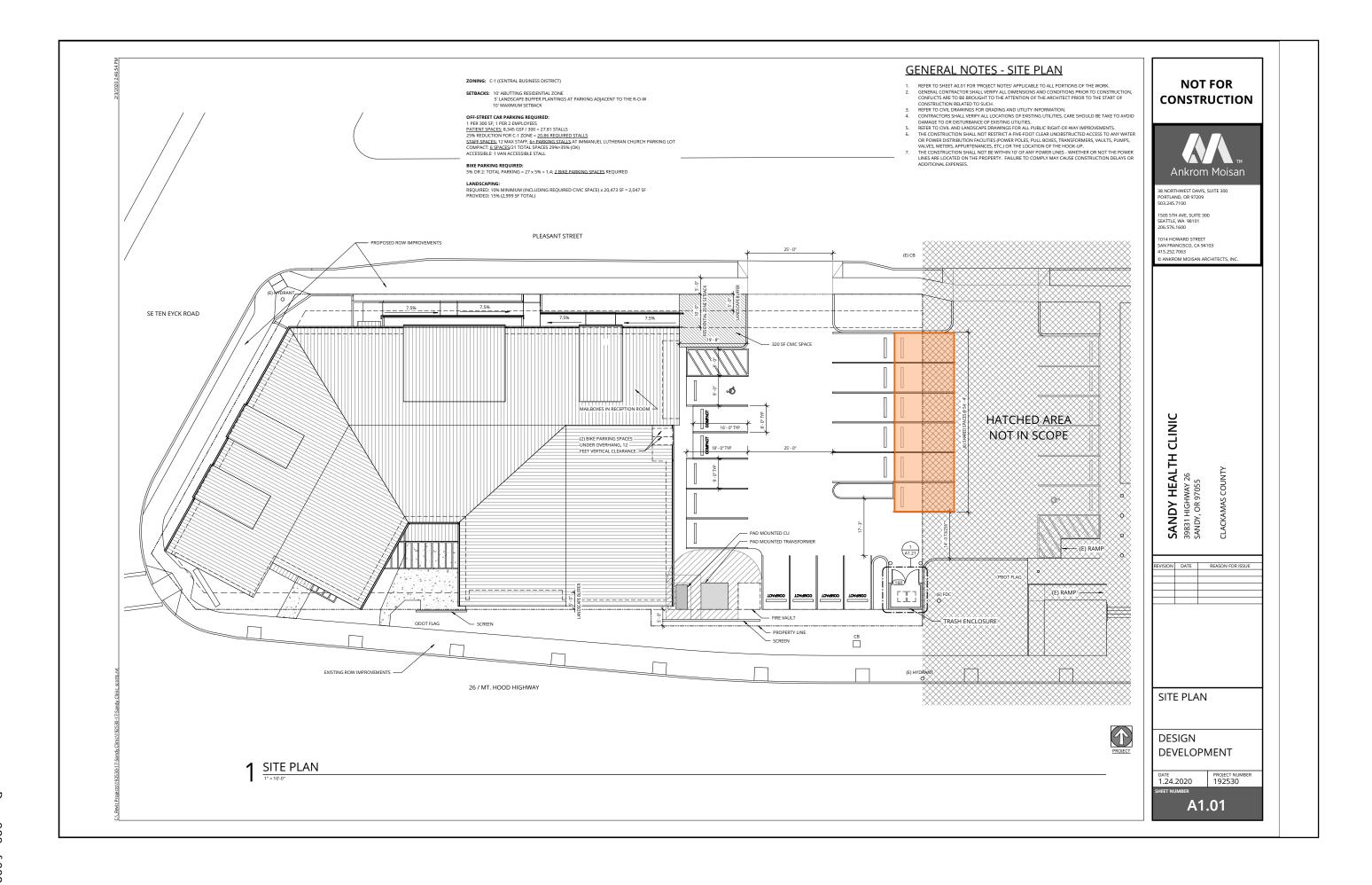


EXHIBIT J

PARKING SPACE LEASE AGREEMENT

This agreement is made and entered into this day of, 2020, by and between Immanuel Lutheran Church ("Owner") and Clackamas County ("County") for the lease of Seventeen (17) parking spaces, in the south parking lot across Pleasant Street from property described as Lots One (1) to including Lot Twelve (12), Block Two (2), Minnie Meinig's Pine View Tracts and commonly known as 39901 Pleasant Street, Sandy OR 97055 (map attached).
Owner represents that it has the right to lease these parking spaces to the County for the County's exclusive use, subject to the following terms and conditions.
1. Term. The lease shall commence on, 2020 and shall end on, 2027. County shall be allowed use of the spaces between the hours of 6am to 8pm, Monday through Friday.
2. Hold Over. If County shall hold over and remain in possession of said premises after expiration of this Lease without any written lease actually being made, such holding over shall not be deemed to operate as a renewal or extension of this Lease but shall only create a tenancy which may be terminated at any time by Owner upon sixty (60) days written notice to County.
3. Renewal . This Lease may be renewed for successive seven (7) year terms ("Renewal Term") by mutual written agreement of the parties, executed not less than six (6) months prior to the expiration of the Initial Term or any Renewal Term, as applicable.
4. Use. County will use the leased parking spaces only for the parking of vehicles. Owner will provide County with exclusive use of the leased parking spaces, and Owner will provide signage indicating that these spaces are reserved for County use during the hours of operation noted in 1. Above. County shall be entitled to full use and possession of the parking spaces for the entire lease term.
5. Fees and Payment . The annual fee will be \$6,000 per year. Payment is due on the first day of and is payable by the 14 th day of A late fee of \$100 will be assessed on the 15 th day of if payment has not been received by the 14 th day. No deposit is required. Checks for payment should be made payable to:
Immanuel Lutheran Church 39901 Pleasant Street
Sandy Parking Lease Page 1 of 5

Sandy, Oregon 97055

- **6. Property Taxes and other expenses.** Owner is solely responsible for property taxes or other expenses such as electric bill for lighting (if any) on the leased parking spaces.
- **7. Maintenance.** County shall be responsible for any necessary trash collection, sweeping, or maintenance of the leased parking spaces. Owner shall be responsible for all other necessary maintenance of the leased parking spaces.

8. Termination.

- **a.** Either party may terminate the Lease upon occurrence of an event of default. An event of default shall be deemed to occur should any of the following events happen:
 - 1. Failure of County to pay lease fees within 30 days from written notice by Owner to County that lease fees are overdue;
 - Repeated failure of County or its employees to obey reasonable rules of the Owner concerning matters of security, safety, or preservation of the Owner's facilities, during the term of the Agreement; or
 - 3. Failure of either party to comply with any term or condition of this Agreement.

In the event of default, the defaulting party shall be given notice of the default in writing by the other party. The party which has been given notice of default shall have 30 days to correct said default. If the default is not corrected within the 30 day notice period, the other party shall have the right to terminate this lease by giving written notice of uncorrected default and termination to the defaulting party. Any notice shall be given by in writing through certified mail, and shall be effective upon receipt. Notice shall be sent to the address for the receiving party as designated herein.

- **b.** County may terminate this Lease in the event the County fails to receive expenditure authority sufficient to allow County, in the exercise of its reasonable administrative discretion, to continue to make payments for performance of this Lease, or if federal or state laws, regulations or guidelines are modified or interpreted in such a way that County is prohibited from performing under the Lease.
- **c.** Any termination of this Lease shall not prejudice any rights or obligations accrued to the parties prior to termination.
- **9. Constitutional Debt Limitation.** This Lease is expressly subject to the debt limitation of Oregon Counties set forth in Article XI, Section 10 of the Oregon Constitution, and is contingent

Sandy Parking Lease

upon funds being appropriated therefor. Any provisions herein which would conflict with law
are deemed inoperative to that extent.
10. No Attorney Fees: In the event any arbitration, action or proceeding, including any bankruptcy
proceeding, is instituted to enforce any term of this Lease, each party shall be responsible for its own
attorneys' fees and expenses.
11. Warrant of Authority: Owner warrants and represents that it is the sole owner of the leased
premises subject to this Lease, and that Owner has full authority to execute this Lease. The undersigned
warrants and represents that he/she has full authority to sign on behalf of Owner.
warrants and represents that he/she has fall authority to sign on behalf of Owner.
[SIGNATURES TO FOLLOW]
,
Sandy Parking Lease Page 3 of 5

CLACKAMAS COUNTY	Lessor: IMMANUEL LUTHERAN CHURCH
Commissioner Jim Bernard, Chair	39901 Pleasant Street
Commissioner Sonya Fischer	Sandy, Oregon 97055
Commissioner Ken Humberston	
Commissioner Paul Savas	
Commissioner Martha Schrader	
Signing on Behalf of the Board.	
Richard Swift, Director	 Dwight Reigert, Legal Signer
Health, Housing, and Human Services Department	Dwight heigert, Legal Signer
Date	 Date
	_
STATE OF OREGON, County of Clackam	as) ss.
BE IT REMEMBERED, that on thisundersigned, a Notary Public in and for REIGERT known to be to be the individu	
BE IT REMEMBERED, that on thisundersigned, a Notary Public in and for REIGERT known to be to be the individu Space Lease Agreement, and acknowled freely and voluntarily.	day of, 2020, before me, the said County and State, personally appeared <u>DWIGHT</u> ual(s) described in and who executed the within Parking
BE IT REMEMBERED, that on thisundersigned, a Notary Public in and for REIGERT known to be to be the individu Space Lease Agreement, and acknowled freely and voluntarily. IN TESTIMONY WHEREOF, I have hereu	day of, 2020, before me, the said County and State, personally appeared <u>DWIGHT</u> ual(s) described in and who executed the within Parking dged to me that said individual(s) executed the same



Sandy Parking Lease

EXHIBIT K



MEMORANDUM

DATE:

03/23/2020

BY:

Norm Scheg

SUBJECT:

Stormwater Utility Narrative

PROJECT:

Sandy Health Clinic

PROJECT NO.:

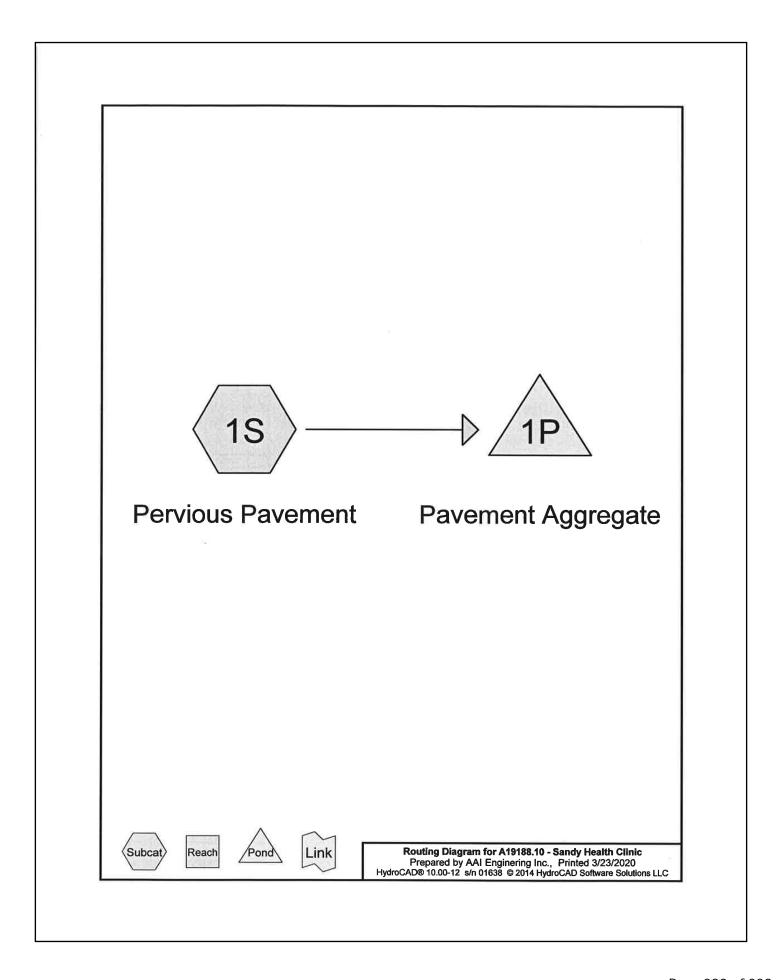
A19188.10

This memorandum is to outline the stormwater requirements for the Sandy Health Clinic project located at 39831 Highway 26, Sandy, OR 97055. The project consists of construction of a 10,940SF building, 5,291SF of pervious parking lot with associated pedestrian pathways and utilities. The total site is 20,204SF and is predominately impervious with an existing building and parking lot. Post construction we will be collecting roof runoff in downspouts that will be hard piped to a drywell. This drywell is "authorized by rule" by Oregon DEQ as it will infiltrate runoff from roofs and a parking lot of less than 50 vehicles. We will be constructing pervious pavement for the parking area as well. Both of these facilities have been modeled utilizing 2" per hour as a basis for storage sizing. Once the site has been cleared, and before construction of any utilities, a Geotechnical Engineer will be retained to test the actual infiltration rates of the native soils to assure they have at least a design infiltration rate of 2" per hour or more. The drywell sizing will be reviewed at that time to see it the annulus of rock can be reduced. The rock under the pervious pavement is the minimum thickness for structural integrity. Attached are HydroCAD calculations verifying the infiltration systems meets the stated requirements. A comprehensive stormwater plan will be submitted with the next submittal.

4875 SW Griffith Drive | Suite 300 | Beaverton, OR | 97005

503.620.3030 | tel 503.620.5539 | fax

www.aaieng.com



Type IA 24-hr 25yr Rainfall=5.00" Printed 3/23/2020

Prepared by AAI Enginering Inc.

HydroCAD® 10.00-12 s/n 01638 © 2014 HydroCAD Software Solutions LLC

Page 2

Time span=5.00-48.00 hrs, dt=0.05 hrs, 861 points
Runoff by SBUH method, Split Pervious/Imperv.
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1S: Pervious Pavement

Runoff Area=5,291 sf 100.00% Impervious Runoff Depth>4.19" Tc=0.0 min CN=0/98 Runoff=0.15 cfs 0.042 af

Pond 1P: Pavement Aggregate

Peak Elev=0.00' Storage=16 cf Inflow=0.15 cfs 0.042 af Outflow=0.15 cfs 0.042 af

Total Runoff Area = 0.121 ac Runoff Volume = 0.042 af Average Runoff Depth = 4.19"
0.00% Pervious = 0.000 ac 100.00% Impervious = 0.121 ac

Type IA 24-hr 25yr Rainfall=5.00"

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Summary for Subcatchment 1S: Pervious Pavement

Runoff

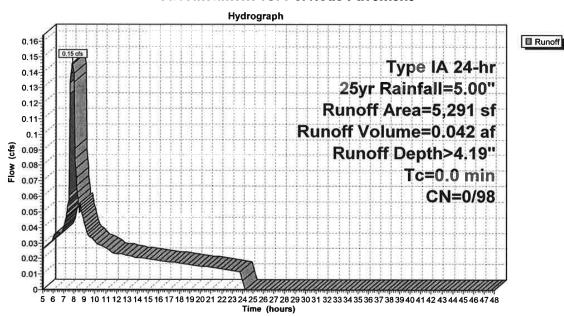
0.15 cfs @ 7.80 hrs, Volume=

0.042 af, Depth> 4.19"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 5.00-48.00 hrs, dt= 0.05 hrs Type IA 24-hr 25yr Rainfall=5.00"

	Area (sf)	CN	Description	
*	5,291	98	Pervious Pavement	
	5 291	98	100 00% Impervious Area	

Subcatchment 1S: Pervious Pavement



Type IA 24-hr 25yr Rainfall=5.00"

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Summary for Pond 1P: Pavement Aggregate

Inflow Area = 0.121 ac,100.00% Impervious, Inflow Depth > 4.19" for 25yr event

Inflow = 0.15 cfs @ 7.80 hrs, Volume= 0.042 af

Outflow = 0.15 cfs @ 7.83 hrs, Volume= 0.042 af, Atten= 0%, Lag= 1.8 min

Discarded = 0.15 cfs @ 7.83 hrs, Volume= 0.042 af

Routing by Stor-Ind method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs Peak Elev= 0.00' @ 7.83 hrs Surf.Area= 211,640 sf Storage= 16 cf

Plug-Flow detention time= 2.6 min calculated for 0.042 af (100% of inflow)

Center-of-Mass det. time= 1.8 min (714.7 - 712.9)

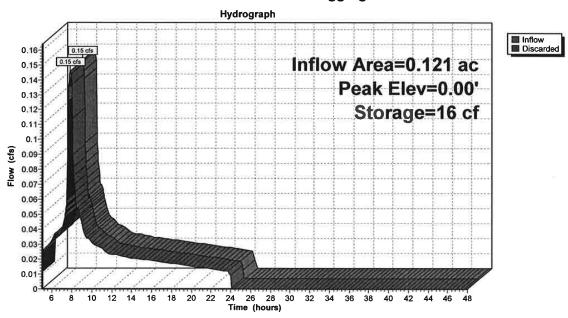
Volume	Invert	Avail.S	torage	Storage	Description	
#1	0.00'	105,	820 cf	Custom	Stage Data (Pr	ismatic) Listed below (Recalc) x 40
Elevation (feet)		.Area sq-ft)		.Store :-feet)	Cum.Store (cubic-feet)	
0.00 0.50		5,291 5,291		0 2,646	0 2,646	

Device Routing Invert Outlet Devices

#1 Discarded 0.00' 2.000 in/hr Exfiltration over Surface area

Discarded OutFlow Max=9.80 cfs @ 7.83 hrs HW=0.00' (Free Discharge) 1=Exfiltration (Exfiltration Controls 9.80 cfs)

Pond 1P: Pavement Aggregate



Type IA 24-hr 100yr Rainfall=6.00" Printed 3/23/2020

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Time span=5.00-48.00 hrs, dt=0.05 hrs, 861 points
Runoff by SBUH method, Split Pervious/Imperv.
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1S: Pervious Pavement

Runoff Area=5,291 sf 100.00% Impervious Runoff Depth>5.05" Tc=0.0 min CN=0/98 Runoff=0.18 cfs 0.051 af

Pond 1P: Pavement Aggregate

Peak Elev=0.00' Storage=19 cf Inflow=0.18 cfs 0.051 af Outflow=0.18 cfs 0.051 af

Total Runoff Area = 0.121 ac Runoff Volume = 0.051 af Average Runoff Depth = 5.05" 0.00% Pervious = 0.000 ac 100.00% Impervious = 0.121 ac

Type IA 24-hr 100yr Rainfall=6.00" Printed 3/23/2020

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Summary for Subcatchment 1S: Pervious Pavement

Runoff

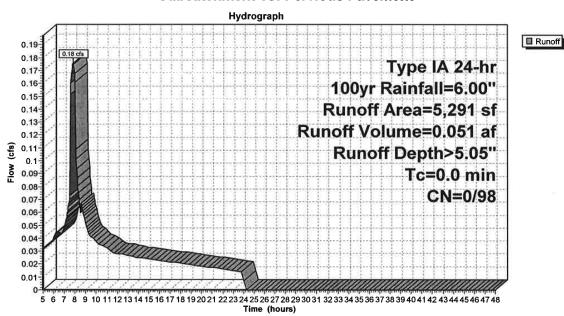
0.18 cfs @ 7.80 hrs, Volume=

0.051 af, Depth> 5.05"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 5.00-48.00 hrs, dt= 0.05 hrs Type IA 24-hr 100yr Rainfall=6.00"

	Area (sf)	CN	Description	
*	5,291	98	Pervious Pavement	
	5 291	98	100 00% Impervious Area	

Subcatchment 1S: Pervious Pavement



Type IA 24-hr 100yr Rainfall=6.00"

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Summary for Pond 1P: Pavement Aggregate

Inflow Area = 0.121 ac,100.00% Impervious, Inflow Depth > 5.05" for 100yr event

Inflow = 0.18 cfs @ 7.80 hrs, Volume= 0.051 af

Outflow = 0.18 cfs @ 7.83 hrs, Volume= 0.051 af, Atten= 0%, Lag= 1.8 min

Discarded = 0.18 cfs @ 7.83 hrs, Volume= 0.051 af

Routing by Stor-Ind method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs Peak Elev= 0.00' @ 7.83 hrs Surf.Area= 211,640 sf Storage= 19 cf

Plug-Flow detention time= 2.6 min calculated for 0.051 af (100% of inflow)

Center-of-Mass det. time= 1.8 min (714.1 - 712.3)

Volume	Invert	Avail.Storage	Storage Description
#1	0.00'	105,820 cf	Custom Stage Data (Prismatic) Listed below (Recalc) x 40

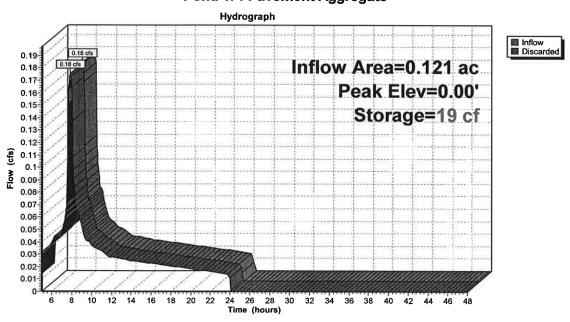
Cum.Store	Inc.Store	Surf.Area	Elevation
(cubic-feet)	(cubic-feet)	(sq-ft)	(feet)
0	0	5,291	0.00
2,646	2,646	5,291	0.50

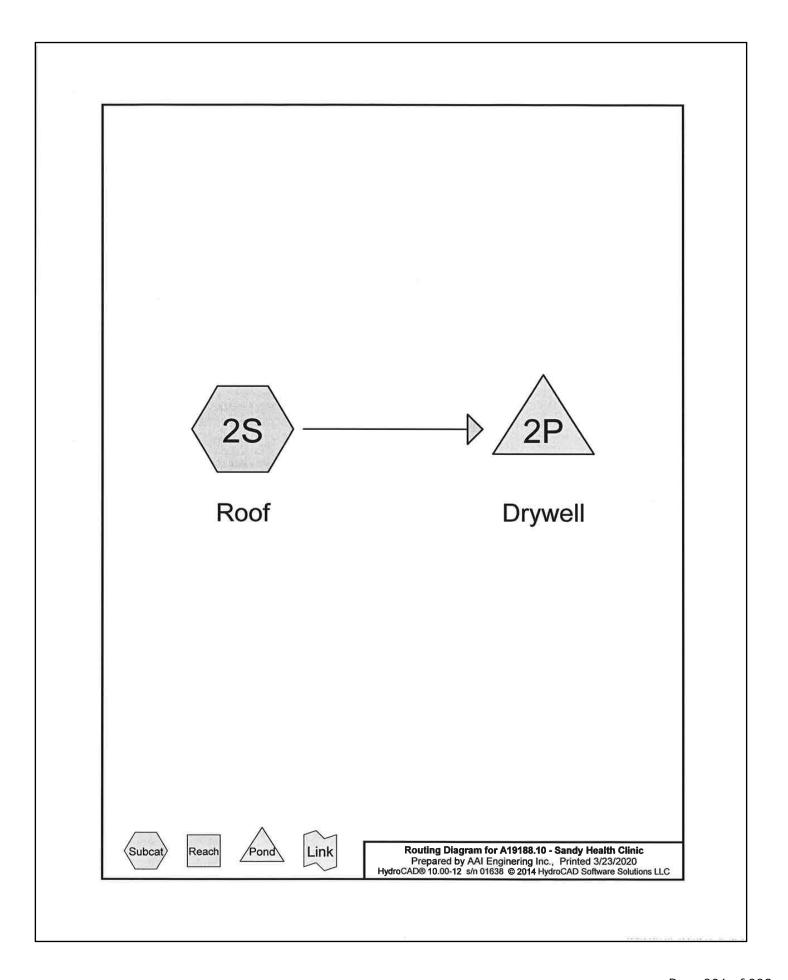
Device Routing Invert Outlet Devices

#1 Discarded 0.00' 2.000 in/hr Exfiltration over Surface area

Discarded OutFlow Max=9.80 cfs @ 7.83 hrs HW=0.00' (Free Discharge) 1=Exfiltration (Exfiltration Controls 9.80 cfs)

Pond 1P: Pavement Aggregate





Type IA 24-hr 25yr Rainfall=5.00" Printed 3/23/2020

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Time span=5.00-48.00 hrs, dt=0.05 hrs, 861 points
Runoff by SBUH method, Split Pervious/Imperv.
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 2S: Roof

Runoff Area=10,940 sf 100.00% Impervious Runoff Depth>4.19" Tc=0.0 min CN=0/98 Runoff=0.30 cfs 0.088 af

Pond 2P: Drywell

Peak Elev=12.83' Storage=2,396 cf Inflow=0.30 cfs 0.088 af Outflow=0.02 cfs 0.074 af

Total Runoff Area = 0.251 ac Runoff Volume = 0.088 af Average Runoff Depth = 4.19"
0.00% Pervious = 0.000 ac 100.00% Impervious = 0.251 ac

Type IA 24-hr 25yr Rainfall=5.00" Printed 3/23/2020

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Summary for Subcatchment 2S: Roof

Runoff

0.30 cfs @

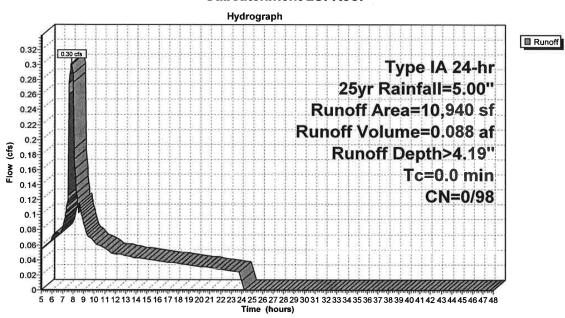
7.80 hrs, Volume=

0.088 af, Depth> 4.19"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 5.00-48.00 hrs, dt= 0.05 hrs Type IA 24-hr 25yr Rainfall=5.00"

	Area (sf)	CN	Description	
*	10,940	98	Roof	
	10 940	98	100 00% Impervious Area	

Subcatchment 2S: Roof



Type IA 24-hr 25yr Rainfall=5.00" Printed 3/23/2020

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Summary for Pond 2P: Drywell

Inflow Area = 0.251 ac,100.00% Impervious, Inflow Depth > 4.19" for 25yr event

Inflow = 0.30 cfs @ 7.80 hrs, Volume= 0.088 af

Outflow = 0.02 cfs @ 5.25 hrs, Volume= 0.074 af, Atten= 93%, Lag= 0.0 min

Discarded = 0.02 cfs @ 5.25 hrs, Volume= 0.074 af

Routing by Stor-Ind method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs Peak Elev= 12.83' @ 23.99 hrs Surf.Area= 452 sf Storage= 2,396 cf

Plug-Flow detention time= 982.6 min calculated for 0.074 af (85% of inflow) Center-of-Mass det. time= 880.4 min (1,593.3 - 712.9)

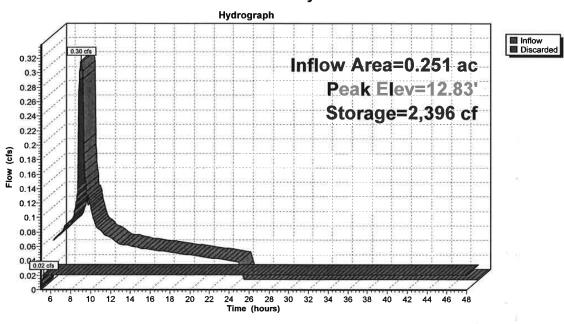
Volume	Invert	Avail.Storage	Storage Description
#1	0.00'	3,482 cf	24.00'D x 20.00'H Vertical Cone/Cylinder
			9,048 cf Overall - 342 cf Embedded = 8,706 cf x 40.0% Voids
#2	0.00'	251 cf	4.00'D x 20.00'H Vertical Cone/Cylinder Inside #1
-			342 cf Overall - 4.0" Wall Thickness = 251 cf

3,734 cf Total Available Storage

Device	Routing	Invert	Outlet Devices	
#1	Discarded	0.00'	2.000 in/hr Exfiltration over Surface area	

Discarded OutFlow Max=0.02 cfs @ 5.25 hrs HW=0.23' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.02 cfs)

Pond 2P: Drywell



Type IA 24-hr 100yr Rainfall=6.00" Printed 3/23/2020

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Time span=5.00-48.00 hrs, dt=0.05 hrs, 861 points
Runoff by SBUH method, Split Pervious/Imperv.
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 2S: Roof

Runoff Area=10,940 sf 100.00% Impervious Runoff Depth>5.05" Tc=0.0 min CN=0/98 Runoff=0.36 cfs 0.106 af

Pond 2P: Drywell

Peak Elev=16.98' Storage=3,170 cf Inflow=0.36 cfs 0.106 af Outflow=0.02 cfs 0.074 af

Total Runoff Area = 0.251 ac Runoff Volume = 0.106 af Average Runoff Depth = 5.05" 0.00% Pervious = 0.000 ac 100.00% Impervious = 0.251 ac

Type IA 24-hr 100yr Rainfall=6.00" Printed 3/23/2020

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■ Runoff

Summary for Subcatchment 2S: Roof

Runoff

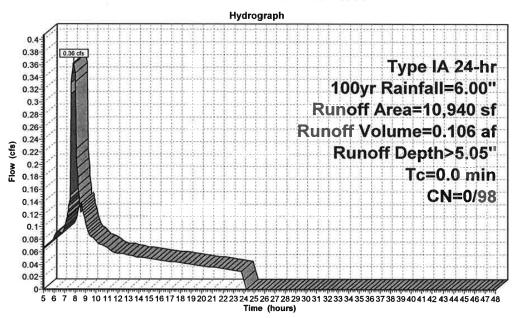
0.36 cfs @ 7.80 hrs, Volume=

0.106 af, Depth> 5.05"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 5.00-48.00 hrs, dt= 0.05 hrs Type IA 24-hr 100yr Rainfall=6.00"

	Area (sf)	CN	Description	
*	10,940	98	Roof	
	10.940	98	100 00% Impervious Area	

Subcatchment 2S: Roof



Type IA 24-hr 100yr Rainfall=6.00" Printed 3/23/2020

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Summary for Pond 2P: Drywell

Inflow Area = 0.251 ac,100.00% Impervious, Inflow Depth > 5.05" for 100yr event

Inflow = 0.36 cfs @ 7.80 hrs, Volume= 0.106 af

Outflow = 0.02 cfs @ 5.20 hrs, Volume= 0.074 af, Atten= 94%, Lag= 0.0 min

Discarded = 0.02 cfs @ 5.20 hrs, Volume= 0.074 af

Routing by Stor-Ind method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs Peak Elev= 16.98' @ 24.00 hrs Surf.Area= 452 sf Storage= 3,170 cf

Plug-Flow detention time= 1,057.8 min calculated for 0.074 af (70% of inflow) Center-of-Mass det. time= 880.3 min (1,592.6 - 712.3)

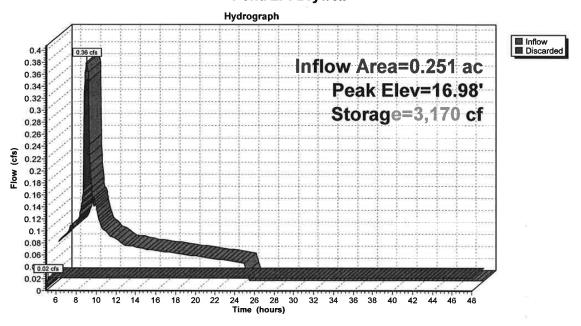
Volume	Invert	Avail.Storage	Storage Description
#1	0.00'	3,482 cf	24.00'D x 20.00'H Vertical Cone/Cylinder
			9,048 cf Overall - 342 cf Embedded = 8,706 cf x 40.0% Voids
#2	0.00'	251 cf	4.00'D x 20.00'H Vertical Cone/Cylinder Inside #1
			342 cf Overall - 4.0" Wall Thickness = 251 cf

3,734 cf Total Available Storage

Device	Routing	Invert	Outlet Devices	
#1	Discarded	0.00'	2.000 in/hr Exfiltration over Surface area	

Discarded OutFlow Max=0.02 cfs @ 5.20 hrs HW=0.24' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.02 cfs)

Pond 2P: Drywell



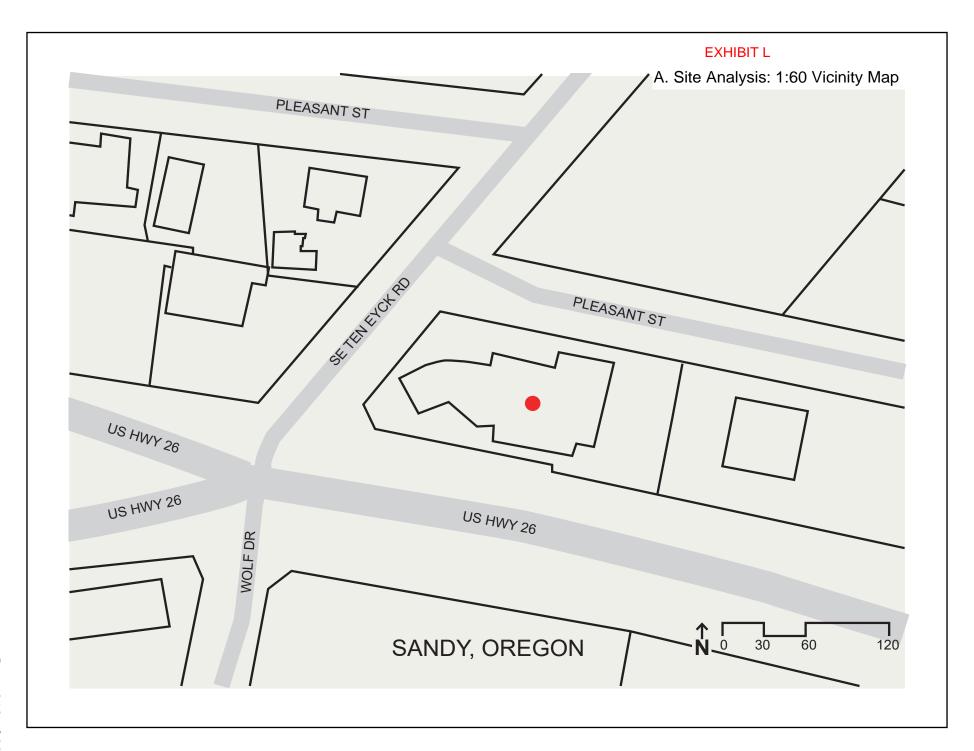
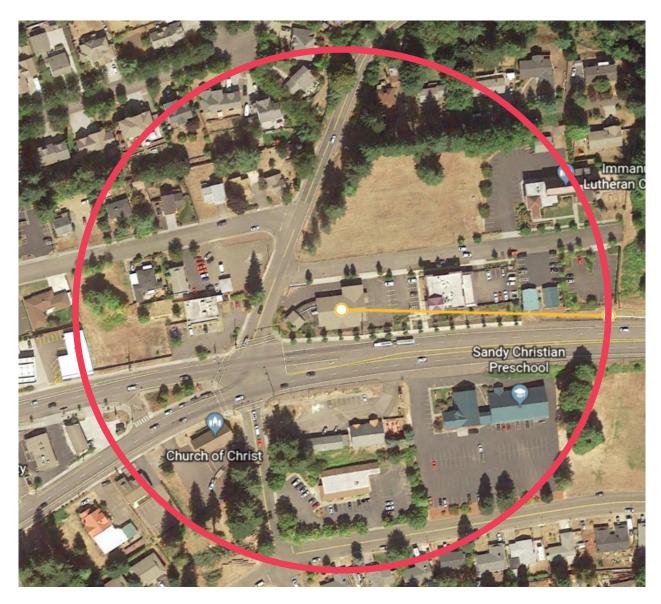


EXHIBIT M

A. Site Analysis: 500 FT Radius



500 ft radius

EXHIBIT N

CURRAN-MCLEOD, INC. CONSULTING ENGINEERS 6655 SW HAMPTON, SUITE 210 PORTLAND, OR 97223

May 29, 2020

Ms. Emily Meharg City of Sandy 39250 Pioneer Blvd. Sandy, OR 97055

RE: CITY OF SANDY

SANDY HEALTH CLINIC (File # 20-006 DR//VAR/DEV/ADJ)

PRELIMINARY REVIEW

Dear Emily:

We have reviewed the preliminary submittal for the above noted development and have the following comments/ recommendations:

- All earthwork activities shall follow the requirements of the most current edition of the Oregon Structural Specialty Code. Site grading shall not in any way impede or impound or inundate the surface drainage flow from the adjoining properties without a proper collection system. The earthwork activities shall be observed and documented under the supervision of the geotechnical Engineer.
- 2. The proposed driveway accesses on Pleasant Street shall be Concrete Commercial Driveway Approach constructed in conformance with the applicable City of Sandy driveway detail and meeting PROWAG requirements.
- 3. Where the existing driveway is removed on Pleasant Street, it shall be replaced with sidewalks and an ADA ramp shall be constructed at the intersection with Ten Eyck Road to current PROPWAG requirements.
- 4. Ten Eyck Road is a County road, we recommend the county requires sidewalks to be constructed along the entire site frontage to match the existing sidewalks on Hwy 26.
- 5. A demolition permit shall be required from the City prior to demoing the existing building.

- 6. We have reviewed the preliminary stormwater calculations that was provided with this submittal. The calculations were found not meeting the water quality/quantity criteria as stated in the City of Sandy Development Code (SDC) 13.18 Standards and the 2016 City of Portland Stormwater Management Manual (SWMM) Standards, that were adopted by reference into the Sandy Development Code. The water quality shall be designed based on 0.19 in/hr rate for 5 minutes time of concentration. While the water quality shall be designed for 2, 5, 10 and 25 year storm events and not 25 and 100 year storm events only. A detailed final report stamped by a licensed professional shall be resubmitted for review with the final construction plans.
- 7. The proposed 6" sanitary sewer service should be adequate to serve this building, unless the City public works department determines a sanitary sewer service exists and a new one is not needed.
- 8. The final construction plans shall be submitted to Sandy Fire Department for review and approval to ensure that the proposed vault has adequate fire protection and also acceptable access is provided to the building.
- 9. The final construction plans shall verify the domestic 2" meter size is adequate based on the meter flow and the building fixture counts found in the 2017 Oregon Plumbing Specialty Code, if larger than 2" meter size is needed shall be verified in the AWWA series 700 and the Oregon Plumbing Specialty Code.

We have no concerns about the proceedings with this project subject to the above stated comments.

Sincerely,

CURRAN-McLEOD, INC.

Hassan A. Ibrahim, P.E.

cc: Mr. Mike Walker, City of Sandy

EXHIBIT O



Emily Meharg <emeharg@ci.sandy.or.us>

Clackamas County Health Clinic (File No. 20-006 DR/VAR/DEV/ADJ)

Gary Boyles <fmboyles.sandyfire@gmail.com>
To: emeharg@ci.sandy.or.us
Cc: Don Patty <d.patty3710@gmail.com>

Wed. Jun 3, 2020 at 12:32 PM

Hi Emily,

The only comment I have regarding this application is that the new fire department connection (FDC) be relocated to the Mt. Hood Highway side of the proposed fire vault and to be as close as possible to the existing fire hydrant located in that area as possible.

Thank you,

Gary Boyles Fire Marshal

Sandy Fire District No. 72

PO Box 518

17460 SE Bruns Ave. Sandy, Oregon 97055

Business line: 503-668-8093 Cell number: 503-891-7042

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EXHIBIT P

REPLINGER & ASSOCIATES LLC

TRANSPORTATION ENGINEERING

June 8, 2020

Ms. Emily Meharg City of Sandy 39250 Pioneer Blvd. Sandy, OR 97055

SUBJECT: REVIEW OF TRANSPORTATION IMPACT ANALYSIS – SANDY HEALTH CLINIC

Dear Emily:

In response to your request, I have reviewed materials submitted in support of the Sandy Health Clinic in the northeast quadrant of Highway 26 and Ten Eyck Road in the east part of Sandy. The Transportation Impact Analysis (TIA), dated March 12, 2020, was prepared under the direction of John Manix, PE of PBS.

The site, currently occupied by a warehouse, is proposed to have a 9600-square foot health clinic. Access will be on Pleasant Street.

Overall

I find the TIA addresses the city's requirements and provides an adequate basis to evaluate impacts of the proposed development.

Comments

- 1. Study Area. The study addresses the appropriate intersections. It includes analyses of:
 - · Highway 26 at SE Ten Eyck Road;
 - Ten Eyck Road at Pleasant Street.
- 2. Traffic Counts. The AM and PM peak hour traffic counts were conducted during February 2020. The engineer adjusted the traffic counts to account for seasonal variations. The engineer adjusted the February counts by 28 percent to estimate the 30th highest hour traffic volumes. The methodology appears consistent with the procedures defined by the Oregon Department of Transportation (ODOT). The adjusted counts appear reasonable.

Ms. Emily Meharg June 8, 2020 Page 2

- 3. Trip Generation. The TIA uses trip generation for a medical clinic and for a warehouse (land use code 630 and 150, respectively) from the Institute of Transportation Engineers' (ITE) Trip Generation Manual. The warehouse calculation was used to calculate traffic from the existing use. After accounting for the warehouse, the engineer calculates that the medical clinic would produce 34 net new AM peak hour trips; 30 net new PM peak hour trips; and 335 net new daily trips. The calculation of trips generated by the development appears reasonable.
- 4. Trip Distribution. The TIA provided information about trip distribution from the site. Among other resources, the engineer consulted with county medical staff about clients. The engineer assumed 90 percent of the traffic would travel to and from the west on US 26 and 10 percent would travel to and from the east on US 26. The trip distribution seems reasonable.
- 5. Traffic Growth. The TIA uses a 2.0 percent annual increase for Highway 26 based on projected volumes from the Transportation System Plan. No adjustments were made for in process developments. The future year background traffic volumes appear reasonable.
- 6. Analysis. Traffic volumes were calculated for the intersections cited in #1, above. Intersection level-of-service (LOS) and the volume-to-capacity (v/c) ratio were provided. The intersection of US 26 with SE Ten Eyck Road is signalized; the intersection of Ten Eyck Road and Pleasant Street is stop-controlled. The analyses were conducted for existing 2020 conditions, 2022 background conditions, 2022 with the development, 2029 background conditions, and 2029 with the development.

The engineer calculated that the intersection of Highway 26 and Ten Eyck Road would operate at LOS B or better and a v/c ratio of 0.66 or better during the AM peak hour. For the PM peak hour, he calculated the intersection would operate at LOS C or better and a v/c ratio of 0.84 or better under all conditions. This meets ODOT's performance standard.

The engineer did not report the LOS or v/c ratio for the intersection of Ten Eyck Road with Pleasant Street. Both the east leg and west leg approaches of Pleasant Street with Ten Eyck Road were analyzed in a simulation that showed the anticipated queues and blockage time. Due to the low traffic volumes and short predicted queues on Pleasant Street, it is apparent that operations of the intersection will be good even with the proposed development. My own calculations using Synchro indicated the intersection will operate at LOS A, meeting city operational standards.

Ms. Emily Meharg June 8, 2020 Page 3

The engineer provides a thorough discussion of queuing issues using traffic volumes for 2029. During the PM peak hour, eastbound left-turn and eastbound right-turn queues are calculated to exceed available storage both with and without the development. In addition, southbound queues on Ten Eyck Road are expected to block nearby driveways and intersections both with and without the development. The engineer notes that the addition of a southbound left-turn lane may shorten queues. He recommended tracking volumes and queues over time to assess queuing storage needs.

The engineer also evaluated the effect of a westbound right-turn lane on Highway 26 at the intersection with Ten Eyck Road. He determined the performance of the intersection is not significantly different with a turn lane and meets v/c standards without it. He recommends against a westbound right-turn lane. He recommends retaining the existing configuration that features a slip lane.

7. Crash Information. The TIA provides information on crashes for the most recent available five-year period covering 2014 through 2018.

At the intersection of US 26 and SE Ten Eyck Road, there were eleven reported crashes. Rear-end crashes were the most common type. This is typical of signalized intersections in an urban area. The intersection has a relatively low crash rate of 0.26 crashes per million entering vehicles. The engineer concluded that no further investigation or mitigation is required. I concur.

- 8. Site Plan and Access. The site plan provides for a single access on Pleasant Street near the parcel's east boundary. The TIA indicates safety will be improved by the elimination of two existing driveways serving the site, including one on Ten Eyck Road. The site access is an improvement relative to existing conditions.
- Sight Distance. The engineer analyzed sight distance at the proposed access and concludes stopping sight distance is met. He recommends maintaining 200 feet of sight distance at the access.
- 10. Conclusions and Recommendations. The engineer concludes that the intersections will meet ODOT and city operational standards for the study area intersections either with or without the development. He also indicates that queuing is not significantly different with or without the development, but that queue storage will be exceeded by 2029 for some movements.

He found crash rates at the intersection of Highway 26 and Ten Eyck Road to be low and did not recommend further investigation. He concluded that a right-turn lane for

Ms. Emily Meharg June 8, 2020 Page 4

Highway 26 westbound was not warranted and recommended retaining the existing slip lane.

He concluded the reduction in accesses to the site, including the elimination of a driveway to Ten Eyck Road, would be beneficial and improve safety.

He recommended monitoring traffic volumes and queuing at the Highway 26 and Ten Eyck Road intersection and reevaluating the intersection in connection with a future Transportation System Plan update.

I concur with the engineer's conclusions.

Conclusion and Recommendations

Based on the information provided by the applicant, I find the TIA meets city requirements. The engineer used appropriate methods and documents his procedures and conclusions.

The intersections of Highway 26 and Ten Eyck Road and Ten Eyck Road and Pleasant Street are calculated to meet ODOT and city performance standards. I do not find a need for mitigation measures to address traffic impacts of the development or to address safety issues.

To the extent that the developer may be required to implement projects or participate in projects involving facilities under the jurisdiction of ODOT, conditions of approval should be included requiring that the development comply with the requirements standards and procedures specified by ODOT. I recommend that that ODOT requirements and standards associated with frontage improvements where the development abuts Highway 26 be made conditions of approval for the development.

If you have any questions or need any further information concerning this review, please contact me at replinger-associates@comcast.net.

Sincerely,

John Replinger, PE Principal

John Keplinger

SandyMedicalTIS060820

EXHIBIT Q



Department of Transportation

Region 1 Headquarters 123 NW Flanders Street Portland, Oregon 97209 (503) 731.8200 FAX (503) 731.8259

June 17, 2020 ODOT #10415

Updated ODOT Response

Project Name: Sandy Health Clinic	Applicant: Steve Kelly
Jurisdiction: City of Sandy	Jurisdiction Case #: 20-006 DR/VAR/DEV/ADJ
Site Address: 39831 US Hwy 26, Sandy, OR 97055	Legal Description: 02S 04E 13ad Tax Lot(s): 01000
State Highway: US 26	

The site of this proposed land use action is adjacent to US 26. ODOT has permitting authority for this facility and an interest in ensuring that this proposed land use is compatible with its safe and efficient operation. ODOT has reviewed the Traffic Impact Analysis for the proposed medical clinic development and determined that a westbound right turn lane at the US 26/SE Ten Eyck Rd intersection is not warranted.

ODOT has determined there will be no significant impacts to state highway facilities and no additional state review is required.

Please send a copy of the Notice of Decision including conditions of approval to:

ODOT Region 1 Planning Development Review 123 NW Flanders St Portland, OR 97209

ODOT_R1_DevRev@odot.state.or.us

Development Review Planner: Marah Danielson	503.731.8258,
	marah.b.danielson@odot.state.or.us

MEMORANDUM

TO: EMILY MEHARG, ASSOCIATE PLANNER

FROM: MIKE WALKER, PUBLIC WORKS DIRECTOR RE: FILE 20-006 CLACKAMAS COUNTY HEALTH CLINIC

DATE: JUNE 18, 2020

The following are Public Works' comments on the above-referenced application:

Transportation

The applicant shall improve the US 26 frontage of the site in compliance with ODOT requirements including modifying the existing slip/right-turn lane to reduce the curb radius at the corner to accommodate east-west and north-south ADA compliant ramps if required by ODOT. This may require right-of-way dedication at the NE corner of US 26 and Ten Eyck Rd.

The existing driveway approach onto Ten Eyck Rd. from the site does not meet the minimum spacing standards in 17.98.80(A) of the Sandy Municipal Code (SMC). The applicant shall abandon the existing driveway approach and improve the Ten Eyck Road frontage of the site including but not limited to: curbs, sidewalks, storm drainage, streetlighting, street trees per the requirements in sections 15.20 and 17.84.30 of the SMC. This section of Ten Eyck Rd. is under the jurisdiction of Clackamas County. The applicant shall coordinate with Clackamas County DTD to determine the required section for Ten Eyck. This may include relocating the existing fire hydrant at the intersection of Ten Eyck Rd. and Pleasant Ave. to install ADA compliant access ramps. Ten Eyck Road is a minor arterial street. Both Clackamas County and City of Sandy require minimum 6 ft. wide sidewalks on arterial streets. The County standard and the proposed ultimate section for Ten Eyck Road will only permit a curb tight sidewalk in the available right-of-way.

The City recently designed a pedestrian improvement project on Ten Eyck Rd. at the subject site. The Clackamas County DTD plan review fee has been paid and the plans have been approved by DTD. The applicant will be responsible for these improvements and is welcome to use the approved planset for this work.

The applicant shall dedicate sufficient right-of-way at the SW corner of the site to accommodate the required street section and pedestrian improvements including ADA-compliant ramp(s) at the intersection with US 26. The exact dedication area shall be determined during construction plan review. The applicant shall be responsible for providing legal descriptions and sketch maps of the dedication area, dedicating the right-of-way using the City's standard documents, and pay all recording costs.

The applicant shall remove the existing west driveway approach onto Pleasant St. The existing driveway approach doesn't to meet the minimum spacing standard in 17.98.80(A) SMC or the maneuvering standard in 17.98.70(B). The applicant shall

improve the Pleasant St. frontage of the site including but not limited to: curbs, sidewalks, storm drainage, streetlighting, street trees per the requirements in sections 15.20 and 17.84.30 of the SMC. The sidewalk shall be curb-tight, minimum 8 ft. in width with street trees specified by the City in tree wells on XX foot centers. The sidewalk shall be five feet wide separated from the curb with a five foot wide planter strip including street trees specified by the City on XX foot centers.

Street tree and landscaping placement shall conform to the sight distance recommendations in the traffic impact analysis submitted by the Applicant.

Utilities

The site is served by the existing 16" water main in US 26 and the existing 8" sanitary sewer main in Pleasant St. The applicant is proposing a drywell for stormwater disposal. Typically, the soils in Sandy do not permit treatment and discharge of stormwater in this manner. Stormwater management shall conform to the requirements in the City of Portland Stormwater Management Manual and the requirements in section 13.18 and 13.20 SMC.

Utility and right-of-way improvement plans are submitted with the land use application solely for conformance with the submittal requirements in Section 17.100.60(D). Land use approval does note connote approval of public improvement plans.

Please let me know if you have any questions or need more information.





Emily Meharg <emeharg@ci.sandy.or.us>

Incompleteness Letter: 20-006 DR/VAR/DEV/ADJ

Scott Soukup <scotts@ankrommoisan.com>

Wed, Apr 1, 2020 at 9:21 AM

To: Emily Meharg <emeharg@ci.sandy.or.us>

Cc: Marisol Martinez martinez@ci.sandv.or.us, "Kelly O'Neill Jr." koneill@ci.sandv.or.us, "Kelly O'Neill Jr." <a href="martinez@ci.sandv.or.us, "Kelly O'Neill Jr." <a href="martinez@ci.sandv.or.us, "Kelly O'Neill Jr." <a href="martinez@ci.s <SteveKel@clackamas.us>, Lori Kellow <lorik@ankrommoisan.com>

Hi Emily,

Here is the credit card authorization form.

For the vertical Nichiha siding, Section 17.90.110(B.3.d) allows composite-wood (concrete fiberboard, panels or shingles). The vertical ribbed Nichiha product is a panelized fiber cement product with 1 5/8" vertical slats and 3/8" reveals between each slat which add depth and rustic texture to the façade. The product is available in custom colors that can match any selected Miller or Sherwin Williams paint. The ribbed fiber cement is not the same as board-andbatten siding or T1-11 sheet siding.

The product can be installed either vertically or horizontally. In terms of design, the vertical siding is a darker earth tone and is the bulk of the siding. The vertical orientation was selected to distinguish it from the horizontal redwood Nichiha material pops of warm color at the bumpouts and recesses in the facade. Both siding products are Nichiha so all the siding can come from a single source manufacturer. This will make the construction process smoother and simplify the detailing. The vertical siding has been selected as the main siding, because the vertical orientation is better for cleaning and maintenance over time.

[Quoted text hidden]

Credit Card Authorization Form_signed 4.1.2020.pdf 337K

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CITY OF SANDY

SE TEN EYCK ROAD & PLEASANT STREET **CURB & SIDEWALK IMPROVEMENTS**



CLACKAMAS COUNTY, OREGON **DECEMBER 2018**

CITY OF SANDY

MAYOR HONORABLE BILL KING COUNCILOR JEREMY PIETZOLD COUNCILOR JOHN HAMBLIN COUNCILOR SCOTT HORSFALL COUNCILOR JAN LEE COUNCILOR CARL EXNER JEAN CUBIC COUNCILOR KIM YAMASHITA CITY MANAGER DIRECTOR OF PUBLIC WORKS MIKE WALKER

SHEET INDEX

- C1 COVER SHEET AND INDEX
- C2 SE TEN EYCK ROAD & PLEASANT STREET **CURB & SIDEWALK IMPROVEMENTS**
- C3 CLACKAMAS COUNTY CONSTRUCTION DETAILS
- C4 CITY OF SANDY CONSTRUCTION DETAILS

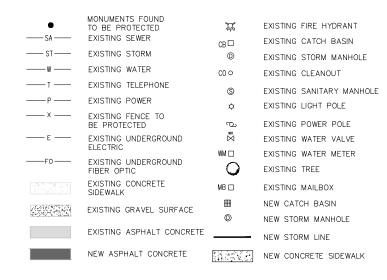
CALL BEFORE YOU DIG

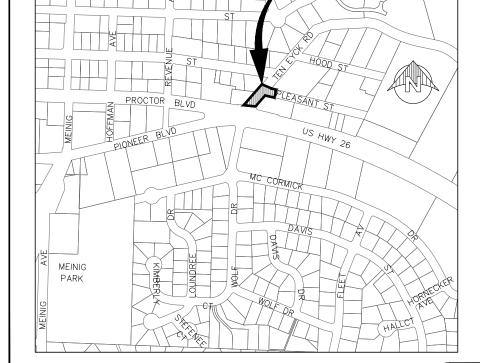
ATTENTION:
OREGON LAW REOUIRES YOU TO FOLLOW RULES ADOPTED BY THE OREGON UTILITY NOTIFICATION CENTER. THOSE RULES ARE SET FORTH IN OAR 952-001-0010 THROUGH OAR 952-001-0090. YOU MAY OBTAIN COPIES OF THESE RULES BY CALLING THE CENTER. (NOTE: THE TELEPHONE NUMBER FOR THE OREGON UTILITY NOTIFICATION CENTER IS (503) 232-1987) OR 811 OR 1-800-332-2344

> ALL UTILITY CROSSINGS ARE APPROXIMATE, CONTRACTOR TO POTHOLE AND FIELD DEPTH VERIFY PRIOR TO CONSTRUCTION AND CONSULT WITH THE ENGINEER REGARDING ANY CONFLICTS

CONTRACTOR TO PROTECT EXISTING POWER, TELEPHONE/TELECOMMUNICATION LINES & COORDINATE RELOCATION OF ANY LINES THAT ARE IN CONFLICT WITH THE CONSTRUCTION WITH APPROPRIATE AGENCY

LEGEND





PROJECT

LOCATION MAP

REVISIONS



CURRAN-McLEOD, INC CONSULTING ENGINEERS

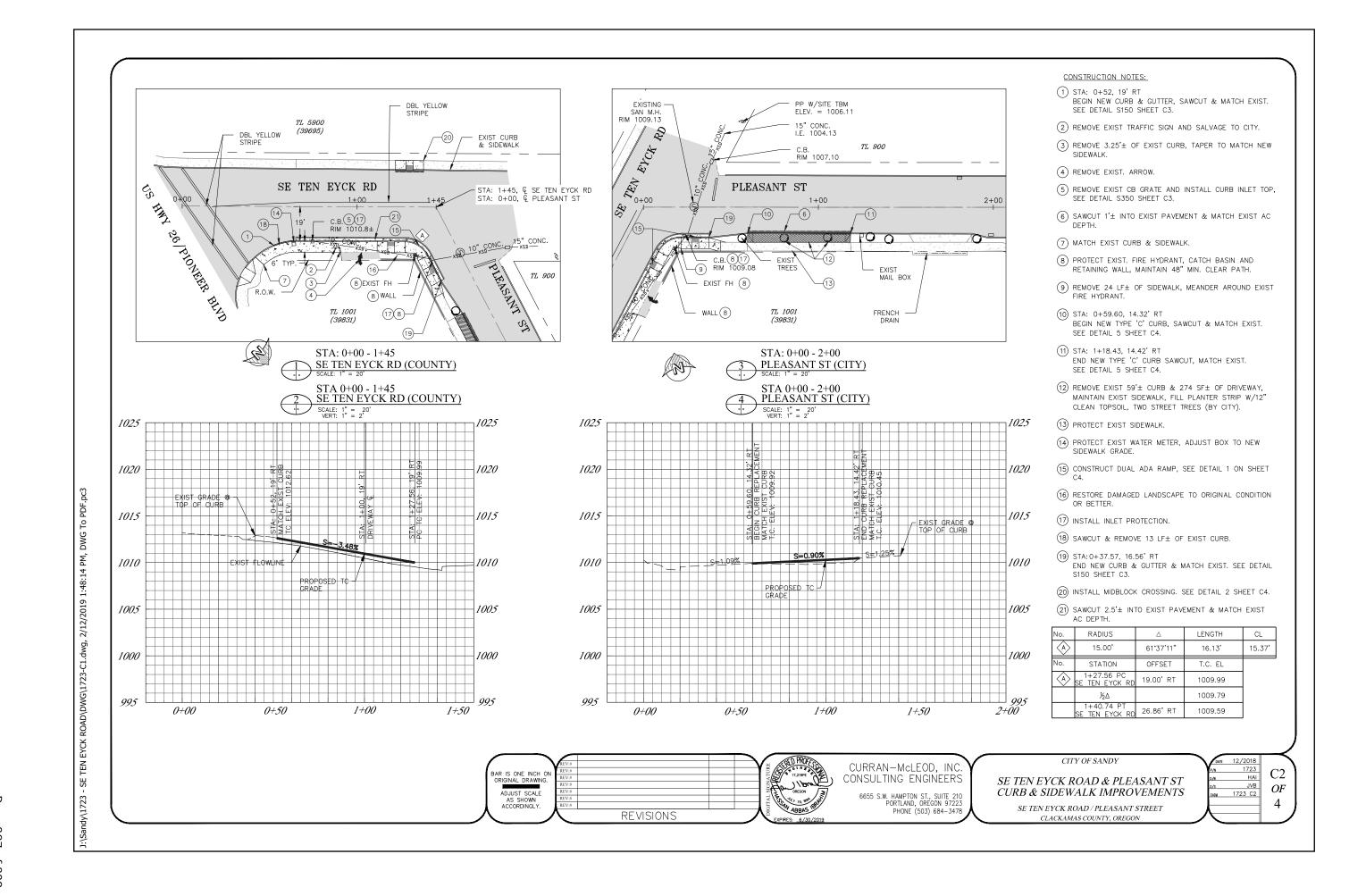
6655 S.W. HAMPTON ST., SUITE 210 PORTLAND, OREGON 97223

CITY OF SANDY

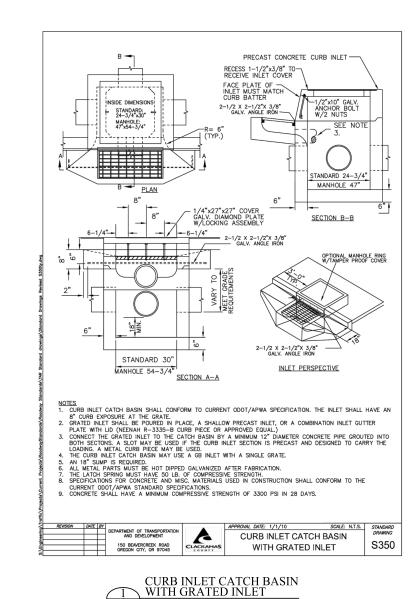
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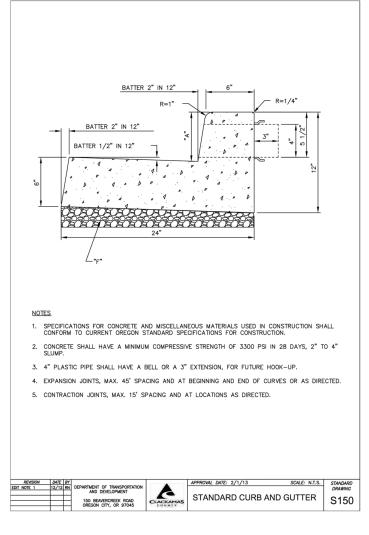
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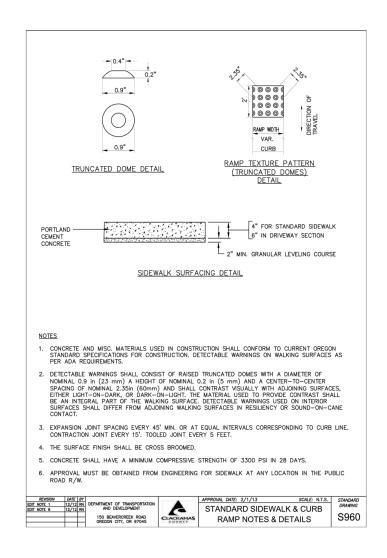
Page 326 of 339





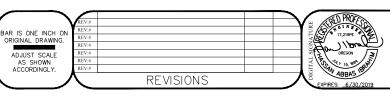












CURRAN-McLEOD, INC. CONSULTING ENGINEERS

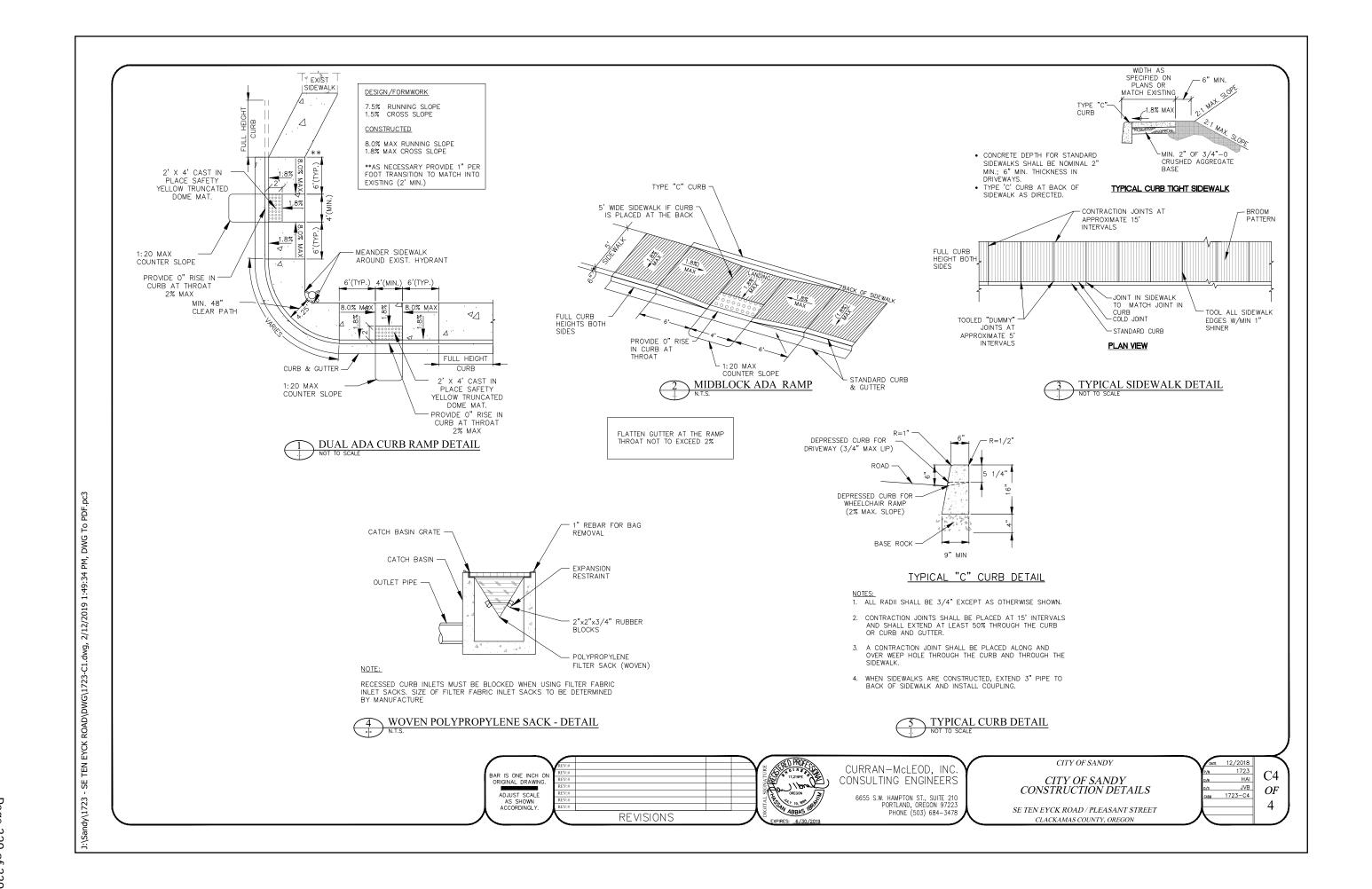
6655 S.W. HAMPTON ST., SUITE 210 PORTLAND, OREGON 97223 PHONE (503) 684-3478

CITY OF SANDY CLACKAMAS COUNTY

CONSTRUCTION DETAILS $SE\ TEN\ EYCK\ ROAD\ /\ PLEASANT\ STREET$ CLACKAMAS COUNTY, OREGON

12/2018 HAI JVB OF 1723-1

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Staff Report

Meeting Date: June 30, 2020

From Shelley Denison, Associate Planner

SUBJECT: 20-012 DCA 5G Small Cell Code Change

Background:

File No. 20-012 DCA proposes an additional section to Title 12 (12.20) of Sandy's Municipal Code to regulate the siting and franchising of 5G small cell "stealth" facilities. New cellular technology has developed with the addition of 5G wireless technology. Rather than requiring cell towers, 5G requires small cell "stealth" facilities. These facilities can be sited on existing utility poles within a city. Because of this, cities are given some degree of legal leeway to regulate the siting and franchising of small cell facilities.

The municipal code does not currently contain any regulatory language related to small cell facilities. The purpose of this proposed Chapter 12.20 is to establish reasonable and nondiscriminatory policies and procedures for the placement of small wireless facilities in the right-of-way within the City's jurisdiction, consistent with and to the extent permitted by federal and state law, in order to provide public benefit consistent with the preservation of the integrity, safe usage, and reasonable aesthetic qualities of the City rights-of-way and the City as a whole.

Recommendation:

Staff recommends the Planning Commission forward a recommendation of approval for the proposed code addition to the City Council.

Code Analysis:

N/A

Budgetary Impact:

N/A

12.20 SMALL WIRELESS FACILITIES

12.20.010 - Purpose and Scope

- (A) <u>Purpose</u>. The purpose of this Chapter is to establish reasonable and nondiscriminatory policies and procedures for the placement of small wireless facilities in the right-of-way within the City's jurisdiction, consistent with and to the extent permitted by federal and state law, in order to provide public benefit consistent with the preservation of the integrity, safe usage, and reasonable aesthetic qualities of the City rights-of-way and the City as a whole.
- (B) <u>Intent</u>. In enacting this Chapter, the City is establishing uniform standards consistent with federal law to address the placement of small wireless facilities and associated poles in the rights-of-way, including without limitation, to manage the public right of way in order to:
 - (1) prevent interference with the use of streets, sidewalks, alleys, parkways and other public ways and places;
 - (2) prevent the creation of obstructions and other conditions that are hazardous to vehicular and pedestrian traffic;
 - (3) prevent interference with the facilities and operations of facilities lawfully located in rights-of-way or public property;
 - (4) protect against environmental damage, including damage to trees;
 - (5) preserve the character of the community, Historic Districts or areas with Decorative Poles; and
 - (6) facilitate technology advancements, such as deployment of small wireless facilities, to provide the benefits of wireless services.

12.20.020 - Definitions

(A) "Accessory equipment" means antenna equipment as defined in 47 C.F.R. § 1.6002(c), as may be amended or superseded, which defines the term to mean equipment, switches, wiring, cabling, power sources, shelters or cabinets associated with an antenna, located at the same fixed location as the antenna, and, when collocated on a structure, is mounted or installed at the same time as such antenna.¹

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¹ The FCC uses the term "antenna equipment" to mean the non-antenna accessory equipment associated with a small cell. The City finds this term confusing because using "antenna equipment" to describe equipment that is not antenna appears contrary to term on its face. In fact, the FCC's full definition of "antenna equipment" refers to 47 C.F.R. § 1320(d), which includes a definition for "antenna" that is essentially a combination of "antenna" and "antenna equipment" as defined in 47 C.F.R. § 1.6002. The reference to § 1320(d) has been omitted from the definition and the City uses "accessory equipment" in this Chapter 12.20 to avoid confusion.

- (B) "Antenna" means the same as defined in 47 C.F.R. § 1.6002(b), as may be amended or superseded, which defines the term to mean an apparatus designed for the purpose of emitting radiofrequency (RF) emission, to be operated or operating from a fixed location pursuant to Federal Communication Commission authorization, for the provision of personal wireless service and any commingled information services. For purposes of this definition, the term antenna does not include an unintentional radiator, mobile station, or device authorized under 47 C.F.R. Part 15.
- (C) "Antenna facility" means the same as defined in 47 C.F.R. § 1.6002(d), as may be amended or superseded, which defines the term to mean an antenna and associated accessory equipment.²
- (D) "Applicable codes" means, without limitation, uniform building, fire, safety, electrical, plumbing, or mechanical codes adopted by a recognized national code organization or state or local amendments to those codes that are of general application and consistent with state and federal law.
- (E) "Applicant" means any person duly authorized to submit an application as or on behalf of a wireless provider.
- (F) "Application" or "applications" means a request(s) submitted by an applicant for permission to collocate small wireless facilities on an existing, modified, new or replacement structure.
- (G) "City structure" means a structure in the rights of way owned, managed or operated by the City including, but not limited to streetlights, traffic signals, utility poles and other structures.
- (H) "Collocate" or "collocation" means the same as defined in 47 C.F.R. § 1.6002(g), as may be amended or superseded, which defines that term to mean (1) mounting or installing an antenna facility on a preexisting structure, and/or (2) modifying a structure for the purpose of mounting or installing an antenna facility on that structure.
- (I) "Day" means calendar day. For purposes of the FCC shot clock, a terminal day that falls on a holiday or weekend shall be deemed to be the next immediate business day.
- (J) "Decorative pole" means a city structure that is specially designed and placed for aesthetic purposes.
- (K) "Historic district" means a group of buildings, properties, or sites that are either: (1) listed in the National Register of Historic Places or formally determined eligible for listing by

² As written, the definition uses the term "accessory equipment" in-lieu of "antenna equipment" for the reasons stated in the previous footnote.

- the Keeper of the National Register in accordance with Section VI.D.1a.i-v of the Nationwide Programmatic Agreement codified at 47 C.F.R. Part 1, Appendix C; or, (2) a locally designated historic district.
- (L) "Permissions" means those authorizations needed for deployment of Small Wireless Facilities.
- (M) "Person" means an individual, corporation, limited liability company, partnership, association, trust, or other entity or organization, including the City.
- (N) "Pole" means a type of structure in the rights-of-way that is or may be used in whole or in part by or for wireline communications, electric distribution, lighting, traffic control, signage, or similar function, or for collocation of small wireless facilities consistent with applicable codes; provided, such term does not include a tower, building or electric transmission structures.
- (O) "Right-of-way" means the same as provided in Chapter 12.02.050.
- (P) "Routine maintenance" means inspections, testing, repair, and modifications that maintain functional capacity, aesthetic and structural integrity of a small wireless facility and/or the associated pole or structure. Any work on a small wireless facility that would not require a permit (e.g., a traffic control permit, building permit, encroachment permit, etc.) qualifies as routine maintenance. As an illustration and not a limitation, routine maintenance would include, without limitation, one-for-one antenna or accessory equipment replacements but would not include technology upgrades that alter or add to the RF emissions from the antenna facility. Similarly, routine maintenance would include, without limitation, the installation of minor brackets or braces to harden an antenna facility but would not include the replacement or reinstallation of the underlying support structure.
- (Q) "Small wireless facility" means a facility that meets each of the following conditions per the 47 C.F.R § 1.6002(1), as may be amended or superseded:
 - (1) The facilities (i) are mounted on structures 50 feet or less in height including the antennas, or (ii) are mounted on structures no more than 10 percent taller than other adjacent structures, or (iii) do not extend existing structures on which they are located to a height of more than 50 feet or by more than 10 percent, whichever is greater; and
 - (2) Each antenna associated with the deployment, excluding associated accessory equipment, is no more than three cubic feet in volume; and

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- (3) All other wireless equipment associated with the structure, including wireless equipment associated with the antenna and any pre-existing associated equipment on the structure, is no more than 28 cubic feet in volume; and
- (4) The facilities do not result in human exposure to radio frequency in excess of the applicable safety standards specified in 47 C.F.R. § 1.1307(b).
- (P) "Structure" means the same as defined in 47 C.F.R. § 1.6002(m), as may be amended or superseded, which defines that term as "a pole, tower, or base station, or other building, whether or not it has an existing antenna facility, that is used or to be used for the provision of personal wireless service (whether on its own or comingled with other types of service)."
- (Q) "Technically feasible" means that the proposed placement, location or design for a small wireless facility can be implemented without a material reduction in the intended service objective of the small wireless facility.
- (R) "Wireless Provider" means either (1) any person who provides "personal wireless services", as defined in 47 U.S.C. § 332(c)(7)(C)(i), as may be amended or superseded; or (2) any person, including a person authorized to provide communications service in the state, that builds or installs wireless communication transmission equipment, wireless facilities, but does not provide personal wireless services.

12.20.030 - Applications and Fees

- (A) <u>Application Required</u>. Except as otherwise provided in this Chapter, no person shall place any small wireless facility in the right-of-way without first filing an application for the facility and obtaining all permits necessary for the installation.
- (B) <u>Application Requirements</u>.

An application filed pursuant to this Chapter shall be on forms provided by the City and, at a minimum, contain the following:

- (1) The applicant's name, address, telephone number, and e-mail address;
- (2) The names, addresses, telephone numbers, and e-mail addresses of all duly authorized representatives and consultants, if any, acting on behalf of the applicant with respect to the filing of the application;
- (3) A general description of the proposed small wireless facility and associated pole, if applicable. The scope and detail of such description shall be appropriate to the

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- nature and character of the work to be performed, with special emphasis on those matters likely to be affected or impacted by the physical work proposed;
- (4) Site plans and engineering drawings to scale that identify the proposed small wireless facility;
- (5) A statement that the small wireless facility shall comply with all applicable codes, regulations and standards, including a certification in a form acceptable to the City that the small wireless facility will comply with applicable FCC regulations for human exposure to RF emissions.
- (6) The applicant shall not be required to provide more information to obtain a small wireless facility permit than is required of other entities who install small wireless facilities in the rights-of-way.
- (C) Routine Maintenance. Routine maintenance in connection with an existing small wireless facility shall be permitted, subject only to any traffic control, encroachment or other regulatory authorizations as may be required for the specific scope of work. Notwithstanding anything to the contrary in this chapter, the applicant and/or permittee conducting routine maintenance shall not be relieved of its obligations to comply with all generally applicable health and safety regulations.
- (D) <u>Application Fees</u>. Application fees in accordance with applicable state and federal law shall be set by resolution of the City Council.

12.20.040 - Decisions; Notice of Decision

- (A) <u>Findings for Denial</u>. The City must process all applications on a nondiscriminatory basis and may deny an application subject to this Chapter if the proposed small wireless facility:
 - (1) Materially and demonstrably interferes with the safe operation of traffic control equipment;
 - (2) Materially and demonstrably interferes with sight lines or clear zones for transportation or pedestrians;
 - (3) Materially fails to comply with the Americans with Disabilities Act or similar federal, state, or local laws, standards and regulations regarding pedestrian access or movement;
 - (4) Fails to comply with applicable codes, standards and regulations, including the City's design standards; or
 - (5) Fails to comply with the provisions in this Chapter.

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- (B) Time for Final Action; Notice of Final Action. The City shall act on an application within the applicable shot clock and advise the applicant in writing of its final action. If the final action is to deny the application,³ The written notice shall state the reasons for denial, with reference to specific code provisions, ordinance, application instructions or otherwise publicly-stated procedures on which the denial was based, and send the notice to the applicant within five (5) days after the City denies the application or before the applicable shot clock expires, whichever occurs first.
- (C) <u>Batched Applications</u>. Applicants may submit batched applications as provided under applicable law, and the City shall act on such applications as provided in 12.20.040 (B) and consistent with applicable law. A batched application that includes only collocations on existing structures shall be subject to a 60-day timeline. A batched application that includes only new support structures shall be subject to a 90-day timeline. A batched application that includes both collocations and new support structures shall be subject to a 90-day timeline

12.20.050 - Special Design Requirements

- (A) <u>Decorative Poles</u>. Subject to the City's discretion, a wireless provider may be permitted to collocate on or replace a decorative pole when necessary to collocate a small wireless facility; provided that any such replacement pole shall, to the extent feasible, replicate the design of the pole being replaced.
- (B) Underground District. [NOTE: ACCORDING TO THE FCC ORDER,
 UNDERGROUNDING REQUIREMENTS ARE SUBJECT TO THE SAME CRITERIA
 AS OTHER AESTHETIC STANDARDS. AESTHETIC REQUIREMENTS
 (INCLUDING UNDERGROUNDING) MUST BE (1) REASONABLE; (2) NO MORE
 BURDENSOME THAN REQUIREMENTS IMPOSED ON OTHER
 INFRASTRUCTURE DEPLOYMENTS; (3) OBJECTIVE; AND (4) PUBLISHED IN
 ADVANCE.

IN THIS REGARD, A REQUIREMENT THAT ALL ELEMENTS OF ALL WIRELESS FACILITIES BE DEPLOYED UNDERGROUND WOULD AMOUNT TO AN EFFECTIVE PROHIBITION GIVEN THE PROPAGATION CHARACTERISTICS OF WIRELESS SIGNALS EMITTED FROM ANTENNAS. CITIES ARE ENCOURAGED TO REVIEW CURRENT UNDERGROUNDING REQUIREMENTS AND WORK WITH THEIR ATTORNEYS/ROW SPECIALISTS TO MAKE SURE THOSE REQUIREMENTS ARE NOT IN CONFLICT WITH THE FCC ORDER.]

(C) <u>Historic District</u>. Small wireless facilities or poles to support collocation of small wireless facilities located in Historic Districts shall be designed to have a substantially

³ Note that a "final action" for the purposes of federal regulations is not the same as a "final decision" for the purposes of Oregon state law.

similar appearance, including coloring and design elements, if technically feasible, of other poles in the rights-of-way within 500 feet of the proposed installation.

12.20.060 - Effect of Construction/Work Permit

(A) <u>Authority Granted</u>. A permit from the City authorizes an applicant to undertake only certain activities in accordance with this Chapter and does not create a property right or grant authority to the applicant to impinge upon the rights of others who may already have an interest in the rights-of-way.

(B) Permit Duration.

- (1) The build-out period for a permit for construction granted pursuant to this Section shall be valid for a period of one year after issuance.
- (2) The permit authorizing the use shall be coterminous with the applicable right-of-way license, franchise or other agreement granting the applicant access to the rights-of-way.
- (3) The installed facility is subject to applicable relocation requirements, termination for material non-compliance after notice and a reasonable opportunity to cure, and an applicant's right to terminate a permit at any time.

12.20.070 - Removal, Relocation or Modification of Small Wireless Facility in the ROW

- (A) Notice. The City shall provide the permittee reasonable advance written notice whenever the City has determined that such removal, relocation, change or alteration, is reasonably necessary for the construction, repair, maintenance, or installation of any City improvement in or upon, or the operations of the City in or upon, the rights-of-way (collectively, "City work"). The City shall specify a reasonable time for such removal, relocation, change or alteration in its notice, taking into account the nature and scope of the work involved and the urgency of the City's need for such work to be performed. Within the time specified in the written notice from the City, the permittee shall, at its own expense, protect, support, temporarily or permanently disconnect, remove, relocate, change or alter the position of any small wireless facilities within the rights-of-way in order to accommodate the City work.
- (B) Emergency Removal or Relocation of Facilities. The City retains the right and privilege to cut or move any small wireless facility located within the rights-of-way of the City in the event of an emergency, as the City may determine to be necessary, appropriate or useful in response to any imminent danger to public health, safety, or property. If practicable under the circumstances, the City shall notify the permittee and provide the permittee an opportunity to move its own facilities prior to cutting or removing a facility and shall notify the permittee promptly when practicable after cutting or removing a small wireless facility.

Page 7 of 9

- (C) Abandonment of Facilities. Within 90 days after a small wireless facility is abandoned, the permittee shall completely remove the small wireless facility and all related improvements and shall restore all affected areas to a condition compliant with all applicable codes. In the event that the permittee does not complete the obligations under this condition, or cause them to be completed, within said 90-day period, the City shall have the right (but not the obligation) to perform such removal and restoration with or without notice, and the permittee shall be liable for all costs and expenses incurred by the City in connection with such removal and/or restoration activities.
- (D) <u>Damage and Repair</u>. The City may require a permittee to repair all damage to the rights-of-way directly caused by the activities of the permittee or third parties acting under the permittee's direction and restore the rights-of-way to its the condition that existed before the damage occurred. All such repair work shall be performed in accordance with applicable laws and to the City Public Works Director's satisfaction. If the permittee fails to make the repairs within 30 days after written notice, the City shall have the right (but not the obligation) to affect those repairs, and the permittee shall be liable for all costs and expenses incurred by the City in connection with such repairs.

12.20.080 - Collocation on City Structures in the ROW

- (A) <u>Collocation on City Structures</u>. Small wireless facilities may be collocated on city structures in the rights-of-way pursuant to this Chapter. No person will be permitted an exclusive arrangement or an arrangement which excludes otherwise qualified applicants to attach to city structures in the rights-of-way. A person who purchases or otherwise acquires a City structure is subject to the requirements of this section.
- (B) <u>Make-Ready</u>. The rates, fees, terms and conditions for the make-ready work to collocate a small wireless facility on a pole owned or controlled by the City must be nondiscriminatory, competitively neutral and be subject to the following:
 - (1) The City or any person owning, managing, or controlling the poles owned by the City will provide a good faith estimate for any make-ready work reasonably necessary to make a specific city pole suitable for attachment of the requested small wireless facility, including pole replacement if necessary, within 60 days after receipt of a completed request. Make-ready work including any pole replacement shall be completed within 60 days of written acceptance of the good faith estimate by the applicant.
 - (2) The City or any person owning, managing, or controlling the poles owned by the city shall not require more make-ready work than required to meet applicable codes or may be reasonably necessary to avoid interference with other attachments on the pole. Fees for make-ready work shall not include costs related

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to pre-existing or prior damage and non-compliance, unless such fees are necessary to accommodate the proposed attachment on the pole. Fees for makeready work including any pole replacement shall not exceed actual and direct costs, or the amount charged to others for similar work and shall not include any contingency based consultant fees or expenses of any kind.

12.20.090 - Rates for ROW and Collocation on City Structures in the ROW

- (A) The recurring rate for use of the ROW and attachment of small wireless facilities to a City structure in the ROW shall be subject to the following requirements:
 - (1) <u>Annual Rate</u>. A person authorized to place small wireless facilities and any related pole in the rights-of-way will pay to the City compensation for use of the rights-of-way and collocation on city structures in the ROW a rate in accordance with applicable state and federal law and set by resolution of the City Council.
 - (2) <u>Cease Payment</u>. A person authorized to use the ROW and/or city structures for a small wireless facility may remove its facilities at any time from the rights-of-way and city structures in the ROW with the required permits and cease paying the City compensation as of the date of the complete removal of the facilities and restoration of the site to the condition that existed prior to the deployment.

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