# City of Sandy 



Agenda
Planning Commission Meeting
Meeting Location: City Hall- Council Chambers, 39250 Pioneer Blvd., Sandy, Oregon 97055
Meeting Date: Tuesday, June 30, 2020
Meeting Time: 6:30 PM

## Page

## 1. MEETING FORMAT NOTICE

Note: The Planning Commission will conduct this meeting electronically using the Zoom video conference platform. Members of the public may listen, view, and/or participate in this meeting using Zoom. Using Zoom is free of charge. See the instructions below:

Please click the link below to join the webinar:
https://us02web.zoom.us/i/81660200390

Or Telephone:
+1 6699006833
Webinar ID: 81660200390
International numbers available: https://us02web.zoom.us/u/kZXUQz8av
2. ROLL CALL
3. APPROVAL OF MINUTES
3.1. Draft Planning Commission Minutes for May 27, 2020
Planning Commission - 27 May 2020 - Minutes - Pdf
4. REQUESTS FROM THE FLOOR - CITIZEN COMMUNICATION ON NON- AGENDA ITEMS

## 5. PUBLIC COMMENT

This meeting will include two public hearings. If you would like to offer testimony during the hearings, see the instructions below:

Testimony for each public hearing will be called for in three groups: testimony in favor of the proposal, testimony opposed to the proposal, and neutral testimony.

If you are participating online, click the "raise hand" button at the appropriate time
and wait to be recognized.

If you are participating via telephone, dial *9 to "raise your hand" at the appropriate time and wait to be recognized.

If you choose to submit testimony in written form, please send to planning@ci.sandy.or.us as soon as possible.

Thank you for your flexibility during the COVID-19 public health emergency. Please call City Hall with any questions: (503) 668-5533.

## 6. NEW BUSINESS

6.1 $\quad$ 20-021 EXT Jewelberry Ridge Subdivision Extension12-1820-021 EXT Jewelberry Ridge Subdivision Extension - Pdf6.2. $20-006$ DR/VAR/DEV/ADJ Clackamas County Health Clinic ..... 19-329
20-006 DR/VAR/DEV/ADJ Clackamas County Health Clinic - Pdf
6.3. 20-012 DCA 5 G Small Cell Code Change ..... 330-339
20-012 DCA 5G Small Cell Code Change - Pdf
7. ITEMS FROM COMMISSION AND STAFF
8. ADJOURN

MINUTES<br>Planning Commission Meeting Wednesday, May 27, 2020 Virtual via Zoom 7:00 PM

COMMISSIONERS PRESENT: Don Carlton, Commissioner, Ron Lesowski, Commissioner, Hollis MacLean-Wenzel, Commissioner, Jerry Crosby, Commissioner, John Logan, Commissioner, Chris Mayton, Commissioner, and Todd Mobley, Commissioner

## COMMISSIONERS ABSENT: None

STAFF PRESENT:
Kelly O'Neill, Development Services Director, Emily Meharg, Senior Planner, David Doughman, City Attorney, and Shelley Denison, Associate Planner

MEDIA PRESENT:
None

## 1. Meeting Format Notice

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+1-669-900-6833
Webinar ID: 83947809040
International numbers available: https://us02web.zoom.us/u/kb7YbrTKLH
2. Roll Call

Chairman Crosby called the meeting to order at 7:02 p.m.
3. Approval of Minutes
3.1. Draft Planning Commission Minutes for April 27, 2020

Motion: To approve minutes for April 27, 2020.
Moved By: Commissioner Logan

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Seconded By: Commissioner Mayton
Yes votes: All Ayes
No votes: None
Abstentions: None
The motion passed.
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## 4. Requests From the Floor - Citizen Communication on Non- Agenda Items ZOOM WEBINAR CHAT: <br> From Kathleen Walker to All panelists:

"Not sure if this is applicable, but Hollis's email on the website is incorrect and bounding back emails. So if you used that to send her zoom invite, it may be what is causing problems....KW"

From Kelly O'Neill Jr. to All panelists:
"Okay thanks for the heads up."

Kathleen Walker
15920 Bluff Road
Sandy, OR 97055
Commented that Hollis Maclean-Wenzel's email address on the City of Sandy website is not working. Kelly O'Neill Jr. said that IT can likely look into the email address concerns. Greg Brewster stated that IT can resolve any potential issues with Commissioner Maclean-Wenzel's email account.

## 5. Public Comment

This meeting will include two public hearings. If you would like to offer testimony during the hearings, see the instructions below:

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## 6. NEW BUSINESS

### 6.1. Gunderson Road \& Parkland Annexation (20-001 ANN/CPA/ZC):

Chairman Crosby opened the public hearing on File No. 20-001 ANN/CPA/ZC at 7:14 p.m. Crosby called for any abstentions, conflicts of interest, ex-parte contact, challenges to the jurisdiction of the Planning Commission, or any challenges to any individual member of the Planning Commission. Commissioner Mobley abstained. Commissioner Mayton declared he received an email from Kathleen Walker concerning the application. Commissioner Carlton also received an email from Kathleen Walker. The email is included in the record so is not ex parte. Chairman Crosby requested that members of the public send comments to staff only, not to the individual Commissioners. No challenges were made, and no declarations were made by the Planning Commissioners.

## Staff Report:

Development Services Director Kelly O'Neill Jr. summarized the staff report. O'Neill updated the Commission on the UGB expansion hearing dates, which were rescheduled due to COVID-19 concerns and the Bailey Meadows Subdivision decision. O'Neill stated the annexation proposal is for parkland, a road, and associated facilities; there will be no additional housing even though some of the land will be zoned residential. Commissioner Carlton mentioned Kathleen Walker's comment regarding houses facing the park and asked Director O'Neill to pull up a picture of the plat. O'Neill stated the proposal does not include a review of Chapter 17.86. Doughman reiterated that the decision about the parkland was part of the subdivision application and that the criteria in Chapter 17.86 aren't included in tonight's decision.

Applicant Testimony:
Mike Robinson
1211 SW 5th Ave. Suite 1900
Portland, OR, 97204
Robinson alerted staff and the Commissioners that they only received Kathleen Walker's letter. O'Neill clarified that the Fair Housing Council did not submit testimony on this application.

## Chris Goodell

12965 SW Herman Rd. Suite 100
Tualatin, OR 97062
Goodell stated the applicant reviewed the staff report and agrees with it.

## Proponent Testimony:

None

## Opponent Testimony:

Kathleen Walker
15920 SE Bluff Rd.

Sandy, OR 97055
Walker stated the parkland doesn't meet the existing code and that the final staff report didn't address it. Walker explained to the residents of Sandy what kind of park they'll end up with. She stated the park will have no vehicle access, no sidewalks for pedestrian access, won't be graded or seeded, and will be vacant land. Walker stated none of the park requirements have been met.

ZOOM WEBINAR CHAT:
From Kathleen Walker to All panelists:
"It's code not my opinion! They have to pay for half od the oroad and the sidewalk is only along Gunderson."

## From Kathleen Walker to All panelists:

"Who is paying for the road? You as a planning commission, staff, and council failed to document (until long after public comment was closed) to address how Bailey Meadows did or did not meet all of 17.86 code requirements. You and the staff ignored us. You implied that the code could be waived without a variance. So don't tell us that we are not accurately reporting this! You ignored it before and now you decided it is out the scope. Great! :)"

## Neutral Testimony:

None

## Staff Recap:

O'Neill stated he's reluctant to go into discussion on Chapter 17.86 as that was part of the Bailey Meadows subdivision land use decision. O'Neill stated sidewalks along parks are typically paid for by the City through SDCs. Doughman reiterated that parkland was a matter germane to the subdivision decision.

## Applicant Rebuttal:

Mike Robinson
1211 SW 5th Ave. Suite 1900
Portland, OR, 97204
Robinson reiterated that the applicant agrees with the staff report and recommendation of approval. Robinson also reiterated that Chapter 17.86 is not relevant.

Chris Goodell
12965 SW Herman Rd. Suite 100
Tualatin, OR 97062
Goodell stated that he had nothing to add.

## Discussion:

Chairman Crosby noted the Commission has more background on this annexation
proposal than is typical of an annexation application. Commissioner Carlton stated he submitted testimony on Bailey Meadows. Carlton stated he was the one who brought up the houses facing the park during the Bailey Meadows subdivision hearing. Carlton stated it appears there's a street adjacent to the park to the west. Lesowski hopes the public realizes the City is trying to make the best out of the situation given the legal confines. Carlton is in favor of the annexation. Logan agreed with what's been said. Mayton stated he listens to the public and reads all the letters but is also bound to follow the code. Mayton and Maclean-Wenzel stated they support the annexation.

Motion: Motion to close the public hearing at 7:57 p.m.
Moved By: Commissioner Carlton
Seconded By: Commissioner Logan
Yes votes: Commissioners Carlton, Lesowski, Maclean-Wenzel, Logan, Mayton, and Crosby.
No votes: None
Abstentions: None
The motion passed at 7:57 p.m.

Motion: Motion to forward a recommendation of approval for 20-001 ANN/CPA/ZC Gunderson Road and Parkland Annexation.
Moved By: Commissioner Mayton
Seconded By: Commissioner Maclean-Wenzel
Yes votes: Commissioners Carlton, Lesowski, Maclean-Wenzel, Logan, Mayton, and Crosby.
No votes: None
Abstentions: None
The motion passed at 8:01 p.m.
5-minute recess

### 6.2. Chapter 17.78 Annexation Code Amendments (20-010 DCA):

Chairman Crosby opened the public hearing on File No. 20-010 DCA at 8:08 p.m. Crosby called for any abstentions, conflicts of interest, ex-parte contact, challenges to the jurisdiction of the Planning Commission, or any challenges to any individual member of the Planning Commission. No challenges were made, and no declarations were made by the Planning Commissioners.

## Staff Report:

Senior Planner Emily Meharg summarized the staff report using a PowerPoint presentation. Commissioner Mayton asked if the commission has the opportunity to ask questions on the code changes. Chairman Crosby stated that the Planning Commission will have an opportunity to ask detailed questions regarding the code changes.

## Proponent Testimony:

Kathleen Walker
15920 Bluff Road
Sandy, OR 97055
Mrs. Walker read her written testimony that she submitted on May 27, 2020.

Erin Findley
37616 Rachel Drive
Sandy, OR 97055
Not too familiar with the planning process. Found it interesting that two former mayors spoke during Bailey Meadows that development should pay for itself. Thanked the Planning Commission and asked for the commission to consider more code changes.

Marie DeBatty
37176 Rachel Drive
Sandy, OR 97055
Would like the Planning Commission to take notice of future annexations. Thinks the City of Sandy should be more careful with future annexations.

## Other Testimony:

None

## Staff Recap:

Meharg stated the intention of the code changes is to clarify annexations and to require more robust analysis. Changes are to address potential weakness. O'Neill stated that staff is trying to make the regulations more robust. Staff has heard from the public and we want to be more selective about future annexations. Doughman said that annexations are both land use regulatory and political in nature. The question really comes down to do we want to expand our territory or not. Annexation proposals are when local decision makers have more discretion. Voter approved annexations are no longer allowed in Oregon. Since voter approved annexations are no longer allowed a lot of cities in Oregon have revised their municipal code to make standards more robust. Before property is annexed the city can require applicants to prove they can meet master plans and other city codes.

## Discussion:

Commissioner Crosby asked is it possible for an applicant to force an annexation into the UGB area. Doughman stated that as long as you have criteria you have the opportunity to deny an annexation request. There have been discussions from both sides of the aisle that the legislature might make changes in the future to the annexation process.

Motion: Motion to close the public hearing at 8:42 p.m.
Moved By: Commissioner Carlton
Seconded By: Commissioner Maclean-Wenzel
Commissioners: All ayes
No votes: None
Abstentions: None
The motion passed at 8:42 p.m.

Commissioner Carlton stated he appreciates public comments. He stated that the UGB expansion analysis was very in-depth and analyzed items, such as city services. He stated that we have a lot of development code regulations to develop your land. Carlton also stated that the proposed language has a lot of subjectivity. Stated that he just heard about the annexation code changes recently. Commissioner Mobley stated there was not a lot of public input and not a lot of public participation. Commissioner Mayton said he was hoping for more analysis. He was surprised there was not more public feedback. Commissioner Lesowski stated he agrees with the other commissioners. He would like to see the code changes to be less subjective. Commissioner Logan said that almost all of the new language that staff has proposed to add are being adopted in other local cities. He said that he would like to see more information and a work group. Commissioner Maclean-Wenzel said she agrees with many of the comments and wants to see responsible growth. She stated it seems that properties that have been added to the UGB have come to expect annexation. She would like to see development pay for itself. Commissioner Logan stated that other cities have 300-foot notification requirements.

Commissioner Lesowski asked if property that is annexed gets any benefits if it's not developed? O'Neill stated that newly annexed property can connect to city utilities (i.e. water and sanitary sewer) and get police service. O'Neill stated that staff is trying to be responsive to requests from the public and the City Council. Staff is trying to be responsive to requests and adopt code quickly. Meharg stated that criterion A in Section 17.78 .50 of the proposed annexation code is already in the existing code. Commissioner Maclean-Wenzel asked Doughman to make some clarification. Doughman stated this is a great discussion where policy hits law. Most development code regulations must be clear and objective. However, annexation code can be written very subjective. Cities can choose to exercise a lot of discretion. The proposed code changes are supposed to assist with responsible growth and are supposed to be subjective and allow for discretion for the decision makers. Commissioner Lesowski asked how some of the criteria are very vague and some of the code is very specific.

Commissioner Mayton asked where does 10 years and 1,000 feet come from? Commissioner Carlton stated he would like to see the tree standards related to 5 years remain the same. He elaborated on the meaning of affordable housing and that it creates more traffic. Does the City have to analyze property being brought into the City via island annexations? Commissioner Maclean-Wenzel provided reasons for why the Tree Code Committee has been delayed in making progress. Developers state that tree retention is not financially viable for them. More than a 5 year waiting period for
significant tree removal seems fine. Commissioner Lesowski stated that the code changes are not decreasing regulations. Commissioner Carlton stated that the proposed code changes transposes work from one place to another place. Commissioner Mayton asked staff to clarify how property development works. O'Neill explained that a lot of property development occurs with contracts and contingencies prior to sale. He explained that individual property owners will not necessarily be on the hook for additional analysis. Meharg stated that 17.78.50 (A) is already in the Development Code. Planning Commission stated that (A) should be removed. Doughman stated it was already in the code. He elaborated there will be more analysis required at annexation but does not believe it will make a big difference on future growth. O'Neill stated that we could look at alternative annexation analysis for smaller properties simply looking to connect to city services. Doughman stated that an annexation agreement could take care of some of the concerns. Lesowski stated he was looking for less code criteria for properties with utility issues. Doughman said that the trade for annexation is higher taxes. Meharg stated the notification area is proposed to be increased to allow more public input and participation. Mayton stated he appreciates staff's time on the code changes, is good with an 8 to 10 year waiting period for significant tree removal, is good with the 1,000 feet notification, and moving subsection A . in 17.78 .50 into the header. Crosby said he suggests passing it along to City Council for approval with some suggested modifications. He summarized the discussion and asked how they can move forward with a motion. The Commissioners talked about tree retention. Commissioner Maclean-Wenzel said she suggests keeping the tree standard at 5 years and possibly modify the code once the Tree Code Committee forwards decisions. Mobley stated that 17.78 .50 (C) should be modified to add the words 'modified if necessary'. Commissioner Carlton encourages staff to notify property owners inside the UGB, but outside city limits. O'Neill stated that staff will consider noticing more property owners prior to the legislative hearing before City Council.

Motion: Motion to move the proposed code changes to Council with leaving A. in 17.78 .50 as an introductory sentence, add additional language for property owners looking to annex for utility reasons or smaller lots, modify C. in 17.78.50 to include the words 'modified if necessary', and leave 5 years for trees instead of 10 years. Moved By: Commissioner Mobley Seconded By: Commissioner Mayton
Commissioners Carlton, Maclean-Wenzel, Logan, Mobley, Mayton, and Crosby. No votes: None
Abstentions: Lesowski (no longer part of the meeting because the battery life on his computer expired)
The motion passed at 10:11 p.m.

## 7. Items from Commission and Staff

Chairman Carlton thanked staff for forwarding the Council report. O'Neill reminded the commission of the meetings in June and July. Commissioner Mayton stated he might miss the June 30, 2020 meeting.
8. Adjourn

Motion: To adjourn
Moved By: Commissioner Carlton
Seconded By: Commissioner Carlton
Yes votes: All Ayes
No votes: None
Abstentions: None
The motion passed.

Chairman Crosby adjourned the meeting at 10:14 p.m.


Chair, Jerry Crosby


Planning Director, Kelly O'Neill Jr

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## Staff Report

Meeting Date: June 30, 2020
From Emily Meharg, Senior Planner
SUBJECT: 20-021 EXT Jewelberry Ridge Subdivision Extension

## Background:

On July 12, 2018 the Planning Commission approved the Jewelberry Ridge Subdivision (File No. 18-014 SUB/VAR/TREE) which is a 9-lot subdivision located northwest of Jewelberry Avenue, east of the Sandy Bluff Annex subdivision, and south of American Street. The tentative subdivision plat approval was valid for one (1) year. On July 25, 2019 the Development Services Director granted an extension to the approval that extended the tentative plat approval for one (1) additional year to July 12, 2020.

The Development Code does not grant the Director authority to grant additional extensions. The applicant was advised by the Director to ask the Planning Commission to grant a tentative plat extension for one additional year to July 12, 2021 or another date as approved by the Planning Commission.

The typical reason that development codes contain expiration dates is because development codes are periodically modified. Modifications to development codes oftentimes lead to alternative findings or conditions of approval than were originally conditioned. In the case of Jewelberry Ridge no applicable development code modifications have occurred that would lead to a different outcome or list of conditions than what was applied to Jewelberry Ridge. Since no applicable development code modifications have occurred the exercise of making the applicant for Jewelberry Ridge seek re-approval for the subdivision would be immaterial.

The legal notice regarding this request was published in the Sandy Post on June 10, 2020.

## Recommendation:

Staff recommends the Planning Commission hear the extension request from the applicant and then make a decision on granting an extension. If Planning Commission grants an extension it shall be to July 12, 2021 or another date as approved by the Planning Commission.

## Code Analysis:

ATTACHMENTS:
A. Land Use Application
B. Narrative
C. Preliminary Plat
D. Director Extension Letter
E. Public Comment - Sally and Jerry Jacobson

Budgetary Impact:
None

EXHIBIT A


## Narrative in Support of One Year Extension Jewelberry Ridge Subdivision Preliminary Plat Approval

## Dear City of Sandy Planning Commission \& Planning Staff,

The Jewelberry Ridge Subdivision (File \# 18-014 SUB/VAR/TREE) was approved on July $12^{\text {th }} 2018$ and per normal approval timelines was good for one year. We had hoped to position the subdivision for construction during summer of 2019 but by the time we had approved engineering plans and other necessary entitlements we were well into the summer of 2019 and missed that dry weather construction season. The preliminary plat approval was then extended for one year through July $12^{\text {th }}, 2020$. We had a buyer lined up to close on the property this spring and move forward with construction this spring/summer but they ended up backing out at the last minute in mid-April, citing concerns with the economic fallout from COVID-19.

Following that setback we re-listed the property and are now under contract to sell the property in early July (2020) to a builder/developer that intends to move forward with construction immediately. Assuming an approximately three-month construction project, they should wrap up in the fall and get a final plat later this year or early next year depending on how everything falls into place.

4-J Land Co., LLC appreciates your consideration of this one year extension to the preliminary plat approval, which will allow enough time for the subdivision to be completed per the approved plans.

Sincerely,
John Schmidt \& Jan Barkley
4-J Land Co., LLC



July 25, 2019

4-J Land Co., LLC
ATTN: Jan Barkley
PO Box 189
Boring, OR 97009

Dear Ms. Barkley and Mr. Schmidt,
This letter regards my email to you on July 12, 2019 and the email request you submitted on July 13, 2019 to extend the tentative subdivision approval for the Jewelberry Ridge subdivision approved with File No. 18-014 SUB. The original expiration date as set forth in the findings of fact and final order was July 12, 2019. The extension I am permitting with this letter allows for a modified expiration date of July 12, 2020.

If you have any questions about this letter, you are welcome to call the City of Sandy Development Services Department at (503) 668-0880 for additional information.

Thank you,

Kelly O’Neill Jr.
Planning \& Building Director
koneill@cityofsandy.com direct line: (503) 489-2163

COMMENT SHEET for File No. 20-021 EXT:


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City of Sandy
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$\frac{15035 \text { SE } 377 \text { hares }}{10}$ Sandy, Ok. 97055
APPLICABLE CRITERIA: Sandy Municipal Code: Chapter 17.12 Procedures for Decision Making; 17.18 Processing Applications; 17.20 Public Hearings; 17.22 Notices.

## Staff Report

Meeting Date: June 30, 2020
From Emily Meharg, Senior Planner
SUBJECT: 20-006 DR/VAR/DEV/ADJ Clackamas County Health Clinic

## Background:

Clackamas County submitted this land use application for a new health clinic facility located at 39831 Highway 26. The proposal includes demolishing the existing building and constructing a new 9,381 square foot building, along with associated site improvements. In addition, the applicant is requesting the following:

- Type II Adjustment to Section 17.90.110(B.3.b) to not include a base material on 18 percent of the building's façade.
- Type III Design Deviation to Section 17.90.110(B.3.d.1) to use vertical grooved sheet siding.
- Type III Design Deviation to Section 17.90.120(D.4) to not provide a primary entrance at the corner or within 40 feet of the corner.
- Type III Design Deviation to Section 17.90.110(D.7) to not provide a primary entrance that faces a public street or civic space.
- Type III Special Variance to Section 17.90.110(E.2) to not meet the 40 percent window requirement along street frontages and where the building abuts a civic space.


## Recommendation:

Staff recommends the Planning Commission approve the requested Design Review, adjustment and variance associated with the Clackamas County Health Clinic as modified by the conditions of approval listed below.

Staff recommends the Planning Commission approve a Type II Adjustment to allow the proposed building to not include a base material on 18 percent of the building's facades.

Staff recommends the Planning Commission approve a Special Variance to Section 17.90.110(E.2) to not provide 40 percent window coverage along street frontages.

Staff recommends the Planning Commission approve the applicant's request for a design deviation to Section $17.90 .110(B .3 . d .1)$ to allow the use of Nichiha siding.

Staff recommends the Planning Commission approve the requested Design Deviation to Section 17.90.110(D.4) to not provide a corner entrance.

Staff recommends the Planning Commission approve the requested Design Deviation to Section 17.90.110(D.7) to not provide a public entrance facing a street or civic space.

## Code Analysis:

See attached staff report and exhibits

## Budgetary Impact:

None

# PLANNING COMMISSION STAFF REPORT TYPE III LAND USE PROPOSAL 

DATE: June 23, 2020
FILE NO.: 20-006 DR/VAR/DEV/ADJ
PROJECT NAME: Clackamas County Health Clinic

APPLICANT: Steve Kelly (Clackamas County)
OWNER: Clackamas County
LEGAL DESCRIPTION: T2S R4E Section 13 AD, Tax Lot 1001
SITUS ADDRESS: 39831 Highway 26

The above-referenced proposal was reviewed as a Type III Design Review with variances, deviations, and adjustments and the following Findings of Fact are adopted supporting approval of the tentative plan in accordance with Chapter 17 of the Sandy Municipal Code.

## EXHIBITS:

## Applicant's Submittals

A. Land Use Application and Supplemental Application
B. Narrative
C. Plan Set

- Sheet CS: Cover Sheet
- Sheet G0.11: Existing Site Survey
- Sheet C0.3: Site Demolition Plan
- Sheet C1.0: Site Hardscape Plan
- Sheet A1.01: Site Plan
- Sheet C2.0: Grading Plan
- Sheet C3.0: Utility Plan
- Sheet A3.11: Building Elevations
- Sheet A4.01: Building Sections
- Sheet G0.34: Floor Plan
- Sheet A2.03: Roof Plan
- Sheet A1.21: Trash Enclosure / Site Details
- Sheet G1.30: Window Area and Base Calculations
- Sheet E0.01: Electrical Symbol Legend
- Sheet E0.02: Lighting Fixture Schedule
- Sheet E1.01: Electrical Site Plan
- Sheet E1.02: Photometric Site Plan
- $\quad$ Sheet L1.01: Landscape Plan
- Sheet L1.02: Landscape Details
- Sheet L4.00: Landscape Notes and Plant Schedule
- Sheet L6.01: Planting Details
D. Exterior Light Fixture Cut Sheets: S1 through S7
E. Renderings
F. Exterior Building Materials
G. Nichiha Details
H. Traffic Impact Analysis Letter
I. Shared Parking Letter to Police
J. Parking Space Lease Agreement with Immanuel Lutheran Church
K. Stormwater Utility Narrative
L. 1:60 Vicinity map
M. 500 foot radius map


## Agency Comments

N. City Engineer (May 29, 2020)
O. Fire Marshal (June 3, 2020)
P. Transportation Engineer (June 8, 2020)
Q. Oregon Department of Transportation ODOT (June 17, 2020)
R. Public Works Director (June 18, 2020)

## Supplemental Documents Provided by Staff

S. Email from applicant regarding Nichiha
T. Ten Eyck Road \& Pleasant Street Curb \& Sidewalk Improvements Final Plans

## FINDINGS OF FACT

## General

1. These findings are based on the applicant's submittal materials received on February 12, 2020 with additional items received on $3 / 13 / 20,3 / 24 / 20,3 / 27 / 20,3 / 31 / 20$, and $4 / 1 / 20$. The application was deemed complete on April 3, 2020. The original 120-day deadline was August 1, 2020; however, in response to COVID-19, the applicant submitted a letter on April 4, 2020 extending the 120-day deadline 30 days to August 31, 2020.
2. The subject site is approximately 0.47 acres and is located on the north side of Highway 26, east of Ten Eyck Road and south of Pleasant Street.
3. The parcel has a Plan Map designation of Commercial and a Zoning Map designation of C-1, Central Business District.
4. Clackamas County submitted this land use application for a new health clinic facility located at 39831 Highway 26. The proposal includes demolishing the existing building and constructing a new 9,381 square foot building, along with associated site improvements. In addition, the applicant is requesting the following:

- Type II Adjustment to Section 17.90.110(B.3.b) to not include a base material on 18 percent of the building's façade.
- Type III Design Deviation to Section 17.90.110(B.3.d.1) to use vertical grooved sheet siding.
- Type III Design Deviation to Section 17.90.120(D.4) to not provide a primary entrance at the corner or within 40 feet of the corner.
- Type III Design Deviation to Section 17.90.110(D.7) to not provide a primary entrance that faces a public street or civic space.
- Type III Special Variance to Section 17.90.110(E.2) to not meet the 40 percent window requirement along street frontages and where the building abuts a civic space.

5. Notification of the proposal was mailed to property owners within 500 feet of the subject property on May 27, 2020 and to affected agencies on May 26, 2020. A legal notice was published in the Sandy Post on June 3, 2020.
6. Per the City Engineer (Exhibit N), a demolition permit is required from the City prior to demolishing the existing building. The applicant applied for a demolition permit on June 15, 20202, which was issued on June 17, 2020.
7. The Planning Commission reviewed the proposal at the June 30, 2020 hearing.

### 17.42 - Central Business District (C-1)

8. The applicant proposes a health clinic, which is a primary use permitted outright in Section 17.42.20(B.2.g).
9. The C-1 zoning district does not require a minimum front yard setback, but does have a maximum front yard setback of 10 feet. The zoning district does not contain any side yard or rear yard setbacks. There is an existing building on the subject property that will be demolished. Chapter 17.10 contains the following definition for "lot line, front": "In the case of an interior lot, a property line that abuts the street. In the case of a corner lot, the front line shall be determined by orientation of the structure based on at least two of the following factors: location of the front door, location of the driveway, or legal street address." The subject property is a corner lot with frontage on three public streets. The proposed front door faces the parking lot to the east, the driveway is located on Pleasant Street, and the address is on Highway 26. Thus, no two factors currently align to determine the front lot line. While the front door faces the parking lot, it is located towards the north end of the east elevation, closest to Pleasant Street. Moreover, the driveway is located off Pleasant Street so that is how patients who drive to the clinic will access the building. Thus, staff believes it makes the most sense to readdress the health clinic with a Pleasant Street address. This will result in Pleasant Street being the front lot line. The proposed new building is set back 10 feet from Pleasant Street (the front lot line), in compliance with setback standards. The applicant shall update the address of the site to be addressed from Pleasant Street and shall pay the re-addressing fee (\$206) to the City of Sandy.
10. Per the submitted Landscape Plan (Exhibit C, Sheet L1.01), the site will include a total of 15 percent landscaping in compliance with the standards of Section 17.42.30(A).

### 17.66 - Adjustments and Variances

11. Chapter 17.66 contains regulations associated with adjustments and variances. Adjustments are a means to vary the development standards normally applied in a particular district and variances
are a means of requesting a complete waiver or major adjustment to certain development standards. The applicant requested the following adjustment and variance:

- Type II Adjustment to Section 17.90.110(B.3.b) to not include a base material on 18 percent of the building's façade.
- Type III Special Variance to Section 17.90.110(E.2) to not meet the 40 percent window requirement along street frontages and where the building abuts a civic space.


## 12. BASE MATERIAL ADJUSTMENT

Section 17.90.110(B.3.b) states: "Strong base materials such as natural stone (e.g., basalt, granite, river stone), split-faced rusticated concrete block, or brick are required. Cultured stone may be allowed if it has a stone texture and is similar in appearance and durability to natural stone. A building's base must extend at least 36 inches but not more than 60 inches above the adjacent finished grade and be included on those sides of the building visible from the abutting public street. If the site contains a grade differential making construction of a minimum 36inch base impracticable, the reviewing body may allow portions of the base to be less than 36inches."

Request: The applicant requests a Type II Adjustment to Section 17.90.110(B.3.b) to not include a base material on 18 percent of the building's facade. The proposed gaps in base material are as follows:

- East elevation: one (1) 10'-4" gap and one (1) 3'-6" gap
- South elevation: one (1) 22'-6" gap and one (1) 26'-3" gap
- West elevation: two (2) 14'-8" gaps

Section 17.66.40 contains Type I and Type II Adjustment criteria. Criteria A of Section 17.66 .40 states "The proposed development will not be contrary to the purposes of this chapter, policies of the Comprehensive Plan, and any other applicable policies and standards adopted by the City." The Comprehensive Plan states that the Central Business (C-1) District is intended to provide the community with a mix of civic, retail, personal services, offices and residential needs of the community and its trade area in the city's traditional commercial core. The intent of Section 17.90.110(B) is to provide building façades, materials and colors consistent with the Sandy Style. The proposed development is consistent with the intent of the Central Business zone by providing a community service. The proposal includes Sandy Style elements and contains base material on 82 percent of the building's facades. Criterion A is met.

Criteria B states "The proposed development will not substantially reduce the amount of privacy enjoyed by users of nearby structures when compared to the same development located as specified by this Code." The proposal to reduce the percent of base material from 100 to 82 percent will not affect the amount of privacy enjoyed by users of nearby structures. Criterion B is met.

Criteria C states "The proposed development will not adversely affect existing physical systems and natural systems, such as traffic, drainage, dramatic land forms, or parks." Based on the Transportation Impact Analysis (TIA), the proposed development will result in 34 net new AM peak hour trips, 30 net new PM peak hour trips, and 335 net new daily trips. The TIA analyzed the intersection of Highway 26 and Ten Eyck Road and the intersection of Pleasant Street and Ten Eyck Road; both intersections were found to meet ODOT and City operational standards
either with or without development. The City's Transportation Engineer (Exhibit P) reviewed the TIA and did not find a need for mitigation measures to address traffic impacts of the development or to address safety issues. Criterion C is met.

Criteria D states "Architectural features of the proposed development will be compatible to the design character of existing structures on adjoining properties and on the proposed development site." The proposed building will be designed to be compatible with Sandy Style and includes gabled roofs, secondary roof forms, a mix of siding material, and strong base material. The subject property is directly west of the Sandy Police Station, which also includes many Sandy Style elements. Criterion D is met.

Staff believes that an 18 percent reduction in base material on the building's frontages is not contrary to the Comprehensive Plan or other City policies, does not reduce the amount of privacy enjoyed by users of nearby structures, will not adversely affect existing systems, and will be compatible to the design character of existing structures on adjoining properties. Staff
recommends the Planning Commission approve a Type II Adjustment to allow the proposed building to not include a base material on 18 percent of the building's facades.

## 13. WINDOW PERCENT SPECIAL VARIANCE

Section 17.90.110(E.2) states: "The ground floor elevation of all new buildings shall contain display areas, windows, and doorways along street frontages and where the building abuts a civic space as follows: Lots with multiple street frontages are required to meet this standard on only two frontages." Buildings up to 10,000 square feet in size are required to provide windows on 40 percent of the ground floor elevation.

Request: The applicant requests a Type III Special Variance to Section 17.90.110(E.2) to not meet the 40 percent window requirement along street frontages and where the building abuts a civic space.

Section 17.66.80 contains criteria for granting a Special Variance. The Planning Commission may grant a special variance waiving a specified provision under the Type III procedure if it finds that the provision is unreasonable and unwarranted due to the specific nature of the proposed development. In submitting an application for a Type III Special Variance, the proposed development explanation shall provide facts and evidence sufficient to enable the Planning Commission to make findings in compliance with the criteria set forth in this section while avoiding conflict with the Comprehensive Plan.

One of the following sets of criteria shall be applied as appropriate.
A. The unique nature of the proposed development is such that:

1. The intent and purpose of the regulations and of the provisions to be waived will not be violated; and
2. Authorization of the special variance will not be materially detrimental to the public welfare and will not be injurious to other property in the area when compared with the effects of development otherwise permitted.
B. The variance approved is the minimum variance needed to permit practical compliance with a requirement of another law or regulation.
C. When restoration or replacement of a nonconforming development is necessary due to damage by fire, flood, or other casual or natural disaster, the restoration or replacement will decrease the degree of the previous noncompliance to the greatest extent possible.

The applicant is proposing to provide windows as follows:

- 43 percent windows on the east façade (parking lot)
- 27 percent windows on the south façade (Highway 26)
- 28 percent windows on the west façade (Ten Eyck Road)
- 23 percent windows on the north façade (Pleasant Street)

The proposed building is approximately 9,381 square feet, which is just under 10,000 square feet. The percent of ground floor windows required for buildings greater than 10,000 square feet is 25 percent of the ground floor elevation. Per the submitted narrative (Exhibit B ), it is not practical to have 40 percent windows on two of the elevations due to the use of the building as a health clinic. The applicant is proposing 43 percent windows on the east elevation; however, the east elevation faces the parking lot and not a street. The applicant is not proposing to meet the 40 percent window requirement on any of the three street-facing facades. Staff believes a reduction below the 40 percent minimum window coverage on the street-facing facades will not violate the intent of the code nor be detrimental to public welfare, in compliance with Criterion A. Because the proposed use is a health clinic, staff understands the need for a certain degree of privacy. Alternatively, the applicant could install additional windows with permanent window coverings to provide privacy, but staff finds this method as overly burdensome and an unnecessary
expense. Staff recommends the Planning Commission approve a Special Variance to Section $17.90 .110(E .2)$ to not provide 40 percent window coverage along street frontages.

### 17.74 - Accessory Development

14. Section 17.74 .40 (B) contains height requirements for fences and retaining walls in commercial and industrial zones. The subject property is zoned Central Business District ( $\mathrm{C}-1$ ). The applicant is proposing two screening fences along the south edge of the site adjacent to Highway 26. The proposed fences are 8 feet in height, which is the maximum height allowed for walls/fences in the rear yards of commercial properties. As discussed in Chapter 17.42 of this document, the applicant will be required to re-address the site to a Pleasant Street address such that Pleasant Street is the front lot line. Once this occurs, Highway 26 will be the rear lot line and the proposed 8 foot tall screening fences will be allowed per the code. If the proposal includes a retaining wall, the applicant shall submit additional details on the proposed retaining wall for staff review and approval. All retaining walls shall have architectural finish.
17.84 - Improvements Required with Development
15. Chapter 17.84 provides general information regarding improvements required in association with development. All required improvements shall be installed or financially guaranteed prior to final occupancy.
16. Section 17.84 .20 provides information on timing of improvements. Submission of preliminary street and utility plans during the land use review process is solely for compliance with the data requirements of Section 17.100 .60 (D). Preliminary plat approval does not connote utility or public improvements plan approval which will be reviewed and approved separately upon submittal of public improvement construction plans.
17. Section 17.84 .30 requires sidewalks and planting areas along all public streets. Per the Public Works Director (Exhibit R), the applicant shall remove the existing west driveway approach onto Pleasant Street. The existing driveway approach doesn't meet the minimum spacing standard in Section 17.98.80(A) or the maneuvering standard in Section 17.98.70(B). The applicant shall improve the Pleasant Street frontage of the site including but not limited to: curbs, sidewalks, storm drainage, streetlighting, and street trees per the requirements in Chapter 15.20 and Section 17.84 .30 of the Sandy Municipal Code. The sidewalk shall be five feet wide separated from the curb with a five-foot-wide planter strip including street trees specified by the City on 30 foot centers. Alternatively, the sidewalk may be curb-tight, minimum 8 feet in width with street trees specified by the City in tree wells on 30 foot centers. Per the City Engineer (Exhibit $N$ ), where the existing driveway is removed on Pleasant Street, it shall be replaced with sidewalks and an ADA ramp shall be constructed at the intersection with Ten Eyck Road to current PROWAG requirements as required by Clackamas County. Ten Eyck Road is a County road so the City Engineer recommends the County require sidewalks to be constructed along the entire site frontage to match the existing sidewalks on Highway 26. Per the Public Works Director (Exhibit R), the existing driveway approach onto Ten Eyck Road from the site does not meet the minimum spacing standards in Section 17.98 .80 (A) of the Sandy Municipal Code (SMC). The applicant shall abandon the existing driveway approach and improve the Ten Eyck Road frontage of the site including but not limited to: curbs, sidewalks, storm drainage, streetlighting, and street trees per the requirements in Chapter $\mathbf{1 5 . 2 0}$ and Section $\mathbf{1 7 . 8 4 . 3 0}$ of the Sandy Municipal Code. This section of Ten Eyck Road is under the jurisdiction of Clackamas County. The applicant shall coordinate with the Clackamas County Department of Transportation and Development (DTD) to determine the required section for Ten Eyck Road. This may include relocating the existing fire hydrant at the intersection of Ten Eyck Road and Pleasant Street to install ADA compliant access ramps. Ten Eyck Road is a minor arterial street. Both Clackamas County and the City of Sandy require minimum 6-foot-wide sidewalks on arterial streets. The County standard and the proposed ultimate section for Ten Eyck Road will only permit a curb tight sidewalk in the available right-of-way. The City recently designed a pedestrian improvement project on Ten Eyck Road at the subject site. The Clackamas County DTD plan review fee has been paid and the plans have been approved by DTD. The applicant shall be responsible for these improvements and is welcome to use the approved plan set (Exhibit T) for this work. The Urban Renewal Agency has agreed to reimburse the applicant $\$ 14,000$ for completing the Ten Eyck Road improvements.
18. 17.84.50 contains standards for street improvements and traffic evaluations. The applicant submitted a Transportation Impact Analysis (TIA; Exhibit H) dated March 12, 2020 and prepared by John Manix of PBS. The City's Transportation Engineer (Exhibit P) reviewed the TIA and concurs with the TIA's conclusions. The Transportation Engineer does not find a need for mitigation measures to address traffic impacts of the development or to address safety issues. The Transportation Engineer recommends that ODOT requirements and standards associated with frontage improvements where the development abuts Highway 26 are made conditions of approval for the development. ODOT (Exhibit Q) reviewed the proposal and Traffic Impact Analysis and determined that a westbound right turn lane at the Highway 26 and Ten Eyck Road intersection is not warranted. ODOT further determined that there will be no significant impacts to state highway facilities and no additional state review is required. Per the Public Works Director (Exhibit R), the applicant shall improve the Highway 26 frontage of the site in
compliance with ODOT requirements including modifying the existing slip/right-turn lane to reduce the curb radius at the corner to accommodate east-west and north-south ADA compliant ramps if required by ODOT. This may require right-of-way dedication at the NE corner of Highway 26 and Ten Eyck Road. The applicant shall dedicate sufficient right-of-way at the SW corner of the site to accommodate the required street section and pedestrian improvements including ADA-compliant ramp(s) at the intersection with Highway 26. The exact dedication area shall be determined during construction plan review. The applicant shall be responsible for providing legal descriptions and sketch maps of the dedication area, dedicating the right-of-way using the City's standard documents, and paying all recording costs.
19. Sections 17.84 .60 and 17.84 .70 relate to public utilities. Utility and right-of-way improvement plans are submitted with the land use application solely for conformance with the submittal requirements in Section 17.100.60(D). Land use approval does note connote approval of public improvement plans. Per the Public Works Director (Exhibit R), the site is served by the existing 16-inch water main in Highway 26 and the existing 8-inch sanitary sewer main in Pleasant Street. The applicant is proposing a drywell for stormwater disposal. Typically, the soils in Sandy do not permit treatment and discharge of stormwater in this manner. Stormwater management shall conform to the requirements in the City of Portland Stormwater Management Manual and the requirements in Sections $\mathbf{1 3 . 1 8}$ and $\mathbf{1 3 . 2 0}$ of the Sandy Municipal Code. The City Engineer reviewed the preliminary stormwater calculations and found that the calculations do not meet the water quality/quantity criteria as stated in the City of Sandy Municipal Code 13.18 Standards and the 2016 City of Portland Stormwater Management Manual (SWMM) Standards that were adopted by reference into the Sandy Development Code. The water quality shall be designed based on $0.19 \mathrm{in} / \mathrm{hr}$ rate for 5 minutes time of concentration. While the water quality shall be designed for $2,5,10$, and 25 -year storm events and not 25 and 100 year storm events only. A detailed final report stamped by a licensed professional shall be resubmitted for review with the final construction plans.
20. Section 17.84 .80 contains standards for franchise utility installation. Private utility services will be submitted for review and approval by service providers and City staff in association with construction plans, and all utility lines will be extended to the perimeter of the site. All franchise utilities shall be installed underground and in conformance with City standards. The applicant shall call the PGE Service Coordinators at 503-323-6700 when the developer is ready to start the project.
21. The Fire Marshal (Exhibit O) reviewed the proposal and states the new fire department connection (FDC) shall be relocated to the Highway 26 side of the proposed fire vault to be as close as possible to the existing fire hydrant located in that area. Per the City Engineer (Exhibit N), the final construction plans shall be submitted to the Sandy Fire Marshal for review and approval to ensure that the proposed vault has adequate fire protection.
22. Section 17.84.100 contains requirements for mail delivery facilities. The applicant will need to coordinate with the United States Postal Service (USPS) to relocate mail facilities and these will be approved by the City and USPS. Mail delivery facilities shall be provided by the applicant in conformance with 17.84 .100 and the standards of the USPS. The applicant shall submit a mail delivery plan to the City and USPS for review and approval prior to installation of mailboxes.
23. The proposal is subject to all the requirements for Design Review as stated in Section 17.90.00.
24. Section 17.90 .70 specifies that design review approval shall be void after two (2) years from the date of the Final Order unless the applicant has submitted plans for building permit approval.
25. Section 17.90.110(A) contains site layout and vehicle access standards intended to provide for compact, walkable development, and to design and manage vehicle access and circulation in a manner that supports pedestrian safety, comfort and convenience. All lots shall abut or have cross access to a dedicated public street. The subject property abuts Highway 26, Ten Eyck Road, and Pleasant Street.
26. Section $17.90 .110(\mathrm{~A})(3)$ requires that off-street parking shall be located to the rear or side of buildings with no portion located within 10 feet of the public right-of-way. All parking spaces are proposed to the east side of the proposed building.
27. Section 17.90.110(A)(5) states that raised walkways or painted crossings shall be provided from the sidewalk to entrances of buildings. The primary building entrance is located on the east elevation facing the parking area. The proposal includes a pedestrian walkway along the east elevation that's connected to the Pleasant Street sidewalk through a civic area. In addition, the proposal includes walkway connections to the Highway 26 sidewalk from two exit doors and an exterior staff break area on the south elevation.
28. Section 17.90.110(A)(9) states that joint use of access points and cross-over easements between parcels shall be required where the City determines it is practicable and necessary. Prior to occupancy the applicant and the City of Sandy shall record cross-over easements for vehicular and pedestrian traffic and a shared parking agreement. The terms of that agreement shall be negotiated by the two parties.
29. Section 17.90.110(B) contains standards regarding building facades, materials, and colors intended to be consistent with the Sandy Style. Section 17.90.110(B.1) requires that buildings be articulated, varied and provide visual interest. The proposed elevations of the structure include varying wall planes that do not exceed 30 feet in length. Variations include secondary roofs, wall projections, and changes in siding material.
30. Section 17.90.110(B.2) requires that buildings incorporate pedestrian shelters over primary building entrances. Pedestrian shelters shall extend at least five feet over the pedestrian area. The proposal includes a gabled primary pedestrian entry on the east façade of the building with a five (5) foot roof overhang. In addition, there are two pedestrian doors for staff use on the south façade and a third on the north façade. Based on the submitted Floor Plan (Exhibit C, Sheet G0.34), the door from the break room on the south facade has a metal canopy overhang that extends at least 5 feet. The canopy overhangs on the door from the hallway on the south side and the door on the north façade are only 4 feet; however, neither door is a primary entrance or a public entrance.
31. Section 17.90.110(B.3) specifies approved building materials. Section 17.90 .110 (B.3.b) requires buildings to include strong base materials such as natural stone, split-faced rusticated concrete block, or brick on all sides of a building visible from an abutting public street. All four elevations are visible from the public right-of-way and, therefore, require a strong base material. The applicant is requesting a Type II Adjustment to not include base material on 18 percent of the building's façade. The Adjustment request is reviewed in Chapter 17.66 of this document.
32. Section 17.90 .110 (B.3.d) contains approved siding. The applicant is proposing to use a mix of vertical and horizontal siding, both of which are Nichiha siding, which is a panelized fiber cement product. Per Section 17.90.110(B.3.d.1) "Vertical grooved (i.e., T1-11) sheet siding and similar materials are prohibited." Therefore, the applicant is requesting a design deviation to Section 17.90.110(B.3.d.1) to allow the use of Nichiha vertical grooved sheet siding. Per email correspondence with the applicant (Exhibit S), "The vertical ribbed Nichiha product is a panelized fiber cement product with $15 / 8 "$ vertical slats and $3 / 8$ " reveals between each slat which add depth and rustic texture to the façade. The product is available in custom colors that can match any selected Miller or Sherwin Williams paint. The ribbed fiber cement is not the same as board-and-batten siding or T1-11 sheet siding. The product can be installed either vertically or horizontally. In terms of design, the vertical siding is a darker earth tone and is the bulk of the siding. The vertical orientation was selected to distinguish it from the horizontal redwood Nichiha material pops of warm color at the bumpouts and recesses in the facade. Both siding products are Nichiha so all the siding can come from a single source manufacturer. This will make the construction process smoother and simplify the detailing. The vertical siding has been selected as the main siding, because the vertical orientation is better for cleaning and maintenance over time." The applicant submitted photo samples of the Nichiha siding (Exhibit G) along with the proposed colors: redwood for the horizontal siding and weathered copper for the vertical siding. Staff believes the Nichiha siding meets the intent of the code. Staff recommends the Planning Commission approve the applicant's request for a design deviation to Section 17.90 .110 (B.3.d.1) to allow the use of Nichiha siding.
33. Section 17.90 .110 (B.4) specifies approved colors. The applicant is proposing a redwood color for the horizontal siding and a weathered copper color for the vertical siding. Staff is unsure if these proposed colors are stains or actual paint colors. If they are paint colors, they are not on the City's approved color palette. The applicant shall provide clarification on the proposed redwood and weathered copper colors. If they are paint colors, the applicant shall choose alternate colors from the City's approved color palette, or shall apply for a Design Deviation. The trim and fascia are Sturgis Gray, which is an approved color on the City's color palette.
34. Section $17.90 .110(\mathrm{C})$ requires gable roofs on new buildings. The proposed building includes a gabled roof on the south elevation as well as a shed roof on the east elevation of the primary entrance. All primary roof pitches are 6:12 or greater in compliance with Section 17.90.110(C.1).
35. Section 17.90 .110 (C.4) requires secondary roof forms based on roof length. The applicant is proposing secondary shed roofs at a different pitch (41/2:12) than the primary roof to break up the roofline in compliance with the code.
36. Section 17.90 .110 (C.5) requires visible roof materials to be wood shingle or architectural grade composition shingle, slate, or concrete tile. Metal with standing or batten seam may also be used conforming to the Color Palette in Appendix D of the development code. The applicant is proposing to use standing seam metal roof panels with the proposed color of Old Town Gray. The applicant shall choose a metal roof color from the City's approved metal roof color palette, or shall apply for a Design Deviation.
37. Section $17.90 .110(\mathrm{D})$ contains standards regarding building orientation and entrances intended to maintain and enhance downtown and village commercial streetscapes as public spaces, emphasizing a pedestrian-scale and character in new development, consistent with the Sandy Style; and to provide for a continuous pedestrian network that promotes pedestrian safety, comfort and convenience, and provides materials and detailing consistent with the Sandy Style. The proposed building has frontage on Highway 26, Ten Eyck Road, and Pleasant Street. Section 17.90.100(D)(1) requires at least 50 percent of the subject site's street frontage to be comprised of building(s) placed within 10 feet of the sidewalk. The building occupies more than 50 percent of the street frontage on all three abutting streets.
38. Section 17.90.110(D.4) specifies buildings located at the intersection of two streets shall use a corner building entrance; where a corner entrance is not practical due to the internal functioning of the building space or due to physical constraints of the site (e.g., topography, accessibility, or similar circumstances), a building entrance must be provided within 40 feet of the corner. The applicant is proposing to locate the public entrance on the east elevation of the proposed building and is therefore requesting a Design Deviation to Section 17.90.110(D.4) to not provide a primary entrance at the corner or within 40 feet of the corner. Per the applicant's narrative (Exhibit B), "a direct corner entrance is not practical due to the nature of the building's use as a health clinic and its relation to the parking lot." Staff recognizes that the parking lot is existing and that it makes sense to have a primary entrance near the parking. While it would be feasible to have a second pedestrian entrance at the corner, staff realizes that this is less practical due to the nature of a health clinic, which typically has one central public entrance with a reception desk and waiting area. Staff recommends the Planning Commission approve the requested Design Deviation to Section 17.90 .110 (D.4) to not provide a corner entrance.
39. Section 17.90.110(D.6) specifies that buildings shall provide at least two elevations where the pedestrian environment is "activated." An elevation is "activated" when it meets the window transparency requirements in Subsection $17.90 .110(\mathrm{E})$ and contains a customer entrance with a pedestrian shelter extending at least five (5) feet over an adjacent sidewalk, walkway or civic space. Where providing a customer entrance on two (2) elevations is not practical, the reviewing body may allow a single entrance. The proposed building does not meet the window percent requirements on any elevations and the applicant has requested a Special Variance, which is reviewed in Section 17.90.110(E) of this document. The proposed building includes a single primary public entrance that faces the parking lot. The applicant's narrative (Exhibit B) states: "There is an entrance with a pedestrian shelter facing Hwy 26 and facing Pleasant Street, however, due to the nature of the clinic, these are staff entrances, not customer entrances. The patient entrance faces the parking lot. Patients may leave the other (2) doors, but these are primarily intended for clinic staff." Staff concurs that it is not practical to provide a second public entrance due to the nature of the health clinic.

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40. Section 17.90.110(D.7) specifies primary entries shall face a public street or a civic space and shall be spaced not more than 30 feet apart on average. The proposed primary entrance faces the parking lot. The applicant is requesting a Design Deviation to Section 17.90.110(D.7) to not provide a primary entrance that faces a public street or civic space. Staff recognizes that the parking lot is existing and that it makes sense to have a primary entrance near the parking area. While it would be feasible to have a second pedestrian entrance at the corner, staff realizes that this is less practical due to the nature of a health clinic, which typically has one central public entrance with a reception desk and waiting area. Staff recommends the Planning Commission approve the requested Design Deviation to Section 17.90 .110(D.7) to not provide a public entrance facing a street or civic space.
41. Section 17.90.110(E) contains standards for construction and placement of windows. The intent of windows is to promote business vitality, public safety, and aesthetics through effective window placement and design. Section 17.90.110(E)(2) specifies that the activated frontage shall contain a minimum of 40 percent of the building elevation in windows and that lots with multiple street frontages are required to meet this standard on two frontages. The proposed percent windows on each elevation are as follows:

- 43 percent windows on the east façade (parking lot)
- 27 percent windows on the south façade (Highway 26)
- 28 percent windows on the west façade (Ten Eyck Road)
- 23 percent windows on the north façade (Pleasant Street)

Only the east elevation, which faces the parking area, meets the 40 percent window requirement. The applicant requested a Special Variance to Section 17.90.110(E.2) to not meet the 40 percent window requirement along two (2) street frontages. The Special Variance request is reviewed in Chapter 17.66 of this document.
42. Section 17.90.110(F) contains additional landscaping and streetscape design standards for parcels along Highway 26 and all other adjacent rights-of-way. This provision requires properties in the C-1 Central Business District to comply with the Downtown Sandy Streetscape Design as contained in Appendix F. The proposal includes street trees planted in tree wells located within the right-of-way as opposed to the traditional landscape strip as these two frontages are not intended to have landscape strips. The streetscape on Highway 26 is required to conform to the standards of Appendix F - Streetscape Design Detail, therefore the applicant shall update the plan set to indicate conformance with the sidewalk tree grate and lighting details identified in Appendix F of the development code. Additionally, the streetscapes on Pleasant Street and Ten Eyck Road shall conform to the Curb \& Sidewalk Improvements final plan (Exhibit T).
43. Section 17.90.110(G) contains standards for civic spaces within developments. The intent of civic space is to connect buildings to the public realm and create comfortable and attractive gathering places and outdoor seating areas for customers and the public. The development code requires developments to provide a civic space area that is at least three percent of the building ground floor area and in no instance is less than 64 square feet. The ground floor area of the proposed use is 9,381 square feet which results in a total need for approximately 281 square feet of civic space. The proposal includes a total of 320 square feet of civic space in the form of a hardscaped area at the northeast corner of the building adjacent to Pleasant Street. The civic space is proposed to contain planters and two benches.

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44. Section $17.90 .110(\mathrm{H})$ contains standards regarding lighting. The applicant shall follow all Dark Sky Ordinance requirements as outlined in Chapter 15.30 of this document.
45. Section 17.90.110(I) contains standards regarding safety and security intended to promote natural surveillance of public spaces. The proposed parking area is located adjacent to the east elevation of the proposed building. The east elevation (Exhibit C, Sheet A3.11) identifies windows along the east façade, which enables both surveillance of the parking area from inside as well as surveillance of the interior from the parking area. In addition, the proposal includes windows on the north, south, and west façades, which provides visibility to the adjacent sidewalks.
46. Section 17.90.110(I.3) requires street address numbers. The applicant shall provide street address numbers measuring a minimum of six (6) inches high, which clearly locate buildings and their entries for patrons and emergency services. The applicant shall verify the location(s) of the address with the Building Official and emergency service providers.
47. Section 17.90.110(J) contains standards regarding external storage and screening intended to promote land use compatibility and aesthetics, particularly where development abuts public spaces. The proposal includes a garbage/recycle enclosure in the parking area along the Highway 26 frontage. The enclosure is an 8 foot by 10 foot concrete block enclosure. Per the submitted narrative (Exhibit B), "trash and recycling storage is in a split face CMU enclosure in the southeast corner of the site. Enclosure CMU matches the color of the concrete block veneer base of the building." In addition, the metal roof of the trash enclosure matches the metal roofing color for the primary structure. Hoodview Disposal and Recycling typically requires the applicant to verify the level of service to determine if the trash/recycling enclosure is adequate size. The applicant shall verify the location of the trash and recycling enclosure with Hoodview.
48. The proposal includes mechanical and fire equipment at the southeast corner of the building along Highway 26. The equipment will be screened from the public right-of-way by an 8 foot tall wood stained fence.

### 17.92 - Landscaping and Screening

49. The subject property is zoned Central Business District, C-1. Section 17.92.20 requires that a minimum of 10 percent of the site be landscaped in the $\mathrm{C}-1$ zoning district. The submitted Landscape Plan (Exhibit C, Sheet L1.01) details 3,250 square feet of landscaped area, which equates to a total of 15 percent of the site.
50. Section $17.92 .10(\mathrm{C})$ requires significant plant and tree specimens to be preserved to the greatest extent practicable and integrated into the design of a development. Per the Existing Site Survey (Exhibit C, Sheet G0.11), there are two (2) existing 8-inch DBH conifers located in the southwest portion of the site. The Demolition Plan (Exhibit C, Sheet C0.3) indicates that these trees will be removed to allow for construction of the proposed building. The subject property is only 0.47 acres; the proposed building and parking area take up almost the entire site. The building size would need to be reduced significantly in order to retain the trees. Thus, staff determined that retention of the two trees is not practicable. The applicant will be planting
additional trees in the parking area and installing additional landscaping around the entire building.
51. Section 17.92.30 requires trees to be planted along public street frontages. There are existing street trees on Highway 26 and the east end of the subject property's Pleasant Street frontage. Medium sized street trees spaced 30 feet on center will be required for both Ten Eyck Road and Pleasant Street. The Landscape Plan (Exhibit C, Sheet L1.01) details street trees along all three frontages of the subject property. The proposed street trees along Pleasant Street and Ten Eyck Road are 2-inch caliper Red Rage Tupelo (Nyssa sylvatica 'Hayman's Red'). All street trees shall be a minimum of 1.5 -inches in caliper measured 6 inches above the ground and shall be planted per the City of Sandy standard planting detail. All street trees shall be staked and tied with loose twine so as not to damage the trunk; the twine shall be removed after one growing season (or a maximum of 1 year). Per the Public Works Director (Exhibit R), street tree and landscaping placement shall conform to the sight distance recommendations in the traffic impact analysis submitted by the applicant.
52. The parking lot is proposed to contain additional Red Rage Tupelo (Nyssa sylvatica 'Hayman's Red') trees. The Landscape Plan (Exhibit C, Sheet L1.01) details a structural tree at the ends of each parking bay with the exception of the parking bay directly in front (east) of the building. The applicant shall update the Landscape Plan to detail one structural tree at the north and south ends of the parking bay east of the building.
53. Section 17.92.40 requires that all landscaping be irrigated, either with a manual or automatic system. The Landscape Plan (Exhibit C, Sheet L1.01) states: "all landscape areas within the site including turf, shrub beds and tree areas shall be irrigated with an automatic irrigation system. The irrigation system shall be adjusted to meet the water requirements of the individual plant material." As required by Section 17.92.140, the developer shall be required to maintain all vegetation planted in the development for two years from the date of completion, and shall replace any dead or dying plants.
54. Section 17.92 .50 specifies the types and sizes of plant materials for landscaping areas. The development code requires that all deciduous trees shall be at least 1.5 inches caliper and all coniferous trees shall be a minimum of 5 feet in height at planting. All shrubs/plants are required to be a minimum of 1 gallon in size or 2 feet in height at planting. The Landscape Plan Plant Schedule (Exhibit C, Sheet L4.00) identifies proposed trees at 2-inch caliper as well as a mix of shrubs, grasses, and vines, all of which meet the minimum 1 gallon in size requirement.
55. Section 17.92.130 details performance bond standards as they associate to required landscaping. The applicant has the option to defer the installation of street trees and/or landscaping for weather-related reasons. Staff recommends the applicant utilize this option rather than install trees and landscaping during the dry summer months. If the applicant chooses to postpone street tree and/or landscaping installation, the applicant shall post a performance bond equal to $\mathbf{1 2 0}$ percent of the cost of the landscaping, assuring installation within $\mathbf{6}$ months. The cost of street trees shall be based on the street tree plan and at least $\$ 500$ per tree. The cost of landscaping shall be based on the average of three estimates from three landscaping contractors; the estimates shall include as separate items all materials, labor, and other costs of the required action, including a two-year maintenance and warranty period.

### 17.98 - Parking, Loading, and Access Requirements

56. Section 17.98.20 contains off-street parking requirements. As detailed on the Site Plan (Exhibit C, Sheet A1.01), the Clackamas County Health Clinic site is proposed to contain 14 parking spaces and two bicycle parking spaces. Two bicycle parking spaces are proposed to be located under the overhang adjacent to the primary entrance. At the time of this land use application submittal medical offices/clinics were required to contain one vehicle parking space per 300 square feet of retail area, plus one parking space per two employees on the largest shift, and 5 percent or 2 bicycle parking spaces, whichever is greater. Per the submitted narrative (Exhibit B), the building will have approximately 8,345 square feet of clinic space (excluding hallways, restrooms, mechanical rooms, etc.). Based on this square footage of clinic area, the site is required to provide 28 parking spaces. The narrative states that the largest shift will have 12 employees requiring 6 additional parking spaces for a total of 34 required parking spaces. The property is in the C-1 zone, which at the time of this land use application submittal the development code allowed for a 25 percent reduction in required parking spaces; thus, 26 parking spaces are required with the allowed parking reduction. In addition to the 14 parking spaces on site, there are six (6) additional spaces shared with the adjacent property to the east. The applicant has also entered into a seven (7) year lease agreement for the use of 17 parking spaces for employees in the Immanuel Lutheran Church parking lot east of the subject property on the south side of Pleasant Street. Moreover, since the applicant submitted this land use application, the parking chapter of the development code (Chapter 17.98) was modified to remove minimum parking requirements for the $\mathrm{C}-1$ zone.
57. ORS 447.233 requires one accessible parking space for each 25 parking spaces up to 100 parking spaces. The Site Plan (Exhibit C, Sheet A1.01) details one proposed ADA parking space on the east side of the proposed building. The accessible parking space is detailed at 9 feet in width with an 8 -foot-wide access aisle on the passenger side of the ADA space as required by ORS.
58. Section 17.98 .60 contains specifications for parking lot design and size of parking spaces. Standard parking spaces shall be at least 9 feet by 18 feet and compact spaces shall be at least 8 feet by 16 feet. The proposal provides a total of 14 parking spaces on site, including seven (7) standard spaces at 9 feet wide by 18 feet deep, six (6) compact parking spaces at 8 feet wide by 16 feet deep, and one (1) ADA space as specified above. The parking spaces are 90 -degree spaces with a minimum 25 -foot-wide drive aisle in compliance with the development code.
59. Section 17.98.100 has specifications for driveways. The site has one existing driveway on Ten Eyck Road and two existing driveways on Pleasant Street. The applicant is proposing to remove the existing driveway on Ten Eyck Road and to remove the existing west driveway on Pleasant Street. The existing driveway on Ten Eyck road is a safety concern due to the spacing to the intersection of Ten Eyck Road and Highway 26. Per the City Engineer (Exhibit N), the proposed driveway access on Pleasant Street shall be a concrete commercial driveway approach constructed in conformance with the applicable City of Sandy driveway details and meeting PROWAG requirements.
60. Section 17.98.120 contains requirements for landscaping and screening in parking areas. Since the proposal is for more than four parking spaces, compliance with the standards of Section 17.98.120 is required. The location of the parking lot already exists. The area between the parking areas and the right-of-way is at least 5 feet in planting depth. As detailed on the Site Plan (Exhibit C, Sheet A1.01), the proposed parking areas are divided into bays of less than 20 spaces.

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Landscaping is proposed at the ends of some but not all parking rows. The parking row directly east of the proposed building is flanked by hardscaped areas - the proposed civic space to the north and a hardscaped area for the fire vault to the south. These areas could still contain structural trees in tree wells. The applicant shall update the Landscape Plan to detail one structural tree at the north and south ends of the parking bay east of the building.
61. Section 17.98.120 (F) contains requirements for wheel stops to protect landscaping and pedestrians. The submitted Site Plan (Exhibit C, Sheet A1.01) details wheel stops in every parking space.
62. Section 17.98.130 requires all parking and vehicular maneuvering areas to be paved with asphalt or concrete. The applicant is proposing to remove and replace the existing parking area. Per the narrative (Exhibit B) preamble asphalt will be used for the new parking lot surface in compliance with the development code.
63. Section 17.98 .140 contains requirements for drainage. Chapter 17.84 of this document details the stormwater requirements.
64. Section 17.98 .160 contains requirements for bicycle parking facilities. The requirement is bicycle parking at 5 percent of the vehicle parking or two bicycle parking spaces, whichever is greater. The Site Plan (Exhibit C, Sheet A1.01) details two (2) bicycle parking spaces under the overhang adjacent to the primary building entrance.

### 15.30 - Dark Sky Ordinance

65. Chapter 15.30 contains the City of Sandy's Dark Sky Ordinance. The applicant submitted a Photometric Plan (Exhibit C, Sheet E1.02) that details foot candles on the subject property and 10 feet beyond the property lines. All outdoor lighting systems must be designed so that the area 10 feet beyond the property line of the premises receives no more than .25 (one quarter) of a foot-candle of light from the proposed listing system. The Photometric Plan details readings that exceed 0.25 (one-quarter) of a foot-candle 10 feet beyond the south property line along Highway 26. The applicant shall adjust the location and/or illumination of the S1 lighting fixture located in the middle of the four (4) compact parking spaces on the south edge of the parking lot such that the area 10 feet beyond the property line does not exceed 0.25 of a foot candle. The applicant shall submit the revised photometric analysis for staff review and approval.
66. The submitted Photometric Plan (Exhibit C, Sheet E1.02) and lighting cut-sheets (Exhibit D) detail all lights at 3,500 Kelvins or less and downward facing. All exterior lights shall be full cut-off and shall not exceed $\mathbf{4 , 1 2 5}$ Kelvins.

### 15.44 - Erosion Control Regulations

67. All erosion control and grading shall comply with Section 15.44 of the Municipal Code. All onsite earthwork activities including any retaining wall construction shall follow the requirements of the most current edition of the Oregon Structural Specialty Code (OSSC). If the proposal includes a retaining wall, the applicant shall submit additional details on the proposed retaining wall for staff review and approval. The earthwork activities shall be observed and documented under the supervision of a geotechnical engineer.
68. Site grading shall not in any way impede, impound, or inundate the surface drainage flow from the adjoining properties without a proper collection system. All the work within the public right-of-way and within the paved area should comply with American Public Works Association (APWA) and City requirements as amended. The applicant shall submit a grading and erosion control permit and request an inspection of installed devices prior to any additional grading onsite. The grading and erosion control plan shall include a revegetation plan for all areas disturbed during construction of the subdivision. All erosion control and grading shall comply with Section 15.44 of the Municipal Code.

## RECOMMENDATION

Staff recommends the Planning Commission approve the requested Design Review, adjustment and variance associated with the Clackamas County Health Clinic as modified by the conditions of approval listed below.

Staff recommends the Planning Commission approve a Type II Adjustment to allow the proposed building to not include a base material on 18 percent of the building's facades.

Staff recommends the Planning Commission approve a Special Variance to Section 17.90.110(E.2) to not provide 40 percent window coverage along street frontages.

Staff recommends the Planning Commission approve the applicant's request for a design deviation to Section 17.90.110(B.3.d.1) to allow the use of Nichiha siding.

Staff recommends the Planning Commission approve the requested Design Deviation to Section 17.90.110(D.4) to not provide a corner entrance.

Staff recommends the Planning Commission approve the requested Design Deviation to Section 17.90.110(D.7) to not provide a public entrance facing a street or civic space.

## CONDITIONS OF APPROVAL

A. Prior to any onsite grading or earthwork:

1. The applicant shall submit a grading and erosion control permit and request an inspection of installed devices prior to any additional grading onsite. The grading and erosion control plan shall include a re-vegetation plan for all areas disturbed during construction. All erosion control and grading shall comply with Section 15.44 of the Municipal Code.
B. Prior to building permit submittal, the applicant shall complete the following and receive necessary approvals as described below:
2. Provide staff with one copy of the final and complete plan set, including elevations, for the proposal to be included in the case file for future reference.
3. Update the plan set to indicate conformance with the sidewalk tree grate and lighting details identified in Appendix F of the development code. Additionally, the streetscapes on Pleasant

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Street and Ten Eyck Road shall conform to the Curb \& Sidewalk Improvements final plan (Exhibit T).
3. Update the Landscape Plan to detail one structural tree at the north and south ends of the parking bay east of the building.
4. Submit a revised photometric plan that adjusts the location and/or illumination of the S1 lighting fixture located in the middle of the four (4) compact parking spaces on the south edge of the parking lot such that the area 10 feet beyond the property line does not exceed 0.25 of a foot candle.
5. If the proposal includes a retaining wall, the applicant shall submit additional details on the proposed retaining wall for staff review and approval. All retaining walls shall have architectural finish.
6. Provide clarification on the proposed redwood and weathered copper colors. If they are paint colors, the applicant shall choose alternate colors from the City's approved color palette, or shall apply for a Design Deviation.
7. Choose a metal roof color from the City's approved metal roof color palette, or apply for a Design Deviation.

## C. The Final Construction Plans shall include the following:

1. Stormwater management shall conform to the requirements in the City of Portland Stormwater Management Manual and the requirements in Sections 13.18 and 13.20 of the Sandy Municipal Code. The water quality shall be designed based on $0.19 \mathrm{in} / \mathrm{hr}$ rate for 5 minutes time of concentration. While the water quality shall be designed for $2,5,10$, and 25 -year storm events and not 25 and 100 year storm events only. A detailed final report stamped by a licensed professional shall be resubmitted for review with the final construction plans.
2. The final construction plans shall be submitted to the Sandy Fire Marshal for review and approval to ensure that the proposed vault has adequate fire protection.

## D. Prior to the certificate of occupancy for the site, the applicant shall complete the following:

1. All required improvements shall be installed or financially guaranteed.
2. The applicant and the City of Sandy shall record cross-over easements for vehicular and pedestrian traffic and a shared parking agreement. The terms of that agreement shall be negotiated by the two parties.
3. Submit a signed copy of the parking space lease agreement with Immanuel Lutheran Church.
4. Abandon the existing driveway approach and improve the Ten Eyck Road frontage of the site including but not limited to: curbs, sidewalks, storm drainage, streetlighting, street trees per the requirements in Chapter 15.20 and Section 17.84 .30 of the Sandy Municipal Code. Coordinate with the Clackamas County Department of Transportation and Development to determine the required section for Ten Eyck Road. This may include relocating the existing fire hydrant at the
intersection of Ten Eyck Road and Pleasant Street to install ADA compliant access ramps. Ten Eyck Road is a minor arterial street. Both Clackamas County and the City of Sandy require minimum 6-foot-wide sidewalks on arterial streets. The County standard and the proposed ultimate section for Ten Eyck Road will only permit a curb tight sidewalk in the available right-of-way. The applicant shall be responsible for these improvements and is welcome to use the approved plan set (Exhibit T) for this work.
5. Remove the existing west driveway approach onto Pleasant Street. Improve the Pleasant Street frontage of the site including but not limited to: curbs, sidewalks, storm drainage, streetlighting, and street trees per the requirements in Chapter 15.20 and Section 17.84.30 of the Sandy Municipal Code. The sidewalk shall be five feet wide separated from the curb with a five-footwide planter strip including street trees specified by the City on 30 foot centers. Alternatively, the sidewalk may be curb-tight, minimum 8 feet in width with street trees specified by the City in tree wells on 30 foot centers. Where the existing driveway is removed on Pleasant Street, it shall be replaced with sidewalks and an ADA ramp shall be constructed at the intersection with Ten Eyck Road to current PROWAG requirements as required by Clackamas County.
6. The proposed driveway access on Pleasant Street shall be a concrete commercial driveway approach constructed in conformance with the applicable City of Sandy driveway details and meeting PROWAG requirements.
7. Improve the Highway 26 frontage of the site in compliance with ODOT requirements including modifying the existing slip/right-turn lane to reduce the curb radius at the corner to accommodate east-west and north-south ADA compliant ramps if required by ODOT. This may require right-of-way dedication at the NE corner of Highway 26 and Ten Eyck Road. The applicant shall dedicate sufficient right-of-way at the SW corner of the site to accommodate the required street section and pedestrian improvements including ADA-compliant ramp(s) at the intersection with Highway 26. The exact dedication area shall be determined during construction plan review. The applicant shall be responsible for providing legal descriptions and sketch maps of the dedication area, dedicating the right-of-way using the City's standard documents, and paying all recording costs.
8. The new fire department connection (FDC) shall be relocated to the Highway 26 side of the proposed fire vault to be as close as possible to the existing fire hydrant located in that area.
9. Update the address of the site to be addressed from Pleasant Street and pay the re-addressing fee (\$206) to the City of Sandy. Install street address numbers measuring a minimum of six (6) inches high, which clearly locate buildings and their entries for patrons and emergency services. The applicant shall verify the location(s) of the address with the Building Official and emergency service providers.
10. Install all landscaping, building improvements, and other site improvements as detailed in this final order.
11. Install mail delivery facilities in conformance with 17.84 .100 and the standards of the USPS. The applicant shall submit a mail delivery plan to the City and USPS for review and approval prior to installation of mailboxes.

## E. General Conditions of Approval

1. Design review approval shall be void after two (2) years from the date of the Final Order unless the applicant has submitted plans for building permit approval.
2. The applicant shall follow all Dark Sky Ordinance requirements as outlined in Chapter 15.30. All exterior lights shall be full cut-off and shall not exceed 4,125 Kelvins.
3. All street trees shall be a minimum of 1.5 -inches in caliper measured 6 inches above the ground and shall be planted per the City of Sandy standard planting detail. All street trees shall be staked and tied with loose twine so as not to damage the trunk; the twine shall be removed after one growing season (or a maximum of 1 year). Street tree and landscaping placement shall conform to the sight distance recommendations in the traffic impact analysis submitted by the applicant.
4. The applicant shall be responsible for the installation of all improvements detailed in Section 17.100.310, including fiber facilities. SandyNet requires the developer to work with the City to ensure that broadband infrastructure meets the design standards and adopted procedures as described in Section 17.84.70.
5. All new utility lines and franchise utilities for future development shall be installed underground and in conformance with City standards.
6. The applicant shall call the PGE Service Coordinators at 503-323-6700 when the developer is ready to start the project.
7. All landscaping shall be continually maintained, including necessary watering, weeding, pruning, and replacing.
8. As required by Section 17.92 .140 , the developer shall be required to maintain all vegetation planted in the development for two years from the date of completion, and shall replace any dead or dying plants during that period.
9. All parking, driveway and maneuvering areas shall be constructed of asphalt, concrete, or other approved material.
10. The applicant will be required to obtain a separate permit for any proposed signage.
11. The applicant shall verify the location of the trash and recycling enclosure with Hoodview.
12. Preliminary plat approval does not connote utility or public improvements plan approval which will be reviewed and approved separately upon submittal of public improvement construction plans. Utility and right-of-way improvement plans are submitted with the land use application solely for conformance with the submittal requirements in Section 17.100.60(D). Land use approval does note connote approval of public improvement plans.
13. All on-site earthwork activities including any retaining wall construction shall follow the requirements of the most current edition of the Oregon Structural Specialty Code (OSSC). If the proposal includes a retaining wall, the applicant shall submit additional details on the proposed retaining wall for staff review and approval. The earthwork activities shall be observed and documented under the supervision of a geotechnical engineer.

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14. Site grading shall not in any way impede, impound, or inundate the surface drainage flow from the adjoining properties without a proper collection system. All the work within the public right-of-way and within the paved area should comply with American Public Works Association (APWA) and City requirements as amended.
15. If the applicant chooses to postpone street tree and/or landscaping installation, the applicant shall post a performance bond equal to 120 percent of the cost of the landscaping, assuring installation within 6 months. The cost of street trees shall be based on the street tree plan and at least $\$ 500$ per tree. The cost of landscaping shall be based on the average of three estimates from three landscaping contractors; the estimates shall include as separate items all materials, labor, and other costs of the required action, including a two-year maintenance and warranty period.
16. Successors-in-interest of the applicant shall comply with site development requirements prior to the issuance of building permits.
17. Comply with all other conditions or regulations imposed by the Sandy Fire District, or state and federal agencies. Compliance is made a part of this approval and any violations of these conditions and/or regulations may result in the review of this approval and/or revocation of approval.


## LAND USE APPLICATION FORM

(Please print or type the information below)
Planning Department 39250 Pioneer Blvd. Sandy OR 97055 503-668-4886

## Name of Project Sandy Health Clinic

Location or Address 39831 Hwy 26, Sandy, OR 97055
Map \& Tax Lot Number T $2 \mathrm{~S}, ~ \mathrm{R} 4 \mathrm{E}$, Section $13 \quad ; \operatorname{Tax} \operatorname{Lot}(\mathrm{s}) \underline{24 E 13 A D 01001}$
Plan Designation C-1 Zoning Designation Commercial Acres $\underline{0.47}$
Request:
Request for a Type II Design Review with (2) Type II Adjustments for a new health clinic. Project will consist of demolition of existing one-story building, construction of a new health clinic building, and associated site work.

I am the (check one) $\square$ owner $\square$ lessee of the property listed above and the statements and information contained herein are in all respects true, complete and correct to the best of my knowledge and belief.

| Applicant Steve Kelly | Owner |
| :---: | :---: |
| Address 2051 Kaen Road | Address |
| City/State/Zip Oregon City, OR 97045 | City/State/Zip |
| Phone 503-655-8591 | Phone |
| Email stevekel@clackamas.us | Email |
| signature ture Nelly | Signature |

If signed by Agent, owner's written authorization must be attached.

| File No. | Date |  | Rec. No. | Fee \$ |
| :--- | :--- | :--- | :--- | :--- | :--- |
| Type of Review (circle one): | Type I | Type II | Type III | Type IV |



# SUPPLEMENTAL APPLICATION DESIGN REVIEW / CONDITIONAL USE PERMIT 

(Please print or type the information below)

## Planning Department 39250 Pioneer Blvd. Sandy OR 97055

 503-668-4886Name of Project Sandy Health Clinic
Location or Address 39831 Hwy 26, Sandy, OR 97055
Type of Business Health clinic with primary care, behavioral health, and dental services
Products Manufactured none

Hours of Operation Monday through Friday, 8am-6pm
No. of Employees per Shift staff of 10-12 individuals

Total Site Size 0.47 acre Total Bldg. Square Footage 9,459 SF

Project Summary
Demolition of existing 6,700 sf, one-story building, construction of a health clinic building and associated site work. Health clinic will provide primary care, behavioral health, and dental services. The clinic will be 9,381 GSF and one-story.

USES WITHIN BUILDING (SQUARE FOOTAGE)
Offices: 3,777 SF
Shop: $\qquad$ Storage: 431 SF

Kitchen: $\qquad$ Laundry: $\qquad$ Rest Rooms: 283 SF

Other: 4,890 SF

## OCCUPANCY \& CONSTRUCTION TYPE

(List all occupancies by square footage)
UBC Occupancy Rating: B

UBC Type of Construction: $\underline{\text { V-B }}$
Will fire sprinklers be installed in the building? $\quad \boxed{\square}$ Yes $\quad \square$ No

| SITE ANALYSIS DATA |  |  |
| :--- | :---: | :---: |
| Type | Lot Coverage <br> (Square Feet) | Lot Coverage <br> (Percent of Site) |
| Buildings | 9,381 | $46 \%$ |
| Parking Lots and Driveways | 5,393 | $27 \%$ |
| Private Walks \& Pedestrian Ways | 2,210 | $11 \%$ |
| Landscaping - Improved Area | 2,999 | $15 \%$ |
| Landscaping - Natural areas | 0 | $0 \%$ |
|  <br> Bioswale Areas | 288 | $1 \%$ |
| Other (describe) |  |  |
| Other (describe) |  |  |

## CONSTRUCTION MATERIAL DETAILS

Color \& Type of Siding Materials: horizontal redwood fiberboard; vertical weathered copper fiberboard Color \& Type of Trim Materials: $\quad$ Sturgis gray painted wood trim; Ebony split face CMU base material Color \& Type of Roof Materials: Gray standing seam metal roof Color \& Type of Exterior Doors: Sturgis gray painted hollow metal doors Color \& Type of Exterior Stairs, Balconies \& Railings: Strugis gray painted exterior railings
Trash \& Recycling Enclosure (describe type, colors, height):
Ebony color split face CMU enclosure; 10 feet wide $\times 8$ feet deep $\times 11$ feet 5 inches tall
Type of Lighting Fixtures (describe):
Pole: D-Series LED, 25 feet tall, charcoal color (S1 fixture) (match pole fixtures at police station)
Wall-Mounted: SY302 Series LED wall up/down light (S3 fixture)

Fencing (height and type): 8 feet tall, vertical stained wood fencing
Mailboxes (location and type): Mail is received internally at the reception area
Private Pedestrian Walkways (type of surfacing): Concrete
Recreational Amenities (describe type and location):
No recreational amenities provided other than outdoor seating.

Other Site Elements (describe):
Boulders, Powder-coated aluminum planters, Heavy timber benches, Monument sign, Steel bike rack

## TYPE OF IRRIGATION SYSTEM

Describe type and brand of irrigation system to be installed. Formal irrigation plans must be submitted with construction plans.

Basis of design is Rain Bird Sales, Inc. electric solenoid controlled underground irrigation system, with low point
self drain

## PROPOSED SOIL AMENDMENTS

Describe soil conditions and proposed plans for soil treatment \& amendments:
Geotechnical engineer will test the native soils once site clearing has been completed, to review the construction parameters.

## ADDITIONAL LANDSCAPING INFORMATION

The shrubs used for parking screen are evergreen and will grow into a hedge (17.92.50.G). Parking lot/new street trees were selected for hardiness and fall color. Per section 17.92.50.A, the plans exceed $75 \%$ of planted required landscaping areas. Sizes indicated on following page meet or exceed caliper, gallon, or height requirements.

Vines indicated are 1 gallon size but are not used as a physical barrier requirement. Groundcovers selected are either flowering or evergreen and will fill in in under the 3 yrs required (17.92.50.I). Other plant selections were made based on regional-appropriateness, owner maintenance, visual interest, and bloom/scent for staff/visitors.

|  |
| :--- |
|  |

## LANDSCAPE MATERIALS

| Quantity | Type - Include botanical and common names. <br> Plants must be keyed to landscape plan. | Size | Height | Spacing |
| :---: | :---: | :---: | :---: | :---: |
| 9 | Nyssa Sylvatica 'Hayman's Red' / Red Rage Tupelo | 2" cal |  |  |
| 18 | Ilex Crenata 'Steeds' / Upright Holly | 7 gal | 6 ft |  |
| 10 | Osmanthus Xfortunei 'San Jose' / Sweet Tea Olive | 3 gal |  |  |
| 66 | Ophiopogon Planiscapus 'Nigrescens' / Mondo Grass | 3 gal |  |  |
| 28 | Delphinium Elatum / Magic Fountain Mix | 3 gal |  |  |
| 55 | Carex Morrowii 'Aurea-Variegata' / Varigated Japanese Sedge | 3 gal |  |  |
| 2 | Clematis Armandii 'Apple Blossom' / White Evergreen Clematis | 1 gal |  |  |
| 110 SF | Carex Obnupta / Slough Sedge | 1 gal |  | 12" |
| 110 SF | Juncus Effusus / Soft Rush | 1 gal |  | 12" |
| 2600 SF | Potentilla N. 'Nana' / Cinquefoil | 1 gal |  | 18" |
| 1500 SF | Euonymus 'Coloratus' / Purple Winter Creeper | 1 gal |  | 18" |
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Sandy Health Clinic<br>39831 Highway 26<br>Sandy, OR 97055

Type II Design Review Project Narrative
February 12, 2020

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Design Review Drawings
Attached separately

## Contacts

| Developer | Clackamas County <br> 2051 Kaen Road <br> Oregon City, OR 97045 |
| :--- | :--- |
| Architect | Ankrom Moisan Architects <br> 38 NW Davis Street |
|  | Suite 300 <br> Portland, OR 97209 <br> 503.245 .7100 |
|  |  |
| City Design Review | Development Services Planning <br>  <br>  <br>  <br>  <br>  <br> Division <br> City of Sandy <br> 39250 Pioneer Blvd <br> Sandy, OR 97055 |
|  |  |

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Kelly O'Neill Jr., Planning \& Building Dir. koneill@cityofsandy.com
503.489.2163

Emily Meharg, Associate Planner emeharg@cityofsandy.com 503.783.2585

## Land Use / Design Reviews

| Application Requested | Type II Design Review <br> (2) Type II Adjustments, please see Sections B. 3 and E. 2 in the Building <br> Design Standards, below. Sections are highlighted in red for clarity. |
| :--- | :--- |
| Pre-Application Conference Conference held December $18^{\text {th }}, 2019$ |  |
| Design Review Application | Submittal: Feb 12, 2020 |

## Site Description

| Site Address: | Sandy Health Clinic 39831 HWY 26 Sandy, OR 97055 |
| :---: | :---: |
| Jurisdiction: | Sandy, OR / Clackamas County |
| Map \& Tax Lot Numbers: | T2S, R4E, Section 13; Tax Lot 24E13AD01001 |
| Parcel Number: | 01830433 |
| Total Site Area: | 0.47 acres (20,473sf) |
| Zoning: | C-1 (Central Business District) |
| Current title Owner: | Clackamas County Health Centers 2051 Kaen Road, 3 rd Floor Oregon City, OR 97045 |
| Street classification |  |
| Hwy 26 / Mt. Hood Highway | Major Arterial roadway |
| SE Ten Eyck Road | Minor Arterial roadway |
| Pleasant Street | Local Street |

## Project Description

## Overview

The project site is located between Hwy 26 / Mt. Hood Highway and Pleasant Street, at the corner of SE Ten Eyck Road. The site is generally flat, with an approximate $21 / 2$-foot slope rising from the north to south at the sidewalk along Hwy 26, and an approximate $23 / 4$-foot slope rising from the north to the south at the sidewalk along Pleasant Street. The parcel currently has a vacant building that was previously a distribution center, and an auto dealership. Zoning is C-1. The building height limit is 45 feet.

The proposed construction is for a one story Type VB, sprinklered health clinic, with primary care, behavioral health, and dental services. The reception / waiting area will be accessed from the parking lot, between the Sandy police station and the new building, at the northeast corner of the building. There will be other access points facing Hwy 26 and Pleasant Street. The Sandy clinic will operate Monday through Friday with hours of $8 \mathrm{am}-6 \mathrm{pm}$. For the proposed development, 6 of the required spaces of the off-street parking will be shared on site with the Sandy Police Station. A parking agreement with Immanuel Lutheran Church, northeast of the property, will accommodate all staff parking and overflow visitor parking.

## Total gross floor area: <br> 9,381 gsf

| Parking stalls: | 22 stalls including 1 accessible van space and 6 compact spaces. |
| :--- | :--- |
| Bike storage: | $5 \%$ or 2 whichever is greater <br> 2 total $(21 / 2$ feet $x 6$ feet $\times 7$ feet vertical) for commercial use |

## Project Code Summary

Occupancy: B-Business
Construction type summary: One-story
Type VB
NFPA 13 Sprinkler System throughout.
Max code height: $\quad 60 \mathrm{ft}$ maximum
Building is approximately 28 ft high

## Zoning Requirements Narrative

| Zoning: | C-1 (Central Business District) <br> Land Use Review process: <br> 17.42.00: <br>  <br> Preapplication Conference (December $18{ }^{\text {th }}$, 2019) <br> Type II Design Review procedure |
| :--- | :--- |
| This district is intended to provide the community with a mix of retail, <br> personal services, offices, and residential needs of the community and its <br> trade area in the city's traditional commercial core. This district is not <br> intended for intensive automobile or industrial uses. This district is <br> intended to provide the principal focus for civil and social functions within <br> the community. |  |
| This commercial district is intended for civic uses and to provide all basic <br> services and amenities required to keep the downtown the vital center of <br> our community. While the district does not permit new low density <br> building types, it is not intended to preclude dwelling units in buildings <br> containing commercial activities. All development and uses shall be <br> consistent with the intent of the district, as well as compatible with the <br> space, access and exposure constraints and opportunities of the central <br> city. |  |

## Zoning Requirements Narrative

Please see attached drawings for additional clarification regarding compliance.
$\left.\left.\begin{array}{|l|l|l|l|}\hline \text { Topic } & \text { Requirement } & \text { Chapter } & \text { Compliance } \\ \hline & & & \\ \hline \text { Permitted Uses } & \begin{array}{l}\text { Commercial in buildings with up to } \\ \text { 30,000 sf of gross floor area and without } \\ \text { drive-through facilities }\end{array} & \begin{array}{l}17.42 .10 . B .2 . \\ \mathrm{g}\end{array} & \begin{array}{l}\text { Medical facilities permitted } \\ \text { outright. }\end{array} \\ \hline \begin{array}{l}\text { Development } \\ \text { Standards }\end{array} & \begin{array}{l}\text { Lot Area: No minimum } \\ \text { Lot Dimensions: No minimum } \\ \text { Setbacks: No minimum; maximum 10 ft } \\ \text { Lot Coverage: No maximum } \\ \text { Landscaping: 10\% minimum } \\ \text { Structure Height: 45 ft maximum }\end{array} & 17.42 .30 . \mathrm{A} & \begin{array}{l}\text { Lot Area: Comply } \\ \text { Lot Dimensions: Comply }\end{array} \\ \hline \text { Special Setbacks } & \begin{array}{l}\text { Property abutting a more restrictive } \\ \text { zoning district shall have the same yard } \\ \text { setback as required by the abutting } \\ \text { district. } \\ \text { Pleasant Street: Across street from more } \\ \text { restrictive R-2., requiring 10 ft minimum } \\ \text { setback and additional 10 ft added for } \\ \text { each 10 ft increment in building height } \\ \text { over 35 ft. }\end{array} & 17.42 .30 . B & \begin{array}{l}\text { Lot Coverage: Comply } \\ \text { Landscaping: } 14 \% \text { provided }\end{array} \\ \text { Structure Height: 28 ft }\end{array}\right\} \begin{array}{l}\text { 10 ft minimum setback } \\ \text { provided from north property } \\ \text { line. }\end{array}\right\}$

| Topic | Requirement | Chapter | Compliance |
| :---: | :---: | :---: | :---: |
| Minimum <br> Landscaping and Screening Improvements | 10\% required for C-1 zone ( $2,047 \mathrm{SF}$ ) | 17.92.20 | 15\% (2,999 SF) provided. |
| Required Tree Plantings | 1 medium canopy street tree at 30 feet on center. 1 medium parking lot tree per 8 cars ( $8 / 21$ total cars $=2.6$ trees) | 17.92.30 | Compliant street trees and parking lot trees provided. See Landscape Plan L1.01. |
| Irrigation | Landscaping to be irrigated to sustain viable plant life | 17.92 .40 | Electric solenoid controlled underground irrigation system, with low point self drain will be used. |
| Types and Sizes of Plant Materials | A. 75\% landscape area to be combination of trees, shrubs, or evergreen ground cover. <br> B. Use native plant materials acclimatized to the Pacific Northwest. <br> C. Trees with a mature spread of crown greater than 15 feet should have trunks with maintained clear condition of over 5 feet of clear wood. <br> D. Deciduous trees to be balled and burlapped, minimum of 7 feet in overall height or $1 \frac{1}{2 \prime \prime}$ in caliper $6^{\prime \prime}$ above the ground. <br> E. Coniferous trees to be minimum of 5 feet above the ground at time of planting. <br> F. Shrubs to be a minimum of 1 gal in size or 2 feet in height. <br> G. Hedges to form a continuous, solid visual screen within 2 years after planting. <br> H. Vines for screening to be minimum of 1 gal. or $30^{\prime \prime}$ in height. <br> I. Groundcover to be fully rooted and well branched or leafed and provide complete coverage in one year. <br> J. Turf areas to be species normally grown as permanent lawns in western Oregon. <br> K. Landscaped areas may include architectural features or artificial ground cover not to exceed $25 \%$ of the required landscaped area. | 17.92.50 | A. Complies <br> B. Complies <br> C. Complies <br> D. Complies <br> E. Complies <br> F. Complies <br> G. No hedges in design. <br> H. Vines not used for site screening. <br> I. Complies <br> J. No turf in design. <br> K. Design includes benches, boulders, aluminum planters, wood fences, dark brown mulch, stone pavers, and a monument sign that do not exceed $25 \%$ of the landscaped area. |
| Landscaping Between Public Right-of-Way and Property Lines | Except for portions allowed for parking, loading, or traffic maneuvering, a required setback area abutting a public street and open area between property and roadway in the public street to be landscaped. | 17.92.70 | Area between property line and public streets are landscaped except where there is a sidewalk or driveway. |


| Topic | Requirement | Chapter | Compliance |
| :---: | :---: | :---: | :---: |
| Screening | A. Height and Opacity <br> B. Chain Link Fencing <br> C. Height Measurement <br> D. Berms | 17.92 .90 | A. Wood fencing is $8^{\prime}-0^{\prime \prime}$ tall and at least $80 \%$ opaque. <br> B. Not in project. <br> C. Screen height is measured from the finished grade of screened improvements. <br> D. Not in project. |
| Screening of Service Facilities | Equipment and trash and recycling areas to be screened. | 17.92.100 | The site transformer and condensing unit are screened, and the trash enclosure is CMU to match the building base material. |
| Off Street Parking Requirements | For medical office / clinic use: Number of car parking spaces: 1 per 300 sf, plus 1 per 2 employees | 17.98 .20 | Car parking spaces: <br> 8,345 GSF / $300=27.81$ stalls <br> 25\% reduction for C-1 zone $=20.86$ (21) required stalls. 6 of these required stalls are shared with the police station. <br> 12 staff, 6 staff parking stalls at Immanuel Lutheran Church parking lot via agreement with church. Bike parking spaces: <br> 27 stalls $\times 5 \%=1.4$ <br> 2 required bike spaces area provided near the main entry. |
| Accessible (ADA) Parking Stalls | 1 accessible parking space required for parking facility with 1 to 25 stalls. | 2010 ADA <br> Standards <br> Table 208.2 | 1 van accessible parking stall provided |


| Topic | Requirement | Chapter | Compliance |
| :---: | :---: | :---: | :---: |
| Parking Lot Design and Size | No more than $35 \%$ of parking stalls shall be compact spaces. <br> 90 degree parking aisle width: 20 feet single sided one-way, 25 feet double sided two-way | 17.98 .60 | 6 compact spaces / 22 total spaces $=27 \%$, therefore ok <br> All parking it 90 degree. 25 foot aisle provided off of Pleasant Street. 17'-3" aisle provided at south end of site, near compact parking stalls. This is larger than the approximately 14 foot aisle that currently exists at the south end of the 6 shared stalls. If a stall needs to be removed to provide a 20 foot space, there will be one parking stall fewer than required. Because these stalls are compact, $17^{\prime}-3^{\prime \prime}$ will be adequate for cars to back up into the aisle. |
| On-Site Circulation | Groups of more than 3 parking spaces shall be provided with adequate aisles or turnaround areas so that all vehicles enter the right-of-way in a forward manner. | 17.98.70 | Complies |
| Driveway | Driveway to an off-street parking area shall be a minimum of 20 feet for a twoway drive for the first 20 feet of the driveway. Shall not traverse a slope in excess of $15 \%$ at any point along the driveway length. | 17.98 .100 | Complies |
| Landscaping and Screening | Between, and at the end of each parking bay, there shall be planters that have a minimum width of 5 feet and a minimum length of 17 feet for a single depth bay and 34 feet for a double bay. Each planter shall contain one major structural tree and ground cover. | 17.98.120.D | Complies. Please see Site Plan and Landscape Plan |
| Paving | Parking areas, driveways, aisles, and turnarounds shall be paved with concrete, asphalt, or comparable surfacing, constructed to city standards for off-street vehicle areas. | 17.98.130.A | Permeable asphalt will be used for the parking lot surface. |

$\left.\begin{array}{|l|l|l|l|}\hline \text { Topic } & \text { Requirement } & \text { Chapter } & \text { Compliance } \\ \hline & & & \\ \hline & \begin{array}{l}\text { Provide adequate provisions for } \\ \text { collection of drainage waters to eliminate } \\ \text { sheet flow of water onto sidewalks, } \\ \text { public rights-of-way, and abutting private } \\ \text { property. }\end{array} & 17.98 .140 & \begin{array}{l}\text { Permeable asphalt will be } \\ \text { used for the parking lot } \\ \text { surface. }\end{array} \\ \hline & \begin{array}{l}\text { Artificial lighting to be provided at } \\ \text { parking. }\end{array} & 17.98 .150 & \begin{array}{l}\text { See sheet E1.02 - Electrical } \\ \text { Site Lighting Plan. }\end{array} \\ \hline \begin{array}{ll}\text { Bighting } \\ \text { Facilities Parking }\end{array} & \text { A. Location } & 17.98 .160 & \begin{array}{l}\text { A. Bike parking is located } \\ \text { next to the primary building } \\ \text { entrance, visible from the }\end{array} \\ \text { reception/waiting room, } \\ \text { separated from the vehicular } \\ \text { parking area, with direct } \\ \text { access to the public right-of- } \\ \text { way. } \\ \text { B. Each of the (2) bike } \\ \text { parking spaces are 2 1/2 feet } \\ \text { by 6 feet, covered with a } \\ \text { vertical clearance of 12 feet. } \\ \text { C. Bikes can be locked to a } \\ \text { provided bike rack with both } \\ \text { cable and U-shaped locks, }\end{array}\right\}$

## Building Design Standards Narrative

| SANDY DESIGN STANDARDS (17.90.110) | PROPOSAL RESPONSE |
| :---: | :---: |
| A Site Layout and Vehicle Access |  |
| A.1. Lot shall abut or have cross access to a dedicated public street | - The lot has access to Hwy 26/Mt Hood Hwy and Pleasant Street |
| A.3. Off street parking located to the rear or side of buildings, with no portion of the lot within required setbacks or within 10 -feet of the public right-of-way. Driveways for ingress or egress shall be limited to one per 150 ft . | - Parking lot is an extension of the police station parking lot and sited between the two buildings. All portions of the lot are within required setbacks. |
| A.4. Adjacent parking lots shall be connected to one another when the City determines it is practicable to do so. | - Parking lot is an extension of the police station parking lot and sited between the two buildings. |
| A.5. Urban design details shall be used to calm traffic and protect pedestrians in parking areas. | - Sidewalk crossing the driveway will be a different paving material than the parking lot. |
| A.8. Raised walkways or painted crossings from public street sidewalk to building entrance(s). | - There is a different paving material used from the sidewalk to the main building entrance. |
| A.12. Free-standing buildings shall be connected to one another with a seamless pedestrian network that provides access to building entrances and civic spaces. | - The sidewalks along Pleasant Street and Hwy 26 connect to the health clinic and police station entrances. |
| B Building Facades, Materials, and Colors |  |
| B.1. Articulation | - 1) All elevations facing public streets are broken into 30 feet or shorter. Each plane projects or recedes 24 inches from the adjacent plane. <br> - 2) Wall planes incorporate visually contrasting and complimentary trim. |
| B.2. Pedestrian Shelters | - a) Shelter over building's main entry is provided by roof projecting over entry. Metal canopies are provided over the other entries. <br> - b) Canopies, roof overhangs, and alcoves are provided. <br> - c) Overhang over entry projects 5 feet to protect pedestrians. <br> - d) Shelter over main entry is extension of roof with same roof slope. |
| B.3. Building Materials | - a) Building is architecturally unified by palette of earth tone material colors, continuous gabled roof form, and repeated dormer/pop-out language. |


|  | - b) Building has a 36 -inch base of Ebony colored, split-faced concrete block veneer with running bond pattern. Type II adjustment, based on notes from pre-application meeting: Percentage of gaps in base material is $18 \%$. Please see Base Material Diagram on Sheet G1.30. <br> - c) Foundation is sheathed with the split-faced concrete block veneer. <br> - d) Siding above veneer base consists of horizontal wood textured fiberboard sizing (redwood color), vertical ribbed fiberboard panels ( $15 / 8^{\prime \prime}$ ribs and weathered copper color) <br> - e) Building elevations facing a public right-of-way or civic space incorporate the following features: <br> - Wood door and window trim and fascia <br> - Metal canopies <br> - Metal brackets - these are cosmetic metal applied to the ends of the extruded framed walls, labeled "break metal trim" on the exterior elevations <br> - f) Materials required on elevations visible from abutting public streets turn the corner and are consistent around the full perimeter of the building. |
| :---: | :---: |
| B.4. Colors | - a) Color palette is warm earth tones and wood look. Material colors are noted in B.3. section above. Exterior paint for trim and fascia is Miller Historic color H119 Sturgis Gray <br> - b) Black color is only used for metal canopies <br> - c) No day-glow or reflective colors are used. |
| C Roof Pitch, Materials, and Parapets |  |
| C.1. Gabled Roof Pitch | - Primary roof forms are $6: 12$, except at the east roof. East roof is 4 feet wider than the rest of the roofs, so the pitch is $55 / 8: 12$ to allow the ridges to be at the same level. 6:12 pitch is only required for new buildings with a span of 50 feet or less. Portion of roofing at $55 / 8: 12$ is $62^{\prime}-8^{\prime \prime}$ wide due to building program needs. Secondary roof forms are $41 / 2: 12$. |
| C.2. Secondary Roof Forms | - Secondary roof forms comprise of $13 \%$ of roof area. (1,514 secondary roof area, 11,624 total roof area) |
| C.3. Gable | - Gable ends of the roof face Hwy 26, the intersection of Hwy 26 and SE Ten Eyck Road, and the parking lot. The gable facing the parking lot, and part of the main entrance, is visible from Pleasant Street. |
| C.4. Secondary Roof Form Quantities | - (2) secondary roof forms on the $76^{\prime}-7^{\prime \prime}$ length of roof facing Ten Eyck. (2) secondary roof forms on the $114^{\prime}-11^{\prime \prime}$ length of roof facing Pleasant Street. |
| C.5. Roof Materials | - Roofing is a standing seam metal roof, color is gray. |
| C.6. Roof and Wall Mounted Equipment | - There is no rooftop equipment other than an approximately 12 " tubular skylight centered on the roof. Mechanical and electrical equipment is internal in dedicated rooms and in the attic space. |


| D Building Orientation and Entrances |  |
| :---: | :---: |
| D.1. Orientation | - More than $50 \%$ of the building's street frontage is within 10 feet or a sidewalk or civic space. The edges of the building are as close to the property line as allowed. No off-street parking is placed between the building and adjacent streets. |
| D.2. Parking Placement | - Not applicable |
| D.3. Ground Floor Connection | - Ground floor spaces are connected to the sidewalks along Pleasant Street and Hwy 26. |
| D.4. Corner Entrance | - There is a door into the building within 40 feet of the corner at the intersection of Ten Eyck and Hwy 26, and an door into the building within 40 feet of the corner at the intersection of Ten Eyck and Pleasant Street. A direct corner entrance is not practical due to the nature of the building's use as a health clinic and its relation to the parking lot. The corners have detailing consistent with the Sandy Style. |
| D.6. Activated Pedestrian Environment on two Elevations | - There is an entrance with a pedestrian shelter facing Hwy 26 and facing Pleasant Street, however, due to the nature of the clinic, these are staff entrances, not customer entrances. The patient entrance faces the parking lot. Patients may leave the other (2) doors, but these are primarily intended for clinic staff. |
| D.7. Primary Entry Spacing | - Due to the nature of the medical clinic, entrances are spaced further than $30^{\prime}$ apart. There is no public retail as a part of this building. |
| D.8. Primary Entrance Architecture | - The roof overhangs the main entrance by $5^{\prime}$ and there is a bench and landscaped elements to emphasize it. There is a metal canopy over the other entrances, with landscaping near these entries. |
| E Windows |  |
| E.1. Unified Design | - There are only 4 different sizes of windows in this design, and their headers, sills, and mullions are in consistent planes in relation to each other. |
| E.2. Ground Floor Windows | - Type II adjustment: Percentage of window area on Hwy 26 elevation is $27 \%$. Percentage of window area on SE Ten Eyck Road elevation is $28 \%$. Percentage of window area facing Pleasant street is $23 \%$. Due to the use of the building as a clinic, it is not practical to have $40 \%$ windows on (2) of these frontages. The building GSF is just shy of $10,000 \mathrm{SF}$, which only requires $25 \%$ window area, which we are meeting on (2) frontages. <br> - a) Typical windows have a sill 3 feet above grade. <br> - b) Windows are square and vertically oriented. <br> - c) Windows are divided into panes that are smaller than 6 feet. <br> - d) Windows have 4 inch wood trim around them. |
| E.4. Prohibited Windows | - a) None of the windows are darkly tinted or mirrored. |


|  | - b) Storefront windows are located at the entrance. The rest of the windows are vinyl. |
| :---: | :---: |
| F Landscaping and Streetscape Design |  |
| F.1. Landscape and Screening General Standards (Chapter 17.92) | - Please see Chapter 17.92 sections in the Zoning Requirements Narrative above. |
| F.2. Streetscape Design Conflicts | - There are no conflicts with the Streetscape design and Landscaping and Screening General Standards. |
| G Civic Space |  |
| G.1. Percent of Ground Floor Area | - The civic space near the main entry to the clinic is $3 \%$ of the ground floor area. ( 320 SF / 9,381 SF). |
| G.2. Dimensions and Area | - The civic space is $19^{\prime}-8^{\prime \prime}$ across and 320 SF. |
| G.3. Type | - The civic space is an extension of the sidewalk in front of the clinic entrance, with benches for seating, and doubles as a zone for patients to await pick-up. |
| G.4. Location | - The civic space is located roughly mid-block, near the entrance, which will be a zone with higher pedestrian activity. |
| G.5. Adjacency to Right-of-Way | - The civic space is directly adjacent to the sidewalk on Pleasant Street, with rectangular stone pavers to identify it. This space is not gated or otherwise closed to public access. |
| H Lighting |  |
| H.1. Chapter 15.30, Dark Sky Ordinance | - Streetscape lighting conforms to the Downtown Sandy Streetscape Design and Dark Sky Ordinance. See sheet E1.02 - Electrical Site Lighting Plan. The area 10 feet beyond the property line receives no more than 0.25 of a foot-candle of light and the new lighting does not exceed 4,125 Kelvins. |
| H.2. Integration | - Site lighting is situated to highlight the architectural design of the clinic and to compliment the site lighting at the nearby police station. |
| H.3. Safety | - Exterior can lights are integrated into the metal canopies over the entries. Walkways, parking lot, and entries are illuminated at 1.5-2.0 foot candles: see sheet E1.02 - Electrical Site Lighting Plan. |
| I Safety and Security |  |
| I.1. Windows | - Windows are located around the full perimeter of the building, facing pedestrian, parking, and loading areas. |
| I.2. Interior Surveillance | - Windows are located so interior activity by be seen from all public right-of-ways, however, due to the nature of the building as a medical clinic, internal shades will be used to provide patients and staff with privacy as needed. The police station is also right |


|  |  | next to the clinic, which will aid in surveillance of the <br> site. |
| :--- | :--- | :--- |
| I.3. | Street Address | -8 " tall street address numbers are near south entry, <br> near the intersection of Hwy 26 and Pleasant Street, <br> shown on sheet A3.11, elevation 6. <br> I.4. <br> On-Site Lighting Orientation <br> J <br> External Storage and Screening <br> J.1. <br> Exterior Storage lighting is placed to provide adequate <br> surveillance of on-site activities. See sheet E1.02 - <br> Electrical Site Lighting Plan. |
| J.3. | Equipment Screening | There will be no exterior storage of merchandise or <br> materials. |
| J.4. | Trash Screening | Mechanical condensing unit and transformer are <br> screened from view with wood landscape fencing. <br> Garbage storage is screened within trash enclosure. |
|  | Trash and recycling storage is in a split face CMU <br> enclosure in the southeast corner of the site. <br> Enclosure CMU matches the color of the concrete <br> block veneer base of the building. And the metal <br> roof of the trash enclosure matches the metal roofing <br> color. |  |



TABLE OF CONTENTS



OWNER TEAM


NOT FOR CONSTRUCTION









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G. Landscape Plan: Supplemental Information

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LANDSCAPE NOTES PLANT SCHEDUL

## DESIGN

DEVELOPMENT

L4.00



S1 Fixture
H. Sandy Health Clinic - Exterior Light Fixture Cut-Sheets

## Introduction

The modern styling of the D-Series is striking yet unobtrusive - making a bold, progressive statement even as it blends seamlessly with its environment. The D-Series distills the benefits of the latest in LED technology into a high performance, high efficacy, long-life luminaire.
The outstanding photometric performance results in sites with excellent uniformity, greater pole spacing and lower power density. It is ideal for replacing up to 750 W metal halide in pedestrian and area lighting applications with typical energy savings of $65 \%$ and expected service life of over 100,000 hours.

Ordering Information
EXAMPLE: DSX1 LED P7 40K T3M MVOLT SPA NLTAIR2 PIRHN DDBXD
DSX1 LED

| Series | LEDS |  |  | Color temperature |  |  | Distribution |  |  |  | Voltage | Mounting |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| DSX1 LED | Forv P1 P2 P3 Rot P10 P11 | $\begin{gathered} \text { d optic } \\ \text { P4 } \\ \text { P5 } \\ \text { P6 } \\ \text { d opti } \\ \text { P12 } \\ \text { P131 } \end{gathered}$ | $\begin{aligned} & \text { P7 } \\ & \text { P8 } \\ & \text { P9 } \end{aligned}$ | $\begin{aligned} & 30 \mathrm{~K} \\ & 40 \mathrm{~K} \end{aligned}$ $50 \mathrm{~K}$ | $\begin{aligned} & 3000 \mathrm{~K} \\ & 4000 \mathrm{~K} \\ & 5000 \mathrm{~K} \end{aligned}$ |  | T15 <br> T2S <br> T2M <br> T3S <br> T3M <br> T4M <br> TFT | Type I short <br> Type II short <br> Type II medium <br> Type III short <br> Type III medium <br> Type IV medium <br> Forward throw <br> medium | T5VS <br> T5S <br> T5M <br> T5W <br> BLC <br> LCCO <br> RCCO | Type V very short <br> Type V short <br> Type V medium <br> Type V wide <br> Backlight control ${ }^{2}$ <br> Left corner cutoff ${ }^{2}$ <br> Right corner cutoff ${ }^{2}$ | $\begin{aligned} & \text { MVOLT }{ }^{3} \\ & 120^{4} \\ & 208^{4} \\ & 240^{4} \\ & 277^{4} \\ & 347^{4} \\ & 480^{4} \end{aligned}$ | Shipped included <br> SPA <br> Squa <br> RPA <br> WBA <br> SPUMBA <br> RPUMBA Rou <br> Shipped separately <br> KMA8 DDBXDU <br> Mas (spe | pole mount ole mountin cket pole univers ole univers <br> $m$ mountin finish $)^{6}$ | mounting adaptor ${ }^{5}$ mounting adaptor ${ }^{5}$ <br> bracket adaptor |
| Control options |  |  |  |  |  |  |  |  |  |  | Other options |  | Finish | reed) |
| Shipped installed |  |  |  |  |  | PIR |  | High/low, motion/ambient sensor, 8-15'mounting height, ambient sensor enabled at $5 f c^{15,16}$ |  |  | Shipped installed |  | DDBXD | Dark bronze |
| NLTAIR2 | nLight AIR generation 2 enabled ${ }^{7}$ |  |  |  |  |  |  |  |  |  | HS H | -side shield ${ }^{17}$ | DBLXD | Black |
| PIRHN | NEMA twist-lock receptacle only (controls ordered separate) ${ }^{9}$ |  |  |  |  | PIRH |  | High/low, motion/ambient sensor, 15-30'mounting height, ambient sensor enabled at $5 f{ }^{515,16}$ |  |  | SF | fuse (120, 277, 347V) ${ }^{4}$ | DNAXD | Natural aluminum |
| PER |  |  |  |  |  | PIR1FC3V |  | High/low, motion/ambient sensor, 8-15'mounting height, ambient sensor enabled at $1 f f^{55,16}$ |  |  | DF | fuse ( $208,240,480 \mathrm{~V})^{4}$ | DWHXD | White |
| PER5 | Five-pin receptacle only (controls ordered separate) ${ }^{\text {9,0 }}$ |  |  |  |  |  |  | 190 | ated optics ${ }^{1}$ | DDBTXD | Textured dark bronze |
| PER7 | Seven-pin receptade only (controls ordered separate) 9,10 |  |  |  |  | PIRH1FC3V |  |  |  |  | Bi-level, motion/ambient sensor, 15-30'mounting height, ambient sensor enabled at $1 \mathrm{fc}{ }^{15,16}$ |  |  | R90 R | otated optics ${ }^{1}$ | DBLBXD | Textured black |
| DMG | $0-10 \mathrm{v}$ dimming wires pulled outside fixture (for use with an external control, ordered separately) " |  |  |  |  |  |  | Field adjustable output ${ }^{14}$ |  |  | Shipped separatelyBS ${ }^{\text {Bird spikes }}{ }^{18}$ |  | DNATXD | Textured natural aluminum |
| DS | Dual switching ${ }^{12,1,14}$ |  |  |  |  |  |  | EGS Ex | al glare shield | DWHGXD | Textured white |

COMMERCIAL OUTDOOR

## Ordering Information

## Accessories

DLLI27F 1.5 Photocell - SSL twist-lock ( $120-277 \mathrm{~V}$ ) DLL347F 1.5CULJU Photocell-SSL twist-lock (347V) ${ }^{19}$ DLL480F 1.5 CULJU Photocell - SSL twist-lock (480V) ${ }^{19}$ DSHORT SBK U Shorting Cap ${ }^{19}$
DSX1HS $30 \mathrm{CU} \quad$ House-side shield for P1, P2, P3, P4 and P5 ${ }^{17}$ DSXIHS $40 \mathrm{CU} \quad$ House-side shield for P6 and P7 DSX1HS $60 \mathrm{CU} \quad$ House-side shield for $\mathrm{P} 8, \mathrm{P9}, \mathrm{P} 10, \mathrm{P} 11$ and $\mathrm{P} 12^{17}$ PUMBA DDBXD U* Square and round pole universal mounting KMA8 DDBXDU $\quad \begin{gathered}\text { Mastarm } \\ \text { finish) } 6\end{gathered}$ mounting bracket adaptor (specify DSXIEGS (FINISH) U External glare shield For more control options, visit DTL and ROAM online.

## NOTES

1 P10, P11, P12 or P13 and rotated optics ( $\mathrm{L} 90, \mathrm{R} 90$ ) only available together.
2 Not available with HS
MVOLT driver operates on any line voltage from $120-277 \mathrm{~V}(50 / 60 \mathrm{~Hz})$
Single fuse (SF requires 120V, 277 V or 347 V . Double fuse (DF) requires $208 \mathrm{~V}, 240 \mathrm{~V}$ or 480 V .
6 Must order fixture with SPA option. Must be ordered as a separate accessory; see Accessories information. For use with 2-3/8" mast arm (not included)
Must be ordered with PIRHN. Sensor cover available only in dark bronze, black, white and natural aluminum colors.
Must be ordered with NLTAIR2. For more information on nLight Air 2 visit this inkic
Photocell ordered and shipped as a separate line item from Acuity Brands Controls. See accessories. Not available with DS option. Shorting cap included
10 If ROAM ${ }^{\oplus}$ node required, it must be ordered and shipped as a separate line item from Acuity Brands Controls. Node with integral dimming
1 DMG not available with PIRHN, PER5, PER7, PIR, PIRH, PIR1FC3V or PIRH1FC3V.
3 Requires (2) separately switched circuits with isolated neutrol. See Outdoor Control Technical Guide for details.
14 Reference Motion Sensor table on page 4.
15 Reference controls options table on page 4 to see functionality.
16 Not available with other dimming controls options
7 Not available with BLC, LCCO and RCCO distribution. Also available as a separate accessory; see Accessories information.
8 Must be ordered with fixture for factory pre-dilling.
9 Requires luminaire to be specified with PER, PER5 or PER7 option. See PER Table on page 3
20 For retrofit use only.

## Options

EGS - External Glare Shield


## Drilling

HANDHOLE ORIENTATION


Tenon Mounting Slipfitter**

| Tenon 0.D. | Mounting | Single Unit | 2 @ 180 | 2@90 | 3 @120 | 3 @ 90 | 4 @ 90 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 2-3/8" | SPA/RPA | AS3-5 190 | AS3-5 280 | AS3-5 290 | AS3-5320 | AS3-5390 | AS3-5 490 |
|  | SPUMBA | AS3-5 190 | AS3-5 280 | AS4-5 290 | AS3-5320 | AS4-5390 | AS4-5490 |
|  | RUPUMBA | AS3-5 190 | AS3-5 280 |  | AS3-5320 |  |  |
| 2-7/8" | SPA/RPA | AST25-190 | AST25-280 | AST25-290 | AST25-320 | AST25-390 | AST25-490 |
|  | SPUMBA | AST25-190 | AST25-280 |  | AST25-320 |  |  |
|  | RUPUMBA | AST25-190 | AST25-280 |  | AST25-320 |  |  |
| $4{ }^{4}$ | SPA/RPA | AST35-190 | AST35-280 | ASTT3-290 | AST35-320 | ASTT35-390 | ASTT35-490 |
|  | SPUMBA | AST35-190 | AST35-280 | AST35-290 | AST35-320 | AST35-390 | AST35-490 |
|  | RUPUMBA | AST35-190 | ASTT3-280 |  | AST35-320 |  |  |


|  |  |  |  |  |  |  |  |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mounting Option | Drilling Template | Single | $2 @ 180$ | $2 @ 90$ | $3 @ 90$ | $3 @ 120$ | $4 @ 90$ |
| Head Location |  | Side B | Side B \& D | Side B \& C | Side B, C \& D | Round Pole Only | Side A, B, C \& D |
| Drill Nomenclature | \#8 | DM19AS | DM28AS | DM29AS | DM39AS | DM32AS | DM49AS |


|  | Drilling Template | Minimum Acceptable Outside Pole Dimension |  |  |  |  |  |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| SPA | $\# 8$ | $2-7 / 8^{\prime \prime}$ | $2-7 / 8^{\prime \prime}$ | $3.5^{\prime \prime}$ | $3.5^{\prime \prime}$ | $3^{\prime \prime}$ | $3.5^{\prime \prime}$ |
| RPA | $\# 8$ | $2-7 / 8^{\prime \prime}$ | $2-7 / 8^{\prime \prime}$ | $3.5^{\prime \prime}$ | $3.5^{\prime \prime}$ | $3^{\prime \prime}$ | $3.5^{\prime \prime}$ |
| SPUMBA | $\# 5$ | $2-7 / 8^{\prime \prime}$ | $3^{\prime \prime}$ | $4^{\prime \prime}$ | $4^{\prime \prime}$ | $3.5^{\prime \prime}$ | $4^{\prime \prime}$ |
| RPUMBA | $\# 5$ | $2-7 / 8^{\prime \prime}$ | $3.5^{\prime \prime}$ | $5^{\prime \prime}$ | $5^{\prime \prime}$ | $3.5^{\prime \prime}$ | $5^{\prime \prime}$ |

## Photometric Diagrams To see complete photometric reports or download .ies files for this product, visit Lithonia Lighting's D-Series Area Size 1 homepage.

Isofootcandle plots for the DSX1 LED 60C 1000 40K. Distances are in units of mounting height ( $25^{\prime}$ ).












## Performance Data

Lumen Ambient Temperature (LAT) Multipliers
Use these factors to determine relative lumen output for average ambient temperatures from $0-40^{\circ} \mathrm{C}\left(32-104^{\circ} \mathrm{F}\right)$.

| Ambient |  | Lumen Multiplier |
| :---: | :---: | :---: |
| $0^{\circ} \mathrm{C}$ | $32^{\circ} \mathrm{F}$ | 1.04 |
| $5^{\circ} \mathrm{C}$ | $41^{\circ} \mathrm{F}$ | 1.04 |
| $10^{\circ} \mathrm{C}$ | $50^{\circ} \mathrm{F}$ | 1.03 |
| $15^{\circ} \mathrm{C}$ | $50^{\circ} \mathrm{F}$ | 1.02 |
| $20^{\circ} \mathrm{C}$ | $68^{\circ} \mathrm{F}$ | 1.01 |
| $25^{\circ} \mathrm{C}$ | $\mathbf{7 7 ^ { \circ } \mathbf { F }}$ | $\mathbf{1 . 0 0}$ |
| $30^{\circ} \mathrm{C}$ | $86^{\circ} \mathrm{F}$ | 0.99 |
| $35^{\circ} \mathrm{C}$ | $95^{\circ} \mathrm{F}$ | 0.98 |
| $40^{\circ} \mathrm{C}$ | $104^{\circ} \mathrm{F}$ | 0.97 |

## Projected LED Lumen Maintenance

Data references the extrapolated performance projections for the platforms noted in a $25^{\circ} \mathrm{C}$ ambient, based on 10,000 hours of LED testing (tested per IESNA LM-80-08 and projected per IESNA TM-21-11).
To calculate LLF, use the lumen maintenance factor that corresponds to the desired number of operating hours below. For other lumen maintenance values, contact factory.

| Operating Hours | Lumen Maintenance Factor |
| :---: | :---: |
| 0 | 1.00 |
| 25,000 | 0.96 |
| 50,000 | 0.92 |
| 100,000 | 0.85 |


| Motion Sensor Default Settings |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Option | Dimmed <br> State | High Level <br> (when <br> triggered) | Phototcell <br> Operation | Dwell <br> Time | Ramp-up <br> Time | Ramp-down <br> Time |  |
| PIR or PIRH | $3 V(37 \%)$ <br> Output | $10 \mathrm{~V}(100 \%)$ <br> Output | Enabled @ 5FC | 5 min | 3 sec | 5 min |  |
| *PIR1FC3V or <br> PIRH1FC3V | $3 V(37 \%)$ <br> Output | $10 \mathrm{~V}(100 \%)$ <br> Output | Enabled @ 1FC | 5 min | 3 sec | 5 min |  |

Electrical Load


Controls Options

| Controls Options |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| Nomenclature | Descripton | Functionality | Primary control device | Notes |
| FAO | Field adjustable output device installed inside the lumiaire; wired to the driver dimming leads. | Allows the lumiaire to be manually dimmed, effectively trimming the light output. | FA0 device | Cannot be used with other controls options that need the $0-10 \mathrm{~V}$ leads |
| DS | Drivers wired independantly for 50/50 luminaire operation | The luminaire is wired to two separate circuits, allowing for $50 / 50$ operation. | Independently wired drivers | Requires two seperately switched circuits. Consider nLight AIR as a more cost effective alternative. |
| PER5 or PER7 | Twist-lock photocell recepticle | Compatible with standard twist-lock photocells for dusk to dawn operation, or advanced control nodes that provide $0-10 \mathrm{~V}$ dimming signals. | Twist-lock photocells such as DLL Elite or advanced control nodes such as ROAM. | Pins 4 \& 5 to dimming leads on driver, Pins 6 \& 7 are capped inside luminaire |
| PIR or PIRH | Motion sensors with integral photocell. PIR for 8-15' mounting; PIRH for 15-30' mounting | Luminaires dim when no occupancy is detected. | Acuity Controls SBGR | Also available with PIRH1FC3V when the sensor photocell is used for dusk-to-dawn operation. |
| NLTAIR2 PIRHN | nLight AIR enabled luminaire for motion sensing, photocell and wireless communication. | Motion and ambient light sensing with group response. Scheduled dimming with motion sensor over-ride when wirelessly connected to the nLight Eclypse. | nLight Air rSDGR | nLight AIR sensors can be programmed and commissioned from the ground using the CIAIRity Pro app. |

COMMERCIAL OUTDOOR

## Performance Data

Lumen Output
Lumen values are from photometric tests performed in accordance with IESNA LM-79-08. Data is considered to be representative of the configurations shown, within the tolerances allowed by Lighting Facts Contact factory for performance data on any configurations not shown here.

| Forward Optics |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| LED Count | Drive | Power | System | Dist. | $\begin{gathered} 30 \mathrm{~K} \\ (3000 \mathrm{~K}, 70 \mathrm{CRI}) \\ \hline \end{gathered}$ |  |  |  |  | $\begin{gathered} 40 \mathrm{~K} \\ (4000 \mathrm{~K}, 70 \mathrm{CRI}) \end{gathered}$ |  |  |  |  | $\begin{gathered} 50 \mathrm{~K} \\ (5000 \mathrm{~K}, 70 \mathrm{CRI}) \\ \hline \end{gathered}$ |  |  |  |  |
|  |  |  |  |  | Lumens | B | U | 6 | LPW | Lumens | B | U | , | LPW | Lumens | B | U | 6 | LPW |
| 30 | 530 | P1 | 54W | T1S | 6,457 | 2 | 0 | 2 | 120 | 6,956 | 2 | 0 | 2 | 129 | 7,044 | 2 | 0 | 2 | 130 |
|  |  |  |  | T2S | 6,450 | 2 | 0 | 2 | 119 | 6,949 | 2 | 0 | 2 | 129 | 7,037 | 2 | 0 | 2 | 130 |
|  |  |  |  | T2M | 6,483 | 1 | 0 | 1 | 120 | 6,984 | 2 | 0 | 2 | 129 | 7,073 | 2 | 0 | 2 | 131 |
|  |  |  |  | T3S | 6,279 | 2 | 0 | 2 | 116 | 6,764 | 2 | 0 | 2 | 125 | 6,850 | 2 | 0 | 2 | 127 |
|  |  |  |  | T3M | 6,468 | 1 | 0 | 2 | 120 | 6,967 | 1 | 0 | 2 | 129 | 7,056 | 1 | 0 | 2 | 131 |
|  |  |  |  | T4M | 6,327 | 1 | 0 | 2 | 117 | 6,816 | 1 | 0 | 2 | 126 | 6,902 | 1 | 0 | 2 | 128 |
|  |  |  |  | TFTM | 6,464 | 1 | 0 | 2 | 120 | 6,963 | 1 | 0 | 2 | 129 | 7,051 | 1 | 0 | 2 | 131 |
|  |  |  |  | T5VS | 6,722 | 2 | 0 | 0 | 124 | 7,242 | 3 | 0 | 0 | 134 | 7,334 | 3 | 0 | 0 | 136 |
|  |  |  |  | T5S | 6,728 | 2 | 0 | 1 | 125 | 7,248 | 2 | 0 | 1 | 134 | 7,340 | 2 | 0 | 1 | 136 |
|  |  |  |  | T5M | 6,711 | 3 | 0 | 1 | 124 | 7,229 | 3 | 0 | 1 | 134 | 7,321 | 3 | 0 | 2 | 136 |
|  |  |  |  | T5W | 6,667 | 3 | 0 | 2 | 123 | 7,182 | 3 | 0 | 2 | 133 | 7,273 | 3 | 0 | 2 | 135 |
|  |  |  |  | BLC | 5,299 | 1 | 0 | 1 | 98 | 5,709 | 1 | 0 | 2 | 106 | 5,781 | 1 | 0 | 2 | 107 |
|  |  |  |  | LCCO | 3,943 | 1 | 0 | 2 | 73 | 4,248 | 1 | 0 | 2 | 79 | 4,302 | 1 | 0 | 2 | 80 |
|  |  |  |  | RCCO | 3,943 | 1 | 0 | 2 | 73 | 4,248 | 1 | 0 | 2 | 79 | 4,302 | 1 | 0 | 2 | 80 |
| 30 | 700 | P2 | 70W | T15 | 8,249 | 2 | 0 | 2 | 118 | 8,886 | 2 | 0 | 2 | 127 | 8,999 | 2 | 0 | 2 | 129 |
|  |  |  |  | T2S | 8,240 | 2 | 0 | 2 | 118 | 8,877 | 2 | 0 | 2 | 127 | 8,989 | 2 | 0 | 2 | 128 |
|  |  |  |  | T2M | 8,283 | 2 | 0 | 2 | 118 | 8,923 | 2 | 0 | 2 | 127 | 9,036 | 2 | 0 | 2 | 129 |
|  |  |  |  | T3S | 8,021 | 2 | 0 | 2 | 115 | 8,641 | 2 | 0 | 2 | 123 | 8,751 | 2 | 0 | 2 | 125 |
|  |  |  |  | T3M | 8,263 | 2 | 0 | 2 | 118 | 8,901 | 2 | 0 | 2 | 127 | 9,014 | 2 | 0 | 2 | 129 |
|  |  |  |  | T4M | 8,083 | 2 | 0 | 2 | 115 | 8,708 | 2 | 0 | 2 | 124 | 8,818 | 2 | 0 | 2 | 126 |
|  |  |  |  | TFTM | 8,257 | 2 | 0 | 2 | 118 | 8,896 | 2 | 0 | 2 | 127 | 9,008 | 2 | 0 | 2 | 129 |
|  |  |  |  | T5VS | 8,588 | 3 | 0 | 0 | 123 | 9,252 | 3 | 0 | 0 | 132 | 9,369 | 3 | 0 | 0 | 134 |
|  |  |  |  | T5S | 8,595 | 3 | 0 | 1 | 123 | 9,259 | 3 | 0 | 1 | 132 | 9,376 | 3 | 0 | 1 | 134 |
|  |  |  |  | T5M | 8,573 | 3 | 0 | 2 | 122 | 9,236 | 3 | 0 | 2 | 132 | 9,353 | 3 | 0 | 2 | 134 |
|  |  |  |  | T5W | 8,517 | 3 | 0 | 2 | 122 | 9,175 | 4 | 0 | 2 | 131 | 9,291 | 4 | 0 | 2 | 133 |
|  |  |  |  | BLC | 6,770 | 1 | 0 | 2 | 97 | 7,293 | 1 | 0 | 2 | 104 | 7,386 | 1 | 0 | 2 | 106 |
|  |  |  |  | LCCO | 5,038 | 1 | 0 | 2 | 72 | 5,427 | 1 | 0 | 2 | 78 | 5,496 | 1 | 0 | 2 | 79 |
|  |  |  |  | RCCO | 5,038 | 1 | 0 | 2 | 72 | 5,427 | 1 | 0 | 2 | 78 | 5,496 | 1 | 0 | 2 | 79 |
| 30 | 1050 | P3 | 102W | T15 | 11,661 | 2 | 0 | 2 | 114 | 12,562 | 3 | 0 | 3 | 123 | 12,721 | 3 | 0 | 3 | 125 |
|  |  |  |  | T2S | 11,648 | 2 | 0 | 2 | 114 | 12,548 | 3 | 0 | 3 | 123 | 12,707 | 3 | 0 | 3 | 125 |
|  |  |  |  | T2M | 11,708 | 2 | 0 | 2 | 115 | 12,613 | 2 | 0 | 2 | 124 | 12,773 | 2 | 0 | 2 | 125 |
|  |  |  |  | T3S | 11,339 | 2 | 0 | 2 | 111 | 12,215 | 3 | 0 | 3 | 120 | 12,370 | 3 | 0 | 3 | 121 |
|  |  |  |  | T3M | 11,680 | 2 | 0 | 2 | 115 | 12,582 | 2 | 0 | 2 | 123 | 12,742 | 2 | 0 | 2 | 125 |
|  |  |  |  | T4M | 11,426 | 2 | 0 | 3 | 112 | 12,309 | 2 | 0 | 3 | 121 | 12,465 | 2 | 0 | 3 | 122 |
|  |  |  |  | TFTM | 11,673 | 2 | 0 | 2 | 114 | 12,575 | 2 | 0 | 3 | 123 | 12,734 | 2 | 0 | 3 | 125 |
|  |  |  |  | T5VS | 12,140 | 3 | 0 | 1 | 119 | 13,078 | 3 | 0 | 1 | 128 | 13,244 | 3 | 0 | 1 | 130 |
|  |  |  |  | T5S | 12,150 | 3 | 0 | 1 | 119 | 13,089 | 3 | 0 | 1 | 128 | 13,254 | 3 | 0 | 1 | 130 |
|  |  |  |  | T5M | 12,119 | 4 | 0 | 2 | 119 | 13,056 | 4 | 0 | 2 | 128 | 13,221 | 4 | 0 | 2 | 130 |
|  |  |  |  | T5W | 12,040 | 4 | 0 | 3 | 118 | 12,970 | 4 | 0 | 3 | 127 | 13,134 | 4 | 0 | 3 | 129 |
|  |  |  |  | BLC | 9,570 | 1 | 0 | 2 | 94 | 10,310 | 1 | 0 | 2 | 101 | 10,440 | 1 | 0 | 2 | 102 |
|  |  |  |  | LCCO | 7,121 | 1 | 0 | 3 | 70 | 7,671 | 1 | 0 | 3 | 75 | 7,768 | 1 | 0 | 3 | 76 |
|  |  |  |  | RCCO | 7,121 | 1 | 0 | 3 | 70 | 7,671 | 1 | 0 | 3 | 75 | 7,768 | 1 | 0 | 3 | 76 |
| 30 | 1250 | P4 | 125W | T15 | 13,435 | 3 | 0 | 3 | 107 | 14,473 | 3 | 0 | 3 | 116 | 14,657 | 3 | 0 | 3 | 117 |
|  |  |  |  | T2S | 13,421 | 3 | 0 | 3 | 107 | 14,458 | 3 | 0 | 3 | 116 | 14,641 | 3 | 0 | 3 | 117 |
|  |  |  |  | T2M | 13,490 | 2 | 0 | 2 | 108 | 14,532 | 3 | 0 | 3 | 116 | 14,716 | 3 | 0 | 3 | 118 |
|  |  |  |  | T3S | 13,064 | 3 | 0 | 3 | 105 | 14,074 | 3 | 0 | 3 | 113 | 14,252 | 3 | 0 | 3 | 114 |
|  |  |  |  | T3M | 13,457 | 2 | 0 | 2 | 108 | 14,497 | 2 | 0 | 2 | 116 | 14,681 | 2 | 0 | 2 | 117 |
|  |  |  |  | T4M | 13,165 | 2 | 0 | 3 | 105 | 14,182 | 2 | 0 | 3 | 113 | 14,362 | 2 | 0 | 3 | 115 |
|  |  |  |  | TFTM | 13,449 | 2 | 0 | 3 | 108 | 14,488 | 2 | 0 | 3 | 116 | 14,672 | 2 | 0 | 3 | 117 |
|  |  |  |  | T5VS | 13,987 | 4 | 0 | 1 | 112 | 15,068 | 4 | 0 | 1 | 121 | 15,259 | 4 | 0 | 1 | 122 |
|  |  |  |  | T5S | 13,999 | 3 | 0 | 1 | 112 | 15,080 | 3 | 0 | 1 | 121 | 15,271 | 3 | 0 | 1 | 122 |
|  |  |  |  | T5M | 13,963 | 4 | 0 | 2 | 112 | 15,042 | 4 | 0 | 2 | 120 | 15,233 | 4 | 0 | 2 | 122 |
|  |  |  |  | T5W | 13,872 | 4 | 0 | 3 | 111 | 14,944 | 4 | 0 | 3 | 120 | 15,133 | 4 | 0 | 3 | 121 |
|  |  |  |  | BLC | 11,027 | 1 | 0 | 2 | 88 | 11,879 | 1 | 0 | 2 | 95 | 12,029 | 1 | 0 | 2 | 96 |
|  |  |  |  | LCCO | 8,205 | 1 | 0 | 3 | 66 | 8,839 | 1 | 0 | 3 | 71 | 8,951 | 1 | 0 | 3 | 72 |
|  |  |  |  | RCCO | 8,205 | 1 | 0 | 3 | 66 | 8,839 | 1 | 0 | 3 | 71 | 8,951 | 1 | 0 | 3 | 72 |
| 30 | 1400 | P5 | 138W | T15 | 14,679 | 3 | 0 | 3 | 106 | 15,814 | 3 | 0 | 3 | 115 | 16,014 | 3 | 0 | 3 | 116 |
|  |  |  |  | T2S | 14,664 | 3 | 0 | 3 | 106 | 15,797 | 3 | 0 | 3 | 114 | 15,997 | 3 | 0 | 3 | 116 |
|  |  |  |  | T2M | 14,739 | 3 | 0 | 3 | 107 | 15,878 | 3 | 0 | 3 | 115 | 16,079 | 3 | 0 | 3 | 117 |
|  |  |  |  | T3S | 14,274 | 3 | 0 | 3 | 103 | 15,377 | 3 | 0 | 3 | 111 | 15,572 | 3 | 0 | 3 | 113 |
|  |  |  |  | T3M | 14,704 | 2 | 0 | 3 | 107 | 15,840 | 3 | 0 | 3 | 115 | 16,040 | 3 | 0 | 3 | 116 |
|  |  |  |  | T4M | 14,384 | 2 | 0 | 3 | 104 | 15,496 | 3 | 0 | 3 | 112 | 15,692 | 3 | 0 | 3 | 114 |
|  |  |  |  | TFTM | 14,695 | 2 | 0 | 3 | 106 | 15,830 | 3 | 0 | 3 | 115 | 16,030 | 3 | 0 | 3 | 116 |
|  |  |  |  | TSVS | 15,283 | 4 | 0 | 1 | 111 | 16,464 | 4 | 0 | 1 | 119 | 16,672 | 4 | 0 | 1 | 121 |
|  |  |  |  | T5S | 15,295 | 3 | 0 | 1 | 111 | 16,477 | 4 | 0 | 1 | 119 | 16,686 | 4 | 0 | 1 | 121 |
|  |  |  |  | T5M | 15,257 | 4 | 0 | 2 | 111 | 16,435 | 4 | 0 | 2 | 119 | 16,644 | 4 | 0 | 2 | 121 |
|  |  |  |  | T5W | 15,157 | 4 | 0 | 3 | 110 | 16,328 | 4 | 0 | 3 | 118 | 16,534 | 4 | 0 | 3 | 120 |
|  |  |  |  | BLC | 12,048 | 1 | 0 | 2 | 87 | 12,979 | 1 | 0 | 2 | 94 | 13,143 | 1 | 0 | 2 | 95 |
|  |  |  |  | LCCO | 8,965 | 1 | 0 | 3 | 65 | 9,657 | 1 | 0 | 3 | 70 | 9,780 | 1 | 0 | 3 | 71 |
|  |  |  |  | RCCO | 8,965 | 1 | 0 | 3 | 65 | 9,657 | 1 | 0 | 3 | 70 | 9,780 | 1 | 0 | 3 | 71 |

COMMERCIAL OUTDOOR

## Performance Data

## Lumen Output

umen values are from photometric tests performed in accordance with IESNA LM-79-08. Data is considered to be representative of the configurations shown, within the tolerances allowed by Lighting Facts. Contact factory for performance data on any configurations not shown here.

| Forward Optics |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| LED Count | Drive | Power | System | Dist. | $\begin{gathered} 30 \mathrm{~K} \\ (3000 \mathrm{~K}, 70 \mathrm{CRI}) \end{gathered}$ |  |  |  |  | $\begin{gathered} 40 \mathrm{~K} \\ (4000 \mathrm{~K}, 70 \mathrm{CRI}) \end{gathered}$ |  |  |  |  | $\begin{gathered} 50 \mathrm{~K} \\ (5000 \mathrm{~K}, 70 \mathrm{CRI}) \\ \hline \end{gathered}$ |  |  |  |  |
|  |  |  |  |  | Lumens | B | U | G | LPW | Lumens | B | U | G | LPW | Lumens | B | U | G | LPW |
| 40 | 1250 | P6 | 163W | T1S | 17,654 | 3 | 0 | 3 | 108 | 19,018 | 3 | 0 | 3 | 117 | 19,259 | 3 | 0 | 3 | 118 |
|  |  |  |  | T2S | 17,635 | 3 | 0 | 3 | 108 | 18,998 | 3 | 0 | 3 | 117 | 19,238 | 3 | 0 | 3 | 118 |
|  |  |  |  | T2M | 17,726 | 3 | 0 | 3 | 109 | 19,096 | 3 | 0 | 3 | 117 | 19,337 | 3 | 0 | 3 | 119 |
|  |  |  |  | T3S | 17,167 | 3 | 0 | 3 | 105 | 18,493 | 3 | 0 | 3 | 113 | 18,727 | 3 | 0 | 3 | 115 |
|  |  |  |  | T3M | 17,683 | 3 | 0 | 3 | 108 | 19,049 | 3 | 0 | 3 | 117 | 19,290 | 3 | 0 | 3 | 118 |
|  |  |  |  | T4M | 17,299 | 3 | 0 | 3 | 106 | 18,635 | 3 | 0 | 4 | 114 | 18,871 | 3 | 0 | 4 | 116 |
|  |  |  |  | TFTM | 17,672 | 3 | 0 | 3 | 108 | 19,038 | 3 | 0 | 4 | 117 | 19,279 | 3 | 0 | 4 | 118 |
|  |  |  |  | T5VS | 18,379 | 4 | 0 | 1 | 113 | 19,800 | 4 | 0 | 1 | 121 | 20,050 | 4 | 0 | 1 | 123 |
|  |  |  |  | T5S | 18,394 | 4 | 0 | 2 | 113 | 19,816 | 4 | 0 | 2 | 122 | 20,066 | 4 | 0 | 2 | 123 |
|  |  |  |  | T5M | 18,348 | 4 | 0 | 2 | 113 | 19,766 | 4 | 0 | 2 | 121 | 20,016 | 4 | 0 | 2 | 123 |
|  |  |  |  | T5W | 18,228 | 5 | 0 | 3 | 112 | 19,636 | 5 | 0 | 3 | 120 | 19,885 | 5 | 0 | 3 | 122 |
|  |  |  |  | BLC | 14,489 | 2 | 0 | 2 | 89 | 15,609 | 2 | 0 | 3 | 96 | 15,806 | 2 | 0 | 3 | 97 |
|  |  |  |  | LCCO | 10,781 | 1 | 0 | 3 | 66 | 11,614 | 1 | 0 | 3 | 71 | 11,761 | 2 | 0 | 3 | 72 |
|  |  |  |  | RCCO | 10,781 | 1 | 0 | 3 | 66 | 11,614 | 1 | 0 | 3 | 71 | 11,761 | 2 | 0 | 3 | 72 |
| 40 | 1400 | P7 | 183W | T15 | 19,227 | 3 | 0 | 3 | 105 | 20,712 | 3 | 0 | 3 | 113 | 20,975 | 3 | 0 | 3 | 115 |
|  |  |  |  | T2S | 19,206 | 3 | 0 | 3 | 105 | 20,690 | 3 | 0 | 3 | 113 | 20,952 | 3 | 0 | 3 | 114 |
|  |  |  |  | T2M | 19,305 | 3 | 0 | 3 | 105 | 20,797 | 3 | 0 | 3 | 114 | 21,060 | 3 | 0 | 3 | 115 |
|  |  |  |  | T3S | 18,696 | 3 | 0 | 3 | 102 | 20,141 | 3 | 0 | 3 | 110 | 20,396 | 3 | 0 | 4 | 111 |
|  |  |  |  | T3M | 19,258 | 3 | 0 | 3 | 105 | 20,746 | 3 | 0 | 3 | 113 | 21,009 | 3 | 0 | 3 | 115 |
|  |  |  |  | T4M | 18,840 | 3 | 0 | 4 | 103 | 20,296 | 3 | 0 | 4 | 111 | 20,553 | 3 | 0 | 4 | 112 |
|  |  |  |  | TFTM | 19,246 | 3 | 0 | 4 | 105 | 20,734 | 3 | 0 | 4 | 113 | 20,996 | 3 | 0 | 4 | 115 |
|  |  |  |  | T5VS | 20,017 | 4 | 0 | 1 | 109 | 21,564 | 4 | 0 | 1 | 118 | 21,837 | 4 | 0 | 1 | 119 |
|  |  |  |  | T5S | 20,033 | 4 | 0 | 2 | 109 | 21,581 | 4 | 0 | 2 | 118 | 21,854 | 4 | 0 | 2 | 119 |
|  |  |  |  | T5M | 19,983 | 4 | 0 | 2 | 109 | 21,527 | 5 | 0 | 3 | 118 | 21,799 | 5 | 0 | 3 | 119 |
|  |  |  |  | T5W | 19,852 | 5 | 0 | 3 | 108 | 21,386 | 5 | 0 | 3 | 117 | 21,656 | 5 | 0 | 3 | 118 |
|  |  |  |  | BLC | 15,780 | 2 | 0 | 3 | 86 | 16,999 | 2 | 0 | 3 | 93 | 17,214 | 2 | 0 | 3 | 94 |
|  |  |  |  | LCCO | 11,742 | 2 | 0 | 3 | 64 | 12,649 | 2 | 0 | 3 | 69 | 12,809 | 2 | 0 | 3 | 70 |
|  |  |  |  | RCCO | 11,742 | 2 | 0 | 3 | 64 | 12,649 | 2 | 0 | 3 | 69 | 12,809 | 2 | 0 | 3 | 70 |
| 60 | 1050 | P8 | 207W | T15 | 22,490 | 3 | 0 | 3 | 109 | 24,228 | 3 | 0 | 3 | 117 | 24,535 | 3 | 0 | 3 | 119 |
|  |  |  |  | T2S | 22,466 | 3 | 0 | 4 | 109 | 24,202 | 3 | 0 | 4 | 117 | 24,509 | 3 | 0 | 4 | 118 |
|  |  |  |  | T2M | 22,582 | 3 | 0 | 3 | 109 | 24,327 | 3 | 0 | 3 | 118 | 24,635 | 3 | 0 | 3 | 119 |
|  |  |  |  | T3S | 21,870 | 3 | 0 | 4 | 106 | 23,560 | 3 | 0 | 4 | 114 | 23,858 | 3 | 0 | 4 | 115 |
|  |  |  |  | T3M | 22,527 | 3 | 0 | 4 | 109 | 24,268 | 3 | 0 | 4 | 117 | 24,575 | 3 | 0 | 4 | 119 |
|  |  |  |  | T4M | 22,038 | 3 | 0 | 4 | 106 | 23,741 | 3 | 0 | 4 | 115 | 24,041 | 3 | 0 | 4 | 116 |
|  |  |  |  | TFTM | 22,513 | 3 | 0 | 4 | 109 | 24,253 | 3 | 0 | 4 | 117 | 24,560 | 3 | 0 | 4 | 119 |
|  |  |  |  | T5VS | 23,415 | 5 | 0 | 1 | 113 | 25,224 | 5 | 0 | 1 | 122 | 25,543 | 5 | 0 | 1 | 123 |
|  |  |  |  | T5S | 23,434 | 4 | 0 | 2 | 113 | 25,244 | 4 | 0 | 2 | 122 | 25,564 | 4 | 0 | 2 | 123 |
|  |  |  |  | T5M | 23,374 | 5 | 0 | 3 | 113 | 25,181 | 5 | 0 | 3 | 122 | 25,499 | 5 | 0 | 3 | 123 |
|  |  |  |  | T5W | 23,221 | 5 | 0 | 4 | 112 | 25,016 | 5 | 0 | 4 | 121 | 25,332 | 5 | 0 | 4 | 122 |
|  |  |  |  | BLC | 18,458 | 2 | 0 | 3 | 89 | 19,885 | 2 | 0 | 3 | 96 | 20,136 | 2 | 0 | 3 | 97 |
|  |  |  |  | LCCO | 13,735 | 2 | 0 | 3 | 66 | 14,796 | 2 | 0 | 4 | 71 | 14,983 | 2 | 0 | 4 | 72 |
|  |  |  |  | RCCO | 13,735 | 2 | 0 | 3 | 66 | 14,796 | 2 | 0 | 4 | 71 | 14,983 | 2 | 0 | 4 | 72 |
| 60 | 1250 | P9 | 241W | T15 | 25,575 | 3 | 0 | 3 | 106 | 27,551 | 3 | 0 | 3 | 114 | 27,900 | 3 | 0 | 3 | 116 |
|  |  |  |  | T2S | 25,548 | 3 | 0 | 4 | 106 | 27,522 | 3 | 0 | 4 | 114 | 27,871 | 3 | 0 | 4 | 116 |
|  |  |  |  | T2M | 25,680 | 3 | 0 | 3 | 107 | 27,664 | 3 | 0 | 3 | 115 | 28,014 | 3 | 0 | 3 | 116 |
|  |  |  |  | T3S | 24,870 | 3 | 0 | 4 | 103 | 26,791 | 3 | 0 | 4 | 111 | 27,130 | 3 | 0 | 4 | 113 |
|  |  |  |  | T3M | 25,617 | 3 | 0 | 4 | 106 | 27,597 | 3 | 0 | 4 | 115 | 27,946 | 3 | 0 | 4 | 116 |
|  |  |  |  | T4M | 25,061 | 3 | 0 | 4 | 104 | 26,997 | 3 | 0 | 4 | 112 | 27,339 | 3 | 0 | 4 | 113 |
|  |  |  |  | TFTM | 25,602 | 3 | 0 | 4 | 106 | 27,580 | 3 | 0 | 4 | 114 | 27,929 | 3 | 0 | 4 | 116 |
|  |  |  |  | T5VS | 26,626 | 5 | 0 | 1 | 110 | 28,684 | 5 | 0 | 1 | 119 | 29,047 | 5 | 0 | 1 | 121 |
|  |  |  |  | T5S | 26,648 | 4 | 0 | 2 | 111 | 28,707 | 5 | 0 | 2 | 119 | 29,070 | 5 | 0 | 2 | 121 |
|  |  |  |  | T5M | 26,581 | 5 | 0 | 3 | 110 | 28,635 | 5 | 0 | 3 | 119 | 28,997 | 5 | 0 | 3 | 120 |
|  |  |  |  | T5W | 26,406 | 5 | 0 | 4 | 110 | 28,447 | 5 | 0 | 4 | 118 | 28,807 | 5 | 0 | 4 | 120 |
|  |  |  |  | BLC | 20,990 | 2 | 0 | 3 | 87 | 22,612 | 2 | 0 | 3 | 94 | 22,898 | 2 | 0 | 3 | 95 |
|  |  |  |  | LCCO | 15,619 | 2 | 0 | 4 | 65 | 16,825 | 2 | 0 | 4 | 70 | 17,038 | 2 | 0 | 4 | 71 |
|  |  |  |  | RCCO | 15,619 | 2 | 0 | 4 | 65 | 16,825 | 2 | 0 | 4 | 70 | 17,038 | 2 | 0 | 4 | 71 |

## Performance Data

Lumen Output
Lumen values are from photometric tests performed in accordance with IESNA LM-79-08. Data is considered to be representative of the configurations shown, within the tolerances allowed by Lighting Facts. Contact factory for
performance data on any configurations not shown here.

| Rotated Optics |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| LED Count | Drive | Power | System | Dist. | $\begin{gathered} 30 \mathrm{~K} \\ (3000 \mathrm{~K}, 70 \mathrm{CRI}) \\ \hline \end{gathered}$ |  |  |  |  | $\begin{gathered} 40 \mathrm{~K} \\ (4000 \mathrm{~K}, 70 \mathrm{CRI}) \\ \hline \end{gathered}$ |  |  |  |  | $\begin{gathered} 50 \mathrm{~K} \\ (5000 \mathrm{~K}, 70 \mathrm{CRI}) \\ \hline \end{gathered}$ |  |  |  |  |
|  |  |  |  |  | Lumens | B | U | G | LPW | Lumens | B | U | G | LPW | Lumens | B | U | G | LPW |
| 60 | 530 | P10 | 106W | T15 | 13,042 | 3 | 0 | 3 | 123 | 14,050 | 3 | 0 | 3 | 133 | 14,228 | 3 | 0 | 3 | 134 |
|  |  |  |  | T2S | 12,967 | 4 | 0 | 4 | 122 | 13,969 | 4 | 0 | 4 | 132 | 14,146 | 4 | 0 | 4 | 133 |
|  |  |  |  | T2M | 13,201 | 3 | 0 | 3 | 125 | 14,221 | 3 | 0 | 3 | 134 | 14,401 | 3 | 0 | 3 | 136 |
|  |  |  |  | T3S | 12,766 | 4 | 0 | 4 | 120 | 13,752 | 4 | 0 | 4 | 130 | 13,926 | 4 | 0 | 4 | 131 |
|  |  |  |  | T3M | 13,193 | 4 | 0 | 4 | 124 | 14,213 | 4 | 0 | 4 | 134 | 14,393 | 4 | 0 | 4 | 136 |
|  |  |  |  | T4M | 12,944 | 4 | 0 | 4 | 122 | 13,945 | 4 | 0 | 4 | 132 | 14,121 | 4 | 0 | 4 | 133 |
|  |  |  |  | TFTM | 13,279 | 4 | 0 | 4 | 125 | 14,305 | 4 | 0 | 4 | 135 | 14,486 | 4 | 0 | 4 | 137 |
|  |  |  |  | T5VS | 13,372 | 3 | 0 | 1 | 126 | 14,405 | 4 | 0 | 1 | 136 | 14,588 | 4 | 0 | 1 | 138 |
|  |  |  |  | T5S | 13,260 | 3 | 0 | 1 | 125 | 14,284 | 3 | 0 | 1 | 135 | 14,465 | 3 | 0 | 1 | 136 |
|  |  |  |  | T5M | 13,256 | 4 | 0 | 2 | 125 | 14,281 | 4 | 0 | 2 | 135 | 14,462 | 4 | 0 | 2 | 136 |
|  |  |  |  | T5W | 13,137 | 4 | 0 | 3 | 124 | 14,153 | 4 | 0 | 3 | 134 | 14,332 | 4 | 0 | 3 | 135 |
|  |  |  |  | BLC | 10,906 | 3 | 0 | 3 | 103 | 11,749 | 3 | 0 | 3 | 111 | 11,898 | 3 | 0 | 3 | 112 |
|  |  |  |  | LCCO | 7,789 | 1 | 0 | 3 | 73 | 8,391 | 1 | 0 | 3 | 79 | 8,497 | 1 | 0 | 3 | 80 |
|  |  |  |  | RCCO | 7,779 | 4 | 0 | 4 | 73 | 8,380 | 4 | 0 | 4 | 79 | 8,486 | 4 | 0 | 4 | 80 |
| 60 | 700 | P11 | 137W | T1S | 16,556 | 3 | 0 | 3 | 121 | 17,835 | 3 | 0 | 3 | 130 | 18,061 | 4 | 0 | 4 | 132 |
|  |  |  |  | T2S | 16,461 | 4 | 0 | 4 | 120 | 17,733 | 4 | 0 | 4 | 129 | 17,957 | 4 | 0 | 4 | 131 |
|  |  |  |  | T2M | 16,758 | 4 | 0 | 4 | 122 | 18,053 | 4 | 0 | 4 | 132 | 18,281 | 4 | 0 | 4 | 133 |
|  |  |  |  | T3S | 16,205 | 4 | 0 | 4 | 118 | 17,457 | 4 | 0 | 4 | 127 | 17,678 | 4 | 0 | 4 | 129 |
|  |  |  |  | T3M | 16,748 | 4 | 0 | 4 | 122 | 18,042 | 4 | 0 | 4 | 132 | 18,271 | 4 | 0 | 4 | 133 |
|  |  |  |  | T4M | 16,432 | 4 | 0 | 4 | 120 | 17,702 | 4 | 0 | 4 | 129 | 17,926 | 4 | 0 | 4 | 131 |
|  |  |  |  | TFTM | 16,857 | 4 | 0 | 4 | 123 | 18,159 | 4 | 0 | 4 | 133 | 18,389 | 4 | 0 | 4 | 134 |
|  |  |  |  | T5VS | 16,975 | 4 | 0 | 1 | 124 | 18,287 | 4 | 0 | 1 | 133 | 18,518 | 4 | 0 | 1 | 135 |
|  |  |  |  | T5S | 16,832 | 4 | 0 | 1 | 123 | 18,133 | 4 | 0 | 2 | 132 | 18,362 | 4 | 0 | 2 | 134 |
|  |  |  |  | T5M | 16,828 | 4 | 0 | 2 | 123 | 18,128 | 4 | 0 | 2 | 132 | 18,358 | 4 | 0 | 2 | 134 |
|  |  |  |  | T5W | 16,677 | 4 | 0 | 3 | 122 | 17,966 | 5 | 0 | 3 | 131 | 18,193 | 5 | 0 | 3 | 133 |
|  |  |  |  | BLC | 13,845 | 3 | 0 | 3 | 101 | 14,915 | 3 | 0 | 3 | 109 | 15,103 | 3 | 0 | 3 | 110 |
|  |  |  |  | LCCO | 9,888 | 1 | 0 | 3 | 72 | 10,652 | 2 | 0 | 3 | 78 | 10,787 | 2 | 0 | 3 | 79 |
|  |  |  |  | RCCO | 9,875 | 4 | 0 | 4 | 72 | 10,638 | 4 | 0 | 4 | 78 | 10,773 | 4 | 0 | 4 | 79 |
| 60 | 1050 | P12 | 207W | T15 | 22,996 | 4 | 0 | 4 | 111 | 24,773 | 4 | 0 | 4 | 120 | 25,087 | 4 | 0 | 4 | 121 |
|  |  |  |  | T2S | 22,864 | 4 | 0 | 4 | 110 | 24,631 | 5 | 0 | 5 | 119 | 24,943 | 5 | 0 | 5 | 120 |
|  |  |  |  | T2M | 23,277 | 4 | 0 | 4 | 112 | 25,075 | 4 | 0 | 4 | 121 | 25,393 | 4 | 0 | 4 | 123 |
|  |  |  |  | T3S | 22,509 | 4 | 0 | 4 | 109 | 24,248 | 5 | 0 | 5 | 117 | 24,555 | 5 | 0 | 5 | 119 |
|  |  |  |  | T3M | 23,263 | 4 | 0 | 4 | 112 | 25,061 | 4 | 0 | 4 | 121 | 25,378 | 4 | 0 | 4 | 123 |
|  |  |  |  | T4M | 22,824 | 5 | 0 | 5 | 110 | 24,588 | 5 | 0 | 5 | 119 | 24,899 | 5 | 0 | 5 | 120 |
|  |  |  |  | TFTM | 23,414 | 5 | 0 | 5 | 113 | 25,223 | 5 | 0 | 5 | 122 | 25,543 | 5 | 0 | 5 | 123 |
|  |  |  |  | T5VS | 23,579 | 5 | 0 | 1 | 114 | 25,401 | 5 | 0 | 1 | 123 | 25,722 | 5 | 0 | 1 | 124 |
|  |  |  |  | T5S | 23,380 | 4 | 0 | 2 | 113 | 25,187 | 4 | 0 | 2 | 122 | 25,506 | 4 | 0 | 2 | 123 |
|  |  |  |  | T5M | 23,374 | 5 | 0 | 3 | 113 | 25,181 | 5 | 0 | 3 | 122 | 25,499 | 5 | 0 | 3 | 123 |
|  |  |  |  | T5W | 23,165 | 5 | 0 | 4 | 112 | 24,955 | 5 | 0 | 4 | 121 | 25,271 | 5 | 0 | 4 | 122 |
|  |  |  |  | BLC | 19,231 | 4 | 0 | 4 | 93 | 20,717 | 4 | 0 | 4 | 100 | 20,979 | 4 | 0 | 4 | 101 |
|  |  |  |  | LCCO | 13,734 | 2 | 0 | 3 | 66 | 14,796 | 2 | 0 | 4 | 71 | 14,983 | 2 | 0 | 4 | 72 |
|  |  |  |  | RCCO | 13,716 | 4 | 0 | 4 | 66 | 14,776 | 4 | 0 | 4 | 71 | 14,963 | 4 | 0 | 4 | 72 |
| 60 | 1250 | P13 | 231W | T15 | 25,400 | 4 | 0 | 4 | 110 | 27,363 | 4 | 0 | 4 | 118 | 27,709 | 4 | 0 | 4 | 120 |
|  |  |  |  | T2S | 25,254 | 5 | 0 | 5 | 109 | 27,205 | 5 | 0 | 5 | 118 | 27,550 | 5 | 0 | 5 | 119 |
|  |  |  |  | T2M | 25,710 | 4 | 0 | 4 | 111 | 27,696 | 4 | 0 | 4 | 120 | 28,047 | 4 | 0 | 4 | 121 |
|  |  |  |  | T3S | 24,862 | 5 | 0 | 5 | 108 | 26,783 | 5 | 0 | 5 | 116 | 27,122 | 5 | 0 | 5 | 117 |
|  |  |  |  | T3M | 25,695 | 5 | 0 | 5 | 111 | 27,680 | 5 | 0 | 5 | 120 | 28,031 | 5 | 0 | 5 | 121 |
|  |  |  |  | T4M | 25,210 | 5 | 0 | 5 | 109 | 27,158 | 5 | 0 | 5 | 118 | 27,502 | 5 | 0 | 5 | 119 |
|  |  |  |  | TFTM | 25,861 | 5 | 0 | 5 | 112 | 27,860 | 5 | 0 | 5 | 121 | 28,212 | 5 | 0 | 5 | 122 |
|  |  |  |  | T5VS | 26,043 | 5 | 0 | 1 | 113 | 28,056 | 5 | 0 | 1 | 121 | 28,411 | 5 | 0 | 1 | 123 |
|  |  |  |  | T5S | 25,824 | 4 | 0 | 2 | 112 | 27,819 | 5 | 0 | 2 | 120 | 28,172 | 5 | 0 | 2 | 122 |
|  |  |  |  | T5M | 25,818 | 5 | 0 | 3 | 112 | 27,813 | 5 | 0 | 3 | 120 | 28,165 | 5 | 0 | 3 | 122 |
|  |  |  |  | T5W | 25,586 | 5 | 0 | 4 | 111 | 27,563 | 5 | 0 | 4 | 119 | 27,912 | 5 | 0 | 4 | 121 |
|  |  |  |  | BLC | 21,241 | 4 | 0 | 4 | 92 | 22,882 | 4 | 0 | 4 | 99 | 23,172 | 4 | 0 | 4 | 100 |
|  |  |  |  | LCCO | 15,170 | 2 | 0 | 4 | 66 | 16,342 | 2 | 0 | 4 | 71 | 16,549 | 2 | 0 | 4 | 72 |
|  |  |  |  | RCCO | 15,150 | 5 | 0 | 5 | 66 | 16,321 | 5 | 0 | 5 | 71 | 16,527 | 5 | 0 | 5 | 72 |

## SA+Capable Luminaire

This item is an A+ capable luminaire, which has been designed and tested to provide consistent color appearance and system-level interoperability.

- All configurations of this luminaire meet the Acuity Brands' specification for chromatic consistency
- This luminaire is A+Certified when ordered with DTL ${ }^{\circledR}$ controls marked by a shaded background DTL DLL equipped luminaires meet the A+ specification for luminaire to photocontrol interoperability1
- This luminaire is part of an A+ Certified solution for ROAM ${ }^{\circledR}$ or XPoint ${ }^{\text {TM }}$ Wireless control networks, providing out-of-the-box control compatibility with simple commissioning, when ordered with drivers and control options marked by a shaded background ${ }^{1}$
To learn more about A+, visit www.acuitybrands.com/aplus.

1. See ordering tree for details.
2. A+ Certified Solutions for ROAM require the order of one ROAM node per luminaire. Sold Separately: Link to Roam; Link to DTL DLL

## FEATURES \& SPECIFICATIONS

## INTENDED USE

The sleek design of the D-Series Size 1 reflects the embedded high performance LED technology. It is ideal for many commercial and municipal applications, such as parking lots, plazas, campuses, and streetscapes.

## CONSTRUCTION

Single-piece die-cast aluminum housing has integral heat sink fins to optimize thermal management through conductive and convective cooling. Modular design allows for ease of maintenance and future light engine upgrades. The LED drivers are mounted in direct contact with the casting to promote low operating temperature and long life. Housing is completely sealed against moisture and environmental contaminants (IP65). Low EPA ( $1.01 \mathrm{ft}^{2}$ ) for optimized pole wind loading.

## FINISH

Exterior parts are protected by a zinc-infused Super Durable TGIC thermoset powder coat finish that provides superior resistance to corrosion and weathering A tightly controlled multi-stage process ensures a minimum 3 mils thickness for a finish that can withstand extreme climate changes without cracking or peeling. Available in both textured and non-textured finishes.

## OPTICS

Precision-molded proprietary acrylic lenses are engineered for superior area lighting distribution, uniformity, and pole spacing. Light engines are available in standard 3000 K, 4000 K and $5000 \mathrm{~K}(70 \mathrm{CRI})$ configurations. The D-Series Size 1 has zero uplight and qualifies as a Nighttime Friendly ${ }^{\text {TM }}$ product, meaning it is consistent with the LEED ${ }^{\oplus}$ and Green Globes $^{\top \mathbb{M}}$ criteria for eliminating wasteful uplight.

## ELECTRICAL

Light engine configurations consist of high-efficacy LEDs mounted to metalcore circuit boards to maximize heat dissipation and promote long life (up to L85/100,000 hours at $25^{\circ} \mathrm{C}$ ). Class 1 electronic drivers are designed to have a power factor $>90 \%$, THD $<20 \%$, and an expected life of 100,000 hours with $<1 \%$ failure rate. Easily serviceable 10 kV surge protection device meets a minimum Category C Low operation (per ANSI/IEEE C62.41.2).

## STANDARD CONTROLS

The DSX1 LED area luminaire has a number of control options. Dusk to dawn controls can be utilized via optional NEMA twist-lock photocell receptacles. Integrated motion sensors with on-board photocells feature field-adjustable programing and are suitable for mounting heights up to 30 feet.

## nLIGHT AIR CONTROLS

The DSX1 LED area luminaire is also available with nLight® AIR for the ultimate in wireless control. This powerful controls platform provides out-of-the-box basic motion sensing and photocontrol functionality and is suitable for mounting heights up to 40 feet. Once commissioned using a smartphone and the easy-to-use CLAIRITY app, nLight AIR equipped luminaries can be grouped, resulting in motion sensor and photocell group response without the need for additional equipment. Scheduled dimming with motion sensor over-ride can be achieved when used with the nLight Eclypse. Additional information about nLight Air can be found here.

## INSTALLATION

Included mounting block and integral arm facilitate quick and easy installation. Stainless steel bolts fasten the mounting block securely to poles and walls, enabling the D-Series Size 1 to withstand up to a 3.0 G vibration load rating per ANSI C136.31. The D-Series Size 1 utilizes the AERIS ${ }^{\text {TM }}$ series pole drilling pattern (template \#8). NEMA photocontrol receptacle are also available.

## LISTINGS

UL Listed for wet locations. Light engines are IP66 rated; luminaire is IP65 rated. Rated for $-40^{\circ} \mathrm{C}$ minimum ambient. U.S. Patent No. D672,492 S. International patent pending.
DesignLights Consortium® (DLC) Premium qualified product and DLC qualified product.
Not all versions of this product may be DLC Premium qualified or DLC qualified. Please check the DLC Qualified Products List at www. designlights. org/QPL to confirm which versions are qualified.
International Dark-Sky Association (IDA) Fixture Seal of Approval (FSA) is available for all products on this page utilizing 3000K color temperature only.

## WARRANTY

5 -year limited warranty. Complete warranty terms located at:
www.acuitybrands.com/CustomerResources/Terms_and_conditions.aspx
Note: Actual performance may differ as a result of end-user environment and application.
All values are design or typical values, measured under laboratory conditions at $25^{\circ} \mathrm{C}$.
Specifications subject to change without notice.

LED bollard $-180^{\circ}$ distribution

## Application

LED bollard with shielded $180^{\circ}$ light distribution. This luminaire is designed to provide one sided illumination of ground surfaces. Provided with
mounting system that allows the luminaire to be adjusted independent of anchor bolt orientation.

## Materials

Luminaire housing and base constructed of die-cast and extruded marine
grade, copper free ( $\leq 0.3 \%$ copper content) A360.0 aluminum alloy
Borosilicate glass lens
Reflector made of pure anodized aluminum
High temperature silicone gasket
Mechanically captive stainless steel fastener
NRTL listed to North American Standards, suitable for wet locations
Protection class IP 65
Weight: 9.7 lbs
Electrical
Operating voltage 120-277VAC
Minimum start temperature
LED module wattage
System wattage
Controllability
Color rendering index
Luminaire lumens
Lifetime at $\mathrm{Ta}=15^{\circ} \mathrm{C}$
Lifetime at $\mathrm{Ta}=55^{\circ} \mathrm{C}$
$-20^{\circ} \mathrm{C}$
7.2 W
10.0W

0-10V dimmable
Ra> 80
411 lumens (3000K)
77,000 h (L70)
53,000 h (L70)

LED color temperature
$\square$ 4000K - Product number + K4
$\square$ 3500K - Product number + K35
$\square 3000 \mathrm{~K}$ - Product number + K3
$\square 2700 \mathrm{~K}$ - Product number + K27
BEGA can supply you with suitable LED replacement modules for up to 20 years after the purchase of LED luminaires - see website for details

## Finish

All BEGA standard finishes are matte, textured polyester powder coat with minimum 3 mil thickness.

| Available colors | $\square$ Black (BLK) | $\square$ White (WHT) | $\square$ RAL: |
| :--- | :--- | :--- | :--- |
|  | $\square$ Bronze (BRZ) | $\square$ Silver (SLV) | $\square$ CUS: |



LED bollard $\cdot 180^{\circ}$ distribution

|  | LED | A | B | Anchorage |
| :--- | :--- | :--- | :--- | ---: |
| $\mathbf{7 7 7 5 2}$ | 7.2 W | $61 / 2$ | $85 / 8$ | $\mathbf{7 9 8 1 7}$ |



BEGA 1000 BEGA Way, Carpinteria, CA 93013 (805) 684-0533 info@bega-us.com
Due to the dynamic nature of lighting products and the associated technologies, luminaire data on this sheet is subject to change at the discretion of BEGA North America. For the most current technical data, please refer to bega-us. com © copyright BEGA 2018

| CATALOG |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | FIXTURE | SUFFIX | REFLECTOR | voltage | FINISH | OPTION | OPTION | OPTION |



ADA Compliant



## MATERIALS

Syrios LED is made of corrosion resistant 356 aluminum alloy with a copper (CU) content of less than 0.1\%.
The main housing is made of seamless extruded aluminum, with an integrally sealed LED light module designed for optimal heat dissipation, and lighting performance.
Syrios LED SY300 series is standard with 29o optic. See options section for alternate selection.

## ELECTRICAL

DRIVER Standard driver is 0-10V dimming-ready (dims to 10\%) with: 120-277 multi-volt compatibility $(50-60 \mathrm{~Hz})$, operating temperature range of $-30^{\circ} \mathrm{C} /-22^{\circ} \mathrm{F}$ to $55^{\circ} \mathrm{C} / 131^{\circ} \mathrm{F}$, output over voltage protection, output over current protection and output short circuit protection with auto-recovery

LED Standard 4000K /80CRI. Optional 2700K, 3000K \& 3500K. Optional Amber LED for turtle sensitive areas.
Wavelengths: 584.5 nm to 597 nm .

## LIFE

60,000hrs $L_{70} B_{50}$ (based on IESNA TM-21 Test Method and LM-80 data). Up to 95,000hrs $L_{70} B_{50}$ (calculated projection from LM-80 data).

## FINISH

Five-stage preparation process including preheating of cast aluminum parts for air extraction, and an environmentally friendly alloy sealant. Polyester powder coating is applied through an electrostatic process and oven cured for long term finish.

## MOUNTING

Maximum weight: $2.5 \mathrm{lbs}(1.1 \mathrm{~kg})$
The mounting plate is designed to fit on a $2 \times 4$ " (51×102) rectangular electrical box using $3.156^{\prime \prime}$ (80) C/C mounting holes.
Optional trimming plate for octagonal jbox (option MT4).

## CERTIFICATION

Tested to UL1598 and CSA 22.2 \#250. ETL listed wet location. Rated IP66. CE Certification on request.



## OPTIONS

| electrical |  | LIGHT \& OPTICS |
| :---: | :---: | :---: |
| $\square \mathrm{FS}$ | Fuse | Alternate CCT ${ }^{\text {ok LED ( }}$ (LCF: Lumen conversion factor) |
| $\square$ REML2-50 | 7W remote emergency battery backup for LED, 90 min . Remote mount 50ft - 12" (305) square enclosure with access cover | $\square$ K27 2700 K CCT 80 CRI (LCF: 0.91) ${ }^{3}$ IDA - Dark Sky Approved <br> $\square$ K3 3000K CCT 80 CRI (LCF: 0.94) IDA - Dark Sky Approved <br> $\square$ K35 3500 K CCT 80 CRI (LCF: 0.983$)^{3}$ |
| MOUNTING |  | NOTE: Other CCT \& higher CRI available, please consult factory. |
| $\begin{aligned} & \square \text { SWK } \\ & \square \text { MT4 } \end{aligned}$ | Adaptor box for surface 3/4" conduit feed Trimming plate for octagonal box |  |
| ACCESSORIES |  |  |
| SL LSL SNT HL | Solite lens Linear spread lens $1.5^{\prime \prime}$ (38) snoot ${ }^{2}$ Hexcell louver |  |

## Notes

1- The remote enclosure must be interior.
2- To prevent reflections, interior painted black when a light color finish is selected (ex. WHT, MST, GRT and CHT).
3- Please consult factory when selecting K27 or K35 in conjuction with R9 (very narrow optics).

## LUMINIS.

SY 300 SERIES
SYRIOS -LED
TYPICAL PHOTOMETRY SUMMARY
ADA Compliant


Descriptive Information
SY300-L1L10-R30
Total Lms: 924 Lumens
Total Input Watts: 13 W
Efficacy: 74 Lumens/Watt
BUG: B1-UO-GO
CCT/CRI: $4000 \mathrm{~K} / 80$
Maximum Candela: 2098@ O deg


## Descriptive Information

SY300-L1L10-R40
Total Lms: 846 Lumens
Total Input Watts: 13 W
Efficacy: 68 Lumens/Watt
BUG: B1-UO-GO
CCT/CRI: $4000 \mathrm{~K} / 80$
Maximum Candela: 1317 @ 0 deg


Descriptive Information
SY300-L1L5NR-R9
Total Lms: 539 Lumens
Total Input Watts: 10 W
Efficacy: 53 Lumens/Watt
BUG: B1-UO-GO
CCT/CRI: $4000 \mathrm{~K} / 80$
Maximum Candela: 12018 @ 0 deg

Please visit our web site www.luminis.com for complete I.E.S. formatted download data.

## LUMINIS.




Reflector aperture: 6-1/4 (15.9)
Wall mount dimensions
Housing diameter: 8-1/8 (20.3)

$$
\begin{aligned}
& A=7-3 / 4(19.7) \\
& B=5-5 / 16(13.5) \\
& C=3-1 / 4(8.3) \\
& D=10-7 / 8(27.6)
\end{aligned}
$$

| WATTAGE CONSUMPTION MATRIX |  |  |  |
| :---: | :---: | :---: | :---: |
| LUMENS | LM ACTUAL | WATTAGE | LUMENS per WATT |
| $\mathbf{1 0 0 0}$ | 1,059 | 11.8 | 90.1 |
| $\mathbf{1 5 0 0}$ | 1,572 | 18.5 | 85.0 |
| $\mathbf{2 0 0 0}$ | 2,058 | 23.2 | 88.9 |
| $\mathbf{2 5 0 0}$ | 2,612 | 29.5 | 88.5 |
| $\mathbf{3 0 0 0}$ | 3,077 | 36.6 | 84.1 |
| $\mathbf{3 5 0 0}$ | 3,591 | 42.1 | 85.3 |
| $\mathbf{4 0 0 0}$ | 4,046 | 48.1 | 84.2 |
| $\mathbf{4 5 0 0}$ | 4,555 | 46.9 | 97.1 |


| ACCESSORIES order as separate catalog numbers (shipped separately) |  |  |
| :---: | :---: | :---: |
| CYS  <br> CRS $^{12}$ $3 / 8^{12}$ stem and canopy with $5^{\circ}$ "hang straight" swivel <br> $3 / 8^{\prime \prime}$ stem and canopy with $45^{\circ}$ swivel  <br> CYSX $^{12}$ $3 / 8^{\prime \prime}$ stem and canopy with $5^{\circ}$ "hang straight" swivel. Use this nomenclature <br>  when ordering EDXB driver | CRSX ${ }^{12}$ <br> SDT 347/120 75VA ${ }^{13}$ | $3 / 8$ " stem and canopy with $45^{\circ}$ swivel. Use this nomenclature when ordering EDXB driver <br> 347V Step-down transformer |


| ORDERING NOTES |  |  |  |
| :---: | :---: | :---: | :---: |
| 1. | Not available with finishes. | 9. | Specify voltage. For use with generator supply EM power. Will require an emer- |
| 2. | Not available with wallwash trim type. |  | gency hot feed and normal hot feed. |
| 3. | Refer to TECH-240 for compatible dimmers. | 10. | Interface remote mounted. |
| 4. | Not available with nLight ${ }^{\text {® }}$ and XPoint options. | 11. | Additional architectural colors available; see www.lithonia.com/archcolors. |
| 5. | XPoint ${ }^{\circledR}$ CMRB ships separately. | 12. | Color and length of stem must be specified (from 6" to 240" in even increments |
| 6. | Specify voltage 120V. |  | in maximum sections of 48"). Ex.: CYS06 DWHG. Ceiling attachment for interior |
| 7. | Access panel (supplied by others) recommended for use with nLight ${ }^{\text {® }}$ and |  | use. Consult factory for exterior use. Wire not included. |
|  | XPoint ${ }^{\text {® }}$. | 13. | Transformer must be field-installed to an accessible remote-mounted junction box. |
| 8. | White cord with white housings. All others black cord. |  |  |



EVO 35/35 6AR LS INPUT WATTS: 42.1, DELIVERED LUMENS: 3591, LM/W=85.3, 1.05 S/MH, TEST NO. LTL27767


|  | Ave Lumens |  |
| :--- | :--- | :--- |
| 0 | 3400 | Zone Lumens \% Lamp |
| $0^{\circ}-30^{\circ}$ | 2579 |  |
| $0^{\circ}-40^{\circ}$ | 3399 |  |

 $\qquad$

|  | Ave | Lumens |  | Zone | Lumens | $\%$ Lamp |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 3400 |  |  | $0^{\circ}-30^{\circ}$ | 2579.3 | 71.8 |  |  |
| 5 | 3390 | 324 |  | $0^{\circ}-40^{\circ}$ | 3399.8 | 94.7 |  |  |
| 15 | 3497 | 981 |  | $0^{\circ}-60^{\circ}$ | 3586.3 | 99.9 |  |  |
| 25 | 2830 | 1274 |  | $0^{\circ}-90^{\circ}$ | 3590.5 | 100.0 |  |  |
| 35 | 1335 | 820 |  | $90^{\circ}-180^{\circ}$ | 0.0 | 0.0 |  |  |
| 45 | 193 | 177 |  | $0^{\circ}-180^{\circ}$ | 3590.5 | $* 100.0$ |  |  |
| 55 | 5 | 9 |  | *Efficiency |  |  |  |  |


| pf | 20\% |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| pc | 80\% |  |  | 70\% |  |  | 50\% |  |  |
| pw | 50\% 30\% 10\% |  |  | 50\% 30\% 10\% |  |  | 50\% 30\% 10\% |  |  |
| 0 | 119 | 119 | 119 | 116 | 116 | 116 | 111 | 111 | 111 |
| 1 | 111 | 108 | 106 | 109 | 106 | 104 | 105 | 103 | 101 |
| 2 | 103 | 99 | 96 | 101 | 98 | 95 | 98 | 95 | 93 |
| 3 | 96 | 91 | 87 | 95 | 90 | 87 | 92 | 88 | 85 |
| 4 | 90 | 84 | 80 | 89 | 84 | 80 | 87 | 82 | 79 |
| 5 | 84 | 78 | 74 | 83 | 78 | 74 | 81 | 77 | 73 |
| 6 | 79 | 73 | 69 | 78 | 72 | 68 | 77 | 72 | 68 |
| 7 | 74 | 68 | 64 | 73 | 68 | 64 | 72 | 67 | 63 |
| 8 | 70 | 64 | 60 | 69 | 63 | 59 | 68 | 63 | 59 |
| 9 | 66 | 60 | 56 | 65 | 60 | 56 | 64 | 59 | 55 |
| 10 | 62 | 56 | 52 | 62 | 56 | 52 | 61 | 56 | 52 |



EVO 35/20 6AR LS
INPUT WATTS: 23.2, DELIVERED LUMENS: 2058, LM/W=88.7, 1.02 S/MH, TEST NO. LTL27777


| LUMEN OUTPUT MULTIPLIER - CRI |  |
| :---: | :---: |
| $\mathbf{C R I}$ | FACTOR |
| $\mathbf{8 0} \mathbf{C R I}$ | 1 |
| $\mathbf{9 0} \mathbf{C R I}$ | 0.79 |


| LUMEN OUTPUT MULTIPLIER - CCT |  |
| :---: | :---: |
| CRI | FACTOR |
| $\mathbf{5 0 0 0} \mathrm{K}$ | 1.101 |
| 4000 K | 1.035 |
| $\mathbf{3 5 0 0} \mathrm{~K}$ | 1 |
| $\mathbf{3 0 0 0} \mathrm{~K}$ | 0.973 |
| 2700 K | 0.938 |


| LUMEN OUTPUT MULTIPLIER - TRIM FINISH |  |  |  |  |  |  |  |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| FINISH | CLEAR <br> (AR) | PEWTER <br> (PR) | WHEAT <br> (WTR) | GOLD <br> (GR) | WHITE <br> (WR/WRAMF) | BLACK <br> (BR) |  |
| Specular (LS) | 1.00 | 0.88 | 0.83 | 0.95 | N/A | N/A |  |
| Semi-specular (LSS) | 0.95 | 0.84 | 0.79 | 0.90 | N/A | N/A |  |
| Matte-diffuse (LD) | 0.85 | 0.73 | 0.69 | 0.80 | N/A | N/A |  |
| Paint | N/A | N/A | N/A | N/A | 0.87 | 0.73 |  |

## PHOTOMETRY NOTES

- Tested in accordance with IESNA LM-79-08.
- Tested to current IES and NEMA standards under stabilized laboratory conditions.
- CRI: 85 typical.

Choose Wall Controls.
nLIGHT o邓ers multiple styles of wall controls - each with varying features and user experience.


Push-Button WallPod Traditional tactile buttons and LED user feedback


Graphic WallPod
full color touch screen provides a sophisticated look and feel


## EXAMPLE

Group Fixture Control*
*Application diagram applies for fixtures with eldoLED drivers only.
nPS 80 EZ Dimming/Control Pack (qty 2 required)
nPODM 2P DX Dual On/Off/Dim Push-Button WallPod nCM ADCX Daylight Sensor with Automatic Dimming Control nCM PDT 9 Dual Technology Occupancy Sensor

Description: This design provides a dual on/off/dim wall station that enables manual control of the fixtures in Row A and Row B separately. Additionally, a daylight harvesting sensor is provided so the lights in row $B$ can be configured to dim automatically when daylight is available. An occupancy sensor turns off all lights when the space is vacant.

| nLight ${ }^{\text {® }}$ Control Accessories: <br> Order as separate catalog number. Visit www.sensorswitch.com/nLight for complete listing of nLight controls. |  |  |  |
| :---: | :---: | :---: | :---: |
| WallPod stations | Model number | Occupancy sensors | Model number |
| On/Off | nPODM [color] | Small motion $360^{\circ}$, ceiling (PIR / dual tech) | nCM 9 / nCM PDT 9 |
| On/Off \& Raise/Lower | $n P O D M$ DX [color] | Large motion $360^{\circ}$, ceiling (PIR / dual tech) | nCM 10 / nCM PDT 10 |
| Graphic Touchscreen | nPOD GFX [color] | Wide view (PIR / dual tech) | nWV 16 / nWV PDT 16 |
| Photocell controls | Model number | Wall Switch w/ Raise/Lower (PIR / dual tech) | nWSX LV DX / nWSX PDT LV DX |
| Dimming | nCM ADCX | Cat-5 cables (plenum rated) | Model number |
|  |  | 10', CAT5 10FT | CAT5 10FT J1 |
|  |  | 15', CAT5 15FT | CAT5 15FT J1 |

## \$4+ Capable Luminaire

This item is an A+ capable luminaire, which has been designed and tested to provide consistent color appearance and out-of-the-box control compatibility with simple commissioning.

- All configurations of this luminaire meet the Acuity Brands' specification for chromatic consistency
- This luminaire is part of an A+Certified solution for nLight control networks when ordered with drivers marked by a shaded background*
- This luminaire is part of an A+Certified solution for nLight control networks, providing advanced control functionality at the luminaire level, when selection includes driver and control options marked by a shaded background*

To learn more about $\mathrm{A}+$, visit www.acuitybrands.com/aplus.
*See ordering tree for details

## TヘスGニ̄ா

## BULLETTO

## Compact Adjustable Flood Light Projector

Concept：Compact projector flood light for high intensity applications．
Materials：Aluminum body and joints for maximum heat dissapation powder coated in Ferrite Grey or Bronze finish．Modular body for toolless maintenance．Lens cover assymbly for simple toolless field interchangability of accessories．
Optics：NSP，SP，FL，MW，and WF use high efficiency LED Chip on Board．Equipped with collimating optic with angle specific holigraphic spread lens filters．
Mounting：Adjustable up to $180^{\circ}$ on the vertical plane with aim locking set screw
Brass strain release gland with $1 / 2^{\prime \prime}$ NPT thread nipple that can be screwed directly to recieving mounting
Installation：Pre－cabled with 10’ Belden direct burial 18ga 3 Conductor Cable for
Connection to remote power supply
Finish：Ferrite Grey／Bronze
Power Supply：Remote Class 2，120V－277VAC power supply required，see page 2
for options
Wattage：10W（NSP）／12W（SP／FL／MF／WF）
Color Temperature： $2700^{\circ} \mathrm{K} / 3000^{\circ} \mathrm{K} / 3500^{\circ} \mathrm{K} / 4000^{\circ} \mathrm{K}$
CRI：Ra84
Delivered Lumens： $3000^{\circ} \mathrm{K}$
Narrow Spot $10^{\circ}=473 \mathrm{Lm}$ IMax： $24,196 \mathrm{~cd} / \mathrm{klm}$
Spot $15^{\circ}=858 \mathrm{Lm}$ IMax：7，292cd／kIm
Flood $25^{\circ}=812 \mathrm{Lm}$ IMax： $2,883 \mathrm{~cd} / \mathrm{klm}$
Medium Wide Flood $30^{\circ}=819 \mathrm{Lm}$ IMax：1，916cd／klm
Wide Flood $43^{\circ}=801 \mathrm{Lm}$ IMax：1，009cd／klm
Lumen Maintenance（L70）：50，000hrs
Calculation for LED fixtures are based on measurements that comply with IES LM－80． CRI：Ra84
Voltage：24V DC
IK Rating：IK10
IP Rating：IP66
Certifications：cULus Class 2 Wet Location Listed


Tested in accordance with LM－79－08
A Title 24 commercial installation compliant
Warranty： 5 year limited warranty

| PRODUCT CODE | DRIVER | FINISH | OUTPUT | OPTICS | COLOR TEMP | ＋ | POWER SUPPLY |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| BLT－BULLETTO | RP－Remote Power | FE－Ferrite Grey | L1－10w | NS－Narrow Spot $10^{\circ}$ | 27－2700K |  | See page 4 |
|  |  | BZ－Bronze | L2－12W | SP－Spot $15^{\circ}$ | $30-3000 \mathrm{~K}$ |  |  |
|  |  |  |  | FL－Flood $25^{\circ}$ | 35－3500K |  |  |
|  |  |  |  | MF－Medium Wide Flood $30^{\circ}$ | 40－4000K |  |  |
|  |  |  |  | WF－Wide Flood $43^{\circ}$ |  |  |  |

Views


## IへRGEォI

## BULLETTO

| INTERNAL OPTICAL ACCESSORIES: |  |
| :--- | :--- |
| Maximum of two optical accessories per fixture. |  |
| $\mathbf{1 E 3 7 9 8}$ | Chromatic filter Red. Dimensions $\varnothing 50 \mathrm{~mm}$ |
| $\mathbf{1 E 3 7 9 9}$ | Chromatic filter Green. Dimensions $\varnothing 50 \mathrm{~mm}$ |
| $\mathbf{1 E 3 8 0 0}$ | Chromatic filter Blue. Dimensions $\varnothing 50 \mathrm{~mm}$ |
| $\mathbf{1 E 3 8 0 1}$ | Chromatic filter Yellow. Dimensions $\varnothing 50 \mathrm{~mm}$ |
| $\mathbf{1 E 3 8 0 2}$ | Chromatic filter Magenta. Dimensions $\varnothing 50 \mathrm{~mm}$ |
| $\mathbf{1 E 3 7 9 0}$ | 'Blade of Light' linear spread lens filter. <br> Dimensions $\varnothing 50 \mathrm{~mm}$ |
| $\mathbf{1 E 3 7 9 2}$ | Honeycomb filter. Dimensions $\varnothing 50 \mathrm{~mm}$ |



Asymmetric Screen

| INSTALLATION ACCESSORIES: |  |  |
| :---: | :---: | :---: |
| Maximum of one installation accessory per fixture. |  |  |
| - | - | Description |
| 1E3786 | 1 E 3804 | Plate for fitting rotation. Powder coated stainless steel. |
| 1E3785 | 1 E 3803 | Earthspike. Powder coated stainless steel. |
| 1BLTSMCVRFE | 1BLTSMCVRBZ | Low Profile surface canopy. Powder coated aluminum with $1 / 2^{\prime \prime}$ NPT to mount over 4" Dia. X .5" H pancake j-box. |



## IへRGEпI

## BULLETTO

## Tree Mounting Accessories

| TREE STRAP (REQUIRED) - CHOOSE $\mathbf{1}$ |  |
| :--- | :--- |
| Durable 1.5 inch wide Nylon webbing Strap is flexible with High Strength Aircraft <br> Aluminum V-ring Buckle in Gunmetal finish and lock in loop to prevent slipping. <br> The strap is made to coordinate with up to 6 fixtures and 1 wiring connection box. |  |
| TTS0101 | Tree strap for trees $39 "$ in circumference or smaller. <br> Olive Green nylon strap with Gunmetal cinch buckle. |
| TTS0102 | Tree strap for trees $39 "$ in circumference or smaller. <br> Coyote Brown nylon strap with Gunmetal cinch buckle. |
| TTS0103 | Tree strap for trees 39" in circumference or smaller. <br> Graphite Grey nylon strap with Gunmetal cinch buckle. |



## INSTALLATION STRAP (OPTIONAL)

Durable 1.5 inch wide Nylon webbing Strap is flexible with Metal Cinch Buckle and lock in loop to prevent slipping. The installation strap comes with 4 each 12 inch Rubber Loops used to hold up to 6 fixtures in place while positioning and tightening the Tree Strap in position during Installation or Maintenance and then removed for operation. For use with 2 or more fixtures.

TTISO101
Installation strap flexible nylon webbing strap with metal cinch buckle and lock in loop. Includes 4 each 150 mm rubber loops to hold up to 6 fixtures in place while positioning

## FIXTURE BRACKETS (REQUIRED) - CHOOSE 1

Buckle style Brackets for securing onto Tree Straps

|  |  | Description |
| :---: | :--- | :--- |
| TTBLT0101 | TTBLT0102 | Bracket with stainless steel mounting screws. |

TREE STRAP CONNECTION BOX (OPTIONAL)
Tree strap connection boxes are used to connect all multiple low voltage cables for one continuous cable connection down the tree, additional cable provided by others.

| TTCB0103 | Tree Strap Connection Box. Grey ABS plastic with internal <br> terminal block for wire connections and 3 each $1 / 2^{\prime \prime}$ knockouts <br> for connections to 1 or 4 fixtures. Dimensions: $31 / 4^{\prime \prime} \times 21 / 2^{\prime \prime} \times 13 / 4^{\prime \prime}$. |
| :--- | :--- |
| TTCB0104 | Tree Strap Connection Box. Grey ABS plastic with internal <br> terminal block for wire connections and 4 each $1 / 2^{\prime \prime}$ knockouts <br> for connections to 5 or 6 fixtures. Dimensions: $41 / 2^{\prime \prime} \times 21 / 2^{\prime \prime} \times 13 / 4^{\prime \prime}$. |

MESH WIRE PROTECTION COVER (OPTIONAL)
A braided polyethylene terepthalate (PET) monofilament yarn resistant to chemical degradation, UV radiation, and abrasion.
TTSJS01 $\quad$ Carbon Grey 3/8" SJO Cord Sleeve (Sold per foot)
TTSJSO2 Brown 3/8" SJO Cord Sleeve (Sold per foot)
TTSJS03 Forest Green 3/8" SJO Cord Sleeve (Sold per foot)

## Wiring Configuration



Adjust the strap every 6 months in order to prevent tree damage

## IへRGEпII

## BULLETTO

| Power Supply (REQUIRED) | Type | Wattage | Input/Output Voltage | Dimmable | IP Rating | Output | Dimensions |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| DEL60PWM | ELECTRONIC PWM DRIVER STANDALONE, UL LISTED ENCLOSURE PROVIDED BY OTHERS | 60W | $\begin{aligned} & 120-277 \mathrm{~V} / \\ & 24 \mathrm{~V} \end{aligned}$ | $\begin{array}{\|l} \hline \text { 0-10V/PWM } \\ 10 \% \end{array}$ | IP67 | UR CLASS 2 | $5.9^{\prime \prime} \times 2.09^{\prime \prime} \times$ |
| DEL90PWM | ELECTRONIC PWM DRIVER STANDALONE, UL LISTED ENCLOSURE PROVIDED BY OTHERS | 90W | $\begin{aligned} & 120-277 \mathrm{~V} / \\ & 24 \mathrm{~V} \end{aligned}$ | $\begin{array}{\|l} 0-10 \mathrm{~V} / \mathrm{PWM} \\ 10 \% \end{array}$ | IP67 | UR CLASS 2 | $\begin{aligned} & 6.73^{\prime \prime} \times 2.48^{\prime \prime} \times \\ & 1.48^{\prime \prime} \end{aligned}$ |
| DEDD10010 | ELECTRONIC 0-10V DRIVER STANDALONE, UL LISTED ENCLOSURE PROVIDED BY OTHERS | 100W | $\begin{aligned} & 120-277 \mathrm{~V} / \\ & 24 \mathrm{~V} \end{aligned}$ | 0-10V DIMMING 0.1\% | IP20 | UR CLASS 2 | $6.02^{\prime \prime} \times 1.97^{\prime \prime} \times 0.91^{\prime \prime}$ |
| DELV40124D | ELECTRONIC LOW VOLTAGE TRANSFORMER | 40W | $\begin{aligned} & 120-277 \mathrm{~V} / \\ & 24 \mathrm{~V} \end{aligned}$ | PHASE (120V ONLY / $0-10 \mathrm{~V}$ DIMMING $<1 \%$ | IP66 / NEMA4 ENCLOSURE | UL CLASS 2 | $11^{\prime \prime} \times 4^{\prime \prime} \times 2.21^{\prime \prime 1}$ |
| DELV60124D | ELECTRONIC LOW VOLTAGE TRANSFORMER | 60W | $\begin{aligned} & 120-277 \mathrm{~V} / \\ & 24 \mathrm{~V} \end{aligned}$ | PHASE (120V ONLY /O-10V DIMMING $<1 \%$ | IP66 / NEMA4 ENCLOSURE | UL CLASS 2 | $11^{\prime \prime} \times 4^{\prime \prime} \times 2.21^{\prime \prime 1}$ |
| DELV96124D | ELECTRONIC LOW VOLTAGE TRANSFORMER | 96W | $\begin{aligned} & 120-277 \mathrm{~V} / \\ & 24 \mathrm{~V} \end{aligned}$ | PHASE (120V ONLY) /0-10V DIMMING < $1 \%$ | IP67 / NEMA4 ENCLOSURE | UL CLASS 2 | $11^{\prime \prime} \times 4^{\prime \prime} \times 2.21^{\prime \prime \prime}$ |
| DELX601241CPWM | ELECTRONIC PWM DRIVER | 60W | $\begin{aligned} & \begin{array}{l} 120-277 \mathrm{~V} / \\ 24 \mathrm{~V} \end{array} \end{aligned}$ | $\begin{aligned} & \text { O-10V/PWM } \\ & 10 \% \end{aligned}$ | IP67 / NEMA3R ENCLOSURE | UL CLASS 2 | $10^{\prime \prime} \times 10^{\prime \prime} \times 4^{\prime \prime}$ |
| DELX901241CPWM | ELECTRONIC PWM DRIVER | 90W | $\begin{aligned} & 120-277 \mathrm{~V} / \\ & 24 \mathrm{~V} \end{aligned}$ | $\begin{aligned} & \text { 0-10V/PWM } \\ & 10 \% \end{aligned}$ | IP67 / NEMA3R ENCLOSURE | UL CLASS 2 | $10^{\prime \prime} \times 10^{\prime \prime} \times 4^{\prime \prime}$ |
| DELX1802242CPWM | ELECTRONIC PWM DRIVER | 2X90W | $\begin{aligned} & 120-277 \mathrm{~V} / \\ & 24 \mathrm{~V} \end{aligned}$ | $\begin{aligned} & \text { 0-10V/PWM } \\ & 10 \% \end{aligned}$ | IP67 / NEMA3R ENCLOSURE | UL CLASS 2 | $12^{\prime \prime} \times 12^{\prime \prime} \times 4^{\prime \prime 1}$ |
| DELX2703243CPWM | ELECTRONIC PWM DRIVER | 3X90w | $\begin{aligned} & 120-277 \mathrm{~V} / \\ & 24 \mathrm{~V} \end{aligned}$ | $\begin{aligned} & \text { O-10V/PWM } \\ & 10 \% \end{aligned}$ | IP67 / NEMA3R ENCLOSURE | UL CLASS 2 | $12^{\prime \prime} \times 12^{\prime \prime} \times 4^{\prime \prime \prime}$ |
| PS060 | LUTRON HI-LUME PREMIER 0.1\% CONSTANT VOLTAGE DRIVER WITH UL LISTED ENCLOSURE | 96W | UNIVERSAL $120-277$ VAC | HI-LUM DIMMABLE 0.1\% | IP20/NOM CERTIFIED | UL CLASS 2 | $10.5^{\prime \prime} \times 5.5^{\prime \prime} \times 2^{\prime \prime 1}$ |
| QOMELED1002410BK | QTRAN QOM-eLED CONSTANT VOLTAGE DRIVER WITH UL LISTED ENCLOSURE | 100W | $\begin{aligned} & 100-277 \mathrm{~V} / \\ & 24 \mathrm{~V} \end{aligned}$ | 0-10V | IP67 | UR CLASS 2 | $\begin{aligned} & 15.4^{\prime \prime \prime} \times 9.22^{\prime \prime} \times \\ & 4.90^{\prime \prime} \end{aligned}$ |
| QOMELED2002410BK | QTRAN QOM-eLED CONSTANT VOLTAGE DRIVER WITH UL LISTED ENCLOSURE | 2X100W | $\begin{aligned} & 100-200 \mathrm{~V} / \\ & 24 \mathrm{~V} \end{aligned}$ | 0-10V | IP67 | UR CLASS 2 | $\begin{aligned} & 15.4^{\prime \prime} \times 9.22^{\prime \prime} \times \\ & 4.90^{\prime \prime} \end{aligned}$ |
| QOMDRIVE10024VBKDMX | QTRAN QOM-DRIVE-PS CONSTANT VOLTAGE DRIVER WITH UL LISTED ENCLOSURE | 100W | $\begin{aligned} & 100-277 \mathrm{~V} / \\ & 24 \mathrm{~V} \end{aligned}$ | DMX CONTROL 4 CHANNEL | IP67 | UR CLASS 2 | $\begin{aligned} & 15.4^{\prime \prime} \times 9.22^{\prime \prime} \times \\ & 4.90^{\prime \prime} \end{aligned}$ |
| QOMDRIVE20024VBKDMX | QTRAN QOM-DRIVE-PS CONSTANT VOLTAGE DRIVER WITH UL LISTED ENCLOSURE | 2X100W | $\begin{aligned} & 100-200 \mathrm{~V} / \\ & 24 \mathrm{~V} \end{aligned}$ | DMX CONTROL 4 CHANNEL | IP67 | UR CLASS 2 | $\begin{aligned} & 15.4^{\prime \prime} \times 9.22^{\prime \prime} \times \\ & 4.90^{\prime \prime} \end{aligned}$ |

${ }^{1}$ Dimensions include enclosure with mounting bracket

* Constant voltage drivers $50 / 60 \mathrm{HZ}$, voltage regulated with short circuit protection. Operating temperature $-40 \mathrm{C}-80^{\circ} \mathrm{C}$
* Installation of power supply must be compliant to Class 2 installation standards. Refer to NEC and local building code requirements.
* Consult factory for additional driver options (ie: DMX, DALI, wattage, size, shape, Lutron, ELDO, or others).


## MAX FIXTURES PER DRIVER

|  | Driver Wattage |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | 40W | 60w | 96W | 90w | 2X90w | 3x90w | 100w | 2×100 |
|  | 10w | 3 | 4 | 7 | 7 | 14 | 21 | 8 | 16 |
|  | 12W | 2 | 4 | 6 | 6 | 12 | 18 | 6 | 13 |

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## IへRGEォI

## BULLETTO

## Photometry

| NARROW SPOT |  | POLAR GRAPH |  |  | CONE OF LIGHT |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Fixture Power | 10W |  |  |  | (Ft) | Atan-42:42: | 6000. |  | Max | Med |
| Fixture Output | 473Lm |  |  |  | ftcd |  |  |  | ftcd |
| Kelvin Temp | $3000^{\circ} \mathrm{K}$ |  |  |  | 4.00 |  |  |  | 945 | 612 |
| Beam Spread | NS 100 |  |  |  | 8.00 |  |  |  | 236 | 153 |
| 1 Max | 24,196cd/klm |  |  |  | 12.00 |  |  |  | 105 | 68 |
| Efficacy | $47 \mathrm{Lm} / \mathrm{W}$ |  |  |  | 16.00 |  |  |  | 59 | 38 |
|  |  |  |  |  | 20.00 |  |  |  | 38 | 24 |




| MEDIUM WIDE FLOOD |  | Polar Graph |  | CONE OF LIGHT |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Fixture Power | 12W |  |  | (Ft) |  | a=00. | 80x $515 \times-15$. | Max | Med |
| Fixture Output | 819Lm |  |  |  |  |  | ftcd | ftcd |
| Kelvin Temp | $3000^{\circ} \mathrm{K}$ |  |  | 4.00 |  |  |  | 138 | 84 |
| Beam Spread | MF $30^{\circ}$ |  |  | 8.00 |  |  |  | 34 | 21 |
| IMax | 1,916cd/klm |  |  | 12.00 |  |  |  | 15 | 9 |
| Efficacy | 68 Lm/W |  |  | 16.00 |  |  |  | 9 | 5 |
|  |  |  |  | 20.00 |  |  |  | 6 | 3 |

## IへRGEォI

## BULLETTO

## Photometry Cont.

| WIDE FLOOD |  | POLAR GRAPH | CONE OF LIGHT |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Fixture Power | 12W |  | (Ft) | Apeoraz 2 -22\% |  | samerezr-22x | Max | Med |
| Fixture Output | 801Lm |  |  |  |  |  | ftcd | ftcd |
| Kelvin Temp | $3000^{\circ} \mathrm{K}$ |  | 4.00 |  |  |  | 72 | 42 |
| Beam Spread | WF $43^{\circ}$ |  | 8.00 |  |  |  | 18 | 10 |
| IMax | 1009cd/klm |  | 12.00 |  |  |  | 8 | 5 |
| Efficacy | $67 \mathrm{Lm} / \mathrm{W}$ |  | 16.00 |  |  |  | 5 | 3 |
|  |  |  | 20.00 |  |  |  | 3 | 2 |

PERFORMANCE 500 (OUTDOOR)
PQ-SERIES | RUBBER COATED \| LINEAR LED LIGHTING
Fixture Type: $\qquad$
Project: $\qquad$
Location: $\qquad$



## PRODUCT FEATURES

- $90+$ CRI
- Dimmable
- 50,000 hour life
- 5-year warranty
- UL-listed for indoor and outdoor use
- $3 \mathrm{M}^{\mathrm{TM}}$ Industrial adhesive backing
- For use with 24 V power supplies

KELVIN COLOR TEMPERATURE SCALE

|  |  |  |  |  |  |  |  | CRI | 90+ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  | Diode | 2835 |
| 등 | 首 | 듬 | 응 | 항 | 흥 | 흥 | 등 | Dimming Options | PWM, Triac, 0-10V, DMX, Hi-lume |
|  |  |  |  |  |  |  |  | Temp Range | $-40^{\circ} \mathrm{F}\left(-40^{\circ} \mathrm{C}\right)$ to $149^{\circ} \mathrm{F}\left(65^{\circ} \mathrm{C}\right)$ |

TOTAL WATTAGE USED AT EACH LENGTH

| 1 ft | 2 ft | 3ft | 4ft | 5 ft | 6ft | 7ft | 8ft | 9ft | 10ft | 11ft | 12ft | 13 ft |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 4.1 | 7.4 | 11.3 | 15.3 | 19.0 | 22.5 | 25.8 | 28.6 | 32.3 | 35.5 | 37.8 | 41.0 | 42.7 |
| 14ft | 15ft | 16 ft | 17ft | 18ft | 19ft | 20 ft | 21 ft | 22 ft | 23ft | 24 ft | 25 ft | 26 ft |
| 45.0 | 48.2 | 50.3 | 52.4 | 54.8 | 56.5 | 58.8 | 60.7 | 61.7 | 62.5 | 63.1 | 63.8 | $n / a$ |


| Conforms to ANSI/UL Standard 2108 <br> Certified to CAN/CSA Standard C22.2 No. 250.0 |  | RoHS | $c \underbrace{\mathrm{U}_{\mathrm{L}}}_{\text {LSTLED }}$ | $\begin{aligned} & 24 \mathrm{~V} \\ & \mathbf{D C} \\ & \hline \end{aligned}$ | WET $\begin{aligned} & \text { WEET } \\ & \text { Weated } \\ & \text { RATED }\end{aligned}$ |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Certified to CAN/CSA Standard C22.2 No. 250.0 |  |  |  |  |  |
| Questions/Support \| 800-789-3810 |  |  |  |  | O42219DM |

## ラへスGĒ！

## S7 Fixture

## ZEDGE LINE

Professional Small Scale LED Steplight

Concept：Recessed wall mounted LED fixture．
Materials：Die－cast anodized aluminum body and external frame；powder coated frame．
Source：LED High Efficiency Board．
Optic：Polycarbonate opal screen．Floor Washer frame allows for uniform optical distribution on the floor and excellent visual comfort．
Mounting：To be completed with a special nylon outer casing fitted for parallel connection．
Driver：Integrated 4／1 driver（Non－dimmable／0－10V／Reverse Phase／Forward
Phase）．Dimmable to $1 \%$ ．
Finish：Ferrite Grey
Wattage：9W
Color Temperature： $2700^{\circ} \mathrm{K} / 3000^{\circ} \mathrm{K} / 3500^{\circ} \mathrm{K} / 4000^{\circ} \mathrm{K}$
CRI：Ra84
Delivered Lumens： $2700^{\circ} \mathrm{K} \quad 3000^{\circ} \mathrm{K} \quad 3500^{\circ} \mathrm{K} \quad 4000^{\circ} \mathrm{K}$
230 Lm 245 Lm 251 Lm 257 Lm

Lumen Maintenance（L70）：50，000hrs
Calculation for LED fixtures are based on measurements that comply with IES LM－80．
Voltage：Universal Voltage 120－277V AC 50／60Hz
IK Rating：IK10
IP Rating：IP66
Certifications：cULus Listed Wet Location
Tested in accordance with LM－79－08
A Title 24 commercial installation compliant．
${ }^{\text {B }}$ Consult factory for marine grade cataphoresis treatment．
Warranty： 5 year limited warranty
Designed in collaboration with Gensler as Product Design Consultant


Views


## IへRGEォI

## ZEDGE LINE

## INSTALLATION (REQUIRED)

```
\begin{tabular}{|l|l|}
\hline \(\mathbf{1 E 3 4 4 7}\) & \begin{tabular}{l} 
Nylon outer casing fitted for parallel \\
connection with feed through-wiring. \\
\(3 / 4 "\) and \(1 / 2^{\prime \prime}\) knock-outs made for EMT \\
connectors and conduit entry. Suitable \\
for concrete pour, drywall, or stucco \\
applications. \\
Dimensions: \(12.13 " \mathrm{~W} \times 2.7^{\prime \prime} \mathrm{H} \times 3\) 3"D
\end{tabular} \\
\hline
\end{tabular}
```


## Photometry







## BASE CONCRETE BLOCK VENEER AND TRASH ENCLOSURE BLOCK

EXHIBIT F


## HORIZONTAL FIBER-CEMENT SIDING



## VERTICAL FIBER-CEMENT PANEL SIDING



## STANDING SEAM METAL ROOFING



SANDY HEALTH CLINIC 192530-MATERIALS

## WINDOW COLOR



## STURGIS GRAY TRIM AND FASCIA PAINT



## COMBINED BUILDING MATERIALS



## Exhibit G



AWP-3030-RIBBED - PANEL DETAIL
SCALE: 1" = 1'-0"


SCALE: $3^{\prime \prime}=1^{\prime}-0^{\prime \prime}$




## AWP-3030 <br> VERTICAL INSTALLATION GUIDE

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## GENERAL

This guide is intended to provide the key information needed to successfully install Nichiha's 3030mm Architectural Wall Panels (AWP3030) in a vertical application. Further installation information and technical resources such as animated instructional videos, Technical Bulletins, three-part specifications, product testing and certifications, architectural details in AutoCAD, Revit, and PDF versions, and other technical documents are available on our website: nichiha.com/resource-center.

Install products in accordance with the latest installation guidelines and all applicable building codes and other laws, rules, regulations, and ordinances. Review all installation instructions and other applicable product documents before installation. This install guide's effective date is July 2019.

## PRODUCT INSPECTION

Inspect all products thoroughly prior to installation. Do not install any product which may have been damaged in shipment or appears to have a damaged or irregular finish. Should you have a question or problem with your order, contact your local dealer or Nichiha Customer Service, toll-free, at 1.866.424.4421. Keep the products dry prior to installation. It is best to store the products indoors, otherwise keep them covered. Do not stack pallets more than two high.

## BASICS OF THE AWP3030 SYSTEM

Nichiha AWP3030 dimensions are 455mm (h) x 3,030mm (I) $\times 16 \mathrm{~mm}(\mathrm{t})$. It is important to keep in mind the actual metric dimensions when considering panel layout, placement of control and compression joints, and with respect to sizing window and door openings. Approximate Imperial dimensions are 17-7/8" (h) x 119-5/16" (I) x 5/8" (t).


VINTAGE WOOD PANEL DETAIL
SCALE: 1" = 1'-0"

AWP3030 panel edges are shiplapped on the long edges and a factory sealant gasket is included on one edge, providing a factory seal on all vertical joints. AWP attachment hardware engages the long edges, holding the panels off the substrate surface by $10 \mathrm{~mm}\left(\sim 3 / 8^{\prime \prime}\right)$ and creating a closed-joint, drained/ back-ventilated rainscreen system with concealed fastening. When accounting for the overall thickness of the AWP system, add this 10 mm plus the thickness of the panel ( 16 mm ) for total system thickness of 26 mm .

AWP3030 may be installed horizontally or vertically. See also Horizontal Installation Guide AWP1818, AWP3030.


PANEL SECTION

## LIMITATIONS AND TECHNICAL DESIGN REVIEWS

Natural limitations on product usage are inherent to any cladding product's design, physical characteristics, and attachment system. Nichiha AWP are intended as a low-to-mid-rise cladding product.

Any project of more than three stories or 45 feet, as well as those located in high wind coastal areas (Exposure Categories C and D with Basic Wind Speed in excess of 130 mph ), or those with any wall assembly not described in Framing \& Sheathing Requirements, require a technical review by Nichiha to evaluate feasibility via our Technical Design Review process.

By evaluating a project's unique criteria and design, we can reference independently test-derived and calculated wind load performance data for our products to determine whether and how the panels can safely be installed on the project. Contact your local rep or Nichiha technical department for details or to initiate a Technical Design Review.

AWP are not to be used in any applications/uses not specified or described in this installation guide or other Nichiha technical documents. Any such use shall not be backed by the manufacturer's product warranty.

Do not use AWP on open screen walls.
Insulated Concrete Forms (ICFs) require additional measures.

Installation of AWP products on modular structures that are factory-constructed and then transported to a final site are not approved; and further, excluded from the Limited Product Warranty, per Section 2.F.

AWP installed as soffit is not covered by the Nichiha Product Warranty or the Nichiha Finish Warranty. Refer to pages 38-41 of the AWP Horizontal Installation Guide.

Please contact Nichiha Technical Services for assistance.

## SAFETY

As with any natural stone, masonry, or concrete based product, when cutting, drilling, sawing, sanding, or abrading fiber cement cladding, proper safety measures must be taken due to the potential for airborne silica dust, an OSHA-identified hazardous substance that can pose serious medical risks.

Always wear safety glasses and a NIOSH/OSHA approved respirator with a rating of $\mathrm{N}, \mathrm{O}$, or P 100. Carefully follow the respirator manufacturer's instructions as well as applicable governmental safety regulations concerning silica. Refer to Nichiha's SDS for more information.

Always cut fiber cement panels outside and with a dust-collecting HEPA system. Do not cut the products in an enclosed area.

Use a dust-reducing circular saw with diamond-tipped or carbide-tipped fiber cement saw blades.

Always clean panels after cutting. Fiber cement dust can bind to the panel finish. Vacuum dust with a HEPA-filtered vacuum.

## FRAMING AND SHEATHING REQUIREMENTS

Prior to Nichiha installation, closely inspect exterior wall substrate and correct any problems. Walls that are out of plumb, for example, can negatively impact the installation quality of AWP. Nichiha Spacer may be used in conjunction with panel attachment hardware if necessary to ensure a smooth, even substrate.

With conventional stud spacing, 7/16" or thicker APA rated OSB or Plywood sheathing must be used as the fastening base for Vertical AWP3030 as the panel size module will not align with framing. If nail-base insulation sheathing is considered, please contact Nichiha Technical as additional measures may be required. Alternatively, studs or furring may be spaced at $45.5 \mathrm{~cm}\left(17-7 / 8^{\prime \prime}\right)$ o.c. to allow fastening of AWP hardware directly to framing.

Refer to our third party building code certification(s) and/or state/local approvals for allowable wind design pressures at nichiha.com/resource-center.

Nichiha AWP cladding may be installed on vertical walls only. No tilted/sloped walls, nor true radius/ curved walls. Vertical AWP installations are not compatible with PEMBs. AWP may be installed on wood or steel framing, concrete/masonry with furring, and Structural Insulating Panels (SIP) meeting the following requirements:

## WOOD STUDS

Structural Sheathing Method
Size: minimum $2^{\prime \prime} \times 4^{\prime \prime}$ studs
Spacing: $16^{\prime \prime}$ o.c. max
Sheathing: APA rated exterior grade minimum
7/16" plywood/OSB required

## Custom Stud/Furring Spacing Method

Size: minimum 2"×4" studs
Spacing: $45.5 \mathrm{~cm}\left(17-7 / 8^{\prime \prime}\right)$ о.c.
Sheathing: APA rated exterior grade minimum
7/16" plywood/OSB, 1/2" or 5/8" gypsum

## METAL STUDS

Structural Sheathing Method
Gauge: minimum 18
Spacing: $16^{\prime \prime}$ o.c. max
Sheathing: APA rated exterior grade minimum 7/16"
plywood/OSB required

## Custom Stud/Furring Spacing Method

Gauge: minimum 18
Spacing: $45.5 \mathrm{~cm}\left(17-7 / 8^{\prime \prime}\right)$ o.c.
Sheathing: APA rated exterior grade minimum 7/16"
plywood/OSB, 1/2" or 5/8" gypsum

## CONCRETE/MASONRY

Furring is required for installation of AWP over concrete and masonry structures.

Wood Furring: pressure treated lumber $2^{\prime \prime} \times 4^{\prime \prime}$, oriented vertically, spaced $45.5 \mathrm{~cm}\left(17-7 / 8^{\prime \prime}\right)$ o.c. max with additional vertical furring segments at Vertical Starter Track locations to enable 9" o.c. fastener spacing for track.

Metal Furring: hat channel, c-stud, or z-furring, minimum 18 gauge with $1^{\prime \prime}$ - 2" flanges, oriented vertically, spaced 45.5 cm (17-7/8") o.c. max. with additional vertical furring segments at Vertical Starter Track locations to enable 9" o.c. fastener spacing for track.

Sheathing: exterior grade minimum 7/16" plywood/ OSB required with furring spacing other than 45.5 cm (17-7/8") o.c.

## STRUCTURAL INSULATING PANELS (SIP)

SIPs should be installed in accordance with manufacturer's instructions and local building codes. Additional special Nichiha installation requirements for SIPs are discussed in the Fasteners and Installing the First Course sections to follow.

For buildings greater than one story, contact
Technical Department for assistance.

## CONTINUOUS INSULATION

When exterior/continuous insulation is to be used with AWP3030 in vertical applications, please contact Nichiha Technical Services for assistance. Framing/ sheathing/furring alternatives will be necessary.

Also refer to the Technical Bulletin:
Continuous Insulation and AWP available at
Nichiha.com/resource-center.

## VERTICAL AWP3030 OVER C.I. ATTACHMENT REOUIREMENTS

When adding a furring grid* to enable AWP installation over c.i., the following general criteria are applicable:

Special attention must be paid to supporting the Vertical Starter Track, which bears the weight of AWP3030 in vertical applications. The clips do not share the dead loads for vertical panels.

1. Shaped metal furrings ( $Z$, hat channel, $C$, etc.)

- Minimum 18 gauge
- Aligned vertically
- Spaced 16" o.c. (max)
- Min. 7/16" APA Rated OSB or Plywood
- or -

2. Pressure treated lumber

- Minimum $2 \times\left(1.5^{\prime \prime}\right)$ thickness
- Aligned vertically
- Spaced 16" o.c. (max.)
- Min. 7/16" APA Rated OSB or Plywood - or -

3. Shaped metal furrings (one layer)

- Minimum 18 gauge
- Aligned vertically at 17-7/8" o.c.
- Additional vertical furring segments at Vertical Starter Track locations to enable 9" o.c. fastener spacing for track
- or -

4. Shaped metal furrings (two layers)
( $Z$, hat channel, $C$, etc.)

- Layer One
- Minimum 18 gauge
- Aligned horizontally
- Spaced per engineer's design
- Layer Two
- Minimum 18 gauge
- Aligned vertically at 17-7/8" o.c.
- Additional vertical furring segments at Vertical Starter Track locations to enable 9" o.c. fastener spacing for track.
- or -

5. CL-TALON ${ }^{\oplus} 300$

- Base Track and Wall Mount T-Tracks (vertical) at $16^{\prime \prime}$ o.c. (aligned with framing), and Therme Clips spaced per project loading requirements
- Wall Mount Supports (horizontal) at $16^{\prime \prime}$ o.c.
*Consult a structural engineer to design the furring system to manage the AWP system dead load of minimum 4 psf and also meet the project wind load design criteria. Furring must account for expected building compression. Nichiha does not provide fastener design for anchoring the furring to structure. Refer to IBC 2015 Table 2603.12.2 for more info.


## ACCESSORY ATTACHMENTS

Nichiha Double and Single Flange Sealant Backers and metal trims, such as H -Mold and Corner Key, must be fastened to furring, blocking, or 18 gauge flat stock. Sealant backers must be fastened every $12^{\prime \prime}-14^{\prime \prime}$ vertically, so any use of flat stock must accommodate this fastening schedule.

Outside corners may be wrapped with 18 gauge flat stock fabricated to fit the corner. Attach the stock to furring on both sides of the corner. Corner Clips are used to secure Nichiha factory panel Corners and may be fastened to the flat stock wrapping, as can metal trim corners.

## IBC 2015 TABLE 2603.12.2

The model building code for 2015 includes information in Chapter 26 about foam plastic insulation/sheathing and furring minimum fastening requirements. Table 2603.12 .2 shows various configurations depending upon framing gauge and spacing, fastener size and spacing, thickness of insulation and cladding weight. As an example, according to the table, 3 inches is the maximum thickness of foam sheathing on which a furring can be added directly on top, spaced at $16^{\prime \prime}$ o.c. and fastened with \#8 screws every 12"-16" (into 18 gauge wall framing), that can support a cladding weight of 3 psf.

## ENERGY CODE FRIENDLY MARKET OPTIONS

A number of engineered third party systems exist that are designed to solve the conflicts between energy code compliance and the safe installation of exterior claddings over continuous insulation.

Nichiha has direct experience with these products:

- Bracket and rail systems:
- Cascadia Clips ${ }^{\circledR}$
- FERO Cladding Support
- ISO Clip
- Knight Wall MFI ${ }^{\circledR}$
- CL-TALON ${ }^{\circledR}$
- Knight Wall $\mathrm{Cl}^{\circledR}$ and $\mathrm{HCl}^{\text {TM }}$ Systems
- SMARTci GreenGirts


## WEATHER RESISTIVE BARRIERS

A weather resistive barrier (WRB) is required when installing Nichiha panels over stud walls and SIPs. For CMU/concrete assemblies, Nichiha defers to local code requirements. Use an approved WRB as defined by the 2015 IBC. Refer to local building codes.

A permeable WRB is highly recommended when installing Nichiha panels for residential applications.

Permeable WRB is required for all commercial applications. A fluid applied WRB is acceptable.

Sheathings and insulations with an integrated code-compliant WRB such as ZIP System ${ }^{\circledR}$ and DensElement ${ }^{\text {TM }}$ are acceptable.

All openings must have appropriate flashing to prevent moisture penetration. Follow manufacturer's guidelines and all local building codes.

## STORAGE \& HANDLING

AWP are a finished product and care must be taken to protect them against damage prior to and during installation. Panels must be stored flat and kept dry. Indoor storage is best. Refer to the storage information included on product pallets.

Ensure panels are completely dry before installing Direct contact between the panels and the ground must be avoided at all times. It is necessary to keep panels clean during the installation process.

Cut panels face down.

Always clean panels with a clean, soft, dry cloth after cutting. Dust can bind to the finish.

When sidewalks are poured after awp installation, take steps to cover/protect panels near grade. Cement dried on AWP cannot be removed.


Always cover pallets with a tarp or store indoors!


Don't unpackage and re-stack panels!
Always carry panels on edge!

## FASTENERS

All Applications
Fasteners must be corrosion resistant. Stainless steel or corrosion resistant screws such as hot-dipped zinc or ceramic coated are recommended. Comply with all local building codes for fastener requirements.

Number 10, pan-head screws (HD .365") were used as clip fasteners for AWP wind load testing. The minimum size for clip fasteners is \#8. Ultimate Clip and Starter Track screws must have a pan, wafer, or hex type full head.

Min. Number 7 finish screws with a bugle or flat head (min. head diameter $0.255^{\prime \prime}$ ) are appropriate for face fastening locations. These must penetrate framing per the minimum requirements below. Refer to the Face Fastening Best Practices section for face fastening procedure.

When installing AWP with the Structural Sheathing Method, ensure clip fasteners are at least $1^{\prime \prime}$ in length to fully penetrate the plywood or OSB. Wherever possible when face fasteners are needed, screws must be long enough to penetrate all the way through the sheathing and into the framing.

For the Custom Stud Spacing Method, the fasteners must always penetrate the studs or furring with minimum 1" penetration for wood or $1 / 2^{\prime \prime}$ for metal.

## FACE FASTENING BEST PRACTICES

To minimize the appearance of face fasteners, utilize the following steps:

1. Apply low adhesive tape such as painters tape to the panel at face fastening locations.
2. Pre-drill panels $1^{\prime \prime}$ from the cut edge to be face fastened. Use a countersink drill bit with chamfer matching the head diameter of the bugle-head type screws to be used for face fastening.
3. Fill counter-sunk fastener holes with exterior cementitious filler, such as MH Ready Patch ${ }^{\circledR}$ and later dab touch-up paint with cotton swabs or artist brush.
4. Remove the painter's tape only after applying patch and touch up paint.


## INSTALLATION HARDWARE \& ACCESSORIES



ULTIMATE VERTICAL STARTER TRACK<br>Ultimate Vertical Starter Track serves as the foundational support for the AWP system while also providing faster and greater ease of installation. With Vertical AWP3030, the Starter Track carries the entirety of the dead loads and is required for each course.

FA 710 T Vertical Starter Track - 10mm rainscreen

## ULTIMATE CLIP II

Ultimate Clips are secured to the vertical panels' shiplaps, securing AWP to the wall while holding their back surface off the substrate to create the $10 \mathrm{~mm}\left(3 / 8^{\prime \prime}\right)$ rainscreen space. In vertical applications, clips do not support panel weight.

JEL 778 CLIP Compatible with all 3030 mm AWP - 10 mm rainscreen

Joint Tab Attachments included with Ultimate Clips are not needed for vertical panel installations.

## CORRUGATED SPACER

At termination points where Ultimate Clips cannot be used, Nichiha Corrugated Spacer is required to maintain the rainscreen space and prevent panel deflection at face fastening locations such as window jambs and outside corners.

FS 1005 SPACER - 5 mm rainscreen

FS 1010 SPACER - 10mm rainscreen


## SEALANT BACKERS

Nichiha Sealant Backers provide exact spacing for expansion and termination joints and the recommended depth of sealant (75-80\%).

They provide faster installation than a foam backer rod and require less sealant. At sealant joints, use a sealant that complies with ASTM C920, Class 35 (min.). Refer to the Sealant section on page 19 for more information.

Single Flange Sealant Backer: FHK 1015 - 10mm rainscreen
Double Flange Sealant Backer: FH 1015 - 10 mm rainscreen

## METAL TRIM OPTIONS

Nichiha metal trim provides aesthetically pleasing design options for corners, openings, and transitions.

| TRIM | APPLICATIONS |
| :--- | :--- |
| Corner Key | Outside Corners |
| H-Mold | Vertical Joints |
| Open Outside Corner | Outside Corners |
| J-Mold | Terminations |
| Inside Corner | Inside Corners |
| ESSENTIAL FLASHING SYSTEM | APPLICATIONS |
| Starter* | Base/Clearance Concealment |
| Compression Joint | Horizontal/Compression Joints |
| Overhang* | Fascia-to-Soffit Transitions |
| * Inside and outside corner segments are available. |  |

## PLANNING AND PANEL LAYOUT

To ensure a successful installation, it is important to first plan how the panels will be laid out, where horizontal/ compression joints will be located for each course, and line of sight regarding inside corners decided.

Reminder: AWP3030 actual dimensions are metric: 455 mm (h) $\times 3,030 \mathrm{~mm}$ (I). Imperial equivalents: 17 7/8" (h) x 119-5/16" (I).

## Horizontal/Compression Joints (Page 25):

1/2" (min.) Horizontal, flashed break detail to allow for building compression at floor lines. Horizontal joints may not be staggered.

Inside Corner Line of Sight (Page 20): Sealant joints at inside corners can be placed out of view from the primary line of sight of a wall. Place the sealant joint on the less-viewed corner wall. Alternatively, utilize Inside Corner metal trim.

Cut Panels: In general, it is best to avoid cutting AWP to short or narrow strips and segments of less than $9^{\prime \prime}$. The hard minimum width or height is $4^{\prime \prime}$ Adjust the layout or use alternate materials when needed to avoid cutting AWP smaller than 4"

Specifically, when an individual panel is taller than a window or other opening and is used over the head or under the sill, do not cut it to less than $9^{\prime \prime}$ in width along the opening jamb. (see image A)

When an opening is taller than an individual panel and two or more are needed to cap over the header or cup the sill, do not cut the panel to less than $4^{\prime \prime}$ in width along the jamb. (see image B)

Design Wind Pressures: Refer to our code approval documents when determining the best vertical panel installation method for a specific project. The Structural Sheathing Method and Custom Stud Spacing methods result in different allowable design pressures, dependent upon thickness of wood sheathing or type/gauge of custom spaced studs/ furring. Refer also to Limitations, Technical Reviews section regarding Nichiha's technical review process.


## AWP3030 - VERTICAL:

 INSTALLING THE FA 710 T VERTICAL STARTER TRACK
## All Applications

Without custom stud/furring spacing outlined in the Framing \& Sheathing Requirements section, 7/16" or thicker APA rated OSB or plywood sheathing MUST be used to enable vertical installation of AWP3030. Plywood/OSB shall be secured to building framing in compliance with best practices and local building codes. In any case, Vertical Starter Track must be secured to framing and never sheathing alone as it fully carries the weight of the vertical panels. With respect to nail-base insulation sheathings, please contact Nichiha Technical Services for guidance.

Starter Track must remain continuous. Staggering of horizontal joints is not permitted.

## MINIMUM CLEARANCES

The Starter must be level and attached at a minimum of 6" above finished soil grade or per local building codes (use a laser level to verify). When installing over a hard surface such as driveways or sidewalks, a $2^{\prime \prime}$ clearance is acceptable.

Keep AWP at least 1 " above roofs.
Essential Starter Flashing may be installed prior to the Starter Track to conceal the clearance gap above hardscape and decking. Beginning with outside and inside corner segments, fasten trim at each stud location or every 10 " o.c. to sill plate. Fasten inside and outside corner segments to framing on both sides of the trim, keeping at least $1^{\prime \prime}$ from trim vertical edges. Main segments will slide into/overlap the corner trim. Position Starter Track to leave 1/4" clearance between the panel edge and trim/flashing.


Vertical Starter Track fastened every 6" to 9" to framing.

## ALL APPLICATIONS

To fully secure Vertical Starter Track, use corrosion resistant screws of sufficient length to ensure full penetration of the sheathing and into framing by 1" for wood or $1 / 2^{\prime \prime}$ for metal. Starter must be level.

## WOOD \& METAL STUDS

Vertical Starter Track must be secured every 6"-9" into the sill plate or to the studs and, if applicable, halfway between into the wood sheathing.

## CONCRETE/MASONRY

When installing over concrete construction, the wall must be furred out with pressure treated lumber, metal hat channel, or z-furring. Install APA rated 7/16" OSB or plywood to furring when the spacing is other than $45.5 \mathrm{~cm}\left(17-7 / 8^{\prime \prime}\right)$ o.c.. Starter Track must be secured at each furring location and halfway between into the sheathing or blocking at an overall fastener spacing of 6"-9" o.c.

FIG. 14

## STRUCTURAL INSULATING PANELS (SIP)

Secure Starter Track every 6"-9" o.c. max into the sill/ horizontal base framing of the SIP.

NAIL-BASE INSULATION SHEATHING
Contact Nichiha Technical Services for guidance.


## GENERAL PANEL \& ACCESSORY BASICS

## PANEL SELECTION

Nichiha AWP are packaged with two panels in a pack, which are placed on pallets consisting of two stacks. Due to alternating patterns of texture and color between individual panels as well as how the panels are manufactured and packaged, it is best to install all panels from each individual stack before taking and installing panels from the second stack on the same pallet. Do not alternate installing from one stack and the second, which may result in undesirable patterns.

## SEALING CUT PANEL EDGES

When cutting AWP, it is best to cut with the panel face down, except when cutting brick finish panels as it is easier to follow the simulated mortar lines.

Cut and exposed panel edges must be primed or sealed with fiber cement sealer (e.g. DryLock ${ }^{\circledR}$ ) or paint such as Kilz Premium ${ }^{\circledR}$ or Kilz Max ${ }^{\circledR}$. Do not use supplied Illumination Touch-Up paint. (Fig. 14)

Be sure to clean panels with a dry, soft, clean cloth after cutting to prevent dust from bonding to the finish.


## CUTTING ULTIMATE CLIPS

JEL778 Ultimate Clips are 26" long. Where full length clips can be used, they are required. However, there may be conditions where clips must be cut to accommodate panels in smaller areas or segments such as short columns, pilasters, or insets/recesses.

Notches on the upward panel engagement flanges indicate where clips can be cut evenly into thirds. These $1 / 3$ segments can be further reduced evenly into two or four pieces each with weep holes serving as dividing points. The smallest segment must include at least one downward panel engagement flange. Always use the widest clip segment possible. Cut with a non-ferrous saw blade on a band or chop saw.


## SEALANT

Sealants to be used with AWP must match the following requirements:

- Comply with ASTM C920
- Have a Class of 35,50 , or $100 / 50$ (minimum $35 \%$ joint movement)
- Be a polyurethane, polyurethane hybrid, or Adfast Adseal 4580
- Provide two-sided adhesion at joints

OSI ${ }^{\circledR}$ QUAD ${ }^{\circledR}$ may not be used for Nichiha expansion joints:

- It is a class 25 product.
- QUAD ${ }^{\circledR}$ MAX is acceptable since it is a Class 50.

Refer to the Technical Bulletin: Sealants available at nichiha.com/resource-center.


## Single Flange Sealant Backer (FHK1015R) (Galvalume)

## SEALANT JOINTS/CAULKING

Fasten Single Flange Sealant Backers at inside corners (one wall at corner), along window and door jambs, and transition points with other cladding. Fasten to framing, blocking or plywood/OSB sheathing at $12^{\prime \prime}-14^{\prime \prime}$ о.c. with the $3 / 8^{\prime \prime}$ bump/ sealant portion butting the corner or jamb.

Sealant complying with ASTM C920, Class 35 (min.) is required where Single and/or Double Flange Sealant Backer is used.

Refer to the sealant manufacturer's instructions or requirements.

Place low-adhesive tape (masking or painter's) over the panel along the areas requiring sealant joints for a clean caulk line.

Fill the gap between the panels with a colormatched/coordinating ASTM C920, Class 35 (min.) sealant. The Nichiha Sealant Backer allows for the proper depth of sealant (75-80\%).

Before removing tape, press the surface of the sealant with a caulk spatula or similar tool to ensure an even surface.

Remove masking tape before sealant cures.
If excess sealant adheres to panel, remove completely using a putty knife or soft cloth.

## AWP3030 - VERTICAL INSTALLATION

Without custom stud/furring spacing outlined in Framing \& Sheathing Requirements section, 7/16" or thicker APA rated OSB or plywood sheathing MUST be used to enable vertical installation of AWP3030. Wall surfaces must be flat.

Use corrosion resistant screws of sufficient length to ensure full penetration of wood sheathing (Structural Sheathing Method), or the 17-7/8" o.c. studs with the Custom Stud/Furring Spacing Method (minimum penetration $1^{\prime \prime}$ into wood, $1 / 2^{\prime \prime}$ into metal), to secure Ultimate Clips. Face fasteners must be at least 1-1/2" in length.

Single Flange Sealant Backer and metal trim should be installed before panels. Refer to Inside Corners, Windows \& Doors and Outside Corners sections.

AWP installation proceeds by working from left to right.
If starting at an inside corner, predetermine which wall will include the Single Flange Sealant Backer. Consider the location to minimize the visibility of the sealant line. Clad the higher visibility wall without the sealant joint first so that the adjoining wall panels can terminate to it with the Single Flange Sealant Backer detail. Or utilize Inside Corner metal trim.

Prior to installing the first vertical panel, add 10 mm corrugated Spacer at the left edge of the wall at the starting point. The Spacer should extend upwards to where the panel will end.

Looking at an AWP3030 oriented horizontally, remove the bottom ship-lapped edge and then rotate the panel 90 degrees clockwise to set the short panel edge on the FA 710T Vertical Starter Track. The freshly cut and sealed edge should butt to the corner/starting point and will cover the 10 mm Spacer. Be sure to clean dust from cut panels with a dry, soft cloth or HEPA vacuum.

Pre-drill panels after applying low-adhesive tape to be removed after patching/touch-up. Fasten every 12"-16" o.c., spaced vertically, with a minimum 1" distance from the edge (Fig. 19a).

Fill counter-sunk fastener holes with exterior cementitious filler, such as MH Ready Patch ${ }^{\circledR}$ and later dab touch-up paint with cotton swabs. Remove painter's tape.

Whenever possible, use face fastening screws long enough to penetrate all the way through the sheathing and into the framing by $1 / 2^{\prime \prime}$ for metal, 1 " into wood. Refer to the Touch-up Paint, Minor Repairs sections for more info on patching face fasteners.

On the right, factory edge, add four Ultimate Clips evenly spaced along the full AWP3030 panel, with the first at the Starter Track edge. Add four fasteners per clip, evenly spaced (Fig. 19b). In the Structural Sheathing Method, the clips will be fastened only to the plywood/osb sheathing. With the Custom Stud Spacing, the clips will align with vertical framing and the fasteners will be secured to the studs or furring (Fig. 19c).

Working from left to right, install the next panel with its ship-lapped edges intact. A rubber mallet or block may be used to seat panels firmly in place and tighten together on vertical panel joints. Do not hammer directly on the panels as direct contact may cause cracks, gouges, or chipping. Install four Ultimate Clips as with the first panel, each with four screws. (Continued p. 20)


Continue likewise until reaching a termination or transition point. The factory edge must be removed from the last panel, and this cut edge must be face fastened over 10 mm Spacer. Space the fasteners every 12"-16" o.c. vertically, with a minimum 1" distance from the edge. Again, whenever possible, use face fastening screws long enough to penetrate all the way through the sheathing and into the framing, 1/2" into metal, $1^{\prime \prime}$ into wood. Refer to Face Fastening Best Practices for info on patching face fasteners.

To begin a second course of panels, install appropriate horizontal joint flashing or Essential Compression Joint Flashing above the top edge of the bottom/first course of panels. Then repeat the steps beginning with FA 710 T Vertical Starter Track a minimum $1 / 2^{\prime \prime}$ above the top edge of the first course of panels (See Horizontal/Compression Joint section). Horizontal joints may not be staggered.

INSIDE CORNERS, WINDOWS \& DOORS All Applications

Appropriate flashing should be used to prevent moisture penetration on all inside corners, doors, and windows. Refer to local building codes for best practices.

Cut and exposed panel edges must be coated with exterior acrylic latex paint.

## INSIDE CORNERS

## Single Flange Sealant Backer (FHK 1015):

Decide primary line of sight in order to minimize visibility of the sealant joint.

Install the panel on the front wall (more visible) first. Ensure panel is butted up tight to the inside corner wall. Fasten the Single Flange Sealant Backer onto the side wall right up against the front wall panel's edge at 12"-14" o.c. to framing, plywood/osb sheathing, or blocking.

Add 10 mm Spacer over the fastening flange of the Sealant Backer.

Install side wall panel, with factory edge removed and sealed, directly against the sealant backer, over the Spacer, and secure with face fasteners*. Fill space with ASTM C920, Class 35 (min.) sealant.


Inside Corner Metal Trim: Install Nichiha Inside Corner metal trim directly against the inside corner sheathing. Fasten metal trim every $12^{\prime \prime}-16^{\prime \prime}$ in a staggered fashion on alternating flanges.

Remove the shiplapped edges that will be inserted into the trim, treating the cuts, and install panels normally, butting to the Inside Corner trim in moderate contact.

Trim Boards: Install trim boards at inside corner first and then add Single Flange Sealant Backer. Add ASTM C920, Class 35 (min.) compliant sealant to the gap.
*Face fasteners should fully penetrate OSB or plywood sheathing and into the framing whenever possible. Refer to Face Fastening Best Practices for info on patching face fasteners.

## WINDOWS AND DOORS

Window Sills (J-Mold optional): For recessed windows, add a flashing where the panels will terminate so that the top edge is covered or capped.

As needed, cut the panel to the required height to fit below the window sill, leaving a $1 / 4^{\prime \prime}$ gap between the top of the cut panel edge and the window sill or trim board.

Cut panel edges must be sealed with $100 \%$ acrylic latex exterior primer or paint, such as Kilz Premium or Kilz Max. Clean any dust off the panels with a dry, soft clean cloth.

Fasten Ultimate Clips along the sides of the panel to sheathing, framing, or furring with a clip positioned within an inch of the top end of the panel meeting the sill and the lowest clip at Vertical Starter Track edge.

If the top edge of the panel is fully sheltered under the sill, it is not necessary to seal the $1 / 4^{\prime \prime}$ gap. For better system performance, Nichiha recommends the vented approach.

If desired, install J-Mold trim, fastened every 12"16", under the sill prior to panels.


## WINDOW/DOOR JAMBS

A minimum gap of $1 / 4^{\prime \prime}$ is required when butting panels into windows, doors, and trim boards. Refer to window/door manufacturer guidelines for spacing trims around windows.

Single Flange Sealant Backer: Install the Single Flange Sealant Backer first, butting to the door/window jamb or trim pieces prior to installing the panels.

The Single Flange Sealant Backer must be fastened a minimum of $12^{\prime \prime}$ to $14^{\prime \prime}$ o.c. to framing, plywood/osb sheathing, or blocking.

Add 10 mm Corrugated Spacer along the jamb.
Remove appropriate ship-lapped edge of panel, clean off dust with soft, dry cloth, and treat cut edge.

Install panels, face fastening through Spacer along the jamb edge every $12^{\prime \prime}-16^{\prime \prime}$, keeping a minimum $1^{\prime \prime}$ from panel edge. Use face fasteners long enough to penetrate framing.

Fill gap with recommended sealant.


J-Mold: Pre-install J-Mold trim, fastening every 12"-16", with a 1/4" gap between it and the jamb or per window/door manufacturer instructions

After installing the next-to-last panel, measure from the edge of the face of this panel to the J-Mold edge (the 90 degree angle edge). From this measurement, subtract $1 / 4^{\prime \prime}$ and cut the last panel to this width. Paint or prime cut edges and clean off dust from panel.

Install 10 mm Corrugated Spacer next to the metal trim.
Install panels by inserting the cut edge into the metal channel and then shifting the panel over onto the side Ultimate Clips along the adjacent panel, fitting ship-lapped edges together.

Face fasten through Spacer along the jamb edge every $12^{\prime \prime}-16^{\prime \prime}$. Use face fasteners long enough to penetrate framing.

Lastly, add foam backer rod and sealant to the 1/4" gap, if applicable, between the J-Mold and jamb.

## WINDOW/DOOR HEADERS

Starter Track: When starting a course of vertical panels above a window or door, add flashing and FA 710 T Vertical Starter Track at the header, installed with fasteners every $6^{\prime \prime}-9$ " into the opening header.


## OUTSIDE CORNERS

There are two primary outside corner installation options for vertical AWP3030:

Trim Boards: Fiber Cement, Wood, or PVC

Metal (Open Outside, Corner Key) or Vinyl Trim Channels.

Appropriate flashing must be used as required to prevent moisture penetration at outside corners.

FIBER CEMENT \& PVC TRIM BOARDS

Nichiha manufactures a full line of fiber cement trim boards - NichiTrim ${ }^{\text {TM }}$, which are available in the Southeast U.S. Refer to Nichiha.com for more information.

When panels are to be butted to fiber cement, wood or other trim pieces, use Nichiha Single Flange Sealant Backer.

Add 10 mm Spacer, remove the appropriate panel shiplap, and face fasten panel edge every $12^{\prime \prime}-16^{\prime \prime}$, vertically, keeping $1^{\prime \prime}$ from edge. Apply sealant to joint width. Sealant must be compliant with ASTM C920, Class 35 (min).

## METAL \& VINYL TRIM

(including Nichiha Corner Key and Open Outside Corner trim)

When installing Nichiha AWP3030 in a vertical orientation, pre-fasten corner trim channels, securing trim to framing every $12^{\prime \prime}-16^{\prime \prime}$, alternating/staggering the fasteners on both flanges.

At the starting point, such as an outside corner, remove the panel edge, add 10 mm Spacer to the wall at the corner. Set the panel on the Starter and into the corner trim channel, and then face fasten the panel as described at the beginning of the AWP3030 Vertical Panel Installation section.

Working from left to right, when reaching the next outside corner, follow the steps for the appropriate trim profile:

Corner Key: After installing the next-to-last panel, measure from the edge of the face of this panel to the Corner Key edge (the 90 degree corner angle edge). From this measurement, subtract 1-3/8" and cut the panel to this width. Paint or prime cut edges and clean off dust from panel. (Fig. 24a)

Open Outside Corner: After installing the next-tolast panel, measure from the edge of the face of this panel to the Open Outside Corner edge (the 90 degree angle edge). From this measurement, subtract $1 / 4^{\prime \prime}$ and cut the panel to this width. Paint
or prime cut edges and clean off dust from panel.
Install 10 mm Spacer next to the metal trim. Install panels by inserting the cut edge into the metal channel, rotating into the wall plane, and then shifting the panel over onto the side clips along the adjacent panel, fitting ship-lapped edges together.

Face fasten panels through Spacer along the corner edge every $12^{\prime \prime}-16^{\prime \prime}$ (Fig. 24b). Use face fasteners long enough to penetrate framing.

Fit panels into channel trim so that panel edges are not exposed.

Nichiha metal trim pieces are each $10^{\prime}$ in length. To cut metal trim, use a non-ferrous carbide miter saw blade. When butting/stacking metal trim pieces, add a bead of polyurethane sealant at the seam/joint.

Metal trim can be pre-finished when purchased to match Nichiha Color Xpressions color(s). Otherwise, for field painting primed metal trim refer to Tamlyn's XtremeTrim Painting Guide.


## NON-90 DEGREE CORNERS

Corners other than 90 degrees can be achieved with custom metal trim, butting panels to trim board with a minimum 1/4" sealant gap, or with the use of Double Flange Sealant Backer to set cut panel edges at the desired corner angle. Please contact Nichiha Technical.

## VERTICAL CONTROL/EXPANSION JOINTS All Applications <br> Because thermal expansion occurs in the long ( 3030 mm ) dimension of the panels, Vertical Control/ Expansion Joints are not required for vertical installations of AWP3030.

HORIZONTAL/COMPRESSION JOINTS All Applications

The module of Vertical AWP3030 necessitates a continuous Horizontal/Compression Joint every 1195/16" (repeating after each course). Do not stagger horizontal joints.

Do not span floor lines with panels.
INSTALLING A HORIZONTAL COMPRESSION JOINT Install Essential Compression Joint Flashing or heavy
 gauge z-shaped metal flashing or drip cap over the top edge of the course of panels terminating under the Horizontal Compression Joint location. Fasten Essential Flashing at each stud location.

Install Vertical Starter Track over the flashing and check for level. Place Vertical Starter at least 1/2" above the course below and $1 / 4^{\prime \prime}$ above flashing/ trim. A best practice is to add flashing tape to cover the fasteners of the flashing, sealing it to the WRB.

Continue to install panels according to these guidelines with compression joints every 119-5/16" (max).


## LARGE OPENINGS

All Applications
Install Vertical Starter Track at the wall base in keeping with standard instructions on both sides of the opening.

Install Vertical Starter Track at the head of the opening, either the width of the opening or all the way across the wall.

Add panels per the standard procedure as in a typical Window or Door Opening for the jamb conditions.

Do not span floor lines with panels. Plan for a Horizontal/Compression Joint at the head of the opening or above, at the same level where the panels along the sides of the opening terminate, assuming the garage or other large opening is shorter than full length panels.


## LAST COURSE

All Applications
Cut panels (horizontally) to properly fit at the roof line under soffit or parapet cap (or at the proper transition point). Ensure Ultimate Clips along factory edges are secured no more than $1^{\prime \prime}-2^{\prime \prime}$ from the top of the panels.

Cover top panel row edge with roof cap/coping, where applicable.


## METAL COPING

NICHIHA PANEL CLIP ON LONG EDGES (FOUR CLIPS PER PANEL EDGE)

GABLE \& OVERHANG

Allow a minimum of $1^{1 "}$ clearance (as per local building codes) above the roof line.

At the top, cut the panel to follow the slope of the gable or overhang.

When installing soffit, the wall panels should be installed first, with the soffit installed over the panels.

Seal all cut panel edges with $100 \%$ acrylic primer or paint. Do not leave any panel edges exposed.

Essential Overhang Flashing may be used at the base of overhangs/bump-outs or porte-cocheres.

Prior to panel installation, fasten Overhang Flashing at each stud location, beginning with corner segments. Main segments will slide under/overlap corner segments.

Use Joint Clip segments to join main segments together. After first piece is secured, add a Joint Clip, fastening through both it and the first main segment. The next main segment will slide behind the Joint Clip.

Position Overhang so that its bottom/return flange butts to or overlaps soffit. The bottom return portion must extend beyond the face of the fascia substrate.


Essential Overhang Flashing \& Joint Clip


Outside Corner



Inside Corner


## TRANSITIONS WITH HORIZONTAL AWP

On projects also utilizing horizontally-installed AWP, expansion and compression type joints will be required as there is no way to naturally joint horizontal and vertical AWP directly.

VERTICAL JOINTS
A Double Flange Sealant Backer or H-Mold trim is necessary at vertical joints/transitions between horizontally oriented panels and vertically oriented ones.

HORIZONTAL JOINTS

A horizontal/compression joint style detail is necessary to transition between horizontal and vertical AWP. Please refer to Horizontal/Compression Joints on page 25.

Horizontal panel to Vertical panel Transitions
Face fasten the top, cut edge of the horizontal AWP, cap it with Essential Compression Joint Flashing or Z-flashing. To then install vertical AWP, add the Vertical Starter Track following the standard procedure and fastening requirements.

Vertical panel to Horizontal panel Transitions Install vertical panels to the desired transition level and cap with Essential Compress Joint Flashing or Z-flashing. Install the Horizontal Starter Track 1-1/4" above the flashing, following the standard procedure and fastening requirements. Refer to the Horizontal Installation Guide AWP1818, AWP3030.


Horizontal transition joint:
Compression Joint details


## CLEANING \& MAINTENANCE

## CLEANING PANELS

After completion of the installation or for periodic maintenance, it may be necessary to clean panels.

When cleaning panels, use no more than 400 psi of water pressure at 10 " to 12 " away. Do not pressure wash custom color panels.

To clean heavily soiled areas, a mild household detergent and/or soft bristle brush may be required.

Do not allow any detergent/cleaner to dry on panels. Rinse immediately after cleaning.

## PAINT TOUCH-UP

Touch up paint must be exterior grade 100\% acrylic latex and can be color matched by taking a panel sample to your local paint or home improvement store.

A small amount of touch-up paint is supplied with your custom color panel order. Do not use touch-up paint for edge treatment/sealing due to the limited quantity provided.

Utilize low-adhesive tape to isolate patching and touchup locations such as face fastened areas. Where face fasteners have been used and patched by cementitious filler, use a cotton swab to lightly dab touch-up paint.

For scratches, use a cotton swab for small ones or 1" foam brush for longer ones, again using a dabbing motion rather than brushing in order to minimize the amount of paint applied.

## REMOVAL OF EXTERIOR ACRYLIC LATEX PAINT

Wet Paint Removal - While the paint is still wet, flush the area with clean water, using mild abrasion with a clean cloth or soft brush.

Semi-Dry Paint Removal - If paint has set, but not dried, flush and clean as above, followed by light scrubbing with alcohol to remove any remaining paint residue. Rinse with water and a clean cloth.

Dry Paint Removal - Please refer to paint-removal guide in the next section.

## OTHER PAINT \& GRAFFITI REMOVAL

The following products have been tested on Nichiha panels to aid in the removal of graffiti type markings.* These citrus-based products can also be used for basic panel cleaning purposes. The panels were sprayed with an indoor/outdoor aerosol spray paint and left to dry overnight, and then the paint removal products were applied following the manufacturer's guidelines.

All products tested achieved good results. However, the outcome may vary depending on the amount of paint that needs to be removed. Be sure to follow all manufacturer's guidelines and first test in an inconspicuous area before working on a larger area.

Do NOT use these cleaners with custom color panels. *Nichiha is not liable for any damage caused by the use of these cleaners.

## CITRISTRIP

www.citristrip.com

Products tested:
Citristrip Striping Gel - One Quart container
Citristrip Stripping Aerosol-18 oz. spray can
GOOF OFF GRAFFITI REMOVER
www.goof-off.com
Products tested:
Goof Off Aerosol - 16 oz. spray can
Goof Off - 22 oz. trigger spray bottle

TAGAWAY
www.tagaway.com
Product tested:
Tagaway-32 oz. trigger spray bottle

## REPAIRING MINOR DAMAGE

Isolate the blemish with a low adhesive tape such as painters tape. This will help protect the surrounding area of the panel and aide in creating a more polished, clean repair.

Lightly brush/abrade the surface within the taped off area in order to remove any loose material.

Carefully fill and smooth the resultant prepped area with cementitious patching material such as MH Ready Patch. Allow to dry/cure fully.

Gently smooth the patch and then apply touch-up paint to the affected area. Allow touch-up paint to dry and remove the tape.

## PANEL REPLACEMENT

Set the depth of the circular saw blade slightly deeper than the panel so the saw blade does not cut into the building wrap or sheathing.

Make cuts into the damaged panel and break into pieces for easier removal.

Remove damaged panel.
If necessary, cut new panel to appropriate height.
Looking at the panel oriented horizontally, cut the top ship-lapped edge off the panel (Figure 33a).

Clean off dust and seal the cut edge.
Add 10 mm Spacer along the right side of the uncovered wall surface. (Figure 33b)

Set the new panel in place on the Vertical Starter Track with the intact factory edge fitting on the exposed
 clips on the left side of the uncovered space.

Pre-drill and face fasten the right edge of panel through the Spacer with a screw every $12^{\prime \prime}-16^{\prime \prime}$ into framing, furring, or blocking (Figure 33c). When only wood sheathing is available for the face fasteners, reduce the screw spacing to $6^{\prime \prime}-8$ " o.c.

Fill countersunk screw heads per Paint Touch Up and Minor Repairs.

## FIG. 33A



## FIG. 33C



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#### Abstract

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EC-58

L.A.R.R. 2608

CERTIFICATION \& TESTING


Code Compliance
CCRR-0299


WUI 8140-2029

29
 Florida Approval 12875

## MIAM-DADE COUNTT

Miami-Dade NOA 18-0522.05

## March 12, 2020

| Steve Kelly, Project Coordinator |  |
| :---: | :---: |
| Clackamas County Community D 2051 Kaen Road, Suite 245 |  |
|  |  |
| Oregon City, Oregon 97045 |  |
| Via email: | stevekel@clackamas.us |
| Regarding: | Traffic Impact Analysis Letter |
|  | Sandy Health Clinic |
|  | 39831 Highway 26 |
|  | Sandy, Oregon 97055 |
|  | PBS Project 71524.000 |

Dear Mr. Kelley:
This document represents a traffic impact analysis (TIA) letter to meet the submittal requirements of the City of Sandy (City) and the Oregon Department of Transportation (ODOT). The TIA letter was scoped based on consultation with the City, ODOT, and Clackamas County.

## PROJECT DESCRIPTION

The Clackamas County Health Department proposes to consolidate its health services from two offices in Sandy, Oregon, to one location. The existing offices are Sandy Health Clinic located at Sandy High School, 37400 Bell Street, and Clackamas County Health Center, located downtown at 38872 Proctor Boulevard. The proposed site has an existing 6,300-square-foot building used as a warehouse that will be replaced with a 9,600 -square-foot health clinic. See Figure 1 for a vicinity map.

The site is within the city under its jurisdiction at the northeast corner of the Highway 26/Ten Eyck Road intersection, and the site will send all of its trips through the Highway 26/Ten Eyck Road intersection. See Figure 2 for the site plan. ODOT recommended a TIA be prepared to determine if a right-turn lane is warranted on westbound Highway 26 at Ten Eyck Road. The intersection of Highway 26 and Ten Eyck Road is under ODOT authority, and Ten Eyck Road is under Clackamas County jurisdiction. After contacting each of the three public agencies and holding discussions with Ankrom Moisan Architects, Inc., the TIA was focused toward two intersections: Highway 26 and Ten Eyck Road, related to the need for a westbound right-turn lane, and Pleasant Street and Ten Eyck Road, related to queueing on southbound Ten Eyck Road.

ODOT recommended that, as a condition of approval, a TIA be prepared to determine if a right-turn lane is warranted for westbound Highway 26 at Ten Eyck Road. If a right-turn lane is not warranted, ODOT recommended that the sidewalk along the site frontage of Highway 26 be extended into the roadway in such a way as to eliminate the existing slip lane and define the bicycle lane through the intersection. Figure 3 shows the existing lane configuration of the intersections in the vicinity of the project.

City staff directed PBS to include an evaluation of queueing on Ten Eyck Road to verify it will not block access to Pleasant Street.

[^1]
## TRAFFIC VOLUMES

PBS contracted with All Traffic Data to collect AM and PM peak hour traffic counts for the adjacent streets at Ten Eyck Road/Highway 26, Ten Eyck Road/Pleasant Street (east leg), and Ten Eyck Road/Pleasant Street (west leg). The traffic data were collected on February 19, 2020. Detailed traffic volume reports are provided in Appendix A

## Background Growth

Background growth is a generic increase in traffic volumes that either is not attributable to specific developments in process (in process) or is attributable to influences outside the study area. No in process projects contributed trips to the studied intersections. A linear background growth rate of $2.0 \%$ per year was applied to 2020 peak hour volumes between public roadways at the studied intersections. The $2.0 \%$ growth rate was based on evaluation of ODOT count volumes on Highway 26 through traffic east and west of Ten Eyck Road between 2013 and 2018. The background growth volumes are included in the 2022 build-out year and the 2029 forecast year traffic volumes. Details of the background growth rate estimate are in Appendix A

## Seasonal Adjustment Factor

A $28 \%$ seasonal adjustment factor (SAF) was applied to the through movements on Highway 26 based on the ODOT Analysis Procedure Manual, Version 2 (APMv2). Since no ODOT automatic traffic recorders (ATRs) are located within the study area, and no ATRs offer similar characteristics or reasonable adjustments, the seasonal trend table method was used. The $28 \%$ SAF was calculated based on the average of the commuter and summer trends, adjusting from the mid-February counts to the seasonal peak period. ${ }^{1}$ Figure 5 presents the 30 th highestvolume hour, including the SAF. Details of the seasonal adjustment factor estimate are in Appendix A

## Baseline Volumes

The 2020, 2022, and 2029 baseline volumes represent the study area traffic volumes without the Sandy Health Clinic project development trips. The baseline volumes are calculated as the sum of existing traffic, background growth, and season factors. The 2029 forecast year is used to compare to the City of Sandy's Transportation System Plan (TSP).

## TRIP GENERATION AND DISTRIBUTION

The following section relies on data provided in the Institute of Transportation Engineers' (ITE) Trip Generation Manual, 10th Edition (2017). Detailed trip generation calculations are provided in Appendix B.

## Trip Generation

The proposed Sandy Health Clinic will utilize a site with an existing building. The net new trips to the site are based on the replacement of a 6,300-square-foot building used for food distribution and warehouse space with a 9,600 -square-foot health clinic. Figure 2 provides a site plan for the proposed site. Trip generation for both the existing warehouse and the proposed health clinic are based on an independent variable of 1,000 square feet in the respective buildings. The trip generation estimates were calculated using the ITE weighted average trip rates for the peak hours of the adjacent streets. Table 1 summarizes the trip generation calculations.

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Table 1. Trip Generation

| ITE Land Use: | Existing Warehouse <br> Code 150 |  | Proposed Clinic <br> Code 630 |  | Net New Trips |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Independent Variable: | 1,000 square feet |  | 1,000 square feet |  |  |  |
| Size: | 6.3 |  | 9.6 |  |  |  |
| Weekday ADT: | 11 |  | 366 |  | 355 |  |
| Total Peak Hour Trips: | AM | PM | AM | PM | AM | PM |
| In: | 1 | 0 | 27 | 9 | 26 | 9 |
| Out: | 0 | 1 | 8 | 22 | 8 | 21 |
| Total: | 1 | 1 | 35 | 31 | 34 | 30 |

Findings: The proposed Sandy Health Clinic project will generate 355 net new trips on an average weekday, 34 net new trips in the AM peak hour of the adjacent streets, and 30 net new trips in the PM peak hour of the adjacent streets

## Proposed Trip Distribution

The proposed distribution of new trips is based on a review of the land uses within the study area, consultation with Clackamas County health clinic staff, and on engineering judgment. See Appendix B for correspondence on the trip distribution of the existing Sandy Health Clinic. All trips from northwest, southwest, and west of the site intersect Highway 26, and approach from the west of the site on Highway 26. All traffic southeast, northeast, and east of the site will approach the site from the east on Highway 26. The overall distribution pattern is proposed as follows:

- $90 \%$ to and from the site via Highway 26 from the west
- $10 \%$ to and from the site via Highway 26 from the east

The distribution pattern above represents an external distribution of the net new trips entering and exiting the study area. The distribution and assignment of the project's net new trips are shown on Figure 4.

## INTERSECTION OPERATIONS AND ROADWAY CAPACITY ANALYSES

## Operation Standards

The City of Sandy Minimum Requirements for Traffic Analysis cites a minimum level of service (LOS) D for signalized intersections and for stop conditions. The LOS is based on the volume-to-capacity ratio (v/c) for signalized intersections when the development is in full service. ODOT has a mobility standard of a v/c ratio 0.85 for highways categorized as Freight Route on a statewide highway for locations inside an urban growth boundary and a posted speed equal to or under 35 miles per hour ( mph ).

## Analysis Methodology

Traffic impacts were estimated to determine the extent of change in traffic conditions caused by future development. In order to make this determination, the following assumptions were employed:

- The individual peak hour volumes were analyzed for 2020 existing year, 2022 assumed full operation, and 2029 forecast year.
- The analysis is based on the AM and PM peak hour of the adjacent streets.
- No in-process trips were included in the analysis.

[^3]- An SAF of $28 \%$ increase was applied to the through movement volumes on Highway 26 based on the ODOT APMv2 Seasonal Trend Table method.
- The peak hour factor (PHF) for the overall intersection, as calculated from the count data, was applied for each analysis scenario.
- A minimum value of $2.0 \%$ was assumed for each movement in the future conditions for heavy vehicle percentage ( HV ) ) .
- As noted previously, trip generation, distribution, and assignment estimates for the project were prepared for the weekday AM and PM peak hours on the surrounding street system.
- Cumulative traffic impacts of the proposed project were determined by superimposing the project-generated traffic onto the baseline volumes for the weekday AM and PM peak hour at studied intersections. This sum is termed the "With Project" conditions.
- The LOS for the signalized intersection was calculated with Trafficware's Synchro software, Version 10, based on Highway Capacity Manual (HCM) 6th Edition (2016) methodologies. The ODOT protocol for Synchro analysis at signalized intersections was used to calculate the intersection v/c ratio.
- Signalized intersection results are reported as the $\mathrm{v} / \mathrm{c}$ ratio for the intersection.
- ODOT right-turn lane warrants at a signalized intersection are based on volume threshold and LOS standards per ODOT APMv2.
- Traffic signal timing values were supplied by ODOT and used throughout the LOS analysis without modification.
- The analysis includes a 2029 With Project alternative with a right-turn lane for westbound Highway 26.
- No site driveways were analyzed for this report.
- The queueing was estimated for 2029 conditions with and without the project using SimTraffic following ODOT APMv2.
- Right-turn lane impacts on bicycle safety are based on bicycle level of traffic stress (LTS) methodology, noted in APMv2.
- The results of the TIA were compared to the City's TSP related to the Highway 26/Ten Eyck Road improvements.


## Level of Service Analyses

Table 2 presents the LOS analysis for the Highway 26/Ten Eyck Road intersection for 2020 existing conditions during the studied peak hours. Detailed LOS calculation reports are provided in Appendix C.

Table 2. Estimated Level of Service at Study Area Intersection for 2020 Existing Conditions

| INTERSECTION | AM Peak Hour |  |  | PM Peak Hour |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | LOS | Delay <br> (sec/veh) | Intersection <br> v/c Ratio | LOS | Delay <br> (sec/veh) | Intersection <br> v/c Ratio |
| Highway 26/Ten Eyck Road | A | 9.1 | 0.56 | C | 20.8 | 0.67 |

Finding: The existing Highway 26/Ten Eyck Road intersection operates above LOS standards.

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Table 3 presents the LOS analysis for the Highway 26/Ten Eyck Road intersection for 2022 without the project during the studied peak hours. Detailed LOS calculation reports are provided in Appendix C.

Table 3. Estimated Level of Service at Study Area Intersection for 2022 Without Project Conditions

| INTERSECTION | AM Peak Hour |  |  | PM Peak Hour |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | LOS | Delay <br> (sec/veh) | Intersection <br> v/c Ratio | LOS | Delay <br> (sec/veh) | Intersection <br> v/c Ratio |
| Highway 26/Ten Eyck Road | A | 9.5 | 0.58 | C | 21.4 | 0.70 |

Finding: In 2022 without the project, the Highway 26/Ten Eyck Road intersection will operate above LOS standards.

Table 4 presents the LOS analysis for the Highway 26/Ten Eyck Road intersection for 2022 with the project during the studied peak hours. Detailed LOS calculation reports are provided in Appendix C.

Table 4. Estimated Level of Service at Study Area Intersection for 2022 With Project Conditions

| INTERSECTION | AM Peak Hour |  |  | PM Peak Hour |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | LOS | Delay <br> (sec/veh) | Intersection <br> v/c Ratio | LOS | Delay <br> (sec/veh) | Intersection <br> v/c Ratio |
| Highway 26/Ten Eyck Road | B | 10.2 | 0.59 | C | 21.8 | 0.72 |

Finding: In 2022 with the project, the Highway 26/Ten Eyck Road intersection will operate above LOS standards.
Table 5 presents the LOS analysis for the Highway 26/Ten Eyck Road intersection for 2029 without the project during the studied peak hours. Detailed LOS calculation reports are provided in Appendix C.

Table 5. Estimated Level of Service at Study Area Intersection for 2029 Without Project Conditions

| INTERSECTION* | AM Peak Hour |  |  | PM Peak Hour |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | LOS | Delay <br> (sec/veh) | Intersection <br> v/c Ratio | LOS | Delay <br> (sec/veh) | Intersection <br> v/c Ratio |
| Highway 26/Ten Eyck Road | B | 11.1 | 0.64 | C | 24.0 | 0.81 |

*This TIA evaluates the intersection in its existing configuration. The northbound and southbound left-turn lanes called for in City of Sandy's TSP are not evaluated.

Finding: In 2029 without the project, the Highway 26/Ten Eyck Road intersection will operate above LOS standards.

Table 6 presents the LOS analysis for the Highway 26/Ten Eyck Road intersection for 2029 with the project during the studied peak hours. It includes the LOS results with a westbound right-turn lane. Detailed LOS calculation reports are provided in Appendix C.

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Table 6. Estimated Level of Service at Study Area Intersection for 2029 With Project Conditions

| INTERSECTION* | AM Peak Hour |  |  | PM Peak Hour |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | LOS | Delay <br> (sec/veh) | Intersection <br> v/c Ratio | LOS | Delay <br> (sec/veh) | Intersection <br> v/c Ratio |
| Highway 26/Ten Eyck Road | B | 12.0 | 0.66 | C | 24.4 | 0.84 |
| Highway 26/Ten Eyck Road <br> With Westbound Right-Turn <br> Lane | B | 11.4 | 0.66 | C | 23.0 | 0.83 |

*This TIA evaluates the intersection in its existing configuration and with the addition of a westbound right-turn lane. The northbound and southbound left-turn lanes called for in City of Sandy's TSP are not evaluated.

Findings: In 2029 with the project, the Highway 26/Ten Eyck Road intersection will operate above LOS standards. The operation of the Highway 26/Ten Eyck Road intersection will not significantly improve with the installation of a westbound right-turn lane on Highway 26.

Recommendation: Do not install a right-turn lane for westbound Highway 26 at Ten Eyck Road.
The City's TSP calls for left-turn lanes on Ten Eyck Road and Wolf Drive at Highway 26, project M8. This improvement alternative was not evaluated for LOS, but the findings of the evaluation without M8 improvements did not support this relatively expensive improvement. The applicant is advised to maintain the existing 39 -foot street width on Ten Eyck Road frontage to allow this improvement in the future.

Finding: Maintain the width of 39 feet on Ten Eyck Road to provide the necessary width a for future left-turn lane.

## Queueing Analysis

Table 7 presents the 95th percentile queue analysis for the Highway 26/Ten Eyck Road and Ten Eyck Road/Pleasant Street intersections to verify the queuing at the intersections do not conflict with each other and that the existing lane storage is not exceeded. The queue analysis is based on procedures and settings outlined in ODOT APMv2 when using Trafficware's SimTraffic (Version 10) simulation software. Table 7 includes a column with queue model results with the inclusion of the southbound left-turn lane on Ten Eyck Road, City TSP project M8, between Highway 26 and the east segment of Pleasant Street. The detailed queuing reports are provided in Appendix C.

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Table 7. 95th Percentile Queue Analysis for 2029 Conditions

| Intersection | Lane/Lane Group | Available Storage ${ }^{2}$ (ft) | Weekday AM Peak Hour Queue (ft) |  |  | Weekday PM Peak Hour Queue (ft) |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | 2029 <br> Without Project |  | Add SB <br> LT Lane | 2029 <br> Without <br> Project |  | Add SB <br> LT Lane |
| Highway 26/Ten Eyck Road | EB LT | 120 | 100 | 125 | 125 | 225 | 250 | 250 |
|  | EB TH | 400 | 250 | 275 | 275 | 375 | 475 | 450 |
|  | EB TH | 1,000 | 200 | 200 | 200 | 350 | 425 | 400 |
|  | EB RT | 100 | 75 | 50 | 50 | 175 | 175 | 175 |
|  | WB LT | 100 | 0 | 0 | 0 | 50 | 75 | 75 |
|  | WB TH | 1,200 | 275 | 275 | 300 | 500 | 500 | 500 |
|  | WB TH+RT | 1,200 | 250 | 250 | 275 | 475 | 475 | 475 |
|  | NB | 275 | 175 | 175 | 175 | 275 | 250 | 250 |
|  | SB | 120 | 200 | 200 | 175 | 325 | 350 | 225 |
| Ten Eyck Road/Pleasant Street ${ }^{1}$ | EB | 120 | 75 | 50 | 50 | 75 | 100 | 75 |
|  | WB | 120 | 0 | 50 | 50 | 50 | 125 | 75 |

BOLD font indicates the queue exceeds the noted available storage.
${ }^{1}$ The queue lengths are reported for both intersections.
${ }^{2}$ For exclusive turn lanes, the available storage noted represents the length of the full-width lane, exclusive of the taper or transition. For continuous lanes, the available storage noted represents the distance from the intersection stop bar to the next upstream intersection or major driveway.

Eastbound left-turn queues at the Highway 26/Ten Eyck Road signal are anticipated to spill beyond the 120 feet of available storage lane during the PM peak hour, both without and with the project trips. The queue will block the eastbound-to-westbound Highway 26 connector and the ARCO gas station driveway. These queues will be made slightly longer by the addition of the Sandy Health Clinic project trips due to the eight new PM peak hour trips turning left on to Ten Eyck Road.

Eastbound through queues at the Highway 26/Ten Eyck Road signal are anticipated to spill beyond several driveways during the PM peak hour; with the addition of the Sandy Health Clinic project trips, the queues could spill beyond the Revenue Avenue intersection, likely due to the eastbound left-turn lane overflow.

Eastbound right-turn queues at the Highway 26/Ten Eyck Road signal are anticipated to spill beyond the 100 feet of available storage lane during the PM peak hour, both without and with the project trips. The Sandy Health Clinic project trips will have negligible effect on these queues.

Southbound queues at the Highway 26/Ten Eyck Road signal are anticipated to spill beyond the upstream intersections, both without and with the project trips. The queue will routinely block the driveways closest to the intersection (serving the lot on the northwest intersection corner) and the east segment of Pleasant Street. During the PM peak hour, the west segment of Pleasant Street also will be blocked by the southbound queue. The southbound queues will be made slightly longer by the addition of the Sandy Health Clinic project trips due to most trips turning right in the PM peak hour (19 of 21 trips). The queueing is much more impacted by the $2.0 \%$

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growth for each turning movement over the next nine years. This may be overly conservative as the $2.0 \%$ growth used in this report was based on ODOT through movements on Highway 26.

Finding: The trips from the Sandy Health Clinic do not significantly contribute to the queueing. The addition of a southbound left-turn lane to Ten Eyck Road with the future City project may reduce southbound queues at Highway 26. However, the 95th percentile queues will still extend beyond the east segment of Pleasant Street.

Recommendation: Collect additional turning movement counts in the future to verify the intersection volumes, especially the eastbound left-turn and southbound traffic movements, are growing at $2.0 \%$ annually. Reevaluate the queuing with the next TSP update.

## SAFETY EVALUATION

The safety evaluation focused on crash history at the existing intersection, bike evaluation, pedestrian safety, and Americans with Disabilities Act (ADA) access.

## Traffic Safety

The proposed Sandy Health Clinic was evaluated for traffic safety based on the existing crash history of the Highway 26/Ten Eyck Road intersection, sight distance of driveways, and driveway/intersection spacing. The crash history was reviewed for the last available 5-year period (January 1, 2014 to December 31, 2018). The records show 11 crashes at the Highway 26/Ten Eyck intersection with 6 rear-end crashes ( 5 of those on Highway 26) and 2 right-angle crashes. None were related to the right-turning vehicles and none were related to bicycles. The intersection crash rate per million vehicles entering is 0.26 . The mean critical crash rate for a four leg, signalized intersection in an urban area is 0.40 . See Appendix $D$ for the crash history and crash rate calculations.

Finding: The intersection crash rate is lower than the mean critical crash rate. No further investigation is necessary.
The stopping sight distance necessary is based on the existing speed limit and the likely travel speeds on the east leg of Pleasant Street. As a local street, Pleasant Street is assumed to have a 25 mph speed limit, but based on the dead end 450 feet to the east of the proposed site driveway and an intersection of Ten Eyck Road 150 feet to the west of the proposed site driveway, speeds are assumed to be 20 mph approaching the driveway. The stopping sight distance is adequate for looking east and west, but care should be taken not to install landscaping or signs along the site frontage that may restrict sight distance below 200 feet.

Finding: The proposed driveway will meet stopping sight distance standards.
Recommendation: The site plan should take care to maintain approximately 200 feet of sight distance in both directions on Pleasant Street.

The applicant proposes elimination of an existing site driveway on Ten Eyck Road. No crashes were noted associated with this driveway, but removing it will likely reduce risk of crashes in the future. The driveway's proximity to the Highway 26 signal ( 70 feet) may have contributed to past crashes. The same is true of eliminating the existing driveway on Pleasant Street that is close to Ten Eyck Road but is much less likely to be a safety concern due to the low traffic volume on Pleasant Street.

Finding: The project should improve safety on Ten Eyck Road and Pleasant Street by eliminating two driveways.

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## Bicycle Safety

The multi-model safety evaluation is related to the bike lane and motor vehicle lane interaction at the westbound bike lane approaching the Highway 26/Ten Eyck Road intersection. The existing and future conditions are evaluated using the bicycle LTS. The westbound Highway 26 approach to Ten Eyck Road has a 135 -foot-long taper (slip lane) with a dropped bike lane. The right-turn lane is approximately 50 feet long with a dashed merge area approximately 50 feet long. Based on ODOT APMv2: "A roadway with no marked bike lanes and a right-turn lane will be a high stress location unless the right-turn lane is short and rarely used. This condition will also occur if a bike lane is dropped ahead of an intersection. If the turn lane is short (less than $75^{\prime}$ ) then there is no impact on the LTS."

With the development, the right-turn volume will increase from 16 vehicles per hour to approximately 22 vehicles in the AM and PM peak hour. In the AM hours only one bicycle used the westbound bike lane. In the PM peak hours, no bicycles were counted. With the short length of the right-turn lane, there is no impact on the bicycle LTS.

Finding: No mitigation is necessary for the existing slip lane and bike lane due to short length of the lane and the low turning movement volumes.

Recommendation: Make no change to the existing westbound slip lane.

## Pedestrian Safety

The current site does not have a sidewalk on the east side Ten Eyck Road. The City has a sidewalk project in process to install a sidewalk on Ten Eyck Road and Pleasant Street, referred to as the SE Ten Eyck Road \& Pleasant Street Curb and Sidewalk Improvements. The project will provide ADA access to the Sandy Health Clinic.

The project may be modified to reduce the turning radius on northeast corner of the Highway 26/Ten Eyck Road intersection. This will reduce vehicle speeds and provide more area for ramp improvements at the corner. This should improve pedestrian safety.

Finding: The proposed modification to the City's sidewalk project will improve pedestrian safety but not reduce the width of Ten Eyck Road below 36 -feet to allow future installation of a left-turn lane.

## FINDINGS

The TIA findings are summarized below:

- The proposed Sandy Health Clinic project will generate 355 net new trips on an average weekday, 34 net new trips in the AM peak hour of the adjacent streets, and 30 net new trips in the PM peak hour of the adjacent streets.
- The existing Highway 26/Ten Eyck Road intersection operates above LOS standards.
- In 2022 without the project, the Highway 26/Ten Eyck Road intersection will operate above LOS standards.
- In 2022 with the project, the Highway 26/Ten Eyck Road intersection will operate at above LOS standards.
- In 2029 without the project, the Highway 26/Ten Eyck Road intersection will operate above LOS standards
- In 2029 with the project, the Highway 26/Ten Eyck Road intersection will operate above LOS standards.
- In 2029 with the project, the operation of the Highway 26/Ten Eyck Road intersection will not significantly improve with the installation of a westbound right-turn lane on Highway 26.

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- The trips from the Sandy Health Clinic do not significantly contribute to queueing at the Highway $26 /$ Ten Eyck Road intersection.
- Maintain the width of 39 feet on Ten Eyck Road to provide the necessary width a for future left-turn lane.
- The intersection crash rate is lower than the mean critical crash rate. No further investigation is necessary.
- The proposed driveway on Pleasant Street will meet stopping sight distance standards.
- The project should improve safety on Ten Eyck Road and Pleasant Street by eliminating two existing driveways (one on each roadway).
- No mitigation is necessary for the existing slip lane and bike lane due to its short length and low turning movements.
- The proposed modification to the City's sidewalk project will improve pedestrian safety but not reduce the width of Ten Eyck Road below 36 -feet to allow future installation of a left-turn lane.


## RECOMMENDATIONS

The TIA recommendations are summarized below:

- Do not install a right-turn lane for westbound Highway 26 at Ten Eyck Road.
- Make no change to the existing westbound slip lane.
- Collect additional turning movement counts in the future to verify the intersection volumes, especially the eastbound left-turn and southbound traffic volumes, are growing at $2.0 \%$ annually. Reevaluate the queuing with the next TSP update.
- The site plan should take care to maintain approximately 200 feet of sight distance in both directions on Pleasant Street.


## CLOSING

Please feel free to contact me at 360.567 .2117 or John.Manix@pbsusa.com with any questions or comments.

Sincerely,


John Manix, PE
Senior Traffic Engineer

Attachments: Figure 1. Vicinity Map
Figure 2. Site Plan


Figure 3. Trip Distribution and Assignment
Figure 4. 2040 With Project Volumes
Appendix A. Traffic Counts
Appendix B. Trip Generation Calculations Appendix C. Level of Service Calculations Appendix D. Crash History

Clackamas County Community Development
TIA Letter - Sandy Health Clinic
March 12, 2020
Page 11

BJ:DAH,JAM:mo


FIGURE $\quad 1$

## Vicinity Map <br> Sandy Health Clinic



| Traffic Impact Analysis | 39831 Highway 26 |
| :--- | ---: |
| Clackamas County Community Development | Sandy, Oregon |



## Existing Lane Configurations Sandy Health Clinic



| Traffic Impact Analysis | 39831 Highway 26 |
| :--- | ---: |
| Clackamas County Community Development | Sandy, Oregon |



## 2020 Existing Traffic Volumes



| Traffic Impact Analysis | 39831 Highway 26 |
| :--- | ---: |
| Clackamas County Community Development | Sandy, Oregon |



## 2022 With Project Traffic Volumes <br> Sandy Health Clinic

| Traffic Impact Analysis | 39831 Highway 26 |
| :--- | ---: |
| Clackamas County Community Development | Sandy, Oregon |



Ten Eyck Road / Pleasant Street (E)


Ten Eyck Rd - Wolf Dr / US 26
FIGURE 8

## 2029 Without Project Traffic Volumes Sandy Health Clinic

| Traffic Impact Analysis | 39831 Highway 26 |
| :--- | ---: |
| Clackamas County Community Development | Sandy, Oregon |



Ten Eyck Road / Pleasant Street (E)


Ten Eyck Rd - Wolf Dr / US 26
FIGURE 9

## 2029 With Project Traffic Volumes <br> Sandy Health Clinic

## Appendix A

Traffic Counts
Total Vehicle Summary
Ten Eyck Rd \& Hwy 26
Wednesday, February 19, 2020 7:00 AM to 9:00 AM
5-Minute Interval Summary

| Interval Start | Northbound Ten Eyck Rd |  |  |  | Southbound Ten Eyck Rd |  |  |  | Eastbound Hwy 26 |  |  |  | Westbound Hwy 26 |  |  |  | Interval Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Time | L | T | R | Bikes | L | T | R | Bikes | L | T | R | Bikes | L | T | R | Bikes |  |
| 7:00 AM | 13 | 0 | 0 | 0 | 3 | 0 | 11 | 0 | 2 | 33 | 2 | 0 | 0 | 61 | 1 | 0 | 126 |
| 7:05 AM | 16 | 0 | 0 | 0 | 0 | 0 | 15 | 0 | 6 | 39 | 0 | 0 | 0 | 78 | 1 | 0 | 155 |
| 7:10 AM | 20 | 0 | 0 | 0 | 3 | 1 | 12 | 0 | 2 | 26 | 2 | 0 | 0 | 83 | 1 | 0 | 150 |
| 7:15 AM | 15 | 0 | 0 | 0 | 2 | 0 | 18 | 0 | 2 | 53 | 3 | 0 | 0 | 85 | 1 | 0 | 179 |
| 7:20 AM | 16 | 2 | 0 | 0 | 2 | 0 | 17 | 0 | 3 | 39 | 3 | 0 | 0 | 64 | 0 | 0 | 146 |
| 7:25 AM | 9 | 0 | 0 | 0 | 1 | 0 | 12 | 0 | 2 | 39 | 4 | 0 | 0 | 74 | 3 | 0 | 144 |
| 7:30 AM | 7 | 0 | 0 | 0 | 0 | 1 | 9 | 0 | 4 | 53 | 4 | 0 | 0 | 58 | 3 | 0 | 139 |
| 7:35 AM | 12 | 1 | 0 | 0 | 0 | 1 | 13 | 0 | 10 | 57 | 4 | 0 | 0 | 45 | 1 | 0 | 144 |
| 7:40 AM | 5 | 0 | 0 | 0 | 3 | 1 | 5 | 0 | 7 | 71 | 2 | 0 | 0 | 55 | 0 | 0 | 149 |
| 7:45 AM | 8 | 0 | 0 | 0 | 3 | 1 | 13 | 0 | 4 | 86 | 3 | 0 | 0 | 49 | 3 | 1 | 170 |
| 7:50 AM | 5 | 0 | 1 | 0 | 4 | 1 | 4 | 0 | 8 | 79 | 4 | 0 | 0 | 41 | 0 | 0 | 147 |
| 7:55 AM | 10 | 0 | 1 | 0 | 0 | 1 | 6 | 0 | 4 | 74 | 2 | 0 | 0 | 38 | 2 | 0 | 138 |
| 8:00 AM | 4 | 0 | 0 | 0 | 1 | 0 | 8 | 0 | 1 | 62 | 4 | 0 | 0 | 45 | 1 | 0 | 126 |
| 8:05 AM | 12 | 0 | 0 | 0 | 1 | 0 | 9 | 0 | 3 | 55 | 1 | 0 | 0 | 43 | 0 | 0 | 124 |
| 8:10 AM | 5 | 0 | 0 | 0 | 0 | 0 | 9 | 0 | 4 | 60 | 2 | 0 | 0 | 46 | 0 | 0 | 126 |
| 8:15 AM | 4 | 0 | 0 | 0 | 0 | 0 | 8 | 0 | 4 | 47 | 1 | 0 | 1 | 51 | 0 | 0 | 116 |
| 8:20 AM | 7 | 0 | 0 | 0 | 3 | 0 | 12 | 0 | 4 | 62 | 1 | 0 | 0 | 48 | 2 | 0 | 139 |
| 8:25 AM | 6 | 2 | 1 | 0 | 1 | 0 | 11 | 0 | 3 | 61 | 4 | 0 | 0 | 40 | 0 | 0 | 129 |
| 8:30 AM | 7 | 1 | 0 | 0 | 6 | 0 | 9 | 0 | 7 | 60 | 1 | 0 | 1 | 47 | 1 | 0 | 140 |
| 8:35 AM | 12 | 0 | 0 | 0 | 1 | 1 | 7 | 0 | 5 | 53 | 3 | 0 | 0 | 53 | 2 | 0 | 137 |
| 8:40 AM | 9 | 1 | 1 | 0 | 2 | 1 | 11 | 0 | 1 | 56 | 2 | 0 | 1 | 53 | 0 | 0 | 138 |
| 8:45 AM | 15 | 0 | 0 | 0 | 5 | 0 | 13 | 0 | 10 | 64 | 3 | 0 | 0 | 51 | 1 | 0 | 162 |
| 8:50 AM | 4 | 1 | 0 | 0 | 2 | 0 | 14 | 0 | 5 | 60 | 3 | 0 | 0 | 62 | 2 | 0 | 153 |
| 8:55 AM | 3 | 0 | 0 | 0 | 3 | 1 | 4 | 0 | 3 | 60 | 2 | 0 | 0 | 45 | 0 | 0 | 121 |
| Total Survey | 224 | 8 | 4 | 0 | 46 | 10 | 250 | 0 | 104 | 1,349 | 60 | 0 | 3 | 1,315 | 25 | 1 | 3,398 |


| Pedestrians <br> Crosswalk |  |  |  |
| :---: | :---: | :---: | :---: |
| North | South | East | West |
| 0 | 0 | 0 | 0 |
| 0 | 1 | 0 | 1 |
| 0 | 0 | 0 | 0 |
| 0 | 0 | 0 | 0 |
| 0 | 0 | 0 | 0 |
| 0 | 0 | 0 | 0 |
| 0 | 0 | 0 | 0 |
| 0 | 0 | 0 | 0 |
| 0 | 0 | 0 | 0 |
| 0 | 0 | 0 | 1 |
| 0 | 0 | 0 | 0 |
| 0 | 0 | 0 | 0 |
| 0 | 0 | 0 | 0 |
| 0 | 0 | 0 | 1 |
| 0 | 0 | 0 | 0 |
| 0 | 0 | 0 | 0 |
| 0 | 0 | 0 | 0 |
| 0 | 0 | 0 | 1 |
| 0 | 0 | 0 | 0 |
| 0 | 0 | 0 | 0 |
| 0 | 0 | 0 | 0 |
| 0 | 0 | 0 | 0 |
| 2 | 0 | 0 | 0 |
| 2 | 1 | 0 | 4 |

15-Minute Interval Summary
7:00 AM to 9:00 AM

| Interval Start Time | Northbound Ten Eyck Rd |  |  |  | Southbound Ten Eyck Rd |  |  |  | Eastbound Hwy 26 |  |  |  | Westbound Hwy 26 |  |  |  | Interval Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | L | T | R | Bikes | L | T | R | Bikes | L | T | R | Bikes | L | T | R | Bikes |  |
| 7:00 AM | 49 | 0 | 0 | 0 | 6 | 1 | 38 | 0 | 10 | 98 | 4 | 0 | 0 | 222 | 3 | 0 | 431 |
| 7:15 AM | 40 | 2 | 0 | 0 | 5 | 0 | 47 | 0 | 7 | 131 | 10 | 0 | 0 | 223 | 4 | 0 | 469 |
| 7:30 AM | 24 | 1 | 0 | 0 | 3 | 3 | 27 | 0 | 21 | 181 | 10 | 0 | 0 | 158 | 4 | 0 | 432 |
| 7:45 AM | 23 | 0 | 2 | 0 | 7 | 3 | 23 | 0 | 16 | 239 | 9 | 0 | 0 | 128 | 5 | 1 | 455 |
| 8:00 AM | 21 | 0 | 0 | 0 | 2 | 0 | 26 | 0 | 8 | 177 | 7 | 0 | 0 | 134 | 1 | 0 | 376 |
| 8:15 AM | 17 | 2 | 1 | 0 | 4 | 0 | 31 | 0 | 11 | 170 | 6 | 0 | 1 | 139 | 2 | 0 | 384 |
| 8:30 AM | 28 | 2 | 1 | 0 | 9 | 2 | 27 | 0 | 13 | 169 | 6 | 0 | 2 | 153 | 3 | 0 | 415 |
| 8:45 AM | 22 | 1 | 0 | 0 | 10 | 1 | 31 | 0 | 18 | 184 | 8 | 0 | 0 | 158 | 3 | 0 | 436 |
| Total Survey | 224 | 8 | 4 | 0 | 46 | 10 | 250 | 0 | 104 | 1,349 | 60 | 0 | 3 | 1,315 | 25 | 1 | 3,398 |


| Pedestrians <br> Crosswalk |  |  |  |
| :---: | :---: | :---: | :---: |
| North | South | East | West |
| 0 | 1 | 0 | 1 |
| 0 | 0 | 0 | 0 |
| 0 | 0 | 0 | 0 |
| 0 | 0 | 0 | 1 |
| 0 | 0 | 0 | 1 |
| 0 | 0 | 0 | 0 |
| 0 | 0 | 0 | 1 |
| 2 | 0 | 0 | 0 |
| 2 | 1 | 0 | 4 |

Peak Hour Summary


| By <br> Movement | Northbound Ten Eyck Rd |  |  |  | Southbound Ten Eyck Rd |  |  |  | $\begin{gathered} \text { Eastbound } \\ \text { Hwy } 26 \end{gathered}$ |  |  |  | Westbound Hwy 26 |  |  |  | Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | L | T | R | Total | L | T | R | Total | L | T | R | Total | L | T | R | Total |  |
| Volume | 136 | 3 | 2 | 141 | 21 | 7 | 135 | 163 | 54 | 649 | 33 | 736 | 0 | 731 | 16 | 747 | 1,787 |
| \%HV | 3.7\% | 0.0\% | 0.0\% | 3.5\% | 23.8\% | 0.0\% | 1.5\% | 4.3\% | 5.6\% | 13.1\% | 6.1\% | 12.2\% | 0.0\% | 4.8\% | 0.0\% | 4.7\% | 7.7\% |
| PHF | 0.67 | 0.38 | 0.25 | 0.67 | 0.53 | 0.58 | 0.72 | 0.74 | 0.64 | 0.68 | 0.69 | 0.70 | 0.00 | 0.74 | 0.57 | 0.75 | 0.92 |

Rolling Hour Summary
7:00 AM to 9:00 AM

| Interval Start Time | Northbound Ten Eyck Rd |  |  |  | Southbound Ten Eyck Rd |  |  |  | Eastbound Hwy 26 |  |  |  | Westbound Hwy 26 |  |  |  | Interval Total | Pedestrians Crosswalk |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | L | T | R | Bikes | L | T | R | Bikes | L | T | R | Bikes | L | T | R | Bikes |  | North | South | East | West |
| 7:00 AM | 136 | 3 | 2 | 0 | 21 | 7 | 135 | 0 | 54 | 649 | 33 | 0 | 0 | 731 | 16 | 1 | 1,787 | 0 | 1 | 0 | 2 |
| 7:15 AM | 108 | 3 | 2 | 0 | 17 | 6 | 123 | 0 | 52 | 728 | 36 | 0 | 0 | 643 | 14 | 1 | 1,732 | 0 | 0 | 0 | 2 |
| 7:30 AM | 85 | 3 | 3 | 0 | 16 | 6 | 107 | 0 | 56 | 767 | 32 | 0 | 1 | 559 | 12 | 1 | 1,647 | 0 | 0 | 0 | 2 |
| 7:45 AM | 89 | 4 | 4 | 0 | 22 | 5 | 107 | 0 | 48 | 755 | 28 | 0 | 3 | 554 | 11 | 1 | 1,630 | 0 | 0 | 0 | 3 |
| 8:00 AM | 88 | 5 | 2 | 0 | 25 | 3 | 115 | 0 | 50 | 700 | 27 | 0 | 3 | 584 | 9 | 0 | 1,611 | 2 | 0 | 0 | 2 |

## Heavy Vehicle Summary



Heavy Vehicle 5-Minute Interval Summary
7:00 AM to 9:00 AM

| $\begin{gathered} \hline \text { Interval } \\ \text { Start } \\ \text { Time } \\ \hline \end{gathered}$ | Northbound Ten Eyck Rd |  |  |  | Southbound Ten Eyck Rd |  |  |  | Eastbound Hwy 26 |  |  |  | Westbound Hwy 26 |  |  |  | Interval Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | L | T | R | Total | L | T | R | Total | L | T | R | Total | L | T | R | Total |  |
| 7:00 AM | 1 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 8 | 1 | 9 | 0 | 5 | 0 | 5 | 15 |
| 7:05 AM | 1 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 13 | 0 | 13 | 0 | 2 | 0 | 2 | 16 |
| 7:10 AM | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 1 | 0 | 6 | 0 | 6 | 0 | 4 | 0 | 4 | 11 |
| 7:15 AM | 2 | 0 | 0 | 2 | 0 | 0 | 0 | 0 | 1 | 7 | 0 | 8 | 0 | 3 | 0 | 3 | 13 |
| 7:20 AM | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 1 | 0 | 5 | 0 | 5 | 0 | 4 | 0 | 4 | 10 |
| 7:25 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 5 | 0 | 5 | 0 | 2 | 0 | 2 | 7 |
| 7:30 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 2 | 0 | 3 | 0 | 0 | 0 | 0 | 3 |
| 7:35 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 3 | 0 | 4 | 0 | 2 | 0 | 2 | 6 |
| 7:40 AM | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 1 | 0 | 10 | 0 | 10 | 0 | 4 | 0 | 4 | 15 |
| 7:45 AM | 0 | 0 | 0 | 0 | 1 | 0 | 1 | 2 | 0 | 8 | 0 | 8 | 0 | 4 | 0 | 4 | 14 |
| 7:50 AM | 0 | 0 | 0 | 0 | 1 | 0 | 1 | 2 | 0 | 9 | 1 | 10 | 0 | 1 | 0 | 1 | 13 |
| 7:55 AM | 1 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 9 | 0 | 9 | 0 | 4 | 0 | 4 | 14 |
| 8:00 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 7 | 0 | 7 | 0 | 4 | 0 | 4 | 11 |
| 8:05 AM | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 1 | 0 | 5 | 1 | 6 | 0 | 5 | 0 | 5 | 12 |
| 8:10 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 5 | 0 | 5 | 0 | 7 | 0 | 7 | 12 |
| 8:15 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 2 | 0 | 2 | 0 | 5 | 0 | 5 | 7 |
| 8:20 AM | 1 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 2 | 0 | 2 | 0 | 8 | 0 | 8 | 11 |
| 8:25 AM | 0 | 0 | 1 | 1 | 0 | 0 | 0 | 0 | 0 | 8 | 0 | 8 | 0 | 6 | 0 | 6 | 15 |
| 8:30 AM | 1 | 0 | 0 | 1 | 1 | 0 | 0 | 1 | 0 | 3 | 0 | 3 | 1 | 11 | 0 | 12 | 17 |
| 8:35 AM | 1 | 0 | 0 | 1 | 0 | 0 | 1 | 1 | 1 | 3 | 0 | 4 | 0 | 2 | 1 | 3 | 9 |
| 8:40 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 3 | 0 | 3 | 0 | 3 | 0 | 3 | 6 |
| 8:45 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 4 | 0 | 4 | 0 | 3 | 0 | 3 | 7 |
| 8:50 AM | 0 | 0 | 0 | 0 | 0 | 0 | 2 | 2 | 0 | 6 | 1 | 7 | 0 | 5 | 0 | 5 | 14 |
| 8:55 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | - | 8 | 0 | 8 | 0 | 3 | 0 | 3 | 11 |
| Total Survey | 8 | 0 | 1 | 9 | 6 | 0 | 6 | 12 | 4 | 141 | 4 | 149 | 1 | 97 | 1 | 99 | 269 |

Heavy Vehicle 15-Minute Interval Summary
7:00 AM to 9:00 AM

| Interval Start Time | Northbound Ten Eyck Rd |  |  |  | Southbound Ten Eyck Rd |  |  |  | Eastbound Hwy 26 |  |  |  | Westbound Hwy 26 |  |  |  | Interval Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | L | T | R | Total | L | T | R | Total | L | T | R | Total | L | T | R | Total |  |
| 7:00 AM | 2 | 0 | 0 | 2 | 1 | 0 | 0 | 1 | 0 | 27 | 1 | 28 | 0 | 11 | 0 | 11 | 42 |
| 7:15 AM | 2 | 0 | 0 | 2 | 1 | 0 | 0 | 1 | 1 | 17 | 0 | 18 | 0 | 9 | 0 | 9 | 30 |
| 7:30 AM | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 1 | 2 | 15 | 0 | 17 | 0 | 6 | 0 | 6 | 24 |
| 7:45 AM | 1 | 0 | 0 | 1 | 2 | 0 | 2 | 4 | 0 | 26 | 1 | 27 | 0 | 9 | 0 | 9 | 41 |
| 8:00 AM | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 1 | 0 | 17 | 1 | 18 | 0 | 16 | 0 | 16 | 35 |
| 8:15 AM | 1 | 0 | 1 | 2 | 0 | 0 | 0 | 0 | 0 | 12 | 0 | 12 | 0 | 19 | 0 | 19 | 33 |
| 8:30 AM | 2 | 0 | 0 | 2 | 1 | 0 | 1 | 2 | 1 | 9 | 0 | 10 | 1 | 16 | 1 | 18 | 32 |
| 8:45 AM | 0 | 0 | 0 | 0 | 0 | 0 | 2 | 2 | 0 | 18 | 1 | 19 | 0 | 11 | 0 | 11 | 32 |
| Total Survey | 8 | 0 | 1 | 9 | 6 | 0 | 6 | 12 | 4 | 141 | 4 | 149 | 1 | 97 | 1 | 99 | 269 |

Heavy Vehicle Peak Hour Summary


| By <br> Movement | Northbound Ten Eyck Rd |  |  |  | Southbound Ten Eyck Rd |  |  |  | $\begin{gathered} \text { Eastbound } \\ \text { Hwy } 26 \end{gathered}$ |  |  |  | Westbound Hwy 26 |  |  |  | Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | L | T | R | Total | L | T | R | Total | L | T | R | Total | L | T | R | Total |  |
| Volume | 5 | 0 | 0 | 5 | 5 | 0 | 2 | 7 | 3 | 85 | 2 | 90 | 0 | 35 | 0 | 35 | 137 |
| PHF | 0.42 | 0.00 | 0.00 | 0.42 | 0.42 | 0.00 | 0.25 | 0.35 | 0.38 | 0.79 | 0.50 | 0.80 | 0.00 | 0.80 | 0.00 | 0.80 | 0.82 |

Heavy Vehicle Rolling Hour Summary
7:00 AM to 9:00 AM

| Interval Start Time | Northbound Ten Eyck Rd |  |  |  | Southbound Ten Eyck Rd |  |  |  | Eastbound Hwy 26 |  |  |  | Westbound Hwy 26 |  |  |  | Interval Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | L | T | R | Total | L | T | R | Total | L | T | R | Total | L | T | R | Total |  |
| 7:00 AM | 5 | 0 | 0 | 5 | 5 | 0 | 2 | 7 | 3 | 85 | 2 | 90 | 0 | 35 | 0 | 35 | 137 |
| 7:15 AM | 3 | 0 | 0 | 3 | 4 | 0 | 3 | 7 | 3 | 75 | 2 | 80 | 0 | 40 | 0 | 40 | 130 |
| 7:30 AM | 2 | 0 | 1 | 3 | 3 | 0 | 3 | 6 | 2 | 70 | 2 | 74 | 0 | 50 | 0 | 50 | 133 |
| 7:45 AM | 4 | 0 | 1 | 5 | 3 | 0 | 4 | 7 | 1 | 64 | 2 | 67 | 1 | 60 | T | 62 | 141 |
| 8:00 AM | 3 | 0 | 1 | 4 | 1 | 0 | 4 | 5 | 1 | 56 | 2 | 59 | 1 | 62 | 1 | 64 | 132 |


Total Vehicle Summary
Ten Eyck Rd \& Hwy 26
Wednesday, February 19, 2020
4:00 PM to 6:00 PM
5-Minute Interval Summary

| Interval Start Time | Northbound Ten Eyck Rd |  |  |  | Southbound Ten Eyck Rd |  |  |  | Eastbound Hwy 26 |  |  |  | Westbound Hwy 26 |  |  |  | Interval Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | L | T | R | Bikes | L | T | R | Bikes | L | T | R | Bikes | L | T | R | Bikes |  |
| 4:00 PM | 5 | 4 | 0 | 0 | 2 | 2 | 14 | 0 | 11 | 55 | 19 | 0 | 1 | 76 | 2 | 0 | 191 |
| 4:05 PM | 10 | 2 | 1 | 0 | 4 | 2 | 6 | 0 | 17 | 78 | 5 | 0 | 0 | 48 | 4 | 0 | 177 |
| 4:10 PM | 4 | 3 | 0 | 0 | 0 | 1 | 7 | 0 | 13 | 73 | 12 | 0 | 0 | 68 | 1 | 0 | 182 |
| 4:15 PM | 4 | 3 | 0 | 0 | 5 | 0 | 5 | 0 | 9 | 67 | 12 | 0 | 0 | 81 | 3 | 0 | 189 |
| 4:20 PM | 6 | 2 | 0 | 0 | 1 | 1 | 5 | 0 | 11 | 73 | 8 | 0 | 0 | 81 | 0 | 0 | 188 |
| 4:25 PM | 11 | 0 | 1 | 0 | 4 | 1 | 8 | 0 | 8 | 60 | 11 | 0 | 0 | 72 | 0 | 0 | 176 |
| 4:30 PM | 6 | 2 | 0 | 0 | 3 | 0 | 4 | 0 | 9 | 70 | 17 | 0 | 0 | 64 | 2 | 0 | 177 |
| 4:35 PM | 14 | 2 | 0 | 0 | 2 | 4 | 6 | 0 | 13 | 70 | 6 | 0 | 0 | 83 | 1 | 0 | 201 |
| 4:40 PM | 11 | 2 | 0 | 0 | 0 | 6 | 10 | 0 | 19 | 80 | 12 | 0 | 1 | 75 | 1 | 0 | 217 |
| 4:45 PM | 10 | 1 | 2 | 0 | 2 | 0 | 4 | 0 | 18 | 85 | 18 | 0 | 1 | 63 | 2 | 0 | 206 |
| 4:50 PM | 12 | 6 | 0 | 0 | 3 | 2 | 15 | 0 | 11 | 55 | 12 | 0 | 1 | 61 | 3 | 0 | 181 |
| 4:55 PM | 11 | 1 | 2 | 0 | 4 | 2 | 6 | 0 | 9 | 52 | 14 | 0 | 1 | 89 | 3 | 0 | 194 |
| 5:00 PM | 12 | 4 | 1 | 0 | 4 | 2 | 9 | 0 | 18 | 60 | 7 | 0 | 1 | 85 | 0 | 0 | 203 |
| 5:05 PM | 7 | 2 | 0 | 0 | 4 | 2 | 14 | 0 | 13 | 99 | 7 | 0 | 0 | 67 | 1 | 0 | 216 |
| 5:10 PM | 7 | 1 | 0 | 0 | 6 | 2 | 7 | 0 | 8 | 72 | 13 | 0 | 0 | 71 | 3 | 0 | 190 |
| 5:15 PM | 9 | 1 | 1 | 0 | 5 | 1 | 12 | 0 | 14 | 78 | 9 | 0 | 0 | 69 | 0 | 0 | 199 |
| 5:20 PM | 8 | 1 | 0 | 0 | 4 | 1 | 4 | 0 | 12 | 59 | 8 | 0 | 1 | 68 | 1 | 0 | 167 |
| 5:25 PM | 6 | 2 | 0 | 0 | 5 | 0 | 7 | 0 | 11 | 71 | 7 | 0 | 1 | 73 | 0 | 0 | 183 |
| 5:30 PM | 4 | 0 | 0 | 0 | 1 | 3 | 9 | 0 | 14 | 77 | 10 | 0 | 0 | 62 | 2 | 0 | 182 |
| 5:35 PM | 8 | 2 | 1 | 0 | 1 | 0 | 6 | 0 | 18 | 80 | 10 | 0 | 0 | 36 | 0 | 0 | 162 |
| 5:40 PM | 5 | 1 | 0 | 0 | 4 | 1 | 10 | 0 | 16 | 57 | 4 | 0 | 0 | 48 | 0 | 0 | 146 |
| 5:45 PM | 7 | 3 | 1 | 0 | 3 | 0 | 7 | 0 | 12 | 65 | 9 | 0 | 0 | 62 | 1 | 0 | 170 |
| 5:50 PM | 12 | 1 | 0 | 0 | 2 | 0 | 8 | 0 | 13 | 72 | 5 | 0 | 0 | 63 | 1 | 0 | 177 |
| 5:55 PM | 4 | 2 | 1 | 0 | 3 | 1 | 9 | 0 | 17 | 63 | 14 | 0 | 0 | 48 | 1 | 0 | 163 |
| Total Survey | 193 | 48 | 11 | 0 | 72 | 34 | 192 | 0 | 314 | 1,671 | 249 | 0 | 8 | 1,613 | 32 | 0 | 4,437 |


| Pedestrians <br> Crosswalk |  |  |  |
| :---: | :---: | :---: | :---: |
| North | South | East | West |
| 0 | 0 | 0 | 1 |
| 0 | 0 | 0 | 0 |
| 4 | 0 | 0 | 0 |
| 1 | 0 | 0 | 0 |
| 2 | 0 | 0 | 0 |
| 0 | 0 | 0 | 0 |
| 0 | 0 | 0 | 0 |
| 0 | 0 | 0 | 0 |
| 0 | 0 | 0 | 0 |
| 0 | 0 | 0 | 3 |
| 0 | 1 | 0 | 1 |
| 0 | 0 | 0 | 0 |
| 0 | 0 | 0 | 0 |
| 0 | 0 | 0 | 0 |
| 0 | 0 | 0 | 1 |
| 0 | 0 | 0 | 0 |
| 0 | 0 | 0 | 0 |
| 1 | 0 | 0 | 0 |
| 0 | 0 | 0 | 0 |
| 0 | 1 | 0 | 0 |
| 1 | 0 | 0 | 0 |
| 0 | 0 | 0 | 1 |
| 1 | 0 | 0 | 2 |
| 0 | 0 | 0 | 0 |
| 10 | 2 | 0 | 11 |

15-Minute Interval Summary

| Interval Start Time | Northbound Ten Eyck Rd |  |  |  | Southbound Ten Eyck Rd |  |  |  | Eastbound Hwy 26 |  |  |  | Westbound Hwy 26 |  |  |  | Interval Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | L | T | R | Bikes | L | T | R | Bikes | L | T | R | Bikes | L | T | R | Bikes |  |
| 4:00 PM | 19 | 9 | 1 | 0 | 6 | 5 | 27 | 0 | 41 | 206 | 36 | 0 | 1 | 192 | 7 | 0 | 550 |
| 4:15 PM | 21 | 5 | 1 | 0 | 10 | 2 | 18 | 0 | 28 | 200 | 31 | 0 | 0 | 234 | 3 | 0 | 553 |
| 4:30 PM | 31 | 6 | 0 | 0 | 5 | 10 | 20 | 0 | 41 | 220 | 35 | 0 | 1 | 222 | 4 | 0 | 595 |
| 4:45 PM | 33 | 8 | 4 | 0 | 9 | 4 | 25 | 0 | 38 | 192 | 44 | 0 | 3 | 213 | 8 | 0 | 581 |
| 5:00 PM | 26 | 7 | 1 | 0 | 14 | 6 | 30 | 0 | 39 | 231 | 27 | 0 | 1 | 223 | 4 | 0 | 609 |
| 5:15 PM | 23 | 4 | 1 | 0 | 14 | 2 | 23 | 0 | 37 | 208 | 24 | 0 | 2 | 210 | 1 | 0 | 549 |
| 5:30 PM | 17 | 3 | 1 | 0 | 6 | 4 | 25 | 0 | 48 | 214 | 24 | 0 | 0 | 146 | 2 | 0 | 490 |
| 5:45 PM | 23 | 6 | 2 | 0 | 8 | 1 | 24 | 0 | 42 | 200 | 28 | 0 | 0 | 173 | 3 | 0 | 510 |
| Total Survey | 193 | 48 | 11 | 0 | 72 | 34 | 192 | 0 | 314 | 1,671 | 249 | 0 | 8 | 1,613 | 32 | 0 | 4,437 |


| Pedestrians Crosswalk |  |  |  |
| :---: | :---: | :---: | :---: |
| North | South | East | West |
| 4 | 0 | 0 | 3 |
| 3 | 0 | 0 | 0 |
| 0 | 0 | 0 | 0 |
| 0 | 1 | 0 | 4 |
| 0 | 0 | 0 | 1 |
| 1 | 0 | 0 | 0 |
| 1 | 1 | 0 | 0 |
| 1 | 0 | 0 | 3 |
| 10 | 2 | 0 | 11 |

Peak Hour Summary


| $\begin{array}{\|c} \text { By } \\ \text { Movement } \end{array}$ | Northbound Ten Eyck Rd |  |  |  | Southbound Ten Eyck Rd |  |  |  | Eastbound Hwy 26 |  |  |  | Westbound Hwy 26 |  |  |  | otal |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | L | T | R | Total | L | T | R | Total | L | T | R | Total | L | T | R | Total |  |
| Volume | 116 | 24 | 7 | 147 | 38 | 23 | 100 | 161 | 151 | 854 | 134 | 1,139 | 5 | 880 | 16 | 901 |  |
| \%HV | 0.9\% | 8.3\% | 0.0\% | 2.0\% | 0.0\% | 0.0\% | 1.0\% | 0.6\% | 0.7\% | 3.3\% | 0.0\% | 2.5\% | 0.0\% | 5.2\% | 0.0\% | 5.1\% | 3.4\% |
| PHF | 0.83 | 0.55 | 0.44 | 0.75 | 0.63 | 0.58 | 0.76 | 0.76 | 76 | 0.86 | 0.76 | 0.89 | 0.42 | 0.91 | 0.5 | 0.91 | 0.9 |

## Rolling Hour Summary

 4:00 PM to 6:00 PM| $\begin{gathered} \hline \text { Interval } \\ \text { Start } \\ \text { Time } \\ \hline \end{gathered}$ | Northbound Ten Eyck Rd |  |  |  | Southbound Ten Eyck Rd |  |  |  | Eastbound Hwy 26 |  |  |  | Westbound Hwy 26 |  |  |  | Interval |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | L | T | R | Bikes | L | T | R | Bikes | L | T | R | Bikes | L | T | R | Bikes |  |
| 4:00 PM | 104 | 28 | 6 | 0 | 30 | 21 | 90 | 0 | 148 | 818 | 146 | 0 | 5 | 861 | 22 | 0 | 2,279 |
| 4:15 PM | 111 | 26 | 6 | 0 | 38 | 22 | 93 | 0 | 146 | 843 | 137 | 0 | 5 | 892 | 19 | 0 | 2,338 |
| 4:30 PM | 113 | 25 | 6 | 0 | 42 | 22 | 98 | 0 | 155 | 851 | 130 | 0 | 7 | 868 | 17 | 0 | 2,334 |
| 4:45 PM | 99 | 22 | 7 | 0 | 43 | 16 | 103 | 0 | 162 | 845 | 119 | 0 | 6 | 792 | 15 | 0 | 2,229 |
| 5:00 PM | 89 | 20 | 5 | 0 | 42 | 13 | 102 | 0 | 166 | 853 | 103 | 0 | 3 | 752 | 10 | 0 | 2.158 |


| Pedestrians <br> Crosswalk <br> North |  |  |  |
| :---: | :---: | :---: | :---: |
| 7 South | East | West |  |
| 7 | 1 | 0 | 7 |
| 1 | 1 | 0 | 5 |
| 1 | 1 | 0 | 5 |
| 2 | 2 | 0 | 5 |
| 3 | 1 | 0 | 4 |



Heavy Vehicle 15-Minute Interval Summary
4:00 PM to 6:00 PM

| $\begin{gathered} \hline \text { Interval } \\ \text { Start } \\ \text { Time } \end{gathered}$ | Northbound Ten Eyck Rd |  |  |  | Southbound Ten Eyck Rd |  |  |  | Eastbound Hwy 26 |  |  |  | Westbound Hwy 26 |  |  |  | Interval Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | L | T | R | Total | L | T | R | Total | L | T | R | Total | L | T | R | Total |  |
| 4:00 PM | 1 | 0 | 0 | 1 | 0 | 0 | 2 | 2 | 0 | 13 | 0 | 13 | 0 | 13 | 1 | 14 | 30 |
| 4:15 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 15 | 0 | 15 | 0 | 17 | 1 | 18 | 33 |
| 4:30 PM | 1 | 2 | 0 | 3 | 0 | 0 | 0 | 0 | 0 | 6 | 0 | 6 | 0 | 10 | 0 | 10 | 19 |
| 4:45 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 4 | 0 | 4 | 0 | 14 | 0 | 14 | 18 |
| 5:00 PM | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 1 | 1 | 6 | 0 | 7 | 0 | 10 | 0 | 10 | 18 |
| 5:15 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 1 | 0 | 6 | 0 | 6 | 7 |
| 5:30 PM | 1 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 4 | 0 | 4 | 0 | 4 | 0 | 4 | 9 |
| 5:45 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 5 | 0 | 6 | 0 | 7 | 0 | 7 | 13 |
| Total Survey | 3 | 2 | 0 | 5 | 0 | 0 | 3 | 3 | 2 | 54 | 0 | 56 | 0 | 81 | 2 | 83 | 147 |

Heavy Vehicle Peak Hour Summary

| By <br> Approach | Northbound Ten Eyck Rd |  |  | Southbound Ten Eyck Rd |  |  | Eastbound Hwy 26 |  |  | Westbound Hwy 26 |  |  | Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | In | Out | Total | In | Out | Total | In | Out | Total | In | Out | Total |  |
| Volume | 3 | 0 | 3 | 1 | 3 | 4 | 29 | 48 | 77 | 46 | 28 | 74 | 79 |
| PHF | 0.25 |  |  | 0.25 |  |  | 0.52 |  |  | 0.77 |  |  | 0.64 |


| By <br> Movement | Northbound Ten Eyck Rd |  |  |  | Southbound Ten Eyck Rd |  |  |  | Eastbound Hwy 26 |  |  |  | Westbound Hwy 26 |  |  |  | Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | L | T | R | Total | L | T | R | Total | L | T | R | Total | L | T | R | Total |  |
| Volume | 1 | 2 | 0 | 3 | 0 | 0 | 1 | 1 | 1 | 28 | 0 | 29 | 0 | 46 | 0 | 46 | 79 |
| PHF | 0.25 | 0.25 | 0.00 | 0.25 | 0.00 | 0.00 | 0.25 | 0.25 | 0.25 | 0.50 | 0.00 | 0.52 | 0.00 | 0.77 | 0.00 | 0.77 | 0.64 |

Heavy Vehicle Rolling Hour Summary 4:00 PM to 6:00 PM

| Interval Start Time | Northbound Ten Eyck Rd |  |  |  | Southbound Ten Eyck Rd |  |  |  | Eastbound Hwy 26 |  |  |  | Westbound Hwy 26 |  |  |  | Interval Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | L | T | R | Total | L | T | R | Total | L | T | R | Total | L | T | R | Total |  |
| 4:00 PM | 2 | 2 | 0 | 4 | 0 | 0 | 2 | 2 | 0 | 38 | 0 | 38 | 0 | 54 | 2 | 56 | 100 |
| 4:15 PM | 1 | 2 | 0 | 3 | 0 | 0 | 1 | 1 | 1 | 31 | 0 | 32 | 0 | 51 | 1 | 52 | 88 |
| 4:30 PM | 1 | 2 | 0 | 3 | 0 | 0 | 1 | 1 | 1 | 17 | 0 | 18 | 0 | 40 | 0 | 40 | 62 |
| 4:45 PM | 1 | 0 | 0 | 1 | 0 | 0 | 1 | 1 | 1 | 15 | 0 | 16 | 0 | 34 | 0 | 34 | 52 |
| 5:00 PM | 1 | 0 | 0 | 1 | 0 | 0 | 1 | 1 | 2 | 16 | 0 | 18 | 0 | 27 | 0 | 27 | 47 |



Ten Eyck Rd \& Pleasant St
Wednesday, February 19, 2020 7:00 AM to 9:00 AM
5-Minute Interval Summary

| Interval Start | Northbound Ten Eyck Rd |  |  |  | Southbound Ten Eyck Rd |  |  |  | Eastbound Pleasant St |  |  |  | Westbound Pleasant St |  |  |  | Interval Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Time | L | T | R | Bikes | L | T | R | Bikes | L | T | R | Bikes | L | T | R | Bikes |  |
| 7:00 AM | 1 | 5 | 2 | 0 | 0 | 11 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 20 |
| 7:05 AM | 1 | 2 | 0 | 0 | 0 | 15 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 20 |
| 7:10 AM | 1 | 0 | 0 | 0 | 0 | 18 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 21 |
| 7:15 AM | 2 | 3 | 1 | 0 | 0 | 19 | 4 | 0 | 0 | 0 | 2 | 0 | 0 | 0 | 0 | 0 | 31 |
| 7:20 AM | 3 | 1 | 1 | 0 | 0 | 16 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 23 |
| 7:25 AM | 3 | 4 | 0 | 0 | 0 | 11 | 1 | 0 | 4 | 0 | 2 | 0 | 0 | 0 | 0 | 0 | 25 |
| 7:30 AM | 1 | 6 | 2 | 0 | 0 | 9 | 3 | 0 | 0 | 0 | 2 | 0 | 0 | 0 | 0 | 0 | 23 |
| 7:35 AM | 1 | 4 | 0 | 0 | 0 | 9 | 2 | 0 | 4 | 0 | 3 | 0 | 0 | 0 | 0 | 0 | 23 |
| 7:40 AM | 0 | 11 | 1 | 0 | 0 | 10 | 0 | 0 | 2 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 25 |
| 7:45 AM | 1 | 3 | 0 | 0 | 0 | 13 | 2 | 0 | 3 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 23 |
| 7:50 AM | 1 | 4 | 1 | 0 | 0 | 4 | 1 | 0 | 1 | 0 | 5 | 0 | 0 | 0 | 0 | 0 | 17 |
| 7:55 AM | 1 | 3 | 1 | 0 | 0 | 8 | 2 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 16 |
| 8:00 AM | 0 | 1 | 2 | 0 | 0 | 6 | 1 | 0 | 3 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 14 |
| 8:05 AM | 0 | 2 | 1 | 0 | 0 | 6 | 1 | 0 | 0 | 0 | 1 | 0 | 1 | 0 | 0 | 0 | 12 |
| 8:10 AM | 0 | 3 | 0 | 0 | 0 | 13 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 17 |
| 8:15 AM | 0 | 5 | 1 | 0 | 0 | 12 | 2 | 0 | 1 | 0 | 2 | 0 | 0 | 1 | 0 | 0 | 24 |
| 8:20 AM | 0 | 6 | 0 | 0 | 0 | 8 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 16 |
| 8:25 AM | 2 | 3 | 1 | 0 | 0 | 13 | 1 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 21 |
| 8:30 AM | 0 | 5 | 2 | 0 | 0 | 13 | 0 | 0 | 2 | 0 | 3 | 0 | 0 | 0 | 0 | 0 | 25 |
| 8:35 AM | 0 | 4 | 0 | 0 | 0 | 8 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 13 |
| 8:40 AM | 1 | 4 | 1 | 0 | 1 | 15 | 2 | 0 | 0 | 1 | 4 | 0 | 0 | 0 | 1 | 0 | 30 |
| 8:45 AM | 0 | 6 | 1 | 0 | 0 | 11 | 2 | 0 | 0 | 0 | 3 | 0 | 0 | 0 | 0 | 0 | 23 |
| $8: 50 \mathrm{AM}$ | 0 | 6 | 1 | 0 | 0 | 10 | 0 | 0 | 0 | 1 | 2 | 0 | 2 | 0 | 0 | 0 | 22 |
| 8:55 AM | 0 | 3 | 0 | 0 | 0 | 10 | 0 | 0 | 0 | 1 | 1 | 0 | 0 | 0 | 0 | 0 | 15 |
| Total Survey | 19 | 94 | 19 | 0 | 1 | 268 | 28 | 0 | 23 | 3 | 38 | 0 | 4 | 1 | 1 | 0 | 499 |


| Pedestrians <br> Crosswalk |  |  |  |
| :---: | :---: | :---: | :---: |
| North | South | East | West |
| 0 | 0 | 0 | 0 |
| 0 | 0 | 0 | 0 |
| 0 | 0 | 0 | 0 |
| 0 | 0 | 0 | 0 |
| 0 | 0 | 0 | 0 |
| 0 | 0 | 0 | 0 |
| 0 | 0 | 0 | 0 |
| 0 | 0 | 0 | 0 |
| 0 | 0 | 0 | 0 |
| 0 | 0 | 0 | 0 |
| 0 | 0 | 0 | 0 |
| 0 | 0 | 0 | 0 |
| 0 | 0 | 0 | 0 |
| 0 | 0 | 0 | 0 |
| 0 | 0 | 0 | 0 |
| 0 | 0 | 0 | 0 |
| 0 | 0 | 0 | 0 |
| 0 | 0 | 0 | 0 |
| 0 | 0 | 0 | 0 |
| 0 | 0 | 0 | 0 |
| 0 | 0 | 0 | 0 |
| 0 | 0 | 0 | 0 |
| 0 | 0 | 0 | 0 |
| 0 | 0 | 0 | 0 |

15-Minute Interval Summary
7:00 AM to 9:00 AM

Peak Hour Summary


| $\begin{gathered} \mathrm{By} \\ \text { Movement } \end{gathered}$ | Northbound Ten Eyck Rd |  |  |  | Southbound Ten Eyck Rd |  |  |  | Eastbound Pleasant St |  |  |  | Westbound Pleasant St |  |  |  | Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | L | T | - | Tot | L | T | , | Total | L | T | R | Tot | L | T | R | Total |  |
| Volu | 16 | 46 | 9 | 71 | 0 | 143 | 18 | 161 | 15 | 0 | 20 | 35 | 0 | 0 | 0 |  |  |
| \%HV | 6.3\% | 2.2\% | 0.0\% | 2.8\% | 0.0\% | 1.4\% | 11.1\% | 2.5\% | 6.7\% | 0.0\% | 25.0\% | 17.1\% | 0.0\% | 0.0\% | $0.0 \%$ | 0.0 | 4.5\% |
| PHF | 0.50 | 0.55 | 0.75 | 0.68 | 0.00 | 0.67 | 0.75 | 0.68 | 0.42 | 0.00 | 0.71 | 0.58 | 0.00 | 0.00 | 0.0 | 0.0 | 0.8 |

Rolling Hour Summary
7:00 AM to 9:00 AM

| $\begin{gathered} \hline \text { Interval } \\ \text { Start } \\ \text { Time } \end{gathered}$ | Northbound Ten Eyck Rd |  |  |  | Southbound Ten Eyck Rd |  |  |  | Eastbound Pleasant St |  |  |  | Westbound Pleasant St |  |  |  | Interval Total | Pedestrians Crosswalk |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | L | T | R | Bikes | L | T | R | Bikes | L | T | R | Bikes | L | T | R | Bikes |  | North | South | East | West |
| 7:00 AM | 16 | 46 | 9 | 0 | 0 | 143 | 18 | 0 | 15 | 0 | 20 | 0 | 0 | 0 | 0 | 0 | 267 | 0 | 0 | 0 | 0 |
| 7:15 AM | 13 | 45 | 10 | 0 | 0 | 124 | 18 | 0 | 19 | 0 | 19 | 0 | 1 | 0 | 0 | 0 | 249 | 0 | 0 | 0 | 0 |
| 7:30 AM | 7 | 51 | 10 | 0 | 0 | 111 | 16 | 0 | 17 | 0 | 17 | 0 | 1 | 1 | 0 | 0 | 231 | 0 | 0 | 0 | 0 |
| 7:45 AM | 6 | 43 | 10 | 0 | 1 | 119 | 13 | 0 | 13 | 1 | 18 | 0 | 2 | 1 | 1 | 0 | 228 | 0 | 0 | 0 | 0 |
| 8:00 AM | 3 | 48 | 10 | 0 | 1 | 125 | 10 | 0 | 8 | 3 | 18 | 0 | 4 | 1 | 1 | 0 | 232 | 0 | 0 | 0 | 0 |

Heavy Vehicle Summary


Ten Eyck Rd \& Pleasant St
Wednesday, February 19, 2020
7:00 AM to 9:00 AM

Heavy Vehicle 5-Minute Interval Summary
7:00 AM to 9:00 AM

| Interval Start Time | Northbound Ten Eyck Rd |  |  |  | Southbound Ten Eyck Rd |  |  |  | Eastbound Pleasant St |  |  |  | Westbound Pleasant St |  |  |  | Interval Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | L | T | R | Total | L | T | R | Total | L | T | R | Total | L | T | R | Total |  |
| 7:00 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 7:05 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 1 | 0 | 0 | 0 | 0 | 1 |
| 7:10 AM | 1 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 |
| 7:15 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 1 | 0 | 0 | 0 | 0 | 1 |
| 7:20 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 7:25 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 7:30 AM | 0 | 1 | 0 | 1 | 0 | 0 | 1 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 2 |
| 7:35 AM | 0 | 0 | 0 | 0 | 0 | 1 | 1 | 2 | 1 | 0 | 1 | 2 | 0 | 0 | 0 | 0 | 4 |
| 7:40 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 7:45 AM | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 1 | 0 | 0 | 1 | 1 | 0 | 0 | 0 | 0 | 2 |
| 7:50 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 1 | 0 | 0 | 0 | 0 | 1 |
| 7:55 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 8:00 AM | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 1 | 1 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 2 |
| 8:05 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 8:10 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 8:15 AM | 0 | 0 | 0 | 0 | 0 | 0 | 2 | 2 | 1 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 3 |
| 8:20 AM | 0 | 1 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 |
| 8:25 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 8:30 AM | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 0 | 1 | 1 | 0 | 0 | 0 | 0 | 3 |
| 8:35 AM | 0 | 1 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 |
| 8:40 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 8:45 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 8:50 AM | 0 | 0 | 0 | 0 | 0 | 2 | 0 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 2 |
| 8:55 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Total Survey | 1 | 4 | 0 | 5 | 0 | 6 | 4 | 10 | 3 | 0 | 6 | 9 | 0 | 0 | 0 | 0 | 24 |

Heavy Vehicle 15-Minute Interval Summary 7:00 AM to 9:00 AM

| $\begin{gathered} \hline \text { Interval } \\ \text { Start } \\ \text { Time } \end{gathered}$ | Northbound Ten Eyck Rd |  |  |  | Southbound Ten Eyck Rd |  |  |  | Eastbound Pleasant St |  |  |  | Westbound Pleasant St |  |  |  | Interval Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | L | T | R | Total | L | T | R | Total | L | T | R | Total | L | T | R | Total |  |
| 7:00 AM | 1 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 1 | 0 | 0 | 0 | 0 | 2 |
| 7:15 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 1 | 0 | 0 | 0 | 0 | 1 |
| 7:30 AM | 0 | 1 | 0 | 1 | 0 | 1 | 2 | 3 | 1 | 0 | 1 | 2 | 0 | 0 | 0 | 0 | 6 |
| 7:45 AM | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 1 | 0 | 0 | 2 | 2 | 0 | 0 | 0 | 0 | 3 |
| 8:00 AM | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 1 | 1 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 2 |
| 8:15 AM | 0 | 1 | 0 | 1 | 0 | 0 | 2 | 2 | 1 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 4 |
| 8:30 AM | 0 | 2 | 0 | 2 | 0 | 1 | 0 | 1 | 0 | 0 | 1 | 1 | 0 | 0 | 0 | 0 | 4 |
| 8:45 AM | 0 | 0 | 0 | 0 | 0 | 2 | 0 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 2 |
| Total Survey | 1 | 4 | 0 | 5 | 0 | 6 | 4 | 10 | 3 | 0 | 6 | 9 | 0 | 0 | 0 | 0 | 24 |

Heavy Vehicle Peak Hour Summary


| By <br> Movement | Northbound Ten Eyck Rd |  |  |  | Southbound Ten Eyck Rd |  |  |  | Eastbound Pleasant St |  |  |  | Westbound Pleasant St |  |  |  | Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | L | T | R | Total | L | T | R | Total | L | T | R | Total | L | T | R | Total |  |
| Volume | 1 | 1 | 0 | 2 | 0 | 2 | 2 | 4 | 1 | 0 | 5 | 6 | 0 | 0 | 0 | 0 | 12 |
| PHF | 0.25 | 0.25 | 0.00 | 0.50 | 0.00 | 0.25 | 0.25 | 0.33 | 0.25 | 0.00 | 0.63 | 0.50 | 0.00 | 0.00 | 0.00 | 0.00 | 0.50 |

Heavy Vehicle Rolling Hour Summary
7:00 AM to 9:00 AM

| Interval Start Time | Northbound Ten Eyck Rd |  |  |  | Southbound Ten Eyck Rd |  |  |  | Eastbound Pleasant St |  |  |  | Westbound Pleasant St |  |  |  | Interval Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | L | T | R | Total | L | T | R | Total | L | T | R | Total | L | T | R | Total |  |
| 7:00 AM | 1 | 1 | 0 | 2 | 0 | 2 | 2 | 4 | 1 | 0 | 5 | 6 | 0 | 0 | 0 | 0 | 12 |
| 7:15 AM | 0 | 1 | 0 | 1 | 0 | 3 | 2 | 5 | 2 | 0 | 4 | 6 | 0 | 0 | 0 | 0 | 12 |
| 7:30 AM | 0 | 2 | 0 | 2 | 0 | 3 | 4 | 7 | 3 | 0 | 3 | 6 | 0 | 0 | 0 | 0 | 15 |
| 7:45 AM | 0 | 3 | 0 | 3 | 0 | 3 | 2 | 5 | 2 | 0 | 3 | 5 | 0 | 0 | 0 | 0 | 13 |
| 8:00 AM | 0 | 3 | 0 | 3 | 0 | 4 | 2 | 6 | 2 | 0 | 1 | 3 | 0 | 0 | 0 | 0 | 12 |


Total Vehicle Summary

Ten Eyck Rd \& Pleasant St
Wednesday, February 19, 2020 4:00 PM to 6:00 PM
5-Minute Interval Summary

| $\begin{array}{\|c\|} \hline \text { Interval } \\ \text { Start } \\ \text { Time } \end{array}$ | Northbound Ten Eyck Rd |  |  |  | Southbound Ten Eyck Rd |  |  |  | Eastbound Pleasant St |  |  |  | Westbound Pleasant St |  |  |  | Interval Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | L | T | R | Bikes | L | T | R | Bikes | L | T | R | Bikes | L | T | R | Bikes |  |
| 4:00 PM | 0 | 9 | 1 | 0 | 0 | 9 | 1 | 0 | 2 | 0 | 4 | 0 | 2 | 0 | 0 | 0 | 28 |
| 4:05 PM | 1 | 20 | 2 | 0 | 0 | 5 | 2 | 0 | 1 | 0 | 2 | 0 | 0 | 2 | 2 | 0 | 37 |
| 4:10 PM | 1 | 10 | 2 | 0 | 0 | 7 | 2 | 0 | 5 | 0 | 3 | 0 | 1 | 0 | 1 | 0 | 32 |
| 4:15 PM | 2 | 11 | 1 | 0 | 0 | 8 | 0 | 0 | 4 | 1 | 1 | 0 | 0 | 2 | 0 | 0 | 30 |
| 4:20 PM | 1 | 9 | 2 | 0 | 0 | 7 | 0 | 0 | 2 | 1 | 1 | 0 | 0 | 1 | 0 | 0 | 24 |
| 4:25 PM | 0 | 11 | 0 | 0 | 0 | 8 | 0 | 0 | 7 | 0 | 4 | 0 | 1 | 0 | 0 | 0 | 31 |
| 4:30 PM | 1 | 14 | 1 | 0 | 0 | 7 | 0 | 0 | 5 | 0 | 3 | 0 | 0 | 0 | 0 | 0 | 31 |
| 4:35 PM | 1 | 16 | 3 | 0 | 0 | 12 | 0 | 0 | 4 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 37 |
| 4:40 PM | 0 | 22 | 1 | 0 | 0 | 6 | 4 | 0 | 1 | 0 | 2 | 0 | 1 | 0 | 0 | 0 | 37 |
| 4:45 PM | 4 | 17 | 1 | 0 | 0 | 9 | 0 | 0 | 5 | 0 | 1 | 0 | 2 | 0 | 1 | 0 | 40 |
| 4:50 PM | 1 | 12 | 2 | 0 | 0 | 18 | 0 | 0 | 3 | 0 | 6 | 0 | 2 | 2 | 0 | 0 | 46 |
| 4:55 PM | 0 | 11 | 0 | 0 | 0 | 10 | 2 | 0 | 3 | 0 | 5 | 0 | 0 | 0 | 0 | 0 | 31 |
| 5:00 PM | 0 | 19 | 1 | 0 | 0 | 10 | 0 | 0 | 4 | 1 | 5 | 0 | 1 | 0 | 0 | 0 | 41 |
| 5:05 PM | 1 | 12 | 4 | 0 | 0 | 14 | 2 | 0 | 5 | 0 | 4 | 0 | 0 | 0 | 0 | 0 | 42 |
| 5:10 PM | 0 | 14 | 2 | 0 | 0 | 10 | 3 | 0 | 3 | 0 | 3 | 0 | 3 | 0 | 0 | 0 | 38 |
| 5:15 PM | 2 | 9 | 1 | 0 | 0 | 6 | 0 | 0 | 3 | 1 | 3 | 0 | 0 | 0 | 0 | 0 | 25 |
| 5:20 PM | 1 | 14 | 2 | 0 | 0 | 9 | 0 | 0 | 4 | 0 | 2 | 0 | 3 | 0 | 0 | 0 | 35 |
| 5:25 PM | 0 | 14 | 0 | 0 | 0 | 7 | 1 | 0 | 1 | 0 | 2 | 0 | 0 | 0 | 0 | 0 | 25 |
| 5:30 PM | 0 | 17 | 1 | 0 | 0 | 12 | 0 | 0 | 2 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 33 |
| 5:35 PM | 0 | 14 | 2 | 0 | 0 | 4 | 0 | 0 | 3 | 0 | 2 | 0 | 2 | 0 | 0 | 0 | 27 |
| 5:40 PM | 1 | 16 | 2 | 0 | 1 | 10 | 0 | 0 | 1 | 0 | 4 | 0 | 1 | 0 | 0 | 0 | 36 |
| 5:45 PM | 0 | 12 | 1 | 0 | 0 | 8 | 1 | 0 | 2 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 25 |
| 5:50 PM | 0 | 12 | 2 | 0 | 0 | 8 | 1 | 0 | 1 | 0 | 4 | 0 | 1 | 1 | 1 | 0 | 31 |
| 5:55 PM | 0 | 13 | 2 | 0 | 0 | 7 | 1 | 0 | 3 | 0 | 4 | 0 | 1 | 0 | 0 | 0 | 31 |
| Total Survey | 17 | 328 | 36 | 0 | 1 | 211 | 20 | 0 | 74 | 4 | 68 | 0 | 21 | 8 | 5 | 0 | 793 |


| Pedestrians <br> Crosswalk |  |  |  |
| :---: | :---: | :---: | :---: |
| North | South | East | West |
| 0 | 0 | 0 | 0 |
| 0 | 0 | 0 | 0 |
| 0 | 0 | 0 | 0 |
| 0 | 0 | 0 | 0 |
| 0 | 0 | 0 | 0 |
| 0 | 0 | 0 | 0 |
| 0 | 0 | 0 | 0 |
| 0 | 0 | 0 | 0 |
| 0 | 0 | 0 | 0 |
| 0 | 0 | 0 | 0 |
| 0 | 0 | 0 | 0 |
| 0 | 0 | 0 | 0 |
| 0 | 0 | 0 | 0 |
| 0 | 0 | 0 | 0 |
| 0 | 0 | 0 | 0 |
| 0 | 0 | 0 | 0 |
| 0 | 0 | 0 | 0 |
| 0 | 0 | 0 | 0 |
| 0 | 0 | 0 | 0 |
| 0 | 0 | 0 | 0 |
| 0 | 0 | 0 | 0 |
| 0 | 0 | 0 | 0 |
| 0 | 0 | 0 | 0 |
| 0 | 0 | 0 | 0 |

15-Minute Interval Summary 4:00 PM to 6:00 PM

Peak Hour Summary


| By <br> Movement | Northbound Ten Eyck Rd |  |  |  | Southbound Ten Eyck Rd |  |  |  | Eastbound Pleasant St |  |  |  | Westbound Pleasant St |  |  |  | Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | L | T | R | Total | L | T | R | Total | L | T | R | Total | L | T | R | Total |  |
| Volume | 11 | 171 | 18 | 200 | 0 | 119 | 11 | 130 | 47 | 2 | 39 | 88 | 13 | 2 | 1 | 16 | 434 |
| \%HV | 0.0\% | 1.8\% | 0.0\% | 1.5\% | 0.0\% | 0.8\% | 9.1\% | 1.5\% | 0.0\% | 0.0\% | 0.0\% | 0.0\% | 0.0\% | 0.0\% | 0.0\% | 0.0\% | 1.2\% |
| PHF | 0.55 | 0.78 | 0.64 | 0.77 | 0.00 | 0.78 | 0.55 | 0.81 | 0.73 | 0.50 | 0.61 | 0.81 | 0.54 | 0.25 | 0.25 | 0.50 | 0.88 |

Rolling Hour Summary 4:00 PM to 6:00 PM

| $\begin{gathered} \hline \text { Interval } \\ \text { Start } \\ \text { Time } \end{gathered}$ | Northbound Ten Eyck Rd |  |  |  | Southbound Ten Eyck Rd |  |  |  | Eastbound Pleasant St |  |  |  | Westbound Pleasant St |  |  |  | Interval Total | Pedestrians Crosswalk |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | L | T | R | Bikes | L | T | R | Bikes | L | T | R | Bikes | L | T | R | Bikes |  | North | South | East | West |
| 4:00 PM | 12 | 162 | 16 | 0 | 0 | 106 | 11 | 0 | 42 | 2 | 33 | 0 | 9 | 7 | 4 | 0 | 404 | 0 | 0 | 0 | 0 |
| 4:15 PM | 11 | 168 | 18 | 0 | 0 | 119 | 11 | 0 | 46 | 3 | 36 | 0 | 10 | 5 | 1 | 0 | 428 | 0 | 0 | 0 | 0 |
| 4:30 PM | 11 | 174 | 18 | 0 | 0 | 118 | 12 | 0 | 41 | 2 | 37 | 0 | 12 | 2 | 1 | 0 | 428 | 0 | 0 | 0 | 0 |
| 4:45 PM | 10 | 169 | 18 | 0 | 1 | 119 | 8 | 0 | 37 | 2 | 38 | 0 | 14 | 2 | 1 | 0 | 419 | 0 | 0 | 0 | 0 |
| 5:00 PM | 5 | 166 | 20 | 0 | 1 | 105 | 9 | 0 | 32 | 2 | 35 | 0 | 12 | 1 | 1 | 0 | 389 | 0 | 0 | 0 | 0 |



Heavy Vehicle 15-Minute Interval Summary
4:00 PM to 6:00 PM

| Interval Start Time | Northbound Ten Eyck Rd |  |  |  | Southbound Ten Eyck Rd |  |  |  | Eastbound Pleasant St |  |  |  | Westbound Pleasant St |  |  |  | Interval Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | L | T | R | Total | L | T | R | Total | L | T | R | Total | L | T | R | Total |  |
| 4:00 PM | 0 | 1 | 0 | 1 | 0 | 3 | 0 | 3 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 4 |
| 4:15 PM | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 |
| 4:30 PM | 0 | 2 | 0 | 2 | 0 | 0 | 1 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 3 |
| 4:45 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 5:00 PM | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 2 |
| 5:15 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 5:30 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 5:45 PM | 0 | 1 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 |
| Total Survey | 0 | 5 | 0 | 5 | 0 | 5 | 1 | 6 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 11 |

Heavy Vehicle Peak Hour Summary


| By <br> Movement | Northbound Ten Eyck Rd |  |  |  | Southbound Ten Eyck Rd |  |  |  | Eastbound Pleasant St |  |  |  | Westbound Pleasant St |  |  |  | Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | L | T | R | Total | L | T | R | Total | L | T | R | Total | L | T | R | Total |  |
| Volume | 0 | 3 | 0 | 3 | 0 | 1 | 1 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 5 |
| PHF | 0.00 | 0.38 | 0.00 | 0.38 | 0.00 | 0.25 | 0.25 | 0.50 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.00 | 0.42 |

Heavy Vehicle Rolling Hour Summary
4:00 PM to 6:00 PM

| Interval Start Time | Northbound Ten Eyck Rd |  |  |  | Southbound Ten Eyck Rd |  |  |  | Eastbound Pleasant St |  |  |  | Westbound Pleasant St |  |  |  | Interval Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | L | T | R | Total | L | T | R | Total | L | T | R | Total | L | T | R | Total |  |
| 4:00 PM | 0 | 3 | 0 | 3 | 0 | 4 | 1 | 5 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 8 |
| 4:15 PM | 0 | 3 | 0 | 3 | 0 | 2 | 1 | 3 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 6 |
| 4:30 PM | 0 | 3 | 0 | 3 | 0 | 1 | 1 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 5 |
| 4:45 PM | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 2 |
| 5:00 PM | 0 | 2 | 0 | 2 | 0 | 1 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 3 |



| Mt. Hood | Highway No. 26 (US 26) |  |  | ODOT T | Data |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| MP | Location | 2013 | 2014 | 2015 | 2016 | 2017 | 2018 |
| 24.59 | EB, just west of Ten Eyck Rd | 12,500 | 12,600 | 11,200 | 11,500 | 11,400 | 12,500 |
| 24.61 | WB, just west of Ten Eyck Rd | 11,600 | 11,700 | 11,700 | 12,100 | 12,000 | 12,600 |
| 25.10 | $E B+W B$, just west of Langensand Rd | 16,900 | 17,100 | 18,000 | 18,500 | 18,400 | 20,700 |
|  | Totals | 41,000 | 41,400 | 40,900 | 42,100 | 41,800 | 45,800 |
|  | Annual Growth from 2013 |  | 1.0\% | -0.1\% | 0.9\% | 0.5\% | 2.3\% |


| SEASONAL TREND TABLE (Updated: 6/26/19) |  |  |  |  |
| :--- | :---: | :---: | :---: | :---: |
| TREND | 1-Feb | 15-Feb | 1-Mar | Seasonal Trend <br> Peak Period <br> Factor |
| (COMMUTER | 1.08 | 1.06 | 1.04 | 0.94 |
| RECREATIONAL SUMMER WINTER | 1.02 | 1.04 | 1.05 | 0.70 |
| SUMMER | 1.24 | 1.21 | 1.15 | 0.83 |

*Seasonal Trend Table factors are based on previous year ATR data. The table is updated yearly. *Grey shading indicates months were seasonal factor is greater than $30 \%$.

| Commuter, Recreational Summer-Winter, \& Summer | 1.11 | 0.83 |
| :--- | :---: | :---: |
|  | $\mathbf{1 . 3 4}$ |  |
| Commuter \& Summer | 1.14 | 0.89 |
| Recreational Summer-Winter only | $\mathbf{1 . 2 8}$ |  |
|  | 1.04 | 0.70 |
| Commuter \& Recreational Summer-Winter | $\mathbf{1 . 4 9}$ |  |
|  | 1.05 | 0.82 |

[^4]Appendix B
Trip Generation Calculations

## Detailed Land Use Data

## For 9.61000 Sq. Ft. GFA of CLINIC 1

$$
\text { ( } 630 \text { ) Clinic }
$$

|  |  |
| :--- | :--- |
| Project: Sandy Medical Clinic | Open Date: |
| $3 / 3 / 2020$ |  |
| Analysis Date: | $3 / 3 / 2020$ |


| Day / Period | $\begin{aligned} & \text { Total } \\ & \text { Trips } \\ & \hline \end{aligned}$ | Pass-By Trips | $\begin{aligned} & \text { Avg } \\ & \text { Rate } \end{aligned}$ | $\begin{gathered} \text { Min } \\ \text { Rate } \\ \hline \end{gathered}$ | $\begin{aligned} & \text { Max } \\ & \text { Rate } \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { Std } \\ & \text { Dev } \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { Avg } \\ & \text { Size } \\ & \hline \end{aligned}$ | $\begin{gathered} \% \\ \text { Enter } \end{gathered}$ | $\begin{gathered} \% \\ \text { Exit } \\ \hline \end{gathered}$ | Use Eq. | Equation | R2 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Weekday Average Daily Trips | 366 | 0 | 38.16 | 25.25 | 86.21 | 30.18 | 21 | 50 | 50 | False |  |  |
| Source: Trip Generation Manual 10th Edition |  |  |  |  |  |  |  |  |  |  |  |  |
| Weekday AM Peak Hour of Adjacent Street Traffic | 35 | 0 | 3.69 | 2.27 | 9.36 | 2.82 | 21 | 78 | 22 | False |  |  |
| Source: Trip Generation Manual 10th Edition |  |  |  |  |  |  |  |  |  |  |  |  |
| Weekday PM Peak Hour of Adjacent Street Traffic | 31 | 0 | 3.28 | 1.93 | 7 | 1.84 | 18 | 29 | 71 | False | $\operatorname{Ln}(\mathrm{T})=0.72 \operatorname{Ln}(\mathrm{X})+1.97$ | 0.7 |

## Detailed Land Use Data

For 6.3 1000 Sq. Ft. GFA of WAREHOUSE 1

$$
\text { ( } 150 \text { ) Warehousing }
$$

Project: Sandy Medical Clinic $\quad$| Open Date: |
| ---: |
| $3 / 3 / 2020$ |
| Analysis Date: |
| $3 / 3 / 2020$ |

| Day / Period | $\begin{aligned} & \text { Total } \\ & \text { Trips } \end{aligned}$ | Pass-By Trips | Avg Rate | Min <br> Rate | $\begin{aligned} & \text { Max } \\ & \text { Rate } \end{aligned}$ | $\begin{aligned} & \text { Std } \\ & \text { Dev } \end{aligned}$ | $\begin{gathered} \text { Avg } \\ \text { Size } \\ \hline \end{gathered}$ | $\begin{gathered} \% \\ \text { Enter } \end{gathered}$ | $\begin{gathered} \% \\ \text { Exit } \end{gathered}$ | Use <br> Eq. | Equation | R2 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Weekday Average Daily Trips | 11 | 0 | 1.74 | 0.15 | 16.93 | 1.55 | 285 | 50 | 50 | False | $\mathrm{T}=1.58(\mathrm{X})+45.54$ | 0.93 |
| Source: Trip Generation Manual 10th Edition |  |  |  |  |  |  |  |  |  |  |  |  |
| Weekday AM Peak Hour of Adjacent Street Traffic <br> Source: Trip Generation Manual 10th Edition | 1 | 0 | 0.17 | 0.02 | 1.93 | 0.2 | 451 | 77 | 23 | False | $\mathrm{T}=0.12(\mathrm{X})+25.32$ | 0.69 |
| Weekday PM Peak Hour of Adjacent Street Traffic | 1 | 0 | 0.19 | 0.01 | 1.8 | 0.18 | 400 | 27 | 73 | False | $\mathrm{T}=0.12(\mathrm{X})+27.82$ | 0.65 |



## John A. Manix

| From: | Wilson, James [jwilson2@clackamas.us](mailto:jwilson2@clackamas.us) |
| :--- | :--- |
| Sent: | Tuesday, February 18, 2020 3:18 PM |
| To: | John A. Manix |
| Cc: | Cockrell, , Eeborah; Kelly, Steve |
| Subject: | RE: PBS Engineers - Traffic Study Question(s) |

Here are estimates for staff members:
trips originate from within the City limits of Sandy
$50 \%$ trips originate from the west of Sandy
\% trips originate from the north of Sandy
\% trips originate from the east of Sandy
$9 \%$ trips originate from the south of Sandy

James
From: John A. Manix [Manix@pbsusa.com](mailto:Manix@pbsusa.com)
Sent: Tuesday, February 18, 2020 10:52 AM
To: Wilson, James [jwilson2@clackamas.us](mailto:jwilson2@clackamas.us)
c: Cockrell, Deborah [DCockrell@clackamas.us](mailto:DCockrell@clackamas.us); Kelly, Steve [SteveKel@clackamas.us](mailto:SteveKel@clackamas.us)
ubject: RE: PBS Engineers - Traffic Study Question(s)
James: This is good. Thank you
I will assume that the origin of staff trips will be about the same. If you have anything similar for staff trips that will be helpful
John Manix, PE \| Senior Traffic Engineer \| PBS Vancouver \| 360.607.1854 (cell)
From: Wilson, James [iwilson2@clackamas.us](mailto:iwilson2@clackamas.us)
ent: Tuesday, February 18, 2020 10:36 AM
To: John A. Manix [Manix@pbsusa.com](mailto:Manix@pbsusa.com)
C: Cockrell, Deborah [DCockrell@clackamas.us](mailto:DCockrell@clackamas.us); Kelly, Steve [SteveKel@clackamas.us](mailto:SteveKel@clackamas.us)
Subject: RE: PBS Engineers - Traffic Study Question(s)
Hi John
ere are estimates of where patients and clients of the new clinic will originate.
These are based on current patient demographics, information from the Health Resources and Services Administration (HRSA), and information from Medicaid / Oregon Health Plan insurance networks.
If you need anything more, please let me know.
$5 \%$ trips originate from within the City limits of Sandy
\% trips originate from the west of Sandy (Kelso, Boring
$5 \%$ trips originate from the north of Sandy
2\% trips originate from the east of Sandy (Mt Hood Village, and Mountain Communities)
$30 \%$ trips originate from the south of Sandy (Estacada / Eagle Creek)

## Have a great morning

James
JMES WILSON
CHIEF OPERATIONS OFFICER
CLACKAMAS HE
$503-655-8697$
EXEETIONAL CAREFOR THE WHOLE PERSON DEINERED WITH DIGNITY AND RESPECT FOR ALL
From: Cockrell, Deborah [DCockrell@clackamas.us](mailto:DCockrell@clackamas.us)
Sent: Tuesday, February 18, 2020 9:30 AM
o: Wilson, James [jwilson2@clackamas.us](mailto:jwilson2@clackamas.us)
FYI data needed
From: John A. Manix [mailto:Manix@pbsusa.com
Fent: Tuesday, February 18, 2020 8:39 AM
Sen. Tuesday, February $18,20208.39$ AM
Subject: RE: PBS Engineers - Traffic Study Question(s)
Deborah: I understand the data will not be easy to produce. If you or the staff can give me a rough estimate of the origin of patrons and/or staff, I would appreciate the effort. It would be best to revise my estimate below in the February 13,2020 email.
John Manix, PE | Senior Traffic Engineer \| PBS Vancouver \| 360.607.1854 (cell)
rom: Cockrell, Deborah [DCockrell@clackamas.us](mailto:DCockrell@clackamas.us)
sent: Monday, February 17, 2020 12:42 PM
o: John A. Manix [Manix@pbsusa.com](mailto:Manix@pbsusa.com)
CC: Kelly, Steve [SteveKel@clackamas.us](mailto:SteveKel@clackamas.us); Wilson, James [jwilson2@clackamas.us](mailto:jwilson2@clackamas.us)
Subject: RE: PBS Engineers - Traffic Study Question(s)
Unfortunately this data will not be easy to produce. We will have to survey staff. JAMES: Can you pull this data for us?
Also, dental is the only new traffic. BH and PC already have a presence in Sandy

## rom: John A. Mani]

Sent: Thursday, February 13, 2020 8:59 AM
To: Cockrell, Deborah <DCockrell@clackamas.us
c: Kelly, Steve [SteveKel@clackamas.us](mailto:SteveKel@clackamas.us)
Subject: RE: PBS Engineers - Traffic Study Question(s)
Deborah: Thank you. This is good information but more important is the were in the community do the trips to your clinic come from. Please edit my estimate below based on your experience.
For example do most patients and staff live in the Sandy or do they live north, east, west or south of town. This is not an exact process but based on residential density and my experience with the community, here is my estimate
$50 \%$ trips originate from within the City limits of Sandy,
$10 \%$ trips originate from the west of Sandy
\% trips orisinate from the north of Sandy
$20 \%$ trips originate from the east of Sandy
$10 \%$ trips originate from the south of Sand

## For the proposed clinic at Hwy 26 and Ten Eyke Road, I need to know which direction the staff and patience approach the clinic. Feel free to call if you have questions.

Thanks
John Manix, PE
Senior Traffic Ensineer
PBS
15 W 6 St St, Suite 601, Vancouver, WA 98660
office: 30.6095 .388 | direct: 360.567 .2117 | cell: 360.607 .185 . ithe. 360.695 .3 .3888 id

』PBS

From: Kelly, Steve [SteveKel@clackamas.us](mailto:SteveKel@clackamas.us)
sent: Wednesday, February 12, 2020 1:20 PM
o: John A. Manix [Manix@pbsusa.com](mailto:Manix@pbsusa.com)
Subject: FW: PBS Engineers - Traffic Study Question(s)
ohn,
Here is what I got from the Director of all of County Health Clinics.
See below, sir.
Steve Kelly, Project Coordinator
Clackamas County Community Development
2051 Kaen Road Suite 245
Oregon City, OR 97045
503. 650 - 5665
stevekel@clackamas.us
From: Cockrell, Deborah [DCockrell@clackamas.us](mailto:DCockrell@clackamas.us)
Sent: Tuesday, February 11, 2020 1:08 PM
To: Kelly, Steve [SteveKel@clackamas.us](mailto:SteveKel@clackamas.us)
Subject: RE: PBS Engineers - Traffic Study Question
MD will see 18 people per day, 4 days a week.
DMD will see 14 people per day, 4 days a week.
Probably 40 people per day for the therapists.
From: Kelly, Steve
Sent: Tuesday, February 11, 2020 11:33 AM
To: Cockrell, Deborah <DCockrell @clackamas us
Subject: PBS Engineers - Traffic Study Question(s)
Importance: High

## John Manix of PBS.

He is getting me a letter with a price today he said.
will provide him our Professional Services Contract and use his letter as an Exhibit A.
Thanks.

Steve Kelly, Project Coordinator
Clackamas County Community Development
2051 Kaen Road, Suite 245
Oregon City, OR 97045
503.650 .5665
stevekel@clackamas.us

NOTE: This message was trained as non-spam. If this is wrong, please correct the training as soon as possible.

Appendix C
Level of Service Calculations

| Critical Intersection Volume-to-Capacity Ratio |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26) |  |  |  |  |  |  |
| 2020 Existing Conditions |  |  | Weekday AM Peak Hour |  |  |  |
|  | EB |  | WB |  | NB | SB |
| Lane Group | EBL | EBT | WBL | WBTR | NBLTR | SBLTR |
| Adj Flow Rate (veh/h) | 59 | 904 | 0 | 1034 | 152 | 69 |
| Sat Flow (veh/h) | 1589 | 2988 | 1667 | 3269 | 1469 | 1618 |
| Critical Flow Ratios | 0.04 | 0.30 | 0.00 | 0.32 | 0.10 | 0.04 |
| critical critical critical |  |  |  |  |  |  |
| Sum of Critical Flow Ratios | 0.46 |  |  |  |  |  |
| Cycle Length | 64.4 | seconds |  |  |  |  |
| Lost Time per Phase |  | seconds |  |  |  |  |
| Total Lost Time |  | seconds |  |  |  |  |
| Critical Intersection V/C Ratio: | 0.56 |  |  |  |  |  |

Method follows ODOT Analysis Procedures Manual, Version 2, Chapter 13.4.4.


Method follows ODOT Analysis Procedures Manual, Version 2, Chapter 13.4.4.

| Critical Intersection Volume-to-Capacity Ratio |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26) |  |  |  |  |  |  |
| 2022 With Project Trips |  |  | Weekday AM Peak Hour |  |  |  |
|  | EB |  | WB |  | NB | SB |
| Lane Group | EBL | EBT | WBL | WBTR | NBLTR | SBLTR |
| Adj Flow Rate (veh/h) | 87 | 940 | 0 | 1080 | 157 | 73 |
| Sat Flow (veh/h) | 1589 | 2988 | 1641 | 3267 | 1447 | 1595 |
| Critical Flow Ratios | 0.05 | 0.31 | 0.00 | 0.33 | 0.11 | 0.05 |
|  | critical |  |  | critical | critical |  |
| Sum of Critical Flow Ratios | 0.49 |  |  |  |  |  |
| Cycle Length | 71.7 | seconds |  |  |  |  |
| Lost Time per Phase |  | seconds |  |  |  |  |
| Total Lost Time |  | seconds |  |  |  |  |
| Critical Intersection V/C Ratio: | 0.59 |  |  |  |  |  |

Method follows ODOT Analysis Procedures Manual, Version 2, Chapter 13.4.4.


Method follows ODOT Analysis Procedures Manual, Version 2, Chapter 13.4.4.

| Critical Intersection Volume-to-Capacity Ratio |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26) |  |  |  |  |  |  |
| 2029 With Project Trips |  |  | Weekday AM Peak Hour |  |  |  |
|  | EB |  | WB |  | NB | SB |
| Lane Group | EBL | EBT | WBL | WBTR | NBLTR | SBLTR |
| Adj Flow Rate (veh/h) | 96 | 1067 | 0 | 1225 | 179 | 76 |
| Sat Flow (veh/h) | 1589 | 2988 | 1641 | 3266 | 1439 | 1606 |
| Critical Flow Ratios | 0.06 | 0.36 | 0.00 | 0.38 | 0.12 | 0.05 |
|  | critical |  |  | critical | critical |  |
| Sum of Critical Flow Ratios | 0.56 |  |  |  |  |  |
| Cycle Length | 78.3 | seconds |  |  |  |  |
| Lost Time per Phase |  | seconds |  |  |  |  |
| Total Lost Time |  | seconds |  |  |  |  |
| Critical Intersection V/C Ratio: | 0.66 |  |  |  |  |  |

Method follows ODOT Analysis Procedures Manual, Version 2, Chapter 13.4.4.


Method follows ODOT Analysis Procedures Manual, Version 2, Chapter 13.4.4.

| Critical Intersection Volume-to-Capacity Ratio |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26) |  |  |  |  |  |  |
| 2020 Existing Conditions |  |  | Weekday PM Peak Hour |  |  |  |
|  | EB |  | WB |  | NB | SB |
| Lane Group | EBL | EBT | WBL | WBTR | NBLTR | SBLTR |
| Adj Flow Rate (veh/h) | 161 | 1165 | 5 | 1216 | 154 | 114 |
| Sat Flow (veh/h) | 1654 | 3247 | 1667 | 3272 | 1219 | 1587 |
| Critical Flow Ratios | 0.10 | 0.36 | 0.00 | 0.37 | 0.13 | 0.07 |
| critical critical critical |  |  |  |  |  |  |
| Sum of Critical Flow Ratios | 0.60 |  |  |  |  |  |
| Cycle Length | 110 | seconds |  |  |  |  |
| Lost Time per Phase |  | seconds |  |  |  |  |
| Total Lost Time |  | seconds |  |  |  |  |
| Critical Intersection V/C Ratio: | 0.67 |  |  |  |  |  |

Method follows ODOT Analysis Procedures Manual, Version 2, Chapter 13.4.4.

| Critical Intersection Volume-to-Capacity Ratio |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26) |  |  |  |  |  |  |
| 2022 Without Project |  |  | Weekday PM Peak Hour |  |  |  |
|  | EB |  | WB |  | NB | SB |
| Lane Group | EBL | EBT | WBL | WBTR | NBLTR | SBLTR |
| Adj Flow Rate (veh/h) | 167 | 1212 | 5 | 1265 | 161 | 127 |
| Sat Flow (veh/h) | 1641 | 3247 | 1641 | 3271 | 1174 | 1575 |
| Critical Flow Ratios | 0.10 | 0.37 | 0.00 | 0.39 | 0.14 | 0.08 |
|  | critical |  |  | critical | critical |  |
| Sum of Critical Flow Ratios | 0.63 |  |  |  |  |  |
| Cycle Length | 110 | seconds |  |  |  |  |
| Lost Time per Phase |  | seconds |  |  |  |  |
| Total Lost Time |  | seconds |  |  |  |  |
| Critical Intersection V/C Ratio: | 0.70 |  |  |  |  |  |

Method follows ODOT Analysis Procedures Manual, Version 2, Chapter 13.4.4.

| Critical Intersection Volume-to-Capacity Ratio |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26) |  |  |  |  |  |  |
| 2022 With Project Trips |  |  | Weekday PM Peak Hour |  |  |  |
|  | EB |  | WB |  | NB | SB |
| Lane Group | EBL | EBT | WBL | WBTR | NBLTR | SBLTR |
| Adj Flow Rate (veh/h) | 176 | 1212 | 5 | 1266 | 161 | 143 |
| Sat Flow (veh/h) | 1641 | 3247 | 1641 | 3270 | 1121 | 1577 |
| Critical Flow Ratios | 0.11 | 0.37 | 0.00 | 0.39 | 0.14 | 0.09 |
| critical critical critical |  |  |  |  |  |  |
| Sum of Critical Flow Ratios | 0.64 |  |  |  |  |  |
| Cycle Length | 110 | seconds |  |  |  |  |
| Lost Time per Phase |  | seconds |  |  |  |  |
| Total Lost Time |  | seconds |  |  |  |  |
| Critical Intersection V/C Ratio: | 0.72 |  |  |  |  |  |

Method follows ODOT Analysis Procedures Manual, Version 2, Chapter 13.4.4.

| Critical Intersection Volume-to-Capacity Ratio |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26) |  |  |  |  |  |  |
| 2029 Without Project | Weekday PM Peak Hour |  |  |  |  |  |
|  | EB |  | WB |  | NB | SB |
| Lane Group | EBL E | EBT | WBL | WBTR | NBLTR | SBLTR |
| Adj Flow Rate (veh/h) | 189 | 1374 | 6 | 1435 | 183 | 149 |
| Sat Flow (veh/h) | 1641 | 3247 | 1641 | 3271 | 1119 | 1578 |
| Critical Flow Ratios | 0.12 | 0.42 | 0.00 | 0.44 | 0.16 | 0.09 |
|  | critical |  |  | critical | critical |  |
| Sum of Critical Flow Ratios | 0.72 |  |  |  |  |  |
| Cycle Length <br> Lost Time per Phase <br> Total Lost Time | 110 s | seconds |  |  |  |  |
|  |  | seconds |  |  |  |  |
|  |  | seconds |  |  |  |  |
| Critical Intersection V/C Ratio: | 0.81 |  |  |  |  |  |

Method follows ODOT Analysis Procedures Manual, Version 2, Chapter 13.4.4.

| Critical Intersection Volume-to-Capacity Ratio |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26) |  |  |  |  |  |  |
| 2029 With Project Trips |  |  | Weekday PM Peak Hour |  |  |  |
|  | EB |  | WB |  | NB | SB |
| Lane Group | EBL | EBT | WBL | WBTR | NBLTR | SBLTR |
| Adj Flow Rate (veh/h) | 198 | 1374 | 6 | 1436 | 183 | 163 |
| Sat Flow (veh/h) | 1630 | 3228 | 1630 | 3160 | 1065 | 1587 |
| Critical Flow Ratios | 0.12 | 0.43 | 0.00 | 0.45 | 0.17 | 0.10 |
| critical critical critical |  |  |  |  |  |  |
| Sum of Critical Flow Ratios | 0.75 |  |  |  |  |  |
| Cycle Length | 110 | seconds |  |  |  |  |
| Lost Time per Phase |  | seconds |  |  |  |  |
| Total Lost Time |  | seconds |  |  |  |  |
| Critical Intersection V/C Ratio: | 0.84 |  |  |  |  |  |

Method follows ODOT Analysis Procedures Manual, Version 2, Chapter 13.4.4.


| Sum of Critical Flow Ratios | 0.74 |
| :--- | ---: |
| Cycle Length | 110 seconds |
| Lost Time per Phase | 4 seconds |
| Total Lost Time | 12 seconds |

Critical Intersection V/C Ratio:
0.83

Method follows ODOT Analysis Procedures Manual, Version 2, Chapter 13.4.4.

Lanes，Volumes，Timings
3：Wolf Drive／Ten Eyck Road \＆Mt．Hood Highway（US 26）

|  | 4 | $\rightarrow$ |  | $\checkmark$ |  |  | 4 | 4 | 7 |  | $\dagger$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \％ | 个4 | 「 | \％ | 中 ${ }_{\text {P }}$ |  |  | \＄ |  |  | $\uparrow$ |  |
| Traffic Volume（vph） | 54 | 832 | 33 | 0 | 937 | 16 | 136 | 3 | 2 | 21 | 7 | 135 |
| Future Volume（vph） | 54 | 832 | 33 | 0 | 937 | 16 | 136 | 3 | 2 | 21 | 7 | 135 |
| Ideal Flow（vphpl） | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 |
| Storage Length（ft） | 115 |  | 100 | 105 |  | 0 | 0 |  | 0 | 0 |  | 0 |
| Storage Lanes | 1 |  | 1 | 1 |  | 0 | 0 |  | 0 | 0 |  | 0 |
| Taper Length（ft） | 100 |  |  | 100 |  |  | 100 |  |  | 100 |  |  |
| Lane Util．Factor | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 0.95 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Ped Bike Factor |  |  | 0.98 |  | 1.00 |  |  | 1.00 |  |  | 0.99 |  |
| Frt |  |  | 0.850 |  | 0.998 |  |  | 0.998 |  |  | 0.889 |  |
| Flt Protected | 0.950 |  |  |  |  |  |  | 0.954 |  |  | 0.994 |  |
| Satd．Flow（prot） | 1568 | 2942 | 1403 | 1750 | 3162 | 0 | 0 | 1604 | 0 | 0 | 1458 | 0 |
| Flt Permitted | 0.950 |  |  |  |  |  |  | 0.592 |  |  | 0.953 |  |
| Satd．Flow（perm） | 1568 | 2942 | 1372 | 1750 | 3162 | 0 | 0 | 994 | 0 | 0 | 1398 | 0 |
| Right Turn on Red |  |  | Yes |  |  | Yes |  |  | Yes |  |  | Yes |
| Satd．Flow（RTOR） |  |  | 77 |  | 2 |  |  | 1 |  |  | 147 |  |
| Link Speed（mph） |  | 25 |  |  | 25 |  |  | 25 |  |  | 25 |  |
| Link Distance（ ft ） |  | 468 |  |  | 302 |  |  | 365 |  |  | 210 |  |
| Travel Time（s） |  | 12.8 |  |  | 8.2 |  |  | 10.0 |  |  | 5.7 |  |
| Confl．Peds．（\＃／hr） |  |  | 1 | 1 |  |  | 2 |  |  |  |  | 2 |
| Confl．Bikes（\＃／hr） |  |  |  |  |  | 1 |  |  |  |  |  |  |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles（\％） | 6\％ | 13\％ | 6\％ | 0\％ | 5\％ | 0\％ | 4\％ | 0\％ | 0\％ | 24\％ | 0\％ | 2\％ |
| Adj．Flow（vph） | 59 | 904 | 36 | 0 | 1018 | 17 | 148 | 3 | 2 | 23 | 8 | 147 |
| Shared Lane Traffic（\％） |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow（vph） | 59 | 904 | 36 | 0 | 1035 | 0 | 0 | 153 | 0 | 0 | 178 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | LNA | R NA | RNA | LNA | R NA | RNA | LNA | R NA | RNA | LNA | R NA | R NA |
| Median Width（ft） |  | 12 |  |  | 12 |  |  | 0 |  |  | 0 |  |
| Link Offset（ft） |  | －12 |  |  | 0 |  |  | 0 |  |  | 4 |  |
| Crosswalk Width（tt） |  | 72 |  |  | 42 |  |  | 32 |  |  | 30 |  |
| Two way Left Turn Lane |  |  |  |  |  |  |  |  |  |  |  |  |
| Headway Factor | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 |
| Number of Detectors | 2 | 1 | 1 | 2 | 1 |  | 2 | 2 |  | 2 | 2 |  |
| Detector Template | Left | Det25 | Right | Left | Det25 |  | Left | Side St |  | Left | Side St |  |
| Leading Detector（ft） | 78 | 153 | 153 | 78 | 153 |  | 78 | 78 |  | 78 | 78 |  |
| Trailing Detector（ft） | 2 | 137 | 137 | 2 | 137 |  | 2 | 2 |  | 2 | 2 |  |
| Detector 1 Position（ft） | 2 | 137 | 137 | 2 | 137 |  | 2 | 2 |  | 2 | 2 |  |
| Detector 1 Size（ft） | 16 | 16 | 16 | 16 | 16 |  | 16 | 16 |  | 16 | 16 |  |
| Detector 1 Type | $\mathrm{Cl}+\mathrm{Ex}$ | $\mathrm{Cl}+\mathrm{Ex}$ | $\mathrm{Cl}+\mathrm{Ex}$ | Cl＋Ex | Cl＋Ex |  | Cl＋Ex | $\mathrm{Cl}+\mathrm{Ex}$ |  | Cl＋Ex | $\mathrm{Cl}+\mathrm{Ex}$ |  |
| Detector 1 Channel |  |  |  |  |  |  |  |  |  |  |  |  |
| Detector 1 Extend（s） | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Detector 1 Queue（s） | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Detector 1 Delay（s） | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Detector 2 Position（ft） | 72 |  |  | 72 |  |  | 72 | 72 |  | 72 | 72 |  |
| Detector 2 Size（ft） | 6 |  |  | 6 |  |  | 6 | 6 |  | 6 | 6 |  |
| Detector 2 Type | Cl＋Ex |  |  | $\mathrm{Cl}+\mathrm{Ex}$ |  |  | Cl＋Ex | Cl＋Ex |  | Cl＋Ex | $\mathrm{Cl}+\mathrm{Ex}$ |  |
| Detector 2 Channel |  |  |  |  |  |  |  |  |  |  |  |  |
| Detector 2 Extend（s） | 0.0 |  |  | 0.0 |  |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |

Sandy Health Clinic－PBS Project 71524.000
2020 Existing Conditions－Weekday PM Peak Hour

Lanes, Volumes, Timings
3: Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26)

|  | $\stackrel{ }{*}$ |  |  |  |  |  | 4 | 4 |  |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Turn Type | Prot | NA | Perm | Prot | NA |  | Perm | NA |  | Perm | NA |  |
| Protected Phases | 5 | 2 |  | 1 | 6 |  |  | 8 |  |  | 4 |  |
| Permitted Phases |  |  | 2 |  |  |  | 8 |  |  | 4 |  |  |
| Detector Phase | 5 | 2 | 2 | 1 | 6 |  | 8 | 8 |  | 4 | 4 |  |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial (s) | 4.0 | 10.0 | 10.0 | 4.0 | 10.0 |  | 6.0 | 6.0 |  | 6.0 | 6.0 |  |
| Minimum Split (s) | 8.5 | 26.0 | 26.0 | 8.5 | 30.0 |  | 11.5 | 11.5 |  | 23.5 | 23.5 |  |
| Total Split (s) | 29.5 | 54.0 | 54.0 | 19.5 | 44.0 |  | 25.5 | 25.5 |  | 25.5 | 25.5 |  |
| Total Split (\%) | 29.8\% | 54.5\% | 54.5\% | 19.7\% | 44.4\% |  | 25.8\% | 25.8\% |  | 25.8\% | 25.8\% |  |
| Maximum Green (s) | 25.0 | 50.0 | 50.0 | 15.0 | 40.0 |  | 20.0 | 20.0 |  | 20.0 | 20.0 |  |
| Yellow Time (s) | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 |  | 4.0 | 4.0 |  | 4.0 | 4.0 |  |
| All-Red Time (s) | 1.0 | 0.5 | 0.5 | 1.0 | 0.5 |  | 1.5 | 1.5 |  | 1.5 | 1.5 |  |
| Lost Time Adjust (s) | -0.5 | 0.0 | 0.0 | -0.5 | 0.0 |  |  | -1.5 |  |  | -1.5 |  |
| Total Lost Time (s) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |  |  | 4.0 |  |  | 4.0 |  |
| Lead/Lag | Lag | Lag | Lag | Lead | Lead |  |  |  |  |  |  |  |
| Lead-Lag Optimize? |  |  |  |  |  |  |  |  |  |  |  |  |
| Vehicle Extension (s) | 2.3 | 5.8 | 5.8 | 2.3 | 5.8 |  | 2.5 | 2.5 |  | 2.5 | 2.5 |  |
| Minimum Gap (s) | 0.5 | 3.8 | 3.8 | 0.5 | 3.8 |  | 2.0 | 2.0 |  | 2.0 | 2.0 |  |
| Time Before Reduce (s) | 8.0 | 10.0 | 10.0 | 8.0 | 10.0 |  | 5.0 | 5.0 |  | 5.0 | 5.0 |  |
| Time To Reduce (s) | 3.0 | 10.0 | 10.0 | 3.0 | 10.0 |  | 5.0 | 5.0 |  | 5.0 | 5.0 |  |
| Recall Mode | None | Min | Min | None | Min |  | None | None |  | None | None |  |
| Walk Time (s) |  | 7.0 | 7.0 |  | 7.0 |  |  |  |  | 7.0 | 7.0 |  |
| Flash Dont Walk (s) |  | 15.0 | 15.0 |  | 19.0 |  |  |  |  | 11.0 | 11.0 |  |
| Pedestrian Calls (\#/hr) |  | 0 | 0 |  | 0 |  |  |  |  | 0 | 0 |  |
| Act Effct Green (s) | 8.3 | 35.2 | 35.2 |  | 28.3 |  |  | 20.5 |  |  | 20.5 |  |
| Actuated g/C Ratio | 0.13 | 0.55 | 0.55 |  | 0.44 |  |  | 0.32 |  |  | 0.32 |  |
| v/c Ratio | 0.29 | 0.56 | 0.05 |  | 0.74 |  |  | 0.48 |  |  | 0.33 |  |
| Control Delay | 34.6 | 10.3 | 0.4 |  | 19.9 |  |  | 29.4 |  |  | 8.8 |  |
| Queue Delay | 0.0 | 0.0 | 0.0 |  | 0.0 |  |  | 0.0 |  |  | 0.0 |  |
| Total Delay | 34.6 | 10.3 | 0.4 |  | 19.9 |  |  | 29.4 |  |  | 8.8 |  |
| LOS | C | B | A |  | B |  |  | C |  |  | A |  |
| Approach Delay |  | 11.4 |  |  | 19.9 |  |  | 29.4 |  |  | 8.8 |  |
| Approach LOS |  | B |  |  | B |  |  | C |  |  | A |  |

Intersection Summary
Area Type: Other
Cycle Length: 99
Actuated Cycle Length: 64.4
Natural Cycle: 65
Control Type: Actuated-Uncoordinated
Maximum v/c Ratio: 0.74
Intersection Signal Delay: 16.1
Intersection LOS: B
Intersection Capacity Utilization 65.0\%
ICU Level of Service C
Analysis Period (min) 15
Splits and Phases: $\quad 3$ : Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26)


Queues
3: Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26)

|  | 4 | $\rightarrow$ | $\cdots$ | $\leftarrow$ | 4 | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBT | NBT | SBT |
| Lane Group Flow (vph) | 59 | 904 | 36 | 1035 | 153 | 178 |
| v/c Ratio | 0.29 | 0.56 | 0.05 | 0.74 | 0.48 | 0.33 |
| Control Delay | 34.6 | 10.3 | 0.4 | 19.9 | 29.4 | 8.8 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Delay | 34.6 | 10.3 | 0.4 | 19.9 | 29.4 | 8.8 |
| Queue Length 50th (ft) | 24 | 112 | 0 | 200 | 54 | 10 |
| Queue Length 95th (ft) | 65 | 151 | 2 | 290 | \#142 | 64 |
| Internal Link Dist (ft) |  | 388 |  | 222 | 285 | 130 |
| Turn Bay Length (ft) | 115 |  | 100 |  |  |  |
| Base Capacity (vph) | 675 | 2332 | 1103 | 2100 | 361 | 601 |
| Starvation Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 |
| Spillback Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 |
| Storage Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 |
| Reduced v/c Ratio | 0.09 | 0.39 | 0.03 | 0.49 | 0.42 | 0.30 |
| Intersection Summary |  |  |  |  |  |  |
| \# 95th percentile volume exceeds capacity, queue may be longer.Queue shown is maximum after two cycles. |  |  |  |  |  |  |
|  |  |  |  |  |  |  |

HCM Signalized Intersection Capacity Analysis
3：Wolf Drive／Ten Eyck Road \＆Mt．Hood Highway（US 26）

|  | $\Rightarrow$ |  |  | 7 |  | 4 | 4 | $\dagger$ | $\pm$ |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ${ }^{*}$ | 个4 | 「 | ${ }^{*}$ | 个t |  |  | $\uparrow$ |  |  | $\uparrow$ |  |
| Traffic Volume（vph） | 54 | 832 | 33 | 0 | 937 | 16 | 136 | 3 | 2 | 21 | 7 | 135 |
| Future Volume（vph） | 54 | 832 | 33 | 0 | 937 | 16 | 136 | 3 | 2 | 21 | 7 | 135 |
| Ideal Flow（vphpl） | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 |
| Total Lost time（s） | 4.0 | 4.0 | 4.0 |  | 4.0 |  |  | 4.0 |  |  | 4.0 |  |
| Lane Util．Factor | 1.00 | 0.95 | 1.00 |  | 0.95 |  |  | 1.00 |  |  | 1.00 |  |
| Frpb，ped／bikes | 1.00 | 1.00 | 0.98 |  | 1.00 |  |  | 1.00 |  |  | 0.99 |  |
| Flpb，ped／bikes | 1.00 | 1.00 | 1.00 |  | 1.00 |  |  | 1.00 |  |  | 1.00 |  |
| Frt | 1.00 | 1.00 | 0.85 |  | 1.00 |  |  | 1.00 |  |  | 0.89 |  |
| Flt Protected | 0.95 | 1.00 | 1.00 |  | 1.00 |  |  | 0.95 |  |  | 0.99 |  |
| Satd．Flow（prot） | 1568 | 2942 | 1373 |  | 3160 |  |  | 1602 |  |  | 1458 |  |
| Flt Permitted | 0.95 | 1.00 | 1.00 |  | 1.00 |  |  | 0.59 |  |  | 0.95 |  |
| Satd．Flow（perm） | 1568 | 2942 | 1373 |  | 3160 |  |  | 994 |  |  | 1398 |  |
| Peak－hour factor，PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj．Flow（vph） | 59 | 904 | 36 | 0 | 1018 | 17 | 148 | 3 |  | 23 | 8 | 147 |
| RTOR Reduction（vph） | 0 | 0 | 16 | 0 | 1 | 0 | 0 | 1 | 0 | 0 | 101 | 0 |
| Lane Group Flow（vph） | 59 | 904 | 20 | 0 | 1034 | 0 | 0 | 152 | 0 | 0 | 77 | 0 |
| Confl．Peds．（\＃／hr） |  |  | 1 | 1 |  |  | 2 |  |  |  |  | 2 |
| Confl．Bikes（\＃／hr） |  |  |  |  |  | 1 |  |  |  |  |  |  |
| Heavy Vehicles（\％） | 6\％ | 13\％ | 6\％ | 0\％ | 5\％ | 0\％ | 4\％ | 0\％ | 0\％ | 24\％ | 0\％ | 2\％ |
| Turn Type | Prot | NA | Perm | Prot | NA |  | Perm | NA |  | Perm | NA |  |
| Protected Phases | 5 | 2 |  | 1 | 6 |  |  | 8 |  |  | 4 |  |
| Permitted Phases |  |  | 2 |  |  |  | 8 |  |  | 4 |  |  |
| Actuated Green，G（s） | 4.6 | 37.4 | 37.4 |  | 28.3 |  |  | 18.9 |  |  | 18.9 |  |
| Effective Green，g（s） | 5.1 | 37.4 | 37.4 |  | 28.3 |  |  | 20.4 |  |  | 20.4 |  |
| Actuated g／C Ratio | 0.08 | 0.57 | 0.57 |  | 0.43 |  |  | 0.31 |  |  | 0.31 |  |
| Clearance Time（s） | 4.5 | 4.0 | 4.0 |  | 4.0 |  |  | 5.5 |  |  | 5.5 |  |
| Vehicle Extension（s） | 2.3 | 5.8 | 5.8 |  | 5.8 |  |  | 2.5 |  |  | 2.5 |  |
| Lane Grp Cap（vph） | 121 | 1672 | 780 |  | 1359 |  |  | 308 |  |  | 433 |  |
| v／s Ratio Prot | 0.04 | c0．31 |  |  | c0．33 |  |  |  |  |  |  |  |
| $\mathrm{v} / \mathrm{s}$ Ratio Perm |  |  | 0.01 |  |  |  |  | c0．15 |  |  | 0.05 |  |
| v／c Ratio | 0.49 | 0.54 | 0.03 |  | 0.76 |  |  | 0.49 |  |  | 0.18 |  |
| Uniform Delay，d1 | 29.1 | 8.8 | 6.2 |  | 15.9 |  |  | 18.5 |  |  | 16.6 |  |
| Progression Factor | 1.00 | 1.00 | 1.00 |  | 1.00 |  |  | 1.00 |  |  | 1.00 |  |
| Incremental Delay，d2 | 1.8 | 0.8 | 0.0 |  | 3.3 |  |  | 0.9 |  |  | 0.1 |  |
| Delay（s） | 30.9 | 9.6 | 6.3 |  | 19.1 |  |  | 19.4 |  |  | 16.7 |  |
| Level of Service | C | A | A |  | B |  |  | B |  |  | B |  |
| Approach Delay（s） |  | 10.8 |  |  | 19.1 |  |  | 19.4 |  |  | 16.7 |  |
| Approach LOS |  | B |  |  | B |  |  | B |  |  | B |  |


| Intersection Summary |  |  |  |
| :--- | ---: | :--- | ---: |
| HCM 2000 Control Delay | 15.4 | HCM 2000 Level of Service | B |
| HCM 2000 Volume to Capacity ratio | 0.66 |  | 12.0 |
| Actuated Cycle Length（s） | 65.8 | Sum of lost time（s） | C |
| Intersection Capacity Utilization | $65.0 \%$ | ICU Level of Service |  |
| Analysis Period（min） | 15 |  |  |
| C Critical Lane Group |  |  |  |

Sandy Health Clinic－PBS Project 71524.000
2020 Existing Conditions－Weekday PM Peak Hour

HCM 6th Signalized Intersection Summary
3: Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26)


Notes

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Lanes, Volumes, Timings
3: Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26)

|  | 4 |  |  | $\checkmark$ |  | 4 | 4 | 4 | $p$ |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ${ }^{*}$ | 革 | F | \% | 中 ${ }_{\text {F }}$ |  |  | \$ |  |  | ¢ |  |
| Traffic Volume (vph) | 151 | 1095 | 134 | 5 | 1128 | 16 | 116 | 24 | 7 | 38 | 23 | 100 |
| Future Volume (vph) | 151 | 1095 | 134 | 5 | 1128 | 16 | 116 | 24 | 7 | 38 | 23 | 100 |
| Ideal Flow (vphpl) | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 |
| Storage Length (ft) | 115 |  | 100 | 105 |  | 0 | 0 |  | 0 | 0 |  | 0 |
| Storage Lanes | 1 |  | 1 | 1 |  | 0 | 0 |  | 0 | 0 |  | 0 |
| Taper Length (ft) | 100 |  |  | 100 |  |  | 100 |  |  | 100 |  |  |
| Lane Util. Factor | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 0.95 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Ped Bike Factor | 1.00 |  | 0.98 | 1.00 | 1.00 |  |  | 1.00 |  |  | 0.99 |  |
| Frt |  |  | 0.850 |  | 0.998 |  |  | 0.994 |  |  | 0.916 |  |
| Flt Protected | 0.950 |  |  | 0.950 |  |  |  | 0.962 |  |  | 0.988 |  |
| Satd. Flow (prot) | 1646 | 3228 | 1488 | 1662 | 3161 | 0 | 0 | 1639 | 0 | 0 | 1553 | 0 |
| Flt Permitted | 0.950 |  |  | 0.950 |  |  |  | 0.548 |  |  | 0.910 |  |
| Satd. Flow (perm) | 1645 | 3228 | 1454 | 1662 | 3161 | 0 | 0 | 930 | 0 | 0 | 1431 | 0 |
| Right Turn on Red |  |  | Yes |  |  | Yes |  |  | Yes |  |  | Yes |
| Satd. Flow (RTOR) |  |  | 95 |  | 2 |  |  | 2 |  |  | 67 |  |
| Link Speed (mph) |  | 25 |  |  | 25 |  |  | 25 |  |  | 25 |  |
| Link Distance (ft) |  | 468 |  |  | 302 |  |  | 365 |  |  | 210 |  |
| Travel Time (s) |  | 12.8 |  |  | 8.2 |  |  | 10.0 |  |  | 5.7 |  |
| Confl. Peds. (\#/hr) | 2 |  | 1 | 1 |  | 2 | 5 |  |  |  |  | 5 |
| Peak Hour Factor | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 |
| Heavy Vehicles (\%) | 1\% | 3\% | 0\% | 0\% | 5\% | 0\% | 1\% | 8\% | 0\% | 0\% | 0\% | 1\% |
| Adj. Flow (vph) | 161 | 1165 | 143 | 5 | 1200 | 17 | 123 | 26 | 7 | 40 | 24 | 106 |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow (vph) | 161 | 1165 | 143 | 5 | 1217 | 0 | 0 | 156 | 0 | 0 | 170 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | L NA | R NA | R NA | LNA | R NA | R NA | LNA | R NA | R NA | LNA | R NA | R NA |
| Median Width(ft) |  | 12 |  |  | 12 |  |  | 0 |  |  | 0 |  |
| Link Offset(ft) |  | -12 |  |  | 0 |  |  | 0 |  |  | 4 |  |
| Crosswalk Width(tt) |  | 72 |  |  | 42 |  |  | 32 |  |  | 30 |  |
| Two way Left Turn Lane |  |  |  |  |  |  |  |  |  |  |  |  |
| Headway Factor | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 |
| Number of Detectors | 2 | 1 | 1 | 2 | 1 |  | 2 | 2 |  | 2 | 2 |  |
| Detector Template | Left | Det25 | Right | Left | Det25 |  | Left | Side St |  | Left | Side St |  |
| Leading Detector (ft) | 78 | 153 | 153 | 78 | 153 |  | 78 | 78 |  | 78 | 78 |  |
| Trailing Detector (ft) | 2 | 137 | 137 | 2 | 137 |  | 2 | 2 |  | 2 | 2 |  |
| Detector 1 Position(ft) | 2 | 137 | 137 | 2 | 137 |  | 2 | 2 |  | 2 | 2 |  |
| Detector 1 Size(ft) | 16 | 16 | 16 | 16 | 16 |  | 16 | 16 |  | 16 | 16 |  |
| Detector 1 Type | Cl+Ex | Cl+Ex | Cl+Ex | Cl+Ex | Cl+Ex |  | Cl+Ex | Cl+Ex |  | Cl+Ex | $\mathrm{Cl}+\mathrm{Ex}$ |  |
| Detector 1 Channel |  |  |  |  |  |  |  |  |  |  |  |  |
| Detector 1 Extend (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Detector 1 Queue (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Detector 1 Delay (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Detector 2 Position(ft) | 72 |  |  | 72 |  |  | 72 | 72 |  | 72 | 72 |  |
| Detector 2 Size(ft) | 6 |  |  | 6 |  |  | 6 | 6 |  | 6 | 6 |  |
| Detector 2 Type | $\mathrm{Cl}+\mathrm{Ex}$ |  |  | Cl+Ex |  |  | Cl+Ex | $\mathrm{Cl}+\mathrm{Ex}$ |  | Cl+Ex | $\mathrm{Cl}+\mathrm{Ex}$ |  |
| Detector 2 Channel |  |  |  |  |  |  |  |  |  |  |  |  |
| Detector 2 Extend (s) | 0.0 |  |  | 0.0 |  |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Turn Type | Prot | NA | Perm | Prot | NA |  | Perm | NA |  | Perm | NA |  |

[^5]Synchro 10 Report - by PBS Engineering and Environmental
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Lanes, Volumes, Timings
3: Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26)

| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Protected Phases | 5 | 2 |  | 1 | 6 |  |  | 8 |  | 4 | 4 |  |


| Detector Phase | 5 | 2 | 2 | 1 | 6 | 8 | 8 | 4 | 4 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Switch Phase |  |  |  |  |  |  |  |  |  |
| Minimum Initial (s) | 4.0 | 10.0 | 10.0 | 4.0 | 10.0 | 6.0 | 6.0 | 6.0 | 6.0 |
| Minimum Split (s) | 8.5 | 26.0 | 26.0 | 8.5 | 30.0 | 11.5 | 11.5 | 23.5 | 23.5 |
| Total Split (s) | 20.0 | 70.0 | 70.0 | 15.0 | 65.0 | 25.0 | 25.0 | 25.0 | 25.0 |
| Total Split (\%) | 18.2\% | 63.6\% | 63.6\% | 13.6\% | 59.1\% | 22.7\% | 22.7\% | 22.7\% | 22.7\% |
| Maximum Green (s) | 15.5 | 66.0 | 66.0 | 10.5 | 61.0 | 19.5 | 19.5 | 19.5 | 19.5 |
| Yellow Time (s) | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 4.0 | 4.0 | 4.0 | 4.0 |
| All-Red Time (s) | 1.0 | 0.5 | 0.5 | 1.0 | 0.5 | 1.5 | 1.5 | 1.5 | 1.5 |
| Lost Time Adjust (s) | -0.5 | 0.0 | 0.0 | -0.5 | 0.0 |  | -1.5 |  | -1.5 |
| Total Lost Time (s) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |  | 4.0 |  | 4.0 |
| Lead/Lag | Lag | Lag | Lag | Lead | Lead |  |  |  |  |
| Lead-Lag Optimize? |  |  |  |  |  |  |  |  |  |
| Vehicle Extension (s) | 2.3 | 5.8 | 5.8 | 2.3 | 5.8 | 2.5 | 2.5 | 2.5 | 2.5 |
| Minimum Gap (s) | 0.5 | 3.8 | 3.8 | 0.5 | 3.8 | 2.0 | 2.0 | 2.0 | 2.0 |
| Time Before Reduce (s) | 8.0 | 10.0 | 10.0 | 8.0 | 10.0 | 5.0 | 5.0 | 5.0 | 5.0 |
| Time To Reduce (s) | 3.0 | 10.0 | 10.0 | 3.0 | 10.0 | 5.0 | 5.0 | 5.0 | 5.0 |
| Recall Mode | None | C-Min | C-Min | None | C-Min | None | None | None | None |
| Walk Time (s) |  | 7.0 | 7.0 |  | 7.0 |  |  | 7.0 | 7.0 |
| Flash Dont Walk (s) |  | 15.0 | 15.0 |  | 19.0 |  |  | 11.0 | 11.0 |
| Pedestrian Calls (\#/hr) |  | 0 | 0 |  | 0 |  |  | 0 | 0 |
| Act Effct Green (s) | 15.0 | 76.5 | 76.5 | 5.8 | 59.7 |  | 23.3 |  | 23.3 |
| Actuated g/C Ratio | 0.14 | 0.70 | 0.70 | 0.05 | 0.54 |  | 0.21 |  | 0.21 |
| v/c Ratio | 0.72 | 0.52 | 0.14 | 0.06 | 0.71 |  | 0.78 |  | 0.48 |
| Control Delay | 63.6 | 9.5 | 2.8 | 50.6 | 22.2 |  | 68.1 |  | 27.7 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 |  | 0.0 |
| Total Delay | 63.6 | 9.5 | 2.8 | 50.6 | 22.2 |  | 68.1 |  | 27.7 |
| LOS | E | A | A | D | C |  | E |  | C |
| Approach Delay |  | 14.8 |  |  | 22.3 |  | 68.1 |  | 27.7 |
| Approach LOS |  | B |  |  | C |  | E |  | C |

Intersection Summary
Area Type: Other
Cycle Length: 110
Actuated Cycle Length: 110
Offset: $0(0 \%)$, Referenced to phase 2:EBT and 6:WBT, Start of Green, Master Intersection
Natural Cycle: 70
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.78
Intersection Signal Delay: 21.3
Intersection LOS: C
Intersection Capacity Utilization 76.9\%
ICU Level of Service D
Analysis Period (min) 15
Splits and Phases: $\quad 3$ : Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26)


2020 Existing Conditions - Weekday PM Peak Hour

Queues
3: Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26)

|  | 4 | $\rightarrow$ | * | $\downarrow$ | - | 4 | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | NBT | SBT |
| Lane Group Flow (vph) | 161 | 1165 | 143 | 5 | 1217 | 156 | 170 |
| $\mathrm{v} / \mathrm{c}$ Ratio | 0.72 | 0.52 | 0.14 | 0.06 | 0.71 | 0.78 | 0.48 |
| Control Delay | 63.6 | 9.5 | 2.8 | 50.6 | 22.2 | 68.1 | 27.7 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Delay | 63.6 | 9.5 | 2.8 | 50.6 | 22.2 | 68.1 | 27.7 |
| Queue Length 50th (ft) | 109 | 205 | 11 | 3 | 366 | 97 | 58 |
| Queue Length 95th (ft) | \#182 | 280 | 35 | 16 | 396 | \#233 | 136 |
| Internal Link Dist (ft) |  | 388 |  |  | 222 | 285 | 130 |
| Turn Bay Length (ft) | 115 |  | 100 | 105 |  |  |  |
| Base Capacity (vph) | 248 | 2246 | 1040 | 166 | 1799 | 202 | 361 |
| Starvation Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Spillback Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Storage Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Reduced v/c Ratio | 0.65 | 0.52 | 0.14 | 0.03 | 0.68 | 0.77 | 0.47 |

Intersection Summary
\# 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis
3: Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26)

c Critical Lane Group

Sandy Health Clinic - PBS Project 71524.000
2020 Existing Conditions - Weekday PM Peak Hour

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HCM 6th Signalized Intersection Summary
3: Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26)


Notes

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Lanes，Volumes，Timings
3：Wolf Drive／Ten Eyck Road \＆Mt．Hood Highway（US 26）

|  | 4 |  |  | $\dagger$ | 4 | 4 | 4 | 4 | $p$ |  | $\dagger$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ＊ | 个4 | 「 | ${ }^{*}$ | 性 |  |  | \＄ |  |  | ¢ |  |
| Traffic Volume（vph） | 56 | 865 | 34 | 0 | 974 | 17 | 141 | 3 | 2 | 22 | 7 | 140 |
| Future Volume（vph） | 56 | 865 | 34 | 0 | 974 | 17 | 141 | 3 | 2 | 22 | 7 | 140 |
| Ideal Flow（vphpl） | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 |
| Storage Length（ft） | 115 |  | 100 | 105 |  | 0 | 0 |  | 0 | 0 |  | 0 |
| Storage Lanes | 1 |  | 1 | 1 |  | 0 | 0 |  | 0 | 0 |  | 0 |
| Taper Length（ft） | 100 |  |  | 100 |  |  | 100 |  |  | 100 |  |  |
| Lane Util．Factor | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 0.95 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Ped Bike Factor |  |  | 0.98 |  | 1.00 |  |  | 1.00 |  |  | 0.99 |  |
| Frt |  |  | 0.850 |  | 0.997 |  |  | 0.998 |  |  | 0.888 |  |
| Flt Protected | 0.950 |  |  |  |  |  |  | 0.954 |  |  | 0.994 |  |
| Satd．Flow（prot） | 1568 | 2942 | 1403 | 1716 | 3158 | 0 | 0 | 1603 | 0 | 0 | 1454 | 0 |
| Flt Permitted | 0.950 |  |  |  |  |  |  | 0.572 |  |  | 0.951 |  |
| Satd．Flow（perm） | 1568 | 2942 | 1372 | 1716 | 3158 | 0 | 0 | 959 | 0 | 0 | 1391 | 0 |
| Right Turn on Red |  |  | Yes |  |  | Yes |  |  | Yes |  |  | Yes |
| Satd．Flow（RTOR） |  |  | 77 |  | 2 |  |  | 1 |  |  | 152 |  |
| Link Speed（mph） |  | 25 |  |  | 25 |  |  | 25 |  |  | 25 |  |
| Link Distance（ ft ） |  | 468 |  |  | 302 |  |  | 365 |  |  | 210 |  |
| Travel Time（s） |  | 12.8 |  |  | 8.2 |  |  | 10.0 |  |  | 5.7 |  |
| Confl．Peds．（\＃／hr） |  |  | 1 | 1 |  |  | 2 |  |  |  |  | 2 |
| Confl．Bikes（\＃／hr） |  |  |  |  |  | 1 |  |  |  |  |  |  |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles（\％） | 6\％ | 13\％ | 6\％ | 2\％ | 5\％ | 2\％ | 4\％ | 2\％ | 2\％ | 24\％ | 2\％ | 2\％ |
| Adj．Flow（vph） | 61 | 940 | 37 | 0 | 1059 | 18 | 153 | 3 | 2 | 24 | 8 | 152 |


| Shared Lane Traffic（\％） |  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Lane Group Flow（vph） | 61 | 940 | 37 | 0 | 1077 | 0 | 0 | 158 | 0 | 0 | 184 | O |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | L NA | R NA | R NA | L NA | R NA | R NA | L NA | R NA | R NA | LNA | RNA | R NA |


| Lane Alignment | NA | R NA | R NA | LNA | R NA | R NA | LNA | R NA | R NA | LNA | R NA | R NA |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Median Width（ft） |  | 12 |  |  | 12 |  |  | 0 |  |  | 0 |  |
| Link Offset（ft） |  | －12 |  |  | 0 |  |  | 0 |  |  | 4 |  |
| Crosswalk Width（ft） |  | 72 |  |  | 42 |  |  | 32 |  |  | 30 |  |
| Two way Left Turn Lane |  |  |  |  |  |  |  |  |  |  |  |  |
| Headway Factor | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 |
| Number of Detectors | 2 | 1 | 1 | 2 | 1 |  | 2 | 2 |  | 2 | 2 |  |
| Detector Template | Left | Det25 | Right | Left | Det25 |  | Left | Side St |  | Left | Side St |  |
| Leading Detector（ft） | 78 | 153 | 153 | 78 | 153 |  | 78 | 78 |  | 78 | 78 |  |
| Trailing Detector（ft） | 2 | 137 | 137 | 2 | 137 |  | 2 | 2 |  | 2 | 2 |  |
| Detector 1 Position（ft） | 2 | 137 | 137 | 2 | 137 |  | 2 | 2 |  | 2 | 2 |  |
| Detector 1 Size（ft） | 16 | 16 | 16 | 16 | 16 |  | 16 | 16 |  | 16 | 16 |  |
| Detector 1 Type | Cl＋Ex | Cl＋Ex | Cl＋Ex | Cl＋Ex | Cl＋Ex |  | Cl＋Ex | Cl＋Ex |  | Cl＋Ex | Cl＋Ex |  |
| Detector 1 Channel |  |  |  |  |  |  |  |  |  |  |  |  |
| Detector 1 Extend（s） | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Detector 1 Queue（s） | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Detector 1 Delay（s） | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Detector 2 Position（ft） | 72 |  |  | 72 |  |  | 72 | 72 |  | 72 | 72 |  |
| Detector 2 Size（ft） | 6 |  |  | 6 |  |  | 6 | 6 |  | 6 | 6 |  |
| Detector 2 Type | $\mathrm{Cl}+\mathrm{Ex}$ |  |  | Cl＋Ex |  |  | Cl＋Ex | $\mathrm{Cl}+\mathrm{Ex}$ |  | $\mathrm{Cl}+\mathrm{Ex}$ | Cl＋Ex |  |
| Detector 2 Channel |  |  |  |  |  |  |  |  |  |  |  |  |
| Detector 2 Extend（s） | 0.0 |  |  | 0.0 |  |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |

Sandy Health Clinic－PBS Project 71524.000
2022 Without Project Conditions－Weekday AM Peak Hour

Synchro 10 Report－by PBS Engineering and Environmental
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Lanes, Volumes, Timings
3: Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26)

|  |  |  |  | $\checkmark$ |  |  | 4 | $\uparrow$ |  |  | $\dagger$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Turn Type | Prot | NA | Perm | Prot | NA |  | Perm | NA |  | Perm | NA |  |
| Protected Phases | 5 | 2 |  | 1 | 6 |  |  | 8 |  |  | 4 |  |
| Permitted Phases |  |  | 2 |  |  |  | 8 |  |  | 4 |  |  |
| Detector Phase | 5 | 2 | 2 | 1 | 6 |  | 8 | 8 |  | 4 | 4 |  |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial (s) | 4.0 | 10.0 | 10.0 | 4.0 | 10.0 |  | 6.0 | 6.0 |  | 6.0 | 6.0 |  |
| Minimum Split (s) | 8.5 | 26.0 | 26.0 | 8.5 | 30.0 |  | 11.5 | 11.5 |  | 23.5 | 23.5 |  |
| Total Split (s) | 29.5 | 54.0 | 54.0 | 19.5 | 44.0 |  | 25.5 | 25.5 |  | 25.5 | 25.5 |  |
| Total Split (\%) | 29.8\% | 54.5\% | 54.5\% | 19.7\% | 44.4\% |  | 25.8\% | 25.8\% |  | 25.8\% | 25.8\% |  |
| Maximum Green (s) | 25.0 | 50.0 | 50.0 | 15.0 | 40.0 |  | 20.0 | 20.0 |  | 20.0 | 20.0 |  |
| Yellow Time (s) | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 |  | 4.0 | 4.0 |  | 4.0 | 4.0 |  |
| All-Red Time (s) | 1.0 | 0.5 | 0.5 | 1.0 | 0.5 |  | 1.5 | 1.5 |  | 1.5 | 1.5 |  |
| Lost Time Adjust (s) | -0.5 | 0.0 | 0.0 | -0.5 | 0.0 |  |  | -1.5 |  |  | -1.5 |  |
| Total Lost Time (s) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |  |  | 4.0 |  |  | 4.0 |  |
| Lead/Lag | Lag | Lag | Lag | Lead | Lead |  |  |  |  |  |  |  |
| Lead-Lag Optimize? |  |  |  |  |  |  |  |  |  |  |  |  |
| Vehicle Extension (s) | 2.3 | 5.8 | 5.8 | 2.3 | 5.8 |  | 2.5 | 2.5 |  | 2.5 | 2.5 |  |
| Minimum Gap (s) | 0.5 | 3.8 | 3.8 | 0.5 | 3.8 |  | 2.0 | 2.0 |  | 2.0 | 2.0 |  |
| Time Before Reduce (s) | 8.0 | 10.0 | 10.0 | 8.0 | 10.0 |  | 5.0 | 5.0 |  | 5.0 | 5.0 |  |
| Time To Reduce (s) | 3.0 | 10.0 | 10.0 | 3.0 | 10.0 |  | 5.0 | 5.0 |  | 5.0 | 5.0 |  |
| Recall Mode | None | Min | Min | None | Min |  | None | None |  | None | None |  |
| Walk Time (s) |  | 7.0 | 7.0 |  | 7.0 |  |  |  |  | 7.0 | 7.0 |  |
| Flash Dont Walk (s) |  | 15.0 | 15.0 |  | 19.0 |  |  |  |  | 11.0 | 11.0 |  |
| Pedestrian Calls (\#/hr) |  | 0 | 0 |  | 0 |  |  |  |  | 0 | 0 |  |
| Act Effct Green (s) | 8.3 | 39.6 | 39.6 |  | 30.0 |  |  | 21.2 |  |  | 21.2 |  |
| Actuated g/C Ratio | 0.12 | 0.57 | 0.57 |  | 0.43 |  |  | 0.31 |  |  | 0.31 |  |
| v/c Ratio | 0.32 | 0.56 | 0.05 |  | 0.79 |  |  | 0.54 |  |  | 0.35 |  |
| Control Delay | 36.8 | 10.1 | 0.4 |  | 22.3 |  |  | 33.3 |  |  | 9.1 |  |
| Queue Delay | 0.0 | 0.0 | 0.0 |  | 0.0 |  |  | 0.0 |  |  | 0.0 |  |
| Total Delay | 36.8 | 10.1 | 0.4 |  | 22.3 |  |  | 33.3 |  |  | 9.1 |  |
| LOS | D | B | A |  | C |  |  | C |  |  | A |  |
| Approach Delay |  | 11.3 |  |  | 22.3 |  |  | 33.3 |  |  | 9.1 |  |
| Approach LOS |  | B |  |  | C |  |  | C |  |  | A |  |

Intersection Summary
Area Type: Other
Cycle Length: 99
Actuated Cycle Length: 69.3
Natural Cycle: 65
Control Type: Actuated-Uncoordinated
Maximum v/c Ratio: 0.79
Intersection Signal Delay: 17.4
Intersection Capacity Utilization 66.8\%
Intersection LOS: B
ICU Level of Service C
Analysis Period (min) 15
Splits and Phases: $\quad 3$ : Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26)


2022 Without Project Conditions - Weekday AM Peak Hour

Queues
3: Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26)

|  | $\Rightarrow$ | $\begin{aligned} & \rightarrow \\ & \text { EBT } \end{aligned}$ |  |  | $\dagger$NBT | ¢SBT |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group |  |  |  |  |  |  |
| Lane Group Flow (vph) | 61 |  | 37 |  | 158 | 184 |
| v/c Ratio | 0.32 | 0.56 | 0.05 | 0.79 | 0.54 | 0.35 |
| Control Delay | 36.8 | 10.1 | 0.4 | 22.3 | 33.3 | 9.1 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Delay | 36.8 | 10.1 | 0.4 | 22.3 | 33.3 | 9.1 |
| Queue Length 50th (ft) | 26 | 118 | 0 | 213 | 59 | 10 |
| Queue Length 95th (ft) | 67 | 160 | 2 | 309 | \#160 | 65 |
| Internal Link Dist (ft) |  | 388 |  | 222 | 285 | 130 |
| Turn Bay Length ( ft ) | 115 |  | 100 |  |  |  |
| Base Capacity (vph) | 613 | 2189 | 1040 | 1940 | 317 | 560 |
| Starvation Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 |
| Spillback Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 |
| Storage Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 |
| Reduced v/c Ratio | 0.10 | 0.43 | 0.04 | 0.56 | 0.50 | 0.33 |

Intersection Summary
\# 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis
3：Wolf Drive／Ten Eyck Road \＆Mt．Hood Highway（US 26）

|  | 4 |  |  | 7 |  | 4 | 4 | $\uparrow$ | $p$ |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ${ }^{*}$ | 个4 | 「 | ＊ | 中 ${ }^{\text {d }}$ |  |  | ¢ |  |  | $\uparrow$ |  |
| Traffic Volume（vph） | 56 | 865 | 34 | 0 | 974 | 17 | 141 | 3 | 2 | 22 | 7 | 140 |
| Future Volume（vph） | 56 | 865 | 34 | 0 | 974 | 17 | 141 | 3 | 2 | 22 | 7 | 140 |
| Ideal Flow（vphpl） | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 |
| Total Lost time（s） | 4.0 | 4.0 | 4.0 |  | 4.0 |  |  | 4.0 |  |  | 4.0 |  |
| Lane Util．Factor | 1.00 | 0.95 | 1.00 |  | 0.95 |  |  | 1.00 |  |  | 1.00 |  |
| Frpb，ped／bikes | 1.00 | 1.00 | 0.98 |  | 1.00 |  |  | 1.00 |  |  | 0.99 |  |
| Flpb，ped／bikes | 1.00 | 1.00 | 1.00 |  | 1.00 |  |  | 1.00 |  |  | 1.00 |  |
| Fit | 1.00 | 1.00 | 0.85 |  | 1.00 |  |  | 1.00 |  |  | 0.89 |  |
| Flt Protected | 0.95 | 1.00 | 1.00 |  | 1.00 |  |  | 0.95 |  |  | 0.99 |  |
| Satd．Flow（prot） | 1568 | 2942 | 1373 |  | 3159 |  |  | 1601 |  |  | 1456 |  |
| Flt Permitted | 0.95 | 1.00 | 1.00 |  | 1.00 |  |  | 0.57 |  |  | 0.95 |  |
| Satd．Flow（perm） | 1568 | 2942 | 1373 |  | 3159 |  |  | 960 |  |  | 1394 |  |
| Peak－hour factor，PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj．Flow（vph） | 61 | 940 | 37 | 0 | 1059 | 18 | 153 | 3 |  | 24 | 8 | 152 |
| RTOR Reduction（vph） | 0 | 0 | 15 | 0 | 1 | 0 | 0 | 1 | 0 | 0 | 106 | 0 |
| Lane Group Flow（vph） | 61 | 940 | 22 | 0 | 1076 | 0 | 0 | 157 | 0 | 0 | 78 | 0 |
| Confl．Peds．（\＃／hr） |  |  | 1 | 1 |  |  | 2 |  |  |  |  | 2 |
| Confl．Bikes（\＃／hr） |  |  |  |  |  | 1 |  |  |  |  |  |  |
| Heavy Vehicles（\％） | 6\％ | 13\％ | 6\％ | 2\％ | 5\％ | 2\％ | 4\％ | 2\％ | 2\％ | 24\％ | 2\％ | 2\％ |
| Turn Type | Prot | NA | Perm | Prot | NA |  | Perm | NA |  | Perm | NA |  |
| Protected Phases | 5 | 2 |  | 1 | 6 |  |  | 8 |  |  | 4 |  |
| Permitted Phases |  |  | 2 |  |  |  | 8 |  |  | 4 |  |  |
| Actuated Green，G（s） | 6.2 | 40.7 | 40.7 |  | 30.0 |  |  | 19.6 |  |  | 19.6 |  |
| Effective Green，g（s） | 6.7 | 40.7 | 40.7 |  | 30.0 |  |  | 21.1 |  |  | 21.1 |  |
| Actuated g／C Ratio | 0.10 | 0.58 | 0.58 |  | 0.43 |  |  | 0.30 |  |  | 0.30 |  |
| Clearance Time（s） | 4.5 | 4.0 | 4.0 |  | 4.0 |  |  | 5.5 |  |  | 5.5 |  |
| Vehicle Extension（s） | 2.3 | 5.8 | 5.8 |  | 5.8 |  |  | 2.5 |  |  | 2.5 |  |
| Lane Grp Cap（vph） | 150 | 1715 | 800 |  | 1357 |  |  | 290 |  |  | 421 |  |
| v／s Ratio Prot | 0.04 | c0．32 |  |  | c0．34 |  |  |  |  |  |  |  |
| v／s Ratio Perm |  |  | 0.02 |  |  |  |  | c0．16 |  |  | 0.06 |  |
| v／c Ratio | 0.41 | 0.55 | 0.03 |  | 0.79 |  |  | 0.54 |  |  | 0.19 |  |
| Uniform Delay，d1 | 29.7 | 8.9 | 6.2 |  | 17.2 |  |  | 20.3 |  |  | 18.0 |  |
| Progression Factor | 1.00 | 1.00 | 1.00 |  | 1.00 |  |  | 1.00 |  |  | 1.00 |  |
| Incremental Delay，d2 | 1.0 | 0.8 | 0.0 |  | 3.9 |  |  | 1.6 |  |  | 0.2 |  |
| Delay（s） | 30.7 | 9.7 | 6.2 |  | 21.1 |  |  | 22.0 |  |  | 18.2 |  |
| Level of Service | C | A | A |  | C |  |  | C |  |  | B |  |
| Approach Delay（s） |  | 10.8 |  |  | 21.1 |  |  | 22.0 |  |  | 18.2 |  |
| Approach LOS |  | B |  |  | C |  |  | C |  |  | B |  |


| Intersection Summary |  |  | B |
| :--- | ---: | :--- | ---: |
| HCM 2000 Control Delay | 16.6 | HCM 2000 Level of Service |  |
| HCM 2000 Volume to Capacity ratio | 0.70 |  | 12.0 |
| Actuated Cycle Length（s） | 69.8 | Sum of lost time（s） | C |
| Intersection Capacity Utilization | $66.8 \%$ | ICU Level of Service |  |
| Analysis Period（min） | 15 |  |  |
| C Critical Lane Group |  |  |  |

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HCM 6th Signalized Intersection Summary
3: Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26)


Notes

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Lanes, Volumes, Timings
3: Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26)

|  | 4 |  |  | $\checkmark$ |  | 4 | 4 | 4 | $p$ |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ${ }^{*}$ | 性 | F | \% | 性 |  |  | \$ |  |  | ¢ |  |
| Traffic Volume (vph) | 157 | 1139 | 139 | 5 | 1173 | 17 | 121 | 25 | 7 | 40 | 24 | 104 |
| Future Volume (vph) | 157 | 1139 | 139 | 5 | 1173 | 17 | 121 | 25 | 7 | 40 | 24 | 104 |
| Ideal Flow (vphpl) | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 |
| Storage Length (ft) | 115 |  | 100 | 105 |  | 0 | 0 |  | 0 | 0 |  | 0 |
| Storage Lanes | 1 |  | 1 | 1 |  | 0 | 0 |  | 0 | 0 |  | 0 |
| Taper Length (ft) | 100 |  |  | 100 |  |  | 100 |  |  | 100 |  |  |
| Lane Util. Factor | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 0.95 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Ped Bike Factor | 1.00 |  | 0.98 | 1.00 | 1.00 |  |  | 1.00 |  |  | 0.99 |  |
| Frt |  |  | 0.850 |  | 0.998 |  |  | 0.994 |  |  | 0.917 |  |
| Flt Protected | 0.950 |  |  | 0.950 |  |  |  | 0.962 |  |  | 0.988 |  |
| Satd. Flow (prot) | 1630 | 3228 | 1458 | 1630 | 3161 | 0 | 0 | 1625 | 0 | 0 | 1534 | 0 |
| Flt Permitted | 0.950 |  |  | 0.950 |  |  |  | 0.549 |  |  | 0.906 |  |
| Satd. Flow (perm) | 1629 | 3228 | 1426 | 1629 | 3161 | 0 | 0 | 924 | 0 | 0 | 1407 | 0 |
| Right Turn on Red |  |  | Yes |  |  | Yes |  |  | Yes |  |  | Yes |
| Satd. Flow (RTOR) |  |  | 95 |  | 2 |  |  | 2 |  |  | 65 |  |
| Link Speed (mph) |  | 25 |  |  | 25 |  |  | 25 |  |  | 25 |  |
| Link Distance (ft) |  | 468 |  |  | 302 |  |  | 365 |  |  | 210 |  |
| Travel Time (s) |  | 12.8 |  |  | 8.2 |  |  | 10.0 |  |  | 5.7 |  |
| Confl. Peds. (\#/hr) | 2 |  | 1 | 1 |  | 2 | 5 |  |  |  |  | 5 |
| Peak Hour Factor | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 |
| Heavy Vehicles (\%) | 2\% | 3\% | 2\% | 2\% | 5\% | 2\% | 2\% | 8\% | 2\% | 2\% | 2\% | 2\% |
| Adj. Flow (vph) | 167 | 1212 | 148 | 5 | 1248 | 18 | 129 | 27 | 7 | 43 | 26 | 111 |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow (vph) | 167 | 1212 | 148 | 5 | 1266 | 0 | 0 | 163 | 0 | 0 | 180 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | L NA | R NA | R NA | LNA | R NA | R NA | LNA | R NA | R NA | L NA | R NA | R NA |
| Median Width(ft) |  | 12 |  |  | 12 |  |  | 0 |  |  | 0 |  |
| Link Offset(ft) |  | -12 |  |  | 0 |  |  | 0 |  |  | 4 |  |
| Crosswalk Width(tt) |  | 72 |  |  | 42 |  |  | 32 |  |  | 30 |  |
| Two way Left Turn Lane |  |  |  |  |  |  |  |  |  |  |  |  |
| Headway Factor | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 |
| Number of Detectors | 2 | 1 | 1 | 2 | 1 |  | 2 | 2 |  | 2 | 2 |  |
| Detector Template | Left | Det25 | Right | Left | Det25 |  | Left | Side St |  | Left | Side St |  |
| Leading Detector (ft) | 78 | 153 | 153 | 78 | 153 |  | 78 | 78 |  | 78 | 78 |  |
| Trailing Detector (ft) | 2 | 137 | 137 | 2 | 137 |  | 2 | 2 |  | 2 | 2 |  |
| Detector 1 Position(ft) | 2 | 137 | 137 | 2 | 137 |  | 2 | 2 |  | 2 | 2 |  |
| Detector 1 Size(ft) | 16 | 16 | 16 | 16 | 16 |  | 16 | 16 |  | 16 | 16 |  |
| Detector 1 Type | Cl+Ex | Cl+Ex | Cl+Ex | Cl+Ex | $\mathrm{Cl}+\mathrm{Ex}$ |  | Cl+Ex | Cl+Ex |  | Cl+Ex | $\mathrm{Cl}+\mathrm{Ex}$ |  |
| Detector 1 Channel |  |  |  |  |  |  |  |  |  |  |  |  |
| Detector 1 Extend (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Detector 1 Queue (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Detector 1 Delay (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Detector 2 Position(ft) | 72 |  |  | 72 |  |  | 72 | 72 |  | 72 | 72 |  |
| Detector 2 Size(ft) | 6 |  |  | 6 |  |  | 6 | 6 |  | 6 | 6 |  |
| Detector 2 Type | $\mathrm{Cl}+\mathrm{Ex}$ |  |  | Cl+Ex |  |  | Cl+Ex | $\mathrm{Cl}+\mathrm{Ex}$ |  | Cl+Ex | $\mathrm{Cl}+\mathrm{Ex}$ |  |
| Detector 2 Channel |  |  |  |  |  |  |  |  |  |  |  |  |
| Detector 2 Extend (s) | 0.0 |  |  | 0.0 |  |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Turn Type | Prot | NA | Perm | Prot | NA |  | Perm | NA |  | Perm | NA |  |

[^6]Synchro 10 Report - by PBS Engineering and Environmental
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Lanes, Volumes, Timings
3: Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26)

|  | 4 |  |  | $\dagger$ |  |  | 4 | $\uparrow$ |  |  | $\frac{1}{*}$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Protected Phases | 5 | 2 |  | 1 | 6 |  |  | 8 |  |  | 4 |  |
| Permitted Phases |  |  | 2 |  |  |  | 8 |  |  | 4 |  |  |
| Detector Phase | 5 | 2 | 2 | 1 | 6 |  | 8 | 8 |  | 4 | 4 |  |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial (s) | 4.0 | 10.0 | 10.0 | 4.0 | 10.0 |  | 6.0 | 6.0 |  | 6.0 | 6.0 |  |
| Minimum Split (s) | 8.5 | 26.0 | 26.0 | 8.5 | 30.0 |  | 11.5 | 11.5 |  | 23.5 | 23.5 |  |
| Total Split (s) | 20.0 | 70.0 | 70.0 | 15.0 | 65.0 |  | 25.0 | 25.0 |  | 25.0 | 25.0 |  |
| Total Split (\%) | 18.2\% | 63.6\% | 63.6\% | 13.6\% | 59.1\% |  | 22.7\% | 22.7\% |  | 22.7\% | 22.7\% |  |
| Maximum Green (s) | 15.5 | 66.0 | 66.0 | 10.5 | 61.0 |  | 19.5 | 19.5 |  | 19.5 | 19.5 |  |
| Yellow Time (s) | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 |  | 4.0 | 4.0 |  | 4.0 | 4.0 |  |
| All-Red Time (s) | 1.0 | 0.5 | 0.5 | 1.0 | 0.5 |  | 1.5 | 1.5 |  | 1.5 | 1.5 |  |
| Lost Time Adjust (s) | -0.5 | 0.0 | 0.0 | -0.5 | 0.0 |  |  | -1.5 |  |  | -1.5 |  |
| Total Lost Time (s) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |  |  | 4.0 |  |  | 4.0 |  |
| Lead/Lag | Lag | Lag | Lag | Lead | Lead |  |  |  |  |  |  |  |
| Lead-Lag Optimize? |  |  |  |  |  |  |  |  |  |  |  |  |
| Vehicle Extension (s) | 2.3 | 5.8 | 5.8 | 2.3 | 5.8 |  | 2.5 | 2.5 |  | 2.5 | 2.5 |  |
| Minimum Gap (s) | 0.5 | 3.8 | 3.8 | 0.5 | 3.8 |  | 2.0 | 2.0 |  | 2.0 | 2.0 |  |
| Time Before Reduce (s) | 8.0 | 10.0 | 10.0 | 8.0 | 10.0 |  | 5.0 | 5.0 |  | 5.0 | 5.0 |  |
| Time To Reduce (s) | 3.0 | 10.0 | 10.0 | 3.0 | 10.0 |  | 5.0 | 5.0 |  | 5.0 | 5.0 |  |
| Recall Mode | None | C-Min | C-Min | None | C-Min |  | None | None |  | None | None |  |
| Walk Time (s) |  | 7.0 | 7.0 |  | 7.0 |  |  |  |  | 7.0 | 7.0 |  |
| Flash Dont Walk (s) |  | 15.0 | 15.0 |  | 19.0 |  |  |  |  | 11.0 | 11.0 |  |
| Pedestrian Calls (\#/hr) |  | 0 | 0 |  | 0 |  |  |  |  | 0 | 0 |  |
| Act Effct Green (s) | 15.2 | 74.6 | 74.6 | 5.8 | 57.5 |  |  | 25.3 |  |  | 25.3 |  |
| Actuated g/C Ratio | 0.14 | 0.68 | 0.68 | 0.05 | 0.52 |  |  | 0.23 |  |  | 0.23 |  |
| v/c Ratio | 0.74 | 0.55 | 0.15 | 0.06 | 0.77 |  |  | 0.76 |  |  | 0.48 |  |
| Control Delay | 65.3 | 10.5 | 2.9 | 50.6 | 24.8 |  |  | 64.2 |  |  | 28.9 |  |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  |  | 0.0 |  |  | 0.0 |  |
| Total Delay | 65.3 | 10.5 | 2.9 | 50.6 | 24.8 |  |  | 64.2 |  |  | 28.9 |  |
| LOS | E | B | A | D | C |  |  | E |  |  | C |  |
| Approach Delay |  | 15.8 |  |  | 24.9 |  |  | 64.2 |  |  | 28.9 |  |
| Approach LOS |  | B |  |  | C |  |  | E |  |  | C |  |

Intersection Summary
Area Type: Other
Cycle Length: 110
Actuated Cycle Length: 110
Offset: $0(0 \%)$, Referenced to phase 2:EBT and 6:WBT, Start of Green, Master Intersection
Natural Cycle: 75
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.77
Intersection Signal Delay: 22.7
Intersection LOS: C
Intersection Capacity Utilization 79.4\%
ICU Level of Service D
Analysis Period (min) 15
Splits and Phases: $\quad 3$ : Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26)


2022 Without Project Conditions - Weekday PM Peak Hour

Queues
3: Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26)

|  | 4 | $\rightarrow$ | \% | 7 | $\stackrel{ }{\sim}$ | 4 | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | NBT | SBT |
| Lane Group Flow (vph) | 167 | 1212 | 148 | 5 | 1266 | 163 | 180 |
| $\mathrm{v} / \mathrm{c}$ Ratio | 0.74 | 0.55 | 0.15 | 0.06 | 0.77 | 0.76 | 0.48 |
| Control Delay | 65.3 | 10.5 | 2.9 | 50.6 | 24.8 | 64.2 | 28.9 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Delay | 65.3 | 10.5 | 2.9 | 50.6 | 24.8 | 64.2 | 28.9 |
| Queue Length 50th (ft) | 113 | 218 | 12 | 3 | 397 | 102 | 66 |
| Queue Length 95th (ft) | \#207 | 297 | 37 | 16 | 416 | \#245 | 147 |
| Internal Link Dist (ft) |  | 388 |  |  | 222 | 285 | 130 |
| Turn Bay Length (ft) | 115 |  | 100 | 105 |  |  |  |
| Base Capacity (vph) | 247 | 2189 | 997 | 163 | 1758 | 214 | 373 |
| Starvation Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Spillback Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Storage Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Reduced v/c Ratio | 0.68 | 0.55 | 0.15 | 0.03 | 0.72 | 0.76 | 0.48 |

Intersection Summary
\# 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis
3: Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26)

c Critical Lane Group

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HCM 6th Signalized Intersection Summary
3: Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26)


Notes

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Lanes，Volumes，Timings
3：Wolf Drive／Ten Eyck Road \＆Mt．Hood Highway（US 26）

|  | 4 |  | 7 | $\checkmark$ |  | 4 | 4 | 4 | 7 |  | $\dagger$ | $\checkmark$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ${ }^{7}$ | 个4 | 「 | \％ | 性 |  |  | \＄ |  |  | $\uparrow$ |  |
| Traffic Volume（vph） | 80 | 865 | 34 | 0 | 974 | 20 | 141 | 3 | 2 | 23 | 7 | 147 |
| Future Volume（vph） | 80 | 865 | 34 | 0 | 974 | 20 | 141 | 3 | 2 | 23 | 7 | 147 |
| Ideal Flow（vphpl） | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 |
| Storage Length（ft） | 115 |  | 100 | 105 |  | 0 | 0 |  | 0 | 0 |  | 0 |
| Storage Lanes | 1 |  | 1 | 1 |  | 0 | 0 |  | 0 | 0 |  | 0 |
| Taper Length（ft） | 100 |  |  | 100 |  |  | 100 |  |  | 100 |  |  |
| Lane Util．Factor | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 0.95 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Ped Bike Factor |  |  | 0.98 |  | 1.00 |  |  | 1.00 |  |  | 0.99 |  |
| Frt |  |  | 0.850 |  | 0.997 |  |  | 0.998 |  |  | 0.888 |  |
| Flt Protected | 0.950 |  |  |  |  |  |  | 0.954 |  |  | 0.994 |  |
| Satd．Flow（prot） | 1568 | 2942 | 1403 | 1716 | 3158 | 0 | 0 | 1603 | 0 | 0 | 1455 | 0 |
| Flt Permitted | 0.950 |  |  |  |  |  |  | 0.555 |  |  | 0.951 |  |
| Satd．Flow（perm） | 1568 | 2942 | 1372 | 1716 | 3158 | 0 | 0 | 931 | 0 | 0 | 1392 | 0 |
| Right Turn on Red |  |  | Yes |  |  | Yes |  |  | Yes |  |  | Yes |
| Satd．Flow（RTOR） |  |  | 77 |  | 2 |  |  | 1 |  |  | 160 |  |
| Link Speed（mph） |  | 25 |  |  | 25 |  |  | 25 |  |  | 25 |  |
| Link Distance（ ft ） |  | 468 |  |  | 302 |  |  | 365 |  |  | 210 |  |
| Travel Time（s） |  | 12.8 |  |  | 8.2 |  |  | 10.0 |  |  | 5.7 |  |
| Confl．Peds．（\＃／hr） |  |  | 1 | 1 |  |  | 2 |  |  |  |  | 2 |
| Confl．Bikes（\＃／hr） |  |  |  |  |  | 1 |  |  |  |  |  |  |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles（\％） | 6\％ | 13\％ | 6\％ | 2\％ | 5\％ | 2\％ | 4\％ | 2\％ | 2\％ | 24\％ | 2\％ | 2\％ |
| Adj．Flow（vph） | 87 | 940 | 37 | 0 | 1059 | 22 | 153 | 3 | 2 | 25 | 8 | 160 |


| Shared Lane Traffic（\％） |  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Lane Group Flow（vph） | 87 | 940 | 37 | 0 | 1081 | 0 | 0 | 158 | 0 | 0 | 193 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | L NA | R NA | R NA | L NA | R NA | R NA | L NA | R NA | R NA | L NA | R NA | R NA |


| Lane Alignment | NA | RNA | RNA | LNA | RNA | RNA | LNA | RNA | RNA | LNA | RNA | R NA |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Median Width（ft） |  | 12 |  |  | 12 |  |  | 0 |  |  | 0 |  |
| Link Offset（ft） |  | －12 |  |  | 0 |  |  | 0 |  |  | 4 |  |
| Crosswalk Width（ft） |  | 72 |  |  | 42 |  |  | 32 |  |  | 30 |  |
| Two way Left Turn Lane |  |  |  |  |  |  |  |  |  |  |  |  |
| Headway Factor | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 |
| Number of Detectors | 2 | 1 | 1 | 2 | 1 |  | 2 | 2 |  | 2 | 2 |  |
| Detector Template | Left | Det25 | Right | Left | Det25 |  | Left | Side St |  | Left | Side St |  |
| Leading Detector（ft） | 78 | 153 | 153 | 78 | 153 |  | 78 | 78 |  | 78 | 78 |  |
| Trailing Detector（ft） | 2 | 137 | 137 | 2 | 137 |  | 2 | 2 |  | 2 | 2 |  |
| Detector 1 Position（ft） | 2 | 137 | 137 | 2 | 137 |  | 2 | 2 |  | 2 | 2 |  |
| Detector 1 Size（ft） | 16 | 16 | 16 | 16 | 16 |  | 16 | 16 |  | 16 | 16 |  |
| Detector 1 Type | Cl＋Ex | Cl＋Ex | Cl＋Ex | Cl＋Ex | Cl＋Ex |  | Cl＋Ex | Cl＋Ex |  | Cl＋Ex | $\mathrm{Cl}+\mathrm{Ex}$ |  |
| Detector 1 Channel |  |  |  |  |  |  |  |  |  |  |  |  |
| Detector 1 Extend（s） | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Detector 1 Queue（s） | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Detector 1 Delay（s） | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Detector 2 Position（ft） | 72 |  |  | 72 |  |  | 72 | 72 |  | 72 | 72 |  |
| Detector 2 Size（ft） | 6 |  |  | 6 |  |  | 6 | 6 |  | 6 | 6 |  |
| Detector 2 Type | Cl＋Ex |  |  | Cl＋Ex |  |  | Cl＋Ex | Cl＋Ex |  | Cl＋Ex | $\mathrm{Cl}+\mathrm{Ex}$ |  |
| Detector 2 Channel |  |  |  |  |  |  |  |  |  |  |  |  |
| Detector 2 Extend（s） | 0.0 |  |  | 0.0 |  |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |

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Lanes, Volumes, Timings
3: Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26)

|  | $\stackrel{ }{*}$ |  |  |  |  |  | 4 | 4 |  |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Turn Type | Prot | NA | Perm | Prot | NA |  | Perm | NA |  | Perm | NA |  |
| Protected Phases | 5 | 2 |  | 1 | 6 |  |  | 8 |  |  | 4 |  |
| Permitted Phases |  |  | 2 |  |  |  | 8 |  |  | 4 |  |  |
| Detector Phase | 5 | 2 | 2 | 1 | 6 |  | 8 | 8 |  | 4 | 4 |  |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial (s) | 4.0 | 10.0 | 10.0 | 4.0 | 10.0 |  | 6.0 | 6.0 |  | 6.0 | 6.0 |  |
| Minimum Split (s) | 8.5 | 26.0 | 26.0 | 8.5 | 30.0 |  | 11.5 | 11.5 |  | 23.5 | 23.5 |  |
| Total Split (s) | 29.5 | 54.0 | 54.0 | 19.5 | 44.0 |  | 25.5 | 25.5 |  | 25.5 | 25.5 |  |
| Total Split (\%) | 29.8\% | 54.5\% | 54.5\% | 19.7\% | 44.4\% |  | 25.8\% | 25.8\% |  | 25.8\% | 25.8\% |  |
| Maximum Green (s) | 25.0 | 50.0 | 50.0 | 15.0 | 40.0 |  | 20.0 | 20.0 |  | 20.0 | 20.0 |  |
| Yellow Time (s) | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 |  | 4.0 | 4.0 |  | 4.0 | 4.0 |  |
| All-Red Time (s) | 1.0 | 0.5 | 0.5 | 1.0 | 0.5 |  | 1.5 | 1.5 |  | 1.5 | 1.5 |  |
| Lost Time Adjust (s) | -0.5 | 0.0 | 0.0 | -0.5 | 0.0 |  |  | -1.5 |  |  | -1.5 |  |
| Total Lost Time (s) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |  |  | 4.0 |  |  | 4.0 |  |
| Lead/Lag | Lag | Lag | Lag | Lead | Lead |  |  |  |  |  |  |  |
| Lead-Lag Optimize? |  |  |  |  |  |  |  |  |  |  |  |  |
| Vehicle Extension (s) | 2.3 | 5.8 | 5.8 | 2.3 | 5.8 |  | 2.5 | 2.5 |  | 2.5 | 2.5 |  |
| Minimum Gap (s) | 0.5 | 3.8 | 3.8 | 0.5 | 3.8 |  | 2.0 | 2.0 |  | 2.0 | 2.0 |  |
| Time Before Reduce (s) | 8.0 | 10.0 | 10.0 | 8.0 | 10.0 |  | 5.0 | 5.0 |  | 5.0 | 5.0 |  |
| Time To Reduce (s) | 3.0 | 10.0 | 10.0 | 3.0 | 10.0 |  | 5.0 | 5.0 |  | 5.0 | 5.0 |  |
| Recall Mode | None | Min | Min | None | Min |  | None | None |  | None | None |  |
| Walk Time (s) |  | 7.0 | 7.0 |  | 7.0 |  |  |  |  | 7.0 | 7.0 |  |
| Flash Dont Walk (s) |  | 15.0 | 15.0 |  | 19.0 |  |  |  |  | 11.0 | 11.0 |  |
| Pedestrian Calls (\#/hr) |  | 0 | 0 |  | 0 |  |  |  |  | 0 | 0 |  |
| Act Effct Green (s) | 9.6 | 41.4 | 41.4 |  | 30.6 |  |  | 21.9 |  |  | 21.9 |  |
| Actuated g/C Ratio | 0.13 | 0.58 | 0.58 |  | 0.43 |  |  | 0.31 |  |  | 0.31 |  |
| v/c Ratio | 0.42 | 0.55 | 0.04 |  | 0.80 |  |  | 0.55 |  |  | 0.36 |  |
| Control Delay | 38.3 | 10.0 | 0.4 |  | 23.7 |  |  | 35.6 |  |  | 9.2 |  |
| Queue Delay | 0.0 | 0.0 | 0.0 |  | 0.0 |  |  | 0.0 |  |  | 0.0 |  |
| Total Delay | 38.3 | 10.0 | 0.4 |  | 23.7 |  |  | 35.6 |  |  | 9.2 |  |
| LOS | D | A | A |  | C |  |  | D |  |  | A |  |
| Approach Delay |  | 11.9 |  |  | 23.7 |  |  | 35.6 |  |  | 9.2 |  |
| Approach LOS |  | B |  |  | C |  |  | D |  |  | A |  |

Intersection Summary
Area Type: Other
Cycle Length: 99
Actuated Cycle Length: 71.7
Natural Cycle: 65
Control Type: Actuated-Uncoordinated
Maximum v/c Ratio: 0.80
Intersection Signal Delay: 18.3
Intersection LOS: B
Intersection Capacity Utilization 68.9\%
ICU Level of Service C
Analysis Period (min) 15
Splits and Phases: $\quad 3$ : Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26)


2022 With Project Conditions - Weekday AM Peak Hour

Queues
3: Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26)

|  | $\rangle$ | $\rightarrow$ | 7 | $\leftrightarrow$ | $\dagger$ | $\frac{1}{7}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBT | NBT | SBT |
| Lane Group Flow (vph) | 87 | 940 | 37 | 1081 | 158 | 193 |
| v/c Ratio | 0.42 | 0.55 | 0.04 | 0.80 | 0.55 | 0.36 |
| Control Delay | 38.3 | 10.0 | 0.4 | 23.7 | 35.6 | 9.2 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Delay | 38.3 | 10.0 | 0.4 | 23.7 | 35.6 | 9.2 |
| Queue Length 50th (ft) | 38 | 118 | 0 | 223 | 63 | 11 |
| Queue Length 95th (ft) | 88 | 160 | 2 | 327 | \#171 | 69 |
| Internal Link Dist (ft) |  | 388 |  | 222 | 285 | 130 |
| Turn Bay Length (ft) | 115 |  | 100 |  |  |  |
| Base Capacity (vph) | 587 | 2179 | 1036 | 1858 | 295 | 549 |
| Starvation Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 |
| Spillback Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 |
| Storage Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 |
| Reduced v/c Ratio | 0.15 | 0.43 | 0.04 | 0.58 | 0.54 | 0.35 |

Intersection Summary
\# 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis
3：Wolf Drive／Ten Eyck Road \＆Mt．Hood Highway（US 26）

|  | 4 | $\rightarrow$ |  | 7 |  | 4 |  | 4 |  |  |  | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \％ | 个个 | 「 | ＊ | 中 ${ }_{\text {c }}$ |  |  | \＄ |  |  | ¢ |  |
| Traffic Volume（vph） | 80 | 865 | 34 | 0 | 974 | 20 | 141 | 3 | 2 | 23 | 7 | 147 |
| Future Volume（vph） | 80 | 865 | 34 | 0 | 974 | 20 | 141 | 3 | 2 | 23 | 7 | 147 |
| Ideal Flow（vphpl） | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 |
| Total Lost time（s） | 4.0 | 4.0 | 4.0 |  | 4.0 |  |  | 4.0 |  |  | 4.0 |  |
| Lane Util．Factor | 1.00 | 0.95 | 1.00 |  | 0.95 |  |  | 1.00 |  |  | 1.00 |  |
| Frpb，ped／bikes | 1.00 | 1.00 | 0.98 |  | 1.00 |  |  | 1.00 |  |  | 0.99 |  |
| Flpb，ped／bikes | 1.00 | 1.00 | 1.00 |  | 1.00 |  |  | 1.00 |  |  | 1.00 |  |
| Frt | 1.00 | 1.00 | 0.85 |  | 1.00 |  |  | 1.00 |  |  | 0.89 |  |
| Flt Protected | 0.95 | 1.00 | 1.00 |  | 1.00 |  |  | 0.95 |  |  | 0.99 |  |
| Satd．Flow（prot） | 1568 | 2942 | 1373 |  | 3157 |  |  | 1601 |  |  | 1455 |  |
| Flt Permitted | 0.95 | 1.00 | 1.00 |  | 1.00 |  |  | 0.55 |  |  | 0.95 |  |
| Satd．Flow（perm） | 1568 | 2942 | 1373 |  | 3157 |  |  | 931 |  |  | 1393 |  |
| Peak－hour factor，PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj．Flow（vph） | 87 | 940 | 37 | 0 | 1059 | 22 | 153 | 3 | 2 | 25 | 8 | 160 |
| RTOR Reduction（vph） | 0 | 0 | 15 | 0 | 1 | 0 | 0 | 1 | 0 | 0 | 112 | 0 |
| Lane Group Flow（vph） | 87 | 940 | 22 | 0 | 1080 | 0 | 0 | 157 | 0 | 0 | 81 | 0 |
| Confl．Peds．（\＃／hr） |  |  | 1 | 1 |  |  | 2 |  |  |  |  | 2 |
| Confl．Bikes（\＃／hr） |  |  |  |  |  | 1 |  |  |  |  |  |  |
| Heavy Vehicles（\％） | 6\％ | 13\％ | 6\％ | 2\％ | 5\％ | 2\％ | 4\％ | 2\％ | 2\％ | 24\％ | 2\％ | 2\％ |
| Turn Type | Prot | NA | Perm | Prot | NA |  | Perm | NA |  | Perm | NA |  |
| Protected Phases | 5 | 2 |  | 1 | 6 |  |  | 8 |  |  | 4 |  |
| Permitted Phases |  |  | 2 |  |  |  | 8 |  |  | 4 |  |  |
| Actuated Green，G（s） | 7.4 | 42.5 | 42.5 |  | 30.6 |  |  | 20.3 |  |  | 20.3 |  |
| Effective Green，g（s） | 7.9 | 42.5 | 42.5 |  | 30.6 |  |  | 21.8 |  |  | 21.8 |  |
| Actuated g／C Ratio | 0.11 | 0.59 | 0.59 |  | 0.42 |  |  | 0.30 |  |  | 0.30 |  |
| Clearance Time（s） | 4.5 | 4.0 | 4.0 |  | 4.0 |  |  | 5.5 |  |  | 5.5 |  |
| Vehicle Extension（s） | 2.3 | 5.8 | 5.8 |  | 5.8 |  |  | 2.5 |  |  | 2.5 |  |
| Lane Grp Cap（vph） | 171 | 1729 | 807 |  | 1336 |  |  | 280 |  |  | 420 |  |
| v／s Ratio Prot | 0.06 | c0．32 |  |  | c0．34 |  |  |  |  |  |  |  |
| v／s Ratio Perm |  |  | 0.02 |  |  |  |  | c0．17 |  |  | 0.06 |  |
| v／c Ratio | 0.51 | 0.54 | 0.03 |  | 0.81 |  |  | 0.56 |  |  | 0.19 |  |
| Uniform Delay，d1 | 30.4 | 9.0 | 6.2 |  | 18.3 |  |  | 21.2 |  |  | 18.7 |  |
| Progression Factor | 1.00 | 1.00 | 1.00 |  | 1.00 |  |  | 1.00 |  |  | 1.00 |  |
| Incremental Delay，d2 | 1.4 | 0.8 | 0.0 |  | 4.4 |  |  | 2.1 |  |  | 0.2 |  |
| Delay（s） | 31.8 | 9.8 | 6.3 |  | 22.7 |  |  | 23.3 |  |  | 18.9 |  |
| Level of Service | C | A | A |  | C |  |  | C |  |  | B |  |
| Approach Delay（s） |  | 11.5 |  |  | 22.7 |  |  | 23.3 |  |  | 18.9 |  |
| Approach LOS |  | B |  |  | C |  |  | C |  |  | B |  |


| Intersection Summary |  |  |  |
| :--- | ---: | :--- | ---: |
| HCM 2000 Control Delay | 17.6 | HCM 2000 Level of Service | B |
| HCM 2000 Volume to Capacity ratio | 0.71 |  | 12.0 |
| Actuated Cycle Length（s） | 72.3 | Sum of lost time（s） | C |
| Intersection Capacity Utilization | $68.9 \%$ | ICU Level of Service |  |
| Analysis Period（min） | 15 |  |  |
| C Critical Lane Group |  |  |  |

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HCM 6th Signalized Intersection Summary
3: Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26)


Notes

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

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Lanes, Volumes, Timings
3: Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26)

|  | $\stackrel{ }{ }$ |  |  | $\checkmark$ |  | 4 | 4 | 4 | $p$ |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \% | 性 | F | \% | 性 |  |  | \$ |  |  | ¢ |  |
| Traffic Volume (vph) | 165 | 1139 | 139 | 5 | 1173 | 18 | 121 | 25 | 7 | 42 | 24 | 124 |
| Future Volume (vph) | 165 | 1139 | 139 | 5 | 1173 | 18 | 121 | 25 | 7 | 42 | 24 | 124 |
| Ideal Flow (vphpl) | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 |
| Storage Length (ft) | 115 |  | 100 | 105 |  | 0 | 0 |  | 0 | 0 |  | 0 |
| Storage Lanes | 1 |  | 1 | 1 |  | 0 | 0 |  | 0 | 0 |  | 0 |
| Taper Length (ft) | 100 |  |  | 100 |  |  | 100 |  |  | 100 |  |  |
| Lane Util. Factor | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 0.95 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Ped Bike Factor | 1.00 |  | 0.98 | 1.00 | 1.00 |  |  | 1.00 |  |  | 0.99 |  |
| Frt |  |  | 0.850 |  | 0.998 |  |  | 0.994 |  |  | 0.912 |  |
| Flt Protected | 0.950 |  |  | 0.950 |  |  |  | 0.962 |  |  | 0.989 |  |
| Satd. Flow (prot) | 1630 | 3228 | 1458 | 1630 | 3161 | 0 | 0 | 1625 | 0 | 0 | 1526 | 0 |
| Flt Permitted | 0.950 |  |  | 0.950 |  |  |  | 0.529 |  |  | 0.910 |  |
| Satd. Flow (perm) | 1629 | 3228 | 1426 | 1629 | 3161 | 0 | 0 | 890 | 0 | 0 | 1404 | 0 |
| Right Turn on Red |  |  | Yes |  |  | Yes |  |  | Yes |  |  | Yes |
| Satd. Flow (RTOR) |  |  | 95 |  | 2 |  |  | 2 |  |  | 75 |  |
| Link Speed (mph) |  | 25 |  |  | 25 |  |  | 25 |  |  | 25 |  |
| Link Distance (ft) |  | 468 |  |  | 302 |  |  | 365 |  |  | 210 |  |
| Travel Time (s) |  | 12.8 |  |  | 8.2 |  |  | 10.0 |  |  | 5.7 |  |
| Confl. Peds. (\#/hr) | 2 |  | 1 | 1 |  | 2 | 5 |  |  |  |  | 5 |
| Peak Hour Factor | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 |
| Heavy Vehicles (\%) | 2\% | 3\% | 2\% | 2\% | 5\% | 2\% | 2\% | 8\% | 2\% | 2\% | 2\% | 2\% |
| Adj. Flow (vph) | 176 | 1212 | 148 | 5 | 1248 | 19 | 129 | 27 | 7 | 45 | 26 | 132 |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow (vph) | 176 | 1212 | 148 | 5 | 1267 | 0 | 0 | 163 | 0 | 0 | 203 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | L NA | R NA | R NA | LNA | R NA | R NA | LNA | R NA | R NA | LNA | R NA | R NA |
| Median Width(ft) |  | 12 |  |  | 12 |  |  | 0 |  |  | 0 |  |
| Link Offset(ft) |  | -12 |  |  | 0 |  |  | 0 |  |  | 4 |  |
| Crosswalk Width(tt) |  | 72 |  |  | 42 |  |  | 32 |  |  | 30 |  |
| Two way Left Turn Lane |  |  |  |  |  |  |  |  |  |  |  |  |
| Headway Factor | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 |
| Number of Detectors | 2 | 1 | 1 | 2 | 1 |  | 2 | 2 |  | 2 | 2 |  |
| Detector Template | Left | Det25 | Right | Left | Det25 |  | Left | Side St |  | Left | Side St |  |
| Leading Detector (ft) | 78 | 153 | 153 | 78 | 153 |  | 78 | 78 |  | 78 | 78 |  |
| Trailing Detector (ft) | 2 | 137 | 137 | 2 | 137 |  | 2 | 2 |  | 2 | 2 |  |
| Detector 1 Position(ft) | 2 | 137 | 137 | 2 | 137 |  | 2 | 2 |  | 2 | 2 |  |
| Detector 1 Size(ft) | 16 | 16 | 16 | 16 | 16 |  | 16 | 16 |  | 16 | 16 |  |
| Detector 1 Type | Cl+Ex | Cl+Ex | Cl+Ex | Cl+Ex | $\mathrm{Cl}+\mathrm{Ex}$ |  | Cl+Ex | Cl+Ex |  | Cl+Ex | $\mathrm{Cl}+\mathrm{Ex}$ |  |
| Detector 1 Channel |  |  |  |  |  |  |  |  |  |  |  |  |
| Detector 1 Extend (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Detector 1 Queue (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Detector 1 Delay (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Detector 2 Position(ft) | 72 |  |  | 72 |  |  | 72 | 72 |  | 72 | 72 |  |
| Detector 2 Size(ft) | 6 |  |  | 6 |  |  | 6 | 6 |  | 6 | 6 |  |
| Detector 2 Type | Cl+Ex |  |  | Cl+Ex |  |  | Cl+Ex | $\mathrm{Cl}+\mathrm{Ex}$ |  | Cl+Ex | $\mathrm{Cl}+\mathrm{Ex}$ |  |
| Detector 2 Channel |  |  |  |  |  |  |  |  |  |  |  |  |
| Detector 2 Extend (s) | 0.0 |  |  | 0.0 |  |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Turn Type | Prot | NA | Perm | Prot | NA |  | Perm | NA |  | Perm | NA |  |

[^7]Lanes, Volumes, Timings
3: Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26)

|  | 4 |  |  |  |  |  | 4 |  |  |  | $\frac{1}{*}$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Protected Phases | 5 | 2 |  | 1 | 6 |  |  | 8 |  |  | 4 |  |
| Permitted Phases |  |  | 2 |  |  |  | 8 |  |  | 4 |  |  |
| Detector Phase | 5 | 2 | 2 | 1 | 6 |  | 8 | 8 |  | 4 | 4 |  |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial (s) | 4.0 | 10.0 | 10.0 | 4.0 | 10.0 |  | 6.0 | 6.0 |  | 6.0 | 6.0 |  |
| Minimum Split (s) | 8.5 | 26.0 | 26.0 | 8.5 | 30.0 |  | 11.5 | 11.5 |  | 23.5 | 23.5 |  |
| Total Split (s) | 20.0 | 70.0 | 70.0 | 15.0 | 65.0 |  | 25.0 | 25.0 |  | 25.0 | 25.0 |  |
| Total Split (\%) | 18.2\% | 63.6\% | 63.6\% | 13.6\% | 59.1\% |  | 22.7\% | 22.7\% |  | 22.7\% | 22.7\% |  |
| Maximum Green (s) | 15.5 | 66.0 | 66.0 | 10.5 | 61.0 |  | 19.5 | 19.5 |  | 19.5 | 19.5 |  |
| Yellow Time (s) | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 |  | 4.0 | 4.0 |  | 4.0 | 4.0 |  |
| All-Red Time (s) | 1.0 | 0.5 | 0.5 | 1.0 | 0.5 |  | 1.5 | 1.5 |  | 1.5 | 1.5 |  |
| Lost Time Adjust (s) | -0.5 | 0.0 | 0.0 | -0.5 | 0.0 |  |  | -1.5 |  |  | -1.5 |  |
| Total Lost Time (s) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |  |  | 4.0 |  |  | 4.0 |  |
| Lead/Lag | Lag | Lag | Lag | Lead | Lead |  |  |  |  |  |  |  |
| Lead-Lag Optimize? |  |  |  |  |  |  |  |  |  |  |  |  |
| Vehicle Extension (s) | 2.3 | 5.8 | 5.8 | 2.3 | 5.8 |  | 2.5 | 2.5 |  | 2.5 | 2.5 |  |
| Minimum Gap (s) | 0.5 | 3.8 | 3.8 | 0.5 | 3.8 |  | 2.0 | 2.0 |  | 2.0 | 2.0 |  |
| Time Before Reduce (s) | 8.0 | 10.0 | 10.0 | 8.0 | 10.0 |  | 5.0 | 5.0 |  | 5.0 | 5.0 |  |
| Time To Reduce (s) | 3.0 | 10.0 | 10.0 | 3.0 | 10.0 |  | 5.0 | 5.0 |  | 5.0 | 5.0 |  |
| Recall Mode | None | C-Min | C-Min | None | C-Min |  | None | None |  | None | None |  |
| Walk Time (s) |  | 7.0 | 7.0 |  | 7.0 |  |  |  |  | 7.0 | 7.0 |  |
| Flash Dont Walk (s) |  | 15.0 | 15.0 |  | 19.0 |  |  |  |  | 11.0 | 11.0 |  |
| Pedestrian Calls (\#/hr) |  | 0 | 0 |  | 0 |  |  |  |  | 0 | 0 |  |
| Act Effct Green (s) | 15.7 | 72.6 | 72.6 | 5.8 | 55.0 |  |  | 27.3 |  |  | 27.3 |  |
| Actuated g/C Ratio | 0.14 | 0.66 | 0.66 | 0.05 | 0.50 |  |  | 0.25 |  |  | 0.25 |  |
| v/c Ratio | 0.76 | 0.57 | 0.15 | 0.06 | 0.80 |  |  | 0.73 |  |  | 0.50 |  |
| Control Delay | 66.2 | 11.4 | 2.9 | 50.6 | 27.1 |  |  | 61.0 |  |  | 28.8 |  |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  |  | 0.0 |  |  | 0.0 |  |
| Total Delay | 66.2 | 11.4 | 2.9 | 50.6 | 27.1 |  |  | 61.0 |  |  | 28.8 |  |
| LOS | E | B | A | D | C |  |  | E |  |  | C |  |
| Approach Delay |  | 16.9 |  |  | 27.2 |  |  | 61.0 |  |  | 28.8 |  |
| Approach LOS |  | B |  |  | C |  |  | E |  |  | C |  |

Intersection Summary
Area Type: Other
Cycle Length: 110
Actuated Cycle Length: 110
Offset: $0(0 \%)$, Referenced to phase 2:EBT and 6:WBT, Start of Green, Master Intersection
Natural Cycle: 80
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.80

Intersection Signal Delay: 24.0
Intersection Capacity Utilization 81.2\%
Analysis Period (min) 15

Intersection LOS: C
ICU Level of Service D

Splits and Phases: 3: Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26)


Queues
3: Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26)

|  | $\stackrel{ }{*}$ | $\rightarrow$ | 7 | $\checkmark$ | 4 | 4 | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | NBT | SBT |
| Lane Group Flow (vph) | 176 | 1212 | 148 | 5 | 1267 | 163 | 203 |
| v/c Ratio | 0.76 | 0.57 | 0.15 | 0.06 | 0.80 | 0.73 | 0.50 |
| Control Delay | 66.2 | 11.4 | 2.9 | 50.6 | 27.1 | 61.0 | 28.8 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Delay | 66.2 | 11.4 | 2.9 | 50.6 | 27.1 | 61.0 | 28.8 |
| Queue Length 50th (ft) | 118 | 195 | 11 | 3 | 378 | 107 | 78 |
| Queue Length 95th (ft) | \#224 | 297 | 37 | 16 | 417 | \#250 | 164 |
| Internal Link Dist (ft) |  | 388 |  |  | 222 | 285 | 130 |
| Turn Bay Length (ft) | 115 |  | 100 | 105 |  |  |  |
| Base Capacity (vph) | 250 | 2130 | 973 | 163 | 1753 | 222 | 404 |
| Starvation Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Spillback Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Storage Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Reduced v/c Ratio | 0.70 | 0.57 | 0.15 | 0.03 | 0.72 | 0.73 | 0.50 |

Intersection Summary
\# 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis
3: Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26)


C Critical Lane Group

Sandy Health Clinic - PBS Project 71524.000
2022 With Project Trips Conditions - Weekday PM Peak Hour

Synchro 10 Report - by PBS Engineering and Environmental
Page 4

HCM 6th Signalized Intersection Summary
3: Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26)


Notes

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Sandy Health Clinic - PBS Project 71524.000
2022 With Project Trips Conditions - Weekday PM Peak Hour

Lanes, Volumes, Timings
3: Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26)

|  | 4 | $\rightarrow$ | 7 | $\checkmark$ | - | 4 | 4 | 4 | 7 |  | $\dagger$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \% | 坐 | F | * | 中 ${ }_{\text {c }}$ |  |  | ¢ |  |  | ¢ |  |
| Traffic Volume (vph) | 64 | 982 | 39 | 0 | 1106 | 19 | 160 | 4 | 2 | 25 | 8 | 159 |
| Future Volume (vph) | 64 | 982 | 39 | 0 | 1106 | 19 | 160 | 4 | 2 | 25 | 8 | 159 |
| Ideal Flow (vphpl) | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 |
| Storage Length (ft) | 115 |  | 100 | 105 |  | 0 | 0 |  | 0 | 0 |  | 0 |
| Storage Lanes | 1 |  | 1 | 1 |  | 0 | 0 |  | 0 | 0 |  | 0 |
| Taper Length (ft) | 100 |  |  | 100 |  |  | 100 |  |  | 100 |  |  |
| Lane Util. Factor | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 0.95 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Ped Bike Factor |  |  | 0.98 |  | 1.00 |  |  | 1.00 |  |  | 0.99 |  |
| Frt |  |  | 0.850 |  | 0.997 |  |  | 0.998 |  |  | 0.888 |  |
| Flt Protected | 0.950 |  |  |  |  |  |  | 0.954 |  |  | 0.994 |  |
| Satd. Flow (prot) | 1568 | 2942 | 1403 | 1716 | 3158 | 0 | 0 | 1603 | 0 | 0 | 1455 | 0 |
| Flt Permitted | 0.950 |  |  |  |  |  |  | 0.515 |  |  | 0.947 |  |
| Satd. Flow (perm) | 1568 | 2942 | 1372 | 1716 | 3158 | 0 | 0 | 864 | 0 | 0 | 1386 | 0 |
| Right Turn on Red |  |  | Yes |  |  | Yes |  |  | Yes |  |  | Yes |
| Satd. Flow (RTOR) |  |  | 77 |  | 2 |  |  | 1 |  |  | 173 |  |
| Link Speed (mph) |  | 25 |  |  | 25 |  |  | 25 |  |  | 25 |  |
| Link Distance ( ft ) |  | 468 |  |  | 302 |  |  | 365 |  |  | 210 |  |
| Travel Time (s) |  | 12.8 |  |  | 8.2 |  |  | 10.0 |  |  | 5.7 |  |
| Confl. Peds. (\#/hr) |  |  | 1 | 1 |  |  | 2 |  |  |  |  | 2 |
| Confl. Bikes (\#/hr) |  |  |  |  |  | 1 |  |  |  |  |  |  |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles (\%) | 6\% | 13\% | 6\% | 2\% | 5\% | 2\% | 4\% | 2\% | 2\% | 24\% | 2\% | 2\% |
| Adj. Flow (vph) | 70 | 1067 | 42 | 0 | 1202 | 21 | 174 | 4 | 2 | 27 | 9 | 173 |


| Shared Lane Traffic (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Lane Group Flow (vph) | 70 | 1067 | 42 | 0 | 1223 | 0 | 0 | 180 | 0 | 0 | 209 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | L NA | R NA | R NA | LNA | R NA | R NA | L NA | R NA | R NA | L NA | R NA | R NA |


| Lane Alignment | NA | R NA | R NA | LNA | R NA | R NA | LNA | R NA | R NA | LNA | R NA | R NA |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Median Width(ft) |  | 12 |  |  | 12 |  |  | 0 |  |  | 0 |  |
| Link Offset(ft) |  | -12 |  |  | 0 |  |  | 0 |  |  | 4 |  |
| Crosswalk Width(ft) |  | 72 |  |  | 42 |  |  | 32 |  |  | 30 |  |
| Two way Left Turn Lane |  |  |  |  |  |  |  |  |  |  |  |  |
| Headway Factor | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 |
| Number of Detectors | 2 | 1 | 1 | 2 | 1 |  | 2 | 2 |  | 2 | 2 |  |
| Detector Template | Left | Det25 | Right | Left | Det25 |  | Left | Side St |  | Left | Side St |  |
| Leading Detector (ft) | 78 | 153 | 153 | 78 | 153 |  | 78 | 78 |  | 78 | 78 |  |
| Trailing Detector (ft) | 2 | 137 | 137 | 2 | 137 |  | 2 | 2 |  | 2 | 2 |  |
| Detector 1 Position(ft) | 2 | 137 | 137 | 2 | 137 |  | 2 | 2 |  | 2 | 2 |  |
| Detector 1 Size(ft) | 16 | 16 | 16 | 16 | 16 |  | 16 | 16 |  | 16 | 16 |  |
| Detector 1 Type | Cl+Ex | Cl+Ex | Cl+Ex | Cl+Ex | Cl+Ex |  | Cl+Ex | Cl+Ex |  | Cl+Ex | Cl+Ex |  |
| Detector 1 Channel |  |  |  |  |  |  |  |  |  |  |  |  |
| Detector 1 Extend (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Detector 1 Queue (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Detector 1 Delay (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Detector 2 Position(ft) | 72 |  |  | 72 |  |  | 72 | 72 |  | 72 | 72 |  |
| Detector 2 Size(ft) | 6 |  |  | 6 |  |  | 6 | 6 |  | 6 | 6 |  |
| Detector 2 Type | $\mathrm{Cl}+\mathrm{Ex}$ |  |  | Cl+Ex |  |  | Cl+Ex | $\mathrm{Cl}+\mathrm{Ex}$ |  | $\mathrm{Cl}+\mathrm{Ex}$ | Cl+Ex |  |
| Detector 2 Channel |  |  |  |  |  |  |  |  |  |  |  |  |
| Detector 2 Extend (s) | 0.0 |  |  | 0.0 |  |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |

[^8]Lanes, Volumes, Timings
3: Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26)

|  | $\stackrel{ }{*}$ |  |  |  |  |  | 4 | 4 |  |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Turn Type | Prot | NA | Perm | Prot | NA |  | Perm | NA |  | Perm | NA |  |
| Protected Phases | 5 | 2 |  | 1 | 6 |  |  | 8 |  |  | 4 |  |
| Permitted Phases |  |  | 2 |  |  |  | 8 |  |  | 4 |  |  |
| Detector Phase | 5 | 2 | 2 | 1 | 6 |  | 8 | 8 |  | 4 | 4 |  |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial (s) | 4.0 | 10.0 | 10.0 | 4.0 | 10.0 |  | 6.0 | 6.0 |  | 6.0 | 6.0 |  |
| Minimum Split (s) | 8.5 | 26.0 | 26.0 | 8.5 | 30.0 |  | 11.5 | 11.5 |  | 23.5 | 23.5 |  |
| Total Split (s) | 29.5 | 54.0 | 54.0 | 19.5 | 44.0 |  | 25.5 | 25.5 |  | 25.5 | 25.5 |  |
| Total Split (\%) | 29.8\% | 54.5\% | 54.5\% | 19.7\% | 44.4\% |  | 25.8\% | 25.8\% |  | 25.8\% | 25.8\% |  |
| Maximum Green (s) | 25.0 | 50.0 | 50.0 | 15.0 | 40.0 |  | 20.0 | 20.0 |  | 20.0 | 20.0 |  |
| Yellow Time (s) | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 |  | 4.0 | 4.0 |  | 4.0 | 4.0 |  |
| All-Red Time (s) | 1.0 | 0.5 | 0.5 | 1.0 | 0.5 |  | 1.5 | 1.5 |  | 1.5 | 1.5 |  |
| Lost Time Adjust (s) | -0.5 | 0.0 | 0.0 | -0.5 | 0.0 |  |  | -1.5 |  |  | -1.5 |  |
| Total Lost Time (s) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |  |  | 4.0 |  |  | 4.0 |  |
| Lead/Lag | Lag | Lag | Lag | Lead | Lead |  |  |  |  |  |  |  |
| Lead-Lag Optimize? |  |  |  |  |  |  |  |  |  |  |  |  |
| Vehicle Extension (s) | 2.3 | 5.8 | 5.8 | 2.3 | 5.8 |  | 2.5 | 2.5 |  | 2.5 | 2.5 |  |
| Minimum Gap (s) | 0.5 | 3.8 | 3.8 | 0.5 | 3.8 |  | 2.0 | 2.0 |  | 2.0 | 2.0 |  |
| Time Before Reduce (s) | 8.0 | 10.0 | 10.0 | 8.0 | 10.0 |  | 5.0 | 5.0 |  | 5.0 | 5.0 |  |
| Time To Reduce (s) | 3.0 | 10.0 | 10.0 | 3.0 | 10.0 |  | 5.0 | 5.0 |  | 5.0 | 5.0 |  |
| Recall Mode | None | Min | Min | None | Min |  | None | None |  | None | None |  |
| Walk Time (s) |  | 7.0 | 7.0 |  | 7.0 |  |  |  |  | 7.0 | 7.0 |  |
| Flash Dont Walk (s) |  | 15.0 | 15.0 |  | 19.0 |  |  |  |  | 11.0 | 11.0 |  |
| Pedestrian Calls (\#/hr) |  | 0 | 0 |  | 0 |  |  |  |  | 0 | 0 |  |
| Act Effct Green (s) | 8.8 | 46.5 | 46.5 |  | 36.2 |  |  | 22.1 |  |  | 22.1 |  |
| Actuated g/C Ratio | 0.11 | 0.61 | 0.61 |  | 0.47 |  |  | 0.29 |  |  | 0.29 |  |
| v/c Ratio | 0.39 | 0.60 | 0.05 |  | 0.82 |  |  | 0.72 |  |  | 0.40 |  |
| Control Delay | 40.5 | 10.4 | 0.5 |  | 23.8 |  |  | 47.8 |  |  | 9.5 |  |
| Queue Delay | 0.0 | 0.0 | 0.0 |  | 0.0 |  |  | 0.0 |  |  | 0.0 |  |
| Total Delay | 40.5 | 10.4 | 0.5 |  | 23.8 |  |  | 47.8 |  |  | 9.5 |  |
| LOS | D | B | A |  | C |  |  | D |  |  | A |  |
| Approach Delay |  | 11.9 |  |  | 23.8 |  |  | 47.8 |  |  | 9.5 |  |
| Approach LOS |  | B |  |  | C |  |  | D |  |  | A |  |

Intersection Summary
Area Type: Other
Cycle Length: 99
Actuated Cycle Length: 76.8
Natural Cycle: 65
Control Type: Actuated-Uncoordinated
Maximum v/c Ratio: 0.82
Intersection Signal Delay: 19.2
Intersection LOS: B
Intersection Capacity Utilization 73.9\%
ICU Level of Service D
Analysis Period (min) 15
Splits and Phases: 3: Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26)


2029 Without Project Conditions - Weekday AM Peak Hour

Queues
3: Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26)

|  | 4 |  | $\cdots$ | 4 | $\dagger$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBT | NBT | SBT |
| Lane Group Flow (vph) | 70 | 1067 | 42 | 1223 | 180 | 209 |
| v/c Ratio | 0.39 | 0.60 | 0.05 | 0.82 | 0.72 | 0.40 |
| Control Delay | 40.5 | 10.4 | 0.5 | 23.8 | 47.8 | 9.5 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Delay | 40.5 | 10.4 | 0.5 | 23.8 | 47.8 | 9.5 |
| Queue Length 50th ( ft ) | 34 | 143 | 0 | 264 | 86 | 14 |
| Queue Length 95th (ft) | 74 | 193 | 4 | 385 | \#210 | 73 |
| Internal Link Dist (ft) |  | 388 |  | 222 | 285 | 130 |
| Turn Bay Length (ft) | 115 |  | 100 |  |  |  |
| Base Capacity (vph) | 535 | 2063 | 985 | 1692 | 249 | 522 |
| Starvation Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 |
| Spillback Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 |
| Storage Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 |
| Reduced v/c Ratio | 0.13 | 0.52 | 0.04 | 0.72 | 0.72 | 0.40 |
| Intersection Summary |  |  |  |  |  |  |
| \# 95th percentile volume exceeds capacity, queue may be longer.Queue shown is maximum after two cycles. |  |  |  |  |  |  |
|  |  |  |  |  |  |  |

HCM Signalized Intersection Capacity Analysis
3：Wolf Drive／Ten Eyck Road \＆Mt．Hood Highway（US 26）

|  | 4 | $\rightarrow$ |  | 7 |  |  | 4 | 4 | $>$ |  |  | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ${ }^{*}$ | 个4 | 「 | ${ }^{*}$ | 性 |  |  | \＄ |  |  | ¢ |  |
| Traffic Volume（vph） | 64 | 982 | 39 | 0 | 1106 | 19 | 160 | ， | 2 | 25 | 8 | 159 |
| Future Volume（vph） | 64 | 982 | 39 | 0 | 1106 | 19 | 160 | 4 | 2 | 25 | 8 | 159 |
| Ideal Flow（vphpl） | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 |
| Total Lost time（s） | 4.0 | 4.0 | 4.0 |  | 4.0 |  |  | 4.0 |  |  | 4.0 |  |
| Lane Util．Factor | 1.00 | 0.95 | 1.00 |  | 0.95 |  |  | 1.00 |  |  | 1.00 |  |
| Frpb，ped／bikes | 1.00 | 1.00 | 0.98 |  | 1.00 |  |  | 1.00 |  |  | 0.99 |  |
| Flpb，ped／bikes | 1.00 | 1.00 | 1.00 |  | 1.00 |  |  | 1.00 |  |  | 1.00 |  |
| Frt | 1.00 | 1.00 | 0.85 |  | 1.00 |  |  | 1.00 |  |  | 0.89 |  |
| Flt Protected | 0.95 | 1.00 | 1.00 |  | 1.00 |  |  | 0.95 |  |  | 0.99 |  |
| Satd．Flow（prot） | 1568 | 2942 | 1373 |  | 3159 |  |  | 1601 |  |  | 1455 |  |
| Flt Permitted | 0.95 | 1.00 | 1.00 |  | 1.00 |  |  | 0.52 |  |  | 0.95 |  |
| Satd．Flow（perm） | 1568 | 2942 | 1373 |  | 3159 |  |  | 865 |  |  | 1387 |  |
| Peak－hour factor，PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj．Flow（vph） | 70 | 1067 | 42 | 0 | 1202 | 21 | 174 | 4 | 2 | 27 | 9 | 173 |
| RTOR Reduction（vph） | 0 | 0 | 16 | 0 | 1 | 0 | 0 | 1 | 0 | 0 | 124 | 0 |
| Lane Group Flow（vph） | 70 | 1067 | 26 | 0 | 1222 | 0 | 0 | 179 | 0 | 0 | 85 | 0 |
| Confl．Peds．（\＃／hr） |  |  | 1 | 1 |  |  | 2 |  |  |  |  | 2 |
| Confl．Bikes（\＃／hr） |  |  |  |  |  | 1 |  |  |  |  |  |  |
| Heavy Vehicles（\％） | 6\％ | 13\％ | 6\％ | 2\％ | 5\％ | 2\％ | 4\％ | 2\％ | 2\％ | 24\％ | 2\％ | 2\％ |
| Turn Type | Prot | NA | Perm | Prot | NA |  | Perm | NA |  | Perm | NA |  |
| Protected Phases | 5 | 2 |  | 1 | 6 |  |  | 8 |  |  | 4 |  |
| Permitted Phases |  |  | 2 |  |  |  | 8 |  |  | 4 |  |  |
| Actuated Green，G（s） | 6.9 | 47.6 | 47.6 |  | 36.2 |  |  | 20.6 |  |  | 20.6 |  |
| Effective Green，g（s） | 7.4 | 47.6 | 47.6 |  | 36.2 |  |  | 22.1 |  |  | 22.1 |  |
| Actuated g／C Ratio | 0.10 | 0.61 | 0.61 |  | 0.47 |  |  | 0.28 |  |  | 0.28 |  |
| Clearance Time（s） | 4.5 | 4.0 | 4.0 |  | 4.0 |  |  | 5.5 |  |  | 5.5 |  |
| Vehicle Extension（s） | 2.3 | 5.8 | 5.8 |  | 5.8 |  |  | 2.5 |  |  | 2.5 |  |
| Lane Grp Cap（vph） | 149 | 1802 | 841 |  | 1471 |  |  | 246 |  |  | 394 |  |
| v／s Ratio Prot | 0.04 | c0．36 |  |  | c0．39 |  |  |  |  |  |  |  |
| v／s Ratio Perm |  |  | 0.02 |  |  |  |  | c0．21 |  |  | 0.06 |  |
| v／c Ratio | 0.47 | 0.59 | 0.03 |  | 0.83 |  |  | 0.73 |  |  | 0.22 |  |
| Uniform Delay，d1 | 33.3 | 9.1 | 5.9 |  | 18.1 |  |  | 25.1 |  |  | 21.2 |  |
| Progression Factor | 1.00 | 1.00 | 1.00 |  | 1.00 |  |  | 1.00 |  |  | 1.00 |  |
| Incremental Delay，d2 | 1.4 | 0.9 | 0.0 |  | 4.8 |  |  | 9.7 |  |  | 0.2 |  |
| Delay（s） | 34.7 | 10.1 | 6.0 |  | 22.9 |  |  | 34.8 |  |  | 21.4 |  |
| Level of Service | C | B | A |  | C |  |  | C |  |  | C |  |
| Approach Delay（s） |  | 11.4 |  |  | 22.9 |  |  | 34.8 |  |  | 21.4 |  |
| Approach LOS |  | B |  |  | C |  |  | C |  |  | C |  |


| Intersection Summary |  |  |  |  |
| :--- | ---: | :--- | ---: | :--- |
| HCM 2000 Control Delay | 18.7 | HCM 2000 Level of Service | B |  |
| HCM 2000 Volume to Capacity ratio | 0.79 |  | 12.0 |  |
| Actuated Cycle Length（s） | 77.7 | Sum of lost time（s） | D |  |
| Intersection Capacity Utilization | $73.9 \%$ | ICU Level of Service |  |  |
| Analysis Period（min） | 15 |  |  |  |
| C Critical Lane Group |  |  |  |  |

Sandy Health Clinic－PBS Project 71524.000 Synchro 10 Report－by PBS Engineering and Environmental
2029 Without Project Conditions－Weekday AM Peak Hour

HCM 6th Signalized Intersection Summary
3: Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26)


Notes

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Lanes, Volumes, Timings
3: Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26)

|  | $\checkmark$ |  |  | $\checkmark$ |  | 4 | 4 | $\uparrow$ | $p$ |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ${ }^{*}$ | 个4 | F | \% | 中 ${ }_{\text {F }}$ |  |  | \$ |  |  | ¢ |  |
| Traffic Volume (vph) | 178 | 1292 | 158 | 6 | 1331 | 19 | 137 | 28 | 8 | 45 | 27 | 118 |
| Future Volume (vph) | 178 | 1292 | 158 | 6 | 1331 | 19 | 137 | 28 | 8 | 45 | 27 | 118 |
| Ideal Flow (vphpl) | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 |
| Storage Length (ft) | 115 |  | 100 | 105 |  | 0 | 0 |  | 0 | 0 |  | 0 |
| Storage Lanes | 1 |  | 1 | 1 |  | 0 | 0 |  | 0 | 0 |  | 0 |
| Taper Length (ft) | 100 |  |  | 100 |  |  | 100 |  |  | 100 |  |  |
| Lane Util. Factor | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 0.95 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Ped Bike Factor | 1.00 |  | 0.98 | 1.00 | 1.00 |  |  | 1.00 |  |  | 0.99 |  |
| Frt |  |  | 0.850 |  | 0.998 |  |  | 0.993 |  |  | 0.916 |  |
| Flt Protected | 0.950 |  |  | 0.950 |  |  |  | 0.962 |  |  | 0.988 |  |
| Satd. Flow (prot) | 1630 | 3228 | 1458 | 1630 | 3161 | 0 | 0 | 1623 | 0 | 0 | 1532 | 0 |
| Flt Permitted | 0.950 |  |  | 0.950 |  |  |  | 0.512 |  |  | 0.905 |  |
| Satd. Flow (perm) | 1629 | 3228 | 1426 | 1629 | 3161 | 0 | 0 | 861 | 0 | 0 | 1404 | 0 |
| Right Turn on Red |  |  | Yes |  |  | Yes |  |  | Yes |  |  | Yes |
| Satd. Flow (RTOR) |  |  | 95 |  | 2 |  |  | 2 |  |  | 66 |  |
| Link Speed (mph) |  | 25 |  |  | 25 |  |  | 25 |  |  | 25 |  |
| Link Distance (ft) |  | 468 |  |  | 302 |  |  | 365 |  |  | 210 |  |
| Travel Time (s) |  | 12.8 |  |  | 8.2 |  |  | 10.0 |  |  | 5.7 |  |
| Confl. Peds. (\#/hr) | 2 |  | 1 | 1 |  | 2 | 5 |  |  |  |  | 5 |
| Peak Hour Factor | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 |
| Heavy Vehicles (\%) | 2\% | 3\% | 2\% | 2\% | 5\% | 2\% | 2\% | 8\% | 2\% | 2\% | 2\% | 2\% |
| Adj. Flow (vph) | 189 | 1374 | 168 | 6 | 1416 | 20 | 146 | 30 | 9 | 48 | 29 | 126 |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow (vph) | 189 | 1374 | 168 | 6 | 1436 | 0 | 0 | 185 | 0 | 0 | 203 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | L NA | R NA | R NA | LNA | R NA | R NA | LNA | R NA | R NA | L NA | R NA | R NA |
| Median Width(ft) |  | 12 |  |  | 12 |  |  | 0 |  |  | 0 |  |
| Link Offset(ft) |  | -12 |  |  | 0 |  |  | 0 |  |  | 4 |  |
| Crosswalk Width(tt) |  | 72 |  |  | 42 |  |  | 32 |  |  | 30 |  |
| Two way Left Turn Lane |  |  |  |  |  |  |  |  |  |  |  |  |
| Headway Factor | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 |
| Number of Detectors | 2 | 1 | 1 | 2 | 1 |  | 2 | 2 |  | 2 | 2 |  |
| Detector Template | Left | Det25 | Right | Left | Det25 |  | Left | Side St |  | Left | Side St |  |
| Leading Detector (ft) | 78 | 153 | 153 | 78 | 153 |  | 78 | 78 |  | 78 | 78 |  |
| Trailing Detector (ft) | 2 | 137 | 137 | 2 | 137 |  | 2 | 2 |  | 2 | 2 |  |
| Detector 1 Position(ft) | 2 | 137 | 137 | 2 | 137 |  | 2 | 2 |  | 2 | 2 |  |
| Detector 1 Size(ft) | 16 | 16 | 16 | 16 | 16 |  | 16 | 16 |  | 16 | 16 |  |
| Detector 1 Type | Cl+Ex | Cl+Ex | Cl+Ex | Cl+Ex | Cl+Ex |  | Cl+Ex | Cl+Ex |  | Cl+Ex | $\mathrm{Cl}+\mathrm{Ex}$ |  |
| Detector 1 Channel |  |  |  |  |  |  |  |  |  |  |  |  |
| Detector 1 Extend (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Detector 1 Queue (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Detector 1 Delay (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Detector 2 Position(ft) | 72 |  |  | 72 |  |  | 72 | 72 |  | 72 | 72 |  |
| Detector 2 Size(ft) | 6 |  |  | 6 |  |  | 6 | 6 |  | 6 | 6 |  |
| Detector 2 Type | $\mathrm{Cl}+\mathrm{Ex}$ |  |  | Cl+Ex |  |  | Cl+Ex | $\mathrm{Cl}+\mathrm{Ex}$ |  | Cl+Ex | $\mathrm{Cl}+\mathrm{Ex}$ |  |
| Detector 2 Channel |  |  |  |  |  |  |  |  |  |  |  |  |
| Detector 2 Extend (s) | 0.0 |  |  | 0.0 |  |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Turn Type | Prot | NA | Perm | Prot | NA |  | Perm | NA |  | Perm | NA |  |

[^9]Synchro 10 Report - by PBS Engineering and Environmental
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Lanes, Volumes, Timings
3: Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26)

| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Protected Phases | 5 | 2 |  | 1 | 6 |  |  | 8 |  | 4 | 4 |  |


| Detector Phase | 5 | 2 | 2 | 1 | 6 | 8 | 8 | 4 | 4 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Switch Phase |  |  |  |  |  |  |  |  |  |
| Minimum Initial (s) | 4.0 | 10.0 | 10.0 | 4.0 | 10.0 | 6.0 | 6.0 | 6.0 | 6.0 |
| Minimum Split (s) | 8.5 | 26.0 | 26.0 | 8.5 | 30.0 | 11.5 | 11.5 | 23.5 | 23.5 |
| Total Split (s) | 20.0 | 70.0 | 70.0 | 15.0 | 65.0 | 25.0 | 25.0 | 25.0 | 25.0 |
| Total Split (\%) | 18.2\% | 63.6\% | 63.6\% | 13.6\% | 59.1\% | 22.7\% | 22.7\% | 22.7\% | 22.7\% |
| Maximum Green (s) | 15.5 | 66.0 | 66.0 | 10.5 | 61.0 | 19.5 | 19.5 | 19.5 | 19.5 |
| Yellow Time (s) | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 4.0 | 4.0 | 4.0 | 4.0 |
| All-Red Time (s) | 1.0 | 0.5 | 0.5 | 1.0 | 0.5 | 1.5 | 1.5 | 1.5 | 1.5 |
| Lost Time Adjust (s) | -0.5 | 0.0 | 0.0 | -0.5 | 0.0 |  | -1.5 |  | -1.5 |
| Total Lost Time (s) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |  | 4.0 |  | 4.0 |
| Lead/Lag | Lag | Lag | Lag | Lead | Lead |  |  |  |  |
| Lead-Lag Optimize? |  |  |  |  |  |  |  |  |  |
| Vehicle Extension (s) | 2.3 | 5.8 | 5.8 | 2.3 | 5.8 | 2.5 | 2.5 | 2.5 | 2.5 |
| Minimum Gap (s) | 0.5 | 3.8 | 3.8 | 0.5 | 3.8 | 2.0 | 2.0 | 2.0 | 2.0 |
| Time Before Reduce (s) | 8.0 | 10.0 | 10.0 | 8.0 | 10.0 | 5.0 | 5.0 | 5.0 | 5.0 |
| Time To Reduce (s) | 3.0 | 10.0 | 10.0 | 3.0 | 10.0 | 5.0 | 5.0 | 5.0 | 5.0 |
| Recall Mode | None | C-Min | C-Min | None | C-Min | None | None | None | None |
| Walk Time (s) |  | 7.0 | 7.0 |  | 7.0 |  |  | 7.0 | 7.0 |
| Flash Dont Walk (s) |  | 15.0 | 15.0 |  | 19.0 |  |  | 11.0 | 11.0 |
| Pedestrian Calls (\#/hr) |  | 0 | 0 |  | 0 |  |  | 0 | 0 |
| Act Effct Green (s) | 15.5 | 75.3 | 75.3 | 5.8 | 58.0 |  | 24.5 |  | 24.5 |
| Actuated g/C Ratio | 0.14 | 0.68 | 0.68 | 0.05 | 0.53 |  | 0.22 |  | 0.22 |
| v/c Ratio | 0.83 | 0.62 | 0.17 | 0.07 | 0.86 |  | 0.96 |  | 0.56 |
| Control Delay | 73.8 | 11.2 | 3.1 | 50.8 | 28.6 |  | 100.2 |  | 33.4 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 |  | 0.0 |
| Total Delay | 73.8 | 11.2 | 3.1 | 50.8 | 28.6 |  | 100.2 |  | 33.4 |
| LOS | E | B | A | D | C |  | F |  | C |
| Approach Delay |  | 17.3 |  |  | 28.7 |  | 100.2 |  | 33.4 |
| Approach LOS |  | B |  |  | C |  | F |  | C |

Intersection Summary
Area Type: Other
Cycle Length: 110
Actuated Cycle Length: 110
Offset: $0(0 \%)$, Referenced to phase 2:EBT and 6:WBT, Start of Green, Master Intersection
Natural Cycle: 90
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.96
Intersection Signal Delay: 27.1
Intersection LOS: C
Intersection Capacity Utilization 87.9\%
ICU Level of Service E
Analysis Period (min) 15
Splits and Phases: $\quad 3$ : Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26)


2029 Without Project Conditions - Weekday PM Peak Hour

Queues
3: Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26)

|  | 4 | $\rightarrow$ | \% | 7 | $\leftrightarrow$ | $\uparrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | NBT | SBT |
| Lane Group Flow (vph) | 189 | 1374 | 168 | 6 | 1436 | 185 | 203 |
| v/c Ratio | 0.83 | 0.62 | 0.17 | 0.07 | 0.86 | 0.96 | 0.56 |
| Control Delay | 73.8 | 11.2 | 3.1 | 50.8 | 28.6 | 100.2 | 33.4 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Delay | 73.8 | 11.2 | 3.1 | 50.8 | 28.6 | 100.2 | 33.4 |
| Queue Length 50th (ft) | 129 | 201 | 12 | 4 | 426 | ~150 | 89 |
| Queue Length 95th (ft) | \#247 | 369 | 44 | 18 | 517 | \#295 | 171 |
| Internal Link Dist (ft) |  | 388 |  |  | 222 | 285 | 130 |
| Turn Bay Length (ft) | 115 |  | 100 | 105 |  |  |  |
| Base Capacity (vph) | 241 | 2210 | 1006 | 163 | 1753 | 193 | 364 |
| Starvation Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Spillback Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Storage Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Reduced v/c Ratio | 0.78 | 0.62 | 0.17 | 0.04 | 0.82 | 0.96 | 0.56 |

## Intersection Summary

~ Volume exceeds capacity, queue is theoretically infinite. Queue shown is maximum after two cycles.
\# 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis
3: Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26)

c Critical Lane Group

Sandy Health Clinic - PBS Project 71524.000
2029 Without Project Conditions - Weekday PM Peak Hour

HCM 6th Signalized Intersection Summary
3：Wolf Drive／Ten Eyck Road \＆Mt．Hood Highway（US 26）

|  | 4 | $\rightarrow$ | V | 7 |  |  | 4 | $\dagger$ | $p$ | $\square$ | $\dagger$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ＊ | 个4 | 「 | ${ }^{*}$ | 性 |  |  | \＄ |  |  | ¢ |  |
| Traffic Volume（veh／h） | 178 | 1292 | 158 | 6 | 1331 | 19 | 137 | 28 | 8 | 45 | 27 | 118 |
| Future Volume（veh／h） | 178 | 1292 | 158 | 6 | 1331 | 19 | 137 | 28 | 8 | 45 | 27 | 118 |
| Initial $Q(Q b)$ ，veh | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Ped－Bike Adj（A＿pbT） | 1.00 |  | 1.00 | 1.00 |  | 1.00 | 0.99 |  | 0.99 | 1.00 |  | 0.99 |
| Parking Bus，Adj | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Work Zone On Approach |  | No |  |  | No |  |  | No |  |  | No |  |
| Adj Sat Flow，veh／h／ln | 1723 | 1709 | 1723 | 1723 | 1682 | 1682 | 1641 | 1641 | 1641 | 1723 | 1723 | 1723 |
| Adj Flow Rate，veh／h | 189 | 1374 | 136 | 6 | 1416 | 19 | 146 | 30 | 7 | 48 | 29 | 72 |
| Peak Hour Factor | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 |
| Percent Heavy Veh，\％ | 2 | 3 | 2 | 2 | 5 | 5 | 8 | 8 | 8 | 2 | 2 | 2 |
| Cap，veh／h | 264 | 2224 | 998 | 17 | 1740 | 23 | 227 | 37 | 8 | 120 | 79 | 146 |
| Arrive On Green | 0.16 | 0.68 | 0.68 | 0.01 | 0.54 | 0.54 | 0.18 | 0.19 | 0.18 | 0.18 | 0.19 | 0.18 |
| Sat Flow，veh／h | 1641 | 3247 | 1458 | 1641 | 3228 | 43 | 882 | 194 | 43 | 401 | 414 | 763 |
| Grp Volume（v），veh／h | 189 | 1374 | 136 | 6 | 700 | 735 | 183 | 0 | 0 | 149 | 0 | 0 |
| Grp Sat Flow（s），veh／h／ln | 1641 | 1624 | 1458 | 1641 | 1598 | 1674 | 1118 | 0 | 0 | 1579 | 0 | 0 |
| Q Serve（g＿s），s | 12.0 | 25.4 | 3.6 | 0.4 | 39.6 | 39.7 | 8.6 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Cycle Q Clear（g＿c），s | 12.0 | 25.4 | 3.6 | 0.4 | 39.6 | 39.7 | 18.0 | 0.0 | 0.0 | 9.4 | 0.0 | 0.0 |
| Prop In Lane | 1.00 |  | 1.00 | 1.00 |  | 0.03 | 0.80 |  | 0.04 | 0.32 |  | 0.48 |
| Lane Grp Cap（c），veh／h | 264 | 2224 | 998 | 17 | 861 | 902 | 257 | 0 | 0 | 323 | 0 | 0 |
| V／C Ratio（X） | 0.72 | 0.62 | 0.14 | 0.34 | 0.81 | 0.81 | 0.71 | 0.00 | 0.00 | 0.46 | 0.00 | 0.00 |
| Avail Cap（c＿a），veh／h | 264 | 2224 | 998 | 164 | 886 | 928 | 257 | 0 | 0 | 323 | 0 | 0 |
| HCM Platoon Ratio | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Upstream Filter（l） | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.00 | 0.00 | 1.00 | 0.00 | 0.00 |
| Uniform Delay（d），s／veh | 43.7 | 9.5 | 6.0 | 54.0 | 20.8 | 20.8 | 44.6 | 0.0 | 0.0 | 40.3 | 0.0 | 0.0 |
| Incr Delay（d2），s／veh | 8.1 | 1.3 | 0.3 | 7.0 | 8.3 | 8.0 | 8.4 | 0.0 | 0.0 | 0.8 | 0.0 | 0.0 |
| Initial Q Delay（d3），s／veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| \％ile BackOfQ（50\％），veh／ln | 5.5 | 8.7 | 1.1 | 0.2 | 16.2 | 17.0 | 5.6 | 0.0 | 0.0 | 3.8 | 0.0 | 0.0 |
| Unsig．Movement Delay，s／veh |  |  |  |  |  |  |  |  |  |  |  |  |
| LnGrp Delay（d），s／veh | 51.9 | 10.8 | 6.3 | 61.0 | 29.1 | 28.8 | 53.0 | 0.0 | 0.0 | 41.1 | 0.0 | 0.0 |
| LnGrp LOS | D | B | A | E | C | C | D | A | A | D | A | A |
| Approach Vol，veh／h |  | 1699 |  |  | 1441 |  |  | 183 |  |  | 149 |  |
| Approach Delay，s／veh |  | 15.0 |  |  | 29.1 |  |  | 53.0 |  |  | 41.1 |  |
| Approach LOS |  | B |  |  | C |  |  | D |  |  | D |  |
| Timer－Assigned Phs | 1 | 2 |  | 4 | 5 | 6 |  | 8 |  |  |  |  |
| Phs Duration（ $G+Y+R \mathrm{c}$ ）， s | 5.2 | 79.8 |  | 25.0 | 21.7 | 63.3 |  | 25.0 |  |  |  |  |
| Change Period（ $\mathrm{Y}+\mathrm{Rc}$ ）， s | 4.5 | ＊ 4.5 |  | 5.5 | 4.5 | 4.0 |  | 5.5 |  |  |  |  |
| Max Green Setting（Gmax），s | 10.5 | ＊ 66 |  | 19.5 | 15.5 | 61.0 |  | 19.5 |  |  |  |  |
| Max Q Clear Time（g＿c c 11 ），s | 2.4 | 27.4 |  | 11.4 | 14.0 | 41.7 |  | 20.0 |  |  |  |  |
| Green Ext Time（p＿c），s | 0.0 | 33.5 |  | 0.3 | 0.1 | 17.6 |  | 0.0 |  |  |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| HCM 6th Ctrl Delay |  |  | 24.0 |  |  |  |  |  |  |  |  |  |
| HCM 6th LOS | C |  |  |  |  |  |  |  |  |  |  |  |

Notes
＊HCM 6th computational engine requires equal clearance times for the phases crossing the barrier．

Lanes，Volumes，Timings
3：Wolf Drive／Ten Eyck Road \＆Mt．Hood Highway（US 26）

|  | 4 | $\rightarrow$ | V | 7 | $\square$ | 4 | 4 | $\uparrow$ | $p$ |  | $\dagger$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \％ | 个个 | F | ＊ | 性 |  |  | ＊ |  |  | ¢ |  |
| Traffic Volume（vph） | 88 | 982 | 39 | 0 | 1106 | 22 | 160 | 4 | 2 | 26 | 8 | 166 |
| Future Volume（vph） | 88 | 982 | 39 | 0 | 1106 | 22 | 160 | 4 | 2 | 26 | 8 | 166 |
| Ideal Flow（vphpl） | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 |
| Storage Length（ft） | 115 |  | 100 | 105 |  | 0 | 0 |  | 0 | 0 |  | 0 |
| Storage Lanes | 1 |  | 1 | 1 |  | 0 | 0 |  | 0 | 0 |  | 0 |
| Taper Length（ft） | 100 |  |  | 100 |  |  | 100 |  |  | 100 |  |  |
| Lane Util．Factor | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 0.95 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Ped Bike Factor |  |  | 0.98 |  | 1.00 |  |  | 1.00 |  |  | 0.99 |  |
| Frt |  |  | 0.850 |  | 0.997 |  |  | 0.998 |  |  | 0.888 |  |
| Flt Protected | 0.950 |  |  |  |  |  |  | 0.954 |  |  | 0.994 |  |
| Satd．Flow（prot） | 1568 | 2942 | 1403 | 1716 | 3158 | 0 | 0 | 1603 | 0 | 0 | 1455 | 0 |
| Flt Permitted | 0.950 |  |  |  |  |  |  | 0.499 |  |  | 0.946 |  |
| Satd．Flow（perm） | 1568 | 2942 | 1372 | 1716 | 3158 | 0 | 0 | 837 | 0 | 0 | 1385 | 0 |
| Right Turn on Red |  |  | Yes |  |  | Yes |  |  | Yes |  |  | Yes |
| Satd．Flow（RTOR） |  |  | 77 |  | 2 |  |  | 1 |  |  | 180 |  |
| Link Speed（mph） |  | 25 |  |  | 25 |  |  | 25 |  |  | 25 |  |
| Link Distance（ ft ） |  | 468 |  |  | 302 |  |  | 365 |  |  | 210 |  |
| Travel Time（s） |  | 12.8 |  |  | 8.2 |  |  | 10.0 |  |  | 5.7 |  |
| Confl．Peds．（\＃／hr） |  |  | 1 | 1 |  |  | 2 |  |  |  |  | 2 |
| Confl．Bikes（\＃／hr） |  |  |  |  |  | 1 |  |  |  |  |  |  |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles（\％） | 6\％ | 13\％ | 6\％ | 2\％ | 5\％ | 2\％ | 4\％ | 2\％ | 2\％ | 24\％ | 2\％ | 2\％ |
| Adj．Flow（vph） | 96 | 1067 | 42 | 0 | 1202 | 24 | 174 | 4 | 2 | 28 | 9 | 180 |


| Shared Lane Traffic（\％） |  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Lane Group Flow（vph） | 96 | 1067 | 42 | 0 | 1226 | 0 | 0 | 180 | 0 | 0 | 217 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | L NA | R NA | R NA | LNA | R NA | R NA | L NA | R NA | R NA | L NA | R NA | R NA |


| Lane Alignment | LNA | NA | R NA | LNA | NA | R NA | LNA | A | RNA | LNA | A | RNA |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Median Width（ft） |  | 12 |  |  | 12 |  |  | 0 |  |  | 0 |  |
| Link Offset（ft） |  | －12 |  |  | 0 |  |  | 0 |  |  | 4 |  |
| Crosswalk Width（ft） |  | 72 |  |  | 42 |  |  | 32 |  |  | 30 |  |
| Two way Left Turn Lane |  |  |  |  |  |  |  |  |  |  |  |  |
| Headway Factor | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 |
| Number of Detectors | 2 | 1 | 1 | 2 | 1 |  | 2 | 2 |  | 2 | 2 |  |
| Detector Template | Left | Det25 | Right | Left | Det25 |  | Left | Side St |  | Left | Side St |  |
| Leading Detector（ft） | 78 | 153 | 153 | 78 | 153 |  | 78 | 78 |  | 78 | 78 |  |
| Trailing Detector（ft） | 2 | 137 | 137 | 2 | 137 |  | 2 | 2 |  | 2 | 2 |  |
| Detector 1 Position（ft） | 2 | 137 | 137 | 2 | 137 |  | 2 | 2 |  | 2 | 2 |  |
| Detector 1 Size（ft） | 16 | 16 | 16 | 16 | 16 |  | 16 | 16 |  | 16 | 16 |  |
| Detector 1 Type | Cl＋Ex | $\mathrm{Cl}+\mathrm{Ex}$ | $\mathrm{Cl}+\mathrm{Ex}$ | Cl＋Ex | Cl＋Ex |  | Cl＋Ex | Cl＋Ex |  | $\mathrm{Cl}+\mathrm{Ex}$ | Cl＋Ex |  |
| Detector 1 Channel |  |  |  |  |  |  |  |  |  |  |  |  |
| Detector 1 Extend（s） | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Detector 1 Queue（s） | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Detector 1 Delay（s） | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Detector 2 Position（ft） | 72 |  |  | 72 |  |  | 72 | 72 |  | 72 | 72 |  |
| Detector 2 Size（ft） | 6 |  |  | 6 |  |  | 6 | 6 |  | 6 | 6 |  |
| Detector 2 Type | Cl＋Ex |  |  | Cl＋Ex |  |  | Cl＋Ex | $\mathrm{Cl}+\mathrm{Ex}$ |  | Cl＋Ex | Cl＋Ex |  |
| Detector 2 Channel |  |  |  |  |  |  |  |  |  |  |  |  |
| Detector 2 Extend（s） | 0.0 |  |  | 0.0 |  |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |

Sandy Health Clinic－PBS Project 71524.000
2029 With Project Conditions－Weekday AM Peak Hour

Synchro 10 Report－by PBS Engineering and Environmental
Page 1

Lanes, Volumes, Timings
3: Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26)

|  | $\stackrel{ }{*}$ |  |  |  |  |  | 4 | 4 |  |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Turn Type | Prot | NA | Perm | Prot | NA |  | Perm | NA |  | Perm | NA |  |
| Protected Phases | 5 | 2 |  | 1 | 6 |  |  | 8 |  |  | 4 |  |
| Permitted Phases |  |  | 2 |  |  |  | 8 |  |  | 4 |  |  |
| Detector Phase | 5 | 2 | 2 | 1 | 6 |  | 8 | 8 |  | 4 | 4 |  |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial (s) | 4.0 | 10.0 | 10.0 | 4.0 | 10.0 |  | 6.0 | 6.0 |  | 6.0 | 6.0 |  |
| Minimum Split (s) | 8.5 | 26.0 | 26.0 | 8.5 | 30.0 |  | 11.5 | 11.5 |  | 23.5 | 23.5 |  |
| Total Split (s) | 29.5 | 54.0 | 54.0 | 19.5 | 44.0 |  | 25.5 | 25.5 |  | 25.5 | 25.5 |  |
| Total Split (\%) | 29.8\% | 54.5\% | 54.5\% | 19.7\% | 44.4\% |  | 25.8\% | 25.8\% |  | 25.8\% | 25.8\% |  |
| Maximum Green (s) | 25.0 | 50.0 | 50.0 | 15.0 | 40.0 |  | 20.0 | 20.0 |  | 20.0 | 20.0 |  |
| Yellow Time (s) | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 |  | 4.0 | 4.0 |  | 4.0 | 4.0 |  |
| All-Red Time (s) | 1.0 | 0.5 | 0.5 | 1.0 | 0.5 |  | 1.5 | 1.5 |  | 1.5 | 1.5 |  |
| Lost Time Adjust (s) | -0.5 | 0.0 | 0.0 | -0.5 | 0.0 |  |  | -1.5 |  |  | -1.5 |  |
| Total Lost Time (s) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |  |  | 4.0 |  |  | 4.0 |  |
| Lead/Lag | Lag | Lag | Lag | Lead | Lead |  |  |  |  |  |  |  |
| Lead-Lag Optimize? |  |  |  |  |  |  |  |  |  |  |  |  |
| Vehicle Extension (s) | 2.3 | 5.8 | 5.8 | 2.3 | 5.8 |  | 2.5 | 2.5 |  | 2.5 | 2.5 |  |
| Minimum Gap (s) | 0.5 | 3.8 | 3.8 | 0.5 | 3.8 |  | 2.0 | 2.0 |  | 2.0 | 2.0 |  |
| Time Before Reduce (s) | 8.0 | 10.0 | 10.0 | 8.0 | 10.0 |  | 5.0 | 5.0 |  | 5.0 | 5.0 |  |
| Time To Reduce (s) | 3.0 | 10.0 | 10.0 | 3.0 | 10.0 |  | 5.0 | 5.0 |  | 5.0 | 5.0 |  |
| Recall Mode | None | Min | Min | None | Min |  | None | None |  | None | None |  |
| Walk Time (s) |  | 7.0 | 7.0 |  | 7.0 |  |  |  |  | 7.0 | 7.0 |  |
| Flash Dont Walk (s) |  | 15.0 | 15.0 |  | 19.0 |  |  |  |  | 11.0 | 11.0 |  |
| Pedestrian Calls (\#/hr) |  | 0 | 0 |  | 0 |  |  |  |  | 0 | 0 |  |
| Act Effct Green (s) | 10.2 | 47.9 | 47.9 |  | 36.5 |  |  | 22.2 |  |  | 22.2 |  |
| Actuated g/C Ratio | 0.13 | 0.61 | 0.61 |  | 0.47 |  |  | 0.28 |  |  | 0.28 |  |
| v/c Ratio | 0.47 | 0.59 | 0.05 |  | 0.83 |  |  | 0.76 |  |  | 0.42 |  |
| Control Delay | 41.9 | 10.2 | 0.5 |  | 25.2 |  |  | 53.0 |  |  | 9.8 |  |
| Queue Delay | 0.0 | 0.0 | 0.0 |  | 0.0 |  |  | 0.0 |  |  | 0.0 |  |
| Total Delay | 41.9 | 10.2 | 0.5 |  | 25.2 |  |  | 53.0 |  |  | 9.8 |  |
| LOS | D | B | A |  | C |  |  | D |  |  | A |  |
| Approach Delay |  | 12.4 |  |  | 25.2 |  |  | 53.0 |  |  | 9.8 |  |
| Approach LOS |  | B |  |  | C |  |  | D |  |  | A |  |

Intersection Summary
Area Type: Other
Cycle Length: 99
Actuated Cycle Length: 78.3
Natural Cycle: 65
Control Type: Actuated-Uncoordinated
Maximum v/c Ratio: 0.83
Intersection Signal Delay: 20.3
Intersection Capacity Utilization 76.0\%
Intersection LOS: C
Analysis Period (min) 15
Splits and Phases: 3: Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26)


2029 With Project Conditions - Weekday AM Peak Hour

Queues
3: Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26)

|  | $\Rightarrow$ | $\rightarrow$ <br> EBT |  | $\frac{\text { WBT }}{1226}$ | $\begin{array}{r} \uparrow \\ \text { NBT } \\ \hline 180 \end{array}$ |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group |  |  |  |  |  |  |
| Lane Group Flow (vph) | 96 | 1067 | 42 |  |  | 217 |
| v/c Ratio | 0.47 | 0.59 | 0.05 | 0.83 | 0.76 | 0.42 |
| Control Delay | 41.9 | 10.2 | 0.5 | 25.2 | 53.0 | 9.8 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Delay | 41.9 | 10.2 | 0.5 | 25.2 | 53.0 | 9.8 |
| Queue Length 50th (ft) | 48 | 143 | 0 | 275 | 89 | 15 |
| Queue Length 95th (ft) | 95 | 191 | 4 | \#406 | \#220 | 76 |
| Internal Link Dist (ft) |  | 388 |  | 222 | 285 | 130 |
| Turn Bay Length (ft) | 115 |  | 100 |  |  |  |
| Base Capacity (vph) | 526 | 2064 | 985 | 1663 | 237 | 520 |
| Starvation Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 |
| Spillback Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 |
| Storage Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 |
| Reduced v/c Ratio | 0.18 | 0.52 | 0.04 | 0.74 | 0.76 | 0.42 |

Intersection Summary
\# 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis
3：Wolf Drive／Ten Eyck Road \＆Mt．Hood Highway（US 26）

|  | 4 | $\rightarrow$ |  | 7 |  | 4 | 4 | 4 | $>$ |  |  | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ${ }^{*}$ | 个4 | 「 | ${ }^{*}$ | 性 |  |  | \＄ |  |  | ¢ |  |
| Traffic Volume（vph） | 88 | 982 | 39 | 0 | 1106 | 22 | 160 | ， | 2 | 26 | 8 | 166 |
| Future Volume（vph） | 88 | 982 | 39 | 0 | 1106 | 22 | 160 |  | 2 | 26 | 8 | 166 |
| Ideal Flow（vphpl） | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 |
| Total Lost time（s） | 4.0 | 4.0 | 4.0 |  | 4.0 |  |  | 4.0 |  |  | 4.0 |  |
| Lane Util．Factor | 1.00 | 0.95 | 1.00 |  | 0.95 |  |  | 1.00 |  |  | 1.00 |  |
| Frpb，ped／bikes | 1.00 | 1.00 | 0.98 |  | 1.00 |  |  | 1.00 |  |  | 0.99 |  |
| Flpb，ped／bikes | 1.00 | 1.00 | 1.00 |  | 1.00 |  |  | 1.00 |  |  | 1.00 |  |
| Frt | 1.00 | 1.00 | 0.85 |  | 1.00 |  |  | 1.00 |  |  | 0.89 |  |
| Flt Protected | 0.95 | 1.00 | 1.00 |  | 1.00 |  |  | 0.95 |  |  | 0.99 |  |
| Satd．Flow（prot） | 1568 | 2942 | 1373 |  | 3158 |  |  | 1601 |  |  | 1455 |  |
| Flt Permitted | 0.95 | 1.00 | 1.00 |  | 1.00 |  |  | 0.50 |  |  | 0.95 |  |
| Satd．Flow（perm） | 1568 | 2942 | 1373 |  | 3158 |  |  | 838 |  |  | 1385 |  |
| Peak－hour factor，PHF | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Adj．Flow（vph） | 96 | 1067 | 42 | 0 | 1202 | 24 | 174 | 4 | 2 | 28 | 9 | 180 |
| RTOR Reduction（vph） | 0 | 0 | 16 | 0 | 1 | 0 | 0 | 1 | 0 | 0 | 130 | 0 |
| Lane Group Flow（vph） | 96 | 1067 | 26 | 0 | 1225 | 0 | 0 | 179 | 0 | 0 | 87 | 0 |
| Confl．Peds．（\＃／hr） |  |  | 1 | 1 |  |  | 2 |  |  |  |  | 2 |
| Confl．Bikes（\＃／hr） |  |  |  |  |  | 1 |  |  |  |  |  |  |
| Heavy Vehicles（\％） | 6\％ | 13\％ | 6\％ | 2\％ | 5\％ | 2\％ | 4\％ | 2\％ | 2\％ | 24\％ | 2\％ | 2\％ |
| Turn Type | Prot | NA | Perm | Prot | NA |  | Perm | NA |  | Perm | NA |  |
| Protected Phases | 5 | 2 |  | 1 | 6 |  |  | 8 |  |  | 4 |  |
| Permitted Phases |  |  | 2 |  |  |  | 8 |  |  | 4 |  |  |
| Actuated Green，G（s） | 8.0 | 49.0 | 49.0 |  | 36.5 |  |  | 20.6 |  |  | 20.6 |  |
| Effective Green，g（s） | 8.5 | 49.0 | 49.0 |  | 36.5 |  |  | 22.1 |  |  | 22.1 |  |
| Actuated g／C Ratio | 0.11 | 0.62 | 0.62 |  | 0.46 |  |  | 0.28 |  |  | 0.28 |  |
| Clearance Time（s） | 4.5 | 4.0 | 4.0 |  | 4.0 |  |  | 5.5 |  |  | 5.5 |  |
| Vehicle Extension（s） | 2.3 | 5.8 | 5.8 |  | 5.8 |  |  | 2.5 |  |  | 2.5 |  |
| Lane Grp Cap（vph） | 168 | 1822 | 850 |  | 1457 |  |  | 234 |  |  | 386 |  |
| v／s Ratio Prot | 0.06 | c0．36 |  |  | c0．39 |  |  |  |  |  |  |  |
| v／s Ratio Perm |  |  | 0.02 |  |  |  |  | c0．21 |  |  | 0.06 |  |
| v／c Ratio | 0.57 | 0.59 | 0.03 |  | 0.84 |  |  | 0.77 |  |  | 0.23 |  |
| Uniform Delay，d1 | 33.6 | 9.0 | 5.8 |  | 18.7 |  |  | 26.1 |  |  | 21.9 |  |
| Progression Factor | 1.00 | 1.00 | 1.00 |  | 1.00 |  |  | 1.00 |  |  | 1.00 |  |
| Incremental Delay，d2 | 3.4 | 0.9 | 0.0 |  | 5.2 |  |  | 13.3 |  |  | 0.2 |  |
| Delay（s） | 37.0 | 9.9 | 5.9 |  | 24.0 |  |  | 39.5 |  |  | 22.1 |  |
| Level of Service | D | A | A |  | C |  |  | D |  |  | C |  |
| Approach Delay（s） |  | 11.9 |  |  | 24.0 |  |  | 39.5 |  |  | 22.1 |  |
| Approach LOS |  | B |  |  | C |  |  | D |  |  | C |  |


| Intersection Summary |  |  |  |
| :--- | ---: | :--- | ---: | :--- |
| HCM 2000 Control Delay | 19.7 | HCM 2000 Level of Service | B |
| HCM 2000 Volume to Capacity ratio | 0.81 |  | 12.0 |
| Actuated Cycle Length（s） | 79.1 | Sum of lost time（s） | D |
| Intersection Capacity Utilization | $76.0 \%$ | ICU Level of Service |  |
| Analysis Period（min） | 15 |  |  |
| C Critical Lane Group |  |  |  |

Sandy Health Clinic－PBS Project 71524.000
2029 With Project Conditions－Weekday AM Peak Hour

Synchro 10 Report－by PBS Engineering and Environmental
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HCM 6th Signalized Intersection Summary
3：Wolf Drive／Ten Eyck Road \＆Mt．Hood Highway（US 26）

|  | 4 | $\rightarrow$ |  | 7 |  | 4 | 4 | 4 | $p$ | $\pm$ | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \％ | 个4 | 「 | ＊ | 㤽 |  |  | \＄ |  |  | ¢ |  |
| Traffic Volume（veh／h） | 88 | 982 | 39 | 0 | 1106 | 22 | 160 | 4 | 2 | 26 | 8 | 166 |
| Future Volume（veh／h） | 88 | 982 | 39 | 0 | 1106 | 22 | 160 | 4 | 2 | 26 | 8 | 166 |
| Initial $Q(Q b)$ ，veh | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Ped－Bike Adj（A＿pbT） | 1.00 |  | 1.00 | 1.00 |  | 0.98 | 1.00 |  | 1.00 | 1.00 |  | 1.00 |
| Parking Bus，Adj | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Work Zone On Approach |  | No |  |  | No |  |  | No |  |  | No |  |
| Adj Sat Flow，veh／h／ln | 1668 | 1573 | 1668 | 1723 | 1682 | 1682 | 1723 | 1723 | 1723 | 1723 | 1723 | 1723 |
| Adj Flow Rate，veh／h | 96 | 1067 | 9 | 0 | 1202 | 23 | 174 | 4 | 1 | 28 | 9 | 39 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Percent Heavy Veh，\％ | 6 | 13 | 6 | 2 | 5 | 5 | 2 | 2 | 2 | 2 | 2 | 2 |
| Cap，veh／h | 135 | 2083 | 985 | 3 | 1792 | 34 | 351 | 6 | 1 | 146 | 64 | 143 |
| Arrive On Green | 0.08 | 0.70 | 0.70 | 0.00 | 0.56 | 0.56 | 0.15 | 0.17 | 0.15 | 0.15 | 0.17 | 0.15 |
| Sat Flow，veh／h | 1589 | 2988 | 1413 | 1641 | 3205 | 61 | 1399 | 32 |  | 411 | 371 | 824 |
| Grp Volume（v），veh／h | 96 | 1067 | 9 | 0 | 599 | 626 | 179 | 0 | 0 | 76 | 0 | 0 |
| Grp Sat Flow（s），veh／h／ln | 1589 | 1494 | 1413 | 1641 | 1598 | 1669 | 1440 | 0 | 0 | 1606 | 0 | 0 |
| Q Serve（g＿s），s | 3.9 | 11.0 | 0.1 | 0.0 | 17.3 | 17.4 | 5.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Cycle Q Clear（g＿c），s | 3.9 | 11.0 | 0.1 | 0.0 | 17.3 | 17.4 | 7.8 | 0.0 | 0.0 | 2.8 | 0.0 | 0.0 |
| Prop In Lane | 1.00 |  | 1.00 | 1.00 |  | 0.04 | 0.97 |  | 0.01 | 0.37 |  | 0.51 |
| Lane Grp Cap（c），veh／h | 135 | 2083 | 985 | 3 | 893 | 933 | 325 | 0 | 0 | 317 | 0 | 0 |
| V／C Ratio（X） | 0.71 | 0.51 | 0.01 | 0.00 | 0.67 | 0.67 | 0.55 | 0.00 | 0.00 | 0.24 | 0.00 | 0.00 |
| Avail Cap（c＿a），veh／h | 617 | 2277 | 1077 | 388 | 974 | 1018 | 535 | 0 | 0 | 543 | 0 | 0 |
| HCM Platoon Ratio | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Upstream Filter（I） | 1.00 | 1.00 | 1.00 | 0.00 | 1.00 | 1.00 | 1.00 | 0.00 | 0.00 | 1.00 | 0.00 | 0.00 |
| Uniform Delay（d），s／veh | 29.2 | 4.7 | 3.0 | 0.0 | 10.2 | 10.2 | 26.2 | 0.0 | 0.0 | 24.2 | 0.0 | 0.0 |
| Incr Delay（d2），s／veh | 4.2 | 0.6 | 0.0 | 0.0 | 3.1 | 3.0 | 1.1 | 0.0 | 0.0 | 0.3 | 0.0 | 0.0 |
| Initial Q Delay（d3），s／veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| \％ile BackOfQ（50\％），veh／ln | 1.6 | 2.6 | 0.0 | 0.0 | 5.9 | 6.2 | 2.8 | 0.0 | 0.0 | 1.1 | 0.0 | 0.0 |
| Unsig．Movement Delay，s／veh |  |  |  |  |  |  |  |  |  |  |  |  |
| LnGrp Delay（d），s／veh | 33.5 | 5.3 | 3.0 | 0.0 | 13.3 | 13.2 | 27.3 | 0.0 | 0.0 | 24.5 | 0.0 | 0.0 |
| LnGrp LOS | C | A | A | A | B | B | C | A | A | C | A | A |
| Approach Vol，veh／h |  | 1172 |  |  | 1225 |  |  | 179 |  |  | 76 |  |
| Approach Delay，s／veh |  | 7.6 |  |  | 13.2 |  |  | 27.3 |  |  | 24.5 |  |
| Approach LOS |  | A |  |  | B |  |  | C |  |  | C |  |
| Timer－Assigned Phs | 1 | 2 |  | 4 | 5 | 6 |  | 8 |  |  |  |  |
| Phs Duration（ $G+Y+R \mathrm{c}$ ），$s$ | 0.0 | 50.2 |  | 15.4 | 9.6 | 40.7 |  | 15.4 |  |  |  |  |
| Change Period（ $Y+R \mathrm{c}$ ），$s$ | 4.5 | ＊ 4.5 |  | 5.5 | 4.5 | 4.0 |  | 5.5 |  |  |  |  |
| Max Green Setting（Gmax），s | 15.0 | ＊ 50 |  | 20.0 | 25.0 | 40.0 |  | 20.0 |  |  |  |  |
| Max Q Clear Time（g＿c＋11），s | 0.0 | 13.0 |  | 4.8 | 5.9 | 19.4 |  | 9.8 |  |  |  |  |
| Green Ext Time（p＿c），s | 0.0 | 25.6 |  | 0.2 | 0.3 | 17.3 |  | 0.5 |  |  |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| HCM 6th Ctrl Delay |  |  | 12.0 |  |  |  |  |  |  |  |  |  |
| HCM 6th LOS |  |  | B |  |  |  |  |  |  |  |  |  |

Notes
＊HCM 6th computational engine requires equal clearance times for the phases crossing the barrier．

Sandy Health Clinic－PBS Project 71524.000
2029 With Project Conditions－Weekday AM Peak Hour

Lanes, Volumes, Timings
3: Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26)

|  | 4 |  |  | $\checkmark$ |  |  | 4 | 4 | $p$ |  | $\dagger$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \% | 个4 | 「 | ${ }^{*}$ |  |  |  | $\uparrow$ |  |  | \$ |  |
| Traffic Volume (vph) | 186 | 1292 | 158 | 6 | 1331 | 20 | 137 | 28 | 8 | 47 | 27 | 138 |
| Future Volume (vph) | 186 | 1292 | 158 | 6 | 1331 | 20 | 137 | 28 | 8 | 47 | 27 | 138 |
| Ideal Flow (vphpl) | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 |
| Storage Length (ft) | 115 |  | 100 | 105 |  | 0 | 0 |  | 0 | 0 |  | 0 |
| Storage Lanes | 1 |  | 1 | 1 |  | 0 | 0 |  | 0 | 0 |  | 0 |
| Taper Length (ft) | 100 |  |  | 100 |  |  | 100 |  |  | 100 |  |  |
| Lane Util. Factor | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 0.95 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Ped Bike Factor | 1.00 |  | 0.98 | 1.00 | 1.00 |  |  | 1.00 |  |  | 0.99 |  |
| Frt |  |  | 0.850 |  | 0.998 |  |  | 0.993 |  |  | 0.912 |  |
| Flt Protected | 0.950 |  |  | 0.950 |  |  |  | 0.962 |  |  | 0.989 |  |
| Satd. Flow (prot) | 1630 | 3228 | 1458 | 1630 | 3161 | 0 | 0 | 1623 | 0 | 0 | 1526 | 0 |
| Flt Permitted | 0.950 |  |  | 0.950 |  |  |  | 0.473 |  |  | 0.910 |  |
| Satd. Flow (perm) | 1629 | 3228 | 1426 | 1629 | 3161 | 0 | 0 | 796 | 0 | 0 | 1404 | 0 |
| Right Turn on Red |  |  | Yes |  |  | Yes |  |  | Yes |  |  | Yes |
| Satd. Flow (RTOR) |  |  | 95 |  | 2 |  |  | 2 |  |  | 75 |  |
| Link Speed (mph) |  | 25 |  |  | 25 |  |  | 25 |  |  | 25 |  |
| Link Distance (ft) |  | 468 |  |  | 302 |  |  | 365 |  |  | 210 |  |
| Travel Time (s) |  | 12.8 |  |  | 8.2 |  |  | 10.0 |  |  | 5.7 |  |
| Confl. Peds. (\#/hr) | 2 |  | 1 | 1 |  | 2 | 5 |  |  |  |  | 5 |
| Peak Hour Factor | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 |
| Heavy Vehicles (\%) | 2\% | 3\% | 2\% | 2\% | 5\% | 2\% | 2\% | 8\% | 2\% | 2\% | 2\% | 2\% |
| Adj. Flow (vph) | 198 | 1374 | 168 | 6 | 1416 | 21 | 146 | 30 | 9 | 50 | 29 | 147 |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow (vph) | 198 | 1374 | 168 | 6 | 1437 | 0 | 0 | 185 | 0 | 0 | 226 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | L NA | R NA | R NA | L NA | R NA | R NA | L NA | R NA | R NA | L NA | R NA | R NA |
| Median Width(ft) |  | 12 |  |  | 12 |  |  | 0 |  |  | 0 |  |
| Link Offset(ft) |  | -12 |  |  | 0 |  |  | 0 |  |  | 4 |  |
| Crosswalk Width(ft) |  | 72 |  |  | 42 |  |  | 32 |  |  | 30 |  |
| Two way Left Turn Lane |  |  |  |  |  |  |  |  |  |  |  |  |
| Headway Factor | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 |
| Number of Detectors | 2 | 1 | 1 | 2 | 1 |  | 2 | 2 |  | 2 | 2 |  |
| Detector Template | Left | Det25 | Right | Left | Det25 |  | Left | Side St |  | Left | Side St |  |
| Leading Detector (ft) | 78 | 153 | 153 | 78 | 153 |  | 78 | 78 |  | 78 | 78 |  |
| Trailing Detector (ft) | 2 | 137 | 137 | 2 | 137 |  | 2 | 2 |  | 2 | 2 |  |
| Detector 1 Position(ft) | 2 | 137 | 137 | 2 | 137 |  | 2 | 2 |  | 2 | 2 |  |
| Detector 1 Size(ft) | 16 | 16 | 16 | 16 | 16 |  | 16 | 16 |  | 16 | 16 |  |
| Detector 1 Type | Cl+Ex | Cl+Ex | Cl+Ex | $\mathrm{Cl}+\mathrm{Ex}$ | $\mathrm{Cl}+\mathrm{Ex}$ |  | Cl+Ex | Cl+Ex |  | Cl+Ex | $\mathrm{Cl}+\mathrm{Ex}$ |  |
| Detector 1 Channel |  |  |  |  |  |  |  |  |  |  |  |  |
| Detector 1 Extend (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Detector 1 Queue (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Detector 1 Delay (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Detector 2 Position(ft) | 72 |  |  | 72 |  |  | 72 | 72 |  | 72 | 72 |  |
| Detector 2 Size(ft) | 6 |  |  | 6 |  |  | 6 | 6 |  | 6 | 6 |  |
| Detector 2 Type | Cl+Ex |  |  | Cl+Ex |  |  | Cl+Ex | Cl+Ex |  | Cl+Ex | $\mathrm{Cl}+\mathrm{Ex}$ |  |
| Detector 2 Channel |  |  |  |  |  |  |  |  |  |  |  |  |
| Detector 2 Extend (s) | 0.0 |  |  | 0.0 |  |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Turn Type | Prot | NA | Perm | Prot | NA |  | Perm | NA |  | Perm | NA |  |

[^10]Lanes, Volumes, Timings
3: Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26)

| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Protected Phases | 5 | 2 |  | 1 | 6 |  |  | 8 |  | 4 | 4 |  |


| Detector Phase | 5 | 2 | 2 | 1 | 6 | 8 | 8 | 4 | 4 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Switch Phase |  |  |  |  |  |  |  |  |  |
| Minimum Initial (s) | 4.0 | 10.0 | 10.0 | 4.0 | 10.0 | 6.0 | 6.0 | 6.0 | 6.0 |
| Minimum Split (s) | 8.5 | 26.0 | 26.0 | 8.5 | 30.0 | 11.5 | 11.5 | 23.5 | 23.5 |
| Total Split (s) | 20.0 | 70.0 | 70.0 | 15.0 | 65.0 | 25.0 | 25.0 | 25.0 | 25.0 |
| Total Split (\%) | 18.2\% | 63.6\% | 63.6\% | 13.6\% | 59.1\% | 22.7\% | 22.7\% | 22.7\% | 22.7\% |
| Maximum Green (s) | 15.5 | 66.0 | 66.0 | 10.5 | 61.0 | 19.5 | 19.5 | 19.5 | 19.5 |
| Yellow Time (s) | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 4.0 | 4.0 | 4.0 | 4.0 |
| All-Red Time (s) | 1.0 | 0.5 | 0.5 | 1.0 | 0.5 | 1.5 | 1.5 | 1.5 | 1.5 |
| Lost Time Adjust (s) | -0.5 | 0.0 | 0.0 | -0.5 | 0.0 |  | -1.5 |  | -1.5 |
| Total Lost Time (s) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |  | 4.0 |  | 4.0 |
| Lead/Lag | Lag | Lag | Lag | Lead | Lead |  |  |  |  |
| Lead-Lag Optimize? |  |  |  |  |  |  |  |  |  |
| Vehicle Extension (s) | 2.3 | 5.8 | 5.8 | 2.3 | 5.8 | 2.5 | 2.5 | 2.5 | 2.5 |
| Minimum Gap (s) | 0.5 | 3.8 | 3.8 | 0.5 | 3.8 | 2.0 | 2.0 | 2.0 | 2.0 |
| Time Before Reduce (s) | 8.0 | 10.0 | 10.0 | 8.0 | 10.0 | 5.0 | 5.0 | 5.0 | 5.0 |
| Time To Reduce (s) | 3.0 | 10.0 | 10.0 | 3.0 | 10.0 | 5.0 | 5.0 | 5.0 | 5.0 |
| Recall Mode | None | C-Min | C-Min | None | C-Min | None | None | None | None |
| Walk Time (s) |  | 7.0 | 7.0 |  | 7.0 |  |  | 7.0 | 7.0 |
| Flash Dont Walk (s) |  | 15.0 | 15.0 |  | 19.0 |  |  | 11.0 | 11.0 |
| Pedestrian Calls (\#/hr) |  | 0 | 0 |  | 0 |  |  | 0 | 0 |
| Act Effct Green (s) | 15.9 | 75.7 | 75.7 | 5.8 | 58.0 |  | 24.1 |  | 24.1 |
| Actuated g/C Ratio | 0.14 | 0.69 | 0.69 | 0.05 | 0.53 |  | 0.22 |  | 0.22 |
| v/c Ratio | 0.84 | 0.62 | 0.17 | 0.07 | 0.86 |  | 1.05 |  | 0.62 |
| Control Delay | 75.7 | 11.0 | 3.1 | 50.8 | 28.6 |  | 126.5 |  | 35.2 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 |  | 0.0 |
| Total Delay | 75.7 | 11.0 | 3.1 | 50.8 | 28.6 |  | 126.5 |  | 35.2 |
| LOS | E | B | A | D | C |  | F |  | D |
| Approach Delay |  | 17.6 |  |  | 28.7 |  | 126.5 |  | 35.2 |
| Approach LOS |  | B |  |  | C |  | F |  | D |

Intersection Summary
Area Type: Other
Cycle Length: 110
Actuated Cycle Length: 110
Offset: $0(0 \%)$, Referenced to phase 2:EBT and 6:WBT, Start of Green, Master Intersection
Natural Cycle: 90
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 1.05
Intersection Signal Delay: 28.8
Intersection Capacity Utilization 89.6\%
Intersection LOS: C
ICU Level of Service E
Analysis Period (min) 15
Splits and Phases: $\quad 3$ : Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26)


Queues
3: Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26)

|  | $\stackrel{ }{*}$ | $\rightarrow$ | 7 | $\checkmark$ | $\leftrightarrow$ | 4 | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | NBT | SBT |
| Lane Group Flow (vph) | 198 | 1374 | 168 | 6 | 1437 | 185 | 226 |
| v/c Ratio | 0.84 | 0.62 | 0.17 | 0.07 | 0.86 | 1.05 | 0.62 |
| Control Delay | 75.7 | 11.0 | 3.1 | 50.8 | 28.6 | 126.5 | 35.2 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Delay | 75.7 | 11.0 | 3.1 | 50.8 | 28.6 | 126.5 | 35.2 |
| Queue Length 50th (ft) | 136 | 201 | 12 | 4 | 426 | ~159 | 100 |
| Queue Length 95th (ft) | \#263 | 369 | 44 | 18 | 518 | \#305 | \#189 |
| Internal Link Dist (ft) |  | 388 |  |  | 222 | 285 | 130 |
| Turn Bay Length (ft) | 115 |  | 100 | 105 |  |  |  |
| Base Capacity (vph) | 244 | 2221 | 1011 | 163 | 1753 | 176 | 366 |
| Starvation Cap Reductn | 0 | 0 | , | 0 | 0 | 0 | 0 |
| Spillback Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Storage Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Reduced v/c Ratio | 0.81 | 0.62 | 0.17 | 0.04 | 0.82 | 1.05 | 0.62 |

## Intersection Summary

~ Volume exceeds capacity, queue is theoretically infinite. Queue shown is maximum after two cycles.
\# 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis
3: Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26)

c Critical Lane Group

Sandy Health Clinic - PBS Project 71524.000
2029 With Project Trips Conditions - Weekday PM Peak Hour

Synchro 10 Report - by PBS Engineering and Environmental
Page 4

HCM 6th Signalized Intersection Summary
3: Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26)


Notes

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Sandy Health Clinic - PBS Project 71524.000
2029 With Project Trips Conditions - Weekday PM Peak Hour

Lanes, Volumes, Timings
3: Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26)

| Shared Lane Traffic (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group Flow (vph) | 96 | 1067 | 42 | 0 | 1202 | 24 | 0 | 180 | 0 | 0 | 217 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | L NA | R NA | R NA | LNA | R NA | R NA | L NA | R NA | R NA | L NA | R NA | R NA |
| Median Width(ft) |  | 12 |  |  | 12 |  |  | 0 |  |  | 0 |  |
| Link Offset(ft) |  | -12 |  |  | 0 |  |  | 0 |  |  | 4 |  |
| Crosswalk Width(ft) |  | 72 |  |  | 42 |  |  | 32 |  |  | 30 |  |
| Two way Left Turn Lane |  |  |  |  |  |  |  |  |  |  |  |  |
| Headway Factor | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 |
| Number of Detectors | 2 | 1 | 1 | 2 | 1 | 1 | 2 | 2 |  | 2 | 2 |  |
| Detector Template | Left | Det25 | Right | Left | Det25 | Right | Left | Side St |  | Left | Side St |  |
| Leading Detector (ft) | 78 | 153 | 153 | 78 | 153 | 153 | 78 | 78 |  | 78 | 78 |  |
| Trailing Detector (ft) | 2 | 137 | 137 | 2 | 137 | 137 | 2 | 2 |  | 2 | 2 |  |
| Detector 1 Position(ft) | 2 | 137 | 137 | 2 | 137 | 137 | 2 | 2 |  | 2 | 2 |  |
| Detector 1 Size(ft) | 16 | 16 | 16 | 16 | 16 | 16 | 16 | 16 |  | 16 | 16 |  |
| Detector 1 Type | $\mathrm{Cl}+\mathrm{Ex}$ | $\mathrm{Cl}+\mathrm{Ex}$ | $\mathrm{Cl}+\mathrm{Ex}$ | Cl+Ex | $\mathrm{Cl}+\mathrm{Ex}$ | $\mathrm{Cl}+\mathrm{Ex}$ | Cl+Ex | $\mathrm{Cl}+\mathrm{Ex}$ |  | $\mathrm{Cl}+\mathrm{Ex}$ | $\mathrm{Cl}+\mathrm{Ex}$ |  |


| Detector 1 Channel |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Detector 1 Extend (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Detector 1 Queue (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Detector 1 Delay (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Detector 2 Position(ft) | 72 |  |  | 72 |  |  | 72 | 72 | 72 | 72 |
| Detector 2 Size(ft) | 6 |  |  | 6 |  |  | 6 | 6 | 6 | 6 |
| Detector 2 Type | Cl+Ex |  |  | $\mathrm{Cl}+\mathrm{Ex}$ |  |  | Cl+Ex | $\mathrm{Cl}+\mathrm{Ex}$ | $\mathrm{Cl}+\mathrm{Ex}$ | $\mathrm{Cl}+\mathrm{Ex}$ |
| Detector 2 Channel |  |  |  |  |  |  |  |  |  |  |
| Detector 2 Extend (s) | 0.0 |  |  | 0.0 |  |  | 0.0 | 0.0 | 0.0 | 0.0 |

[^11]Lanes, Volumes, Timings
3: Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26)

|  | $\rangle$ | $\rightarrow$ |  | 7 |  |  | 4 | $\dagger$ | P |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Turn Type | Prot | NA | Perm | Prot | NA | Perm | Perm | NA |  | Perm | NA |  |
| Protected Phases | 5 | 2 |  | 1 | 6 |  |  | 8 |  |  | 4 |  |
| Permitted Phases |  |  | 2 |  |  | 6 | 8 |  |  | 4 |  |  |
| Detector Phase | 5 | 2 | 2 | 1 | 6 | 6 | 8 | 8 |  | 4 | 4 |  |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial (s) | 4.0 | 10.0 | 10.0 | 4.0 | 10.0 | 10.0 | 6.0 | 6.0 |  | 6.0 | 6.0 |  |
| Minimum Split (s) | 8.5 | 26.0 | 26.0 | 8.5 | 30.0 | 30.0 | 11.5 | 11.5 |  | 23.5 | 23.5 |  |
| Total Split (s) | 29.5 | 54.0 | 54.0 | 19.5 | 44.0 | 44.0 | 25.5 | 25.5 |  | 25.5 | 25.5 |  |
| Total Split (\%) | 29.8\% | 54.5\% | 54.5\% | 19.7\% | 44.4\% | 44.4\% | 25.8\% | 25.8\% |  | 25.8\% | 25.8\% |  |
| Maximum Green (s) | 25.0 | 50.0 | 50.0 | 15.0 | 40.0 | 40.0 | 20.0 | 20.0 |  | 20.0 | 20.0 |  |
| Yellow Time (s) | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 4.0 | 4.0 |  | 4.0 | 4.0 |  |
| All-Red Time (s) | 1.0 | 0.5 | 0.5 | 1.0 | 0.5 | 0.5 | 1.5 | 1.5 |  | 1.5 | 1.5 |  |
| Lost Time Adjust (s) | -0.5 | 0.0 | 0.0 | -0.5 | 0.0 | 0.0 |  | -1.5 |  |  | -1.5 |  |
| Total Lost Time (s) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |  | 4.0 |  |  | 4.0 |  |
| Lead/Lag | Lag | Lag | Lag | Lead | Lead | Lead |  |  |  |  |  |  |
| Lead-Lag Optimize? |  |  |  |  |  |  |  |  |  |  |  |  |
| Vehicle Extension (s) | 2.3 | 5.8 | 5.8 | 2.3 | 5.8 | 5.8 | 2.5 | 2.5 |  | 2.5 | 2.5 |  |
| Minimum Gap (s) | 0.5 | 3.8 | 3.8 | 0.5 | 3.8 | 3.8 | 2.0 | 2.0 |  | 2.0 | 2.0 |  |
| Time Before Reduce (s) | 8.0 | 10.0 | 10.0 | 8.0 | 10.0 | 10.0 | 5.0 | 5.0 |  | 5.0 | 5.0 |  |
| Time To Reduce (s) | 3.0 | 10.0 | 10.0 | 3.0 | 10.0 | 10.0 | 5.0 | 5.0 |  | 5.0 | 5.0 |  |
| Recall Mode | None | Min | Min | None | Min | Min | None | None |  | None | None |  |
| Walk Time (s) |  | 7.0 | 7.0 |  | 7.0 | 7.0 |  |  |  | 7.0 | 7.0 |  |
| Flash Dont Walk (s) |  | 15.0 | 15.0 |  | 19.0 | 19.0 |  |  |  | 11.0 | 11.0 |  |
| Pedestrian Calls (\#/hr) |  | 0 | 0 |  | 0 | 0 |  |  |  | 0 | 0 |  |
| Act Effct Green (s) | 10.1 | 46.7 | 46.7 |  | 35.4 | 35.4 |  | 22.3 |  |  | 22.3 |  |
| Actuated g/C Ratio | 0.13 | 0.60 | 0.60 |  | 0.46 | 0.46 |  | 0.29 |  |  | 0.29 |  |
| v/c Ratio | 0.47 | 0.60 | 0.05 |  | 0.83 | 0.03 |  | 0.74 |  |  | 0.41 |  |
| Control Delay | 41.5 | 10.4 | 0.5 |  | 25.1 | 0.1 |  | 50.4 |  |  | 9.7 |  |
| Queue Delay | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 |  |  | 0.0 |  |
| Total Delay | 41.5 | 10.4 | 0.5 |  | 25.1 | 0.1 |  | 50.4 |  |  | 9.7 |  |
| LOS | D | B | A |  | C | A |  | D |  |  | A |  |
| Approach Delay |  | 12.5 |  |  | 24.6 |  |  | 50.4 |  |  | 9.7 |  |
| Approach LOS |  | B |  |  | C |  |  | D |  |  | A |  |

Intersection Summary
Area Type: Other
Cycle Length: 99
Actuated Cycle Length: 77.3
Natural Cycle: 65
Control Type: Actuated-Uncoordinated
Maximum v/c Ratio: 0.83
Intersection Signal Delay: 20.0
Intersection LOS: B
Intersection Capacity Utilization 75.2\%
ICU Level of Service D
Analysis Period (min) 15
Splits and Phases: 3: Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26)


2029 With Project Conditions - Weekday AM Peak Hour + With WB Right-Turn Lane

Queues
3: Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26)


Intersection Summary
\# 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis
3: Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26)


Sandy Health Clinic - PBS Project 71524.000 Synchro 10 Report - by PBS Engineering and Environmental
2029 With Project Conditions - Weekday AM Peak Hour + With WB Right-Turn Lane
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HCM 6th Signalized Intersection Summary
3: Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26)


Notes

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

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Lanes, Volumes, Timings
3: Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26)

|  | 4 |  |  | 7 |  |  | 4 | 4 | 1 |  | $\dagger$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \% | ¢4 | 「 | \% | 个4 | F' |  | \$ |  |  | $\uparrow$ |  |
| Traffic Volume (vph) | 186 | 1292 | 158 | 6 | 1331 | 20 | 137 | 28 | 8 | 47 | 27 | 138 |
| Future Volume (vph) | 186 | 1292 | 158 | 6 | 1331 | 20 | 137 | 28 | 8 | 47 | 27 | 138 |
| Ideal Flow (vphpl) | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 | 1750 |
| Storage Length (ft) | 115 |  | 100 | 105 |  | 100 | 0 |  | 0 | 0 |  | 0 |
| Storage Lanes | 1 |  | 1 | 1 |  | 0 | 0 |  | 0 | 0 |  | 0 |
| Taper Length (ft) | 100 |  |  | 100 |  |  | 100 |  |  | 100 |  |  |
| Lane Util. Factor | 1.00 | 0.95 | 1.00 | 1.00 | 0.95 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Ped Bike Factor | 1.00 |  | 0.98 | 1.00 |  | 0.98 |  | 1.00 |  |  | 0.99 |  |
| Frt |  |  | 0.850 |  |  | 0.850 |  | 0.993 |  |  | 0.912 |  |
| Flt Protected | 0.950 |  |  | 0.950 |  |  |  | 0.962 |  |  | 0.989 |  |
| Satd. Flow (prot) | 1630 | 3228 | 1458 | 1630 | 3167 | 1458 | 0 | 1623 | 0 | 0 | 1526 | 0 |
| Flt Permitted | 0.950 |  |  | 0.950 |  |  |  | 0.477 |  |  | 0.909 |  |
| Satd. Flow (perm) | 1629 | 3228 | 1426 | 1629 | 3167 | 1423 | 0 | 802 | 0 | 0 | 1403 | 0 |
| Right Turn on Red |  |  | Yes |  |  | Yes |  |  | Yes |  |  | Yes |
| Satd. Flow (RTOR) |  |  | 95 |  |  | 69 |  | 2 |  |  | 75 |  |
| Link Speed (mph) |  | 25 |  |  | 25 |  |  | 25 |  |  | 25 |  |
| Link Distance ( ft ) |  | 468 |  |  | 302 |  |  | 365 |  |  | 210 |  |
| Travel Time (s) |  | 12.8 |  |  | 8.2 |  |  | 10.0 |  |  | 5.7 |  |
| Confl. Peds. (\#/hr) | 2 |  | 1 | 1 |  | 2 | 5 |  |  |  |  | 5 |
| Peak Hour Factor | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 |
| Heavy Vehicles (\%) | 2\% | 3\% | 2\% | 2\% | 5\% | 2\% | 2\% | 8\% | 2\% | 2\% | 2\% | 2\% |
| Adj. Flow (vph) | 198 | 1374 | 168 | 6 | 1416 | 21 | 146 | 30 | 9 | 50 | 29 | 147 |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow (vph) | 198 | 1374 | 168 | 6 | 1416 | 21 | 0 | 185 | 0 | 0 | 226 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | L NA | R NA | R NA | LNA | R NA | R NA | L NA | R NA | R NA | LNA | RNA | R NA |
| Median Width(ft) |  | 12 |  |  | 12 |  |  | 0 |  |  | 0 |  |
| Link Offset(ft) |  | -12 |  |  | 0 |  |  | 0 |  |  | 4 |  |
| Crosswalk Width(ft) |  | 72 |  |  | 42 |  |  | 32 |  |  | 30 |  |
| Two way Left Turn Lane |  |  |  |  |  |  |  |  |  |  |  |  |
| Headway Factor | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 | 1.11 |
| Number of Detectors | 2 | 1 | 1 | 2 | 1 | 1 | 2 | 2 |  | 2 | 2 |  |
| Detector Template | Left | Det25 | Right | Left | Det25 | Right | Left | Side St |  | Left | Side St |  |
| Leading Detector (ft) | 78 | 153 | 153 | 78 | 153 | 153 | 78 | 78 |  | 78 | 78 |  |
| Trailing Detector (ft) | 2 | 137 | 137 | 2 | 137 | 137 | 2 | 2 |  | 2 | 2 |  |
| Detector 1 Position(ft) | 2 | 137 | 137 | 2 | 137 | 137 | 2 | 2 |  | 2 | 2 |  |
| Detector 1 Size(ft) | 16 | 16 | 16 | 16 | 16 | 16 | 16 | 16 |  | 16 | 16 |  |
| Detector 1 Type | Cl+Ex | $\mathrm{Cl}+\mathrm{Ex}$ | $\mathrm{Cl}+\mathrm{Ex}$ | $\mathrm{Cl}+\mathrm{Ex}$ | $\mathrm{Cl}+\mathrm{Ex}$ | $\mathrm{Cl}+\mathrm{Ex}$ | $\mathrm{Cl}+\mathrm{Ex}$ | Cl+Ex |  | $\mathrm{Cl}+\mathrm{Ex}$ | $\mathrm{Cl}+\mathrm{Ex}$ |  |
| Detector 1 Channel |  |  |  |  |  |  |  |  |  |  |  |  |
| Detector 1 Extend (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Detector 1 Queue (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Detector 1 Delay (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Detector 2 Position(ft) | 72 |  |  | 72 |  |  | 72 | 72 |  | 72 | 72 |  |
| Detector 2 Size(ft) | 6 |  |  | 6 |  |  | 6 | 6 |  | 6 | 6 |  |
| Detector 2 Type | Cl+Ex |  |  | Cl+Ex |  |  | Cl+Ex | Cl+Ex |  | Cl+Ex | $\mathrm{Cl}+\mathrm{Ex}$ |  |
| Detector 2 Channel |  |  |  |  |  |  |  |  |  |  |  |  |
| Detector 2 Extend (s) | 0.0 |  |  | 0.0 |  |  | 0.0 | 0.0 |  | 0.0 | 0.0 |  |
| Turn Type | Prot | NA | Perm | Prot | NA | Perm | Perm | NA |  | Perm | NA |  |

[^12]2029 With Project Trips Conditions - Weekday PM Peak Hour + With Right-Turn Lane
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Lanes, Volumes, Timings
3: Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26)

| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Protected Phases | 5 | 2 |  | 1 | 6 |  |  | 8 |  | 4 | 4 |  |


| Detector Phase | 5 | 2 | 2 | 1 | 6 | 6 | 8 | 8 | 4 | 4 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial (s) | 4.0 | 10.0 | 10.0 | 4.0 | 10.0 | 10.0 | 6.0 | 6.0 | 6.0 | 6.0 |
| Minimum Split (s) | 8.5 | 26.0 | 26.0 | 8.5 | 30.0 | 30.0 | 11.5 | 11.5 | 23.5 | 23.5 |
| Total Split (s) | 20.0 | 70.0 | 70.0 | 15.0 | 65.0 | 65.0 | 25.0 | 25.0 | 25.0 | 25.0 |
| Total Split (\%) | 18.2\% | 63.6\% | 63.6\% | 13.6\% | 59.1\% | 59.1\% | 22.7\% | 22.7\% | 22.7\% | 22.7\% |
| Maximum Green (s) | 15.5 | 66.0 | 66.0 | 10.5 | 61.0 | 61.0 | 19.5 | 19.5 | 19.5 | 19.5 |
| Yellow Time (s) | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 4.0 | 4.0 | 4.0 | 4.0 |
| All-Red Time (s) | 1.0 | 0.5 | 0.5 | 1.0 | 0.5 | 0.5 | 1.5 | 1.5 | 1.5 | 1.5 |
| Lost Time Adjust (s) | -0.5 | 0.0 | 0.0 | -0.5 | 0.0 | 0.0 |  | -1.5 |  | -1.5 |
| Total Lost Time (s) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |  | 4.0 |  | 4.0 |
| Lead/Lag | Lag | Lag | Lag | Lead | Lead | Lead |  |  |  |  |
| Lead-Lag Optimize? |  |  |  |  |  |  |  |  |  |  |
| Vehicle Extension (s) | 2.3 | 5.8 | 5.8 | 2.3 | 5.8 | 5.8 | 2.5 | 2.5 | 2.5 | 2.5 |
| Minimum Gap (s) | 0.5 | 3.8 | 3.8 | 0.5 | 3.8 | 3.8 | 2.0 | 2.0 | 2.0 | 2.0 |
| Time Before Reduce (s) | 8.0 | 10.0 | 10.0 | 8.0 | 10.0 | 10.0 | 5.0 | 5.0 | 5.0 | 5.0 |
| Time To Reduce (s) | 3.0 | 10.0 | 10.0 | 3.0 | 10.0 | 10.0 | 5.0 | 5.0 | 5.0 | 5.0 |
| Recall Mode | None | C-Min | C-Min | None | C-Min | C-Min | None | None | None | None |
| Walk Time (s) |  | 7.0 | 7.0 |  | 7.0 | 7.0 |  |  | 7.0 | 7.0 |
| Flash Dont Walk (s) |  | 15.0 | 15.0 |  | 19.0 | 19.0 |  |  | 11.0 | 11.0 |
| Pedestrian Calls (\#/hr) |  | 0 | 0 |  | 0 | 0 |  |  | 0 | 0 |
| Act Effct Green (s) | 16.0 | 75.4 | 75.4 | 5.8 | 57.5 | 57.5 |  | 24.5 |  | 24.5 |
| Actuated g/C Ratio | 0.15 | 0.69 | 0.69 | 0.05 | 0.52 | 0.52 |  | 0.22 |  | 0.22 |
| v/c Ratio | 0.84 | 0.62 | 0.17 | 0.07 | 0.86 | 0.03 |  | 1.03 |  | 0.61 |
| Control Delay | 74.8 | 11.2 | 3.1 | 50.8 | 28.5 | 0.1 |  | 120.1 |  | 34.8 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 |  | 0.0 |
| Total Delay | 74.8 | 11.2 | 3.1 | 50.8 | 28.5 | 0.1 |  | 120.1 |  | 34.8 |
| LOS | E | B | A | D | C | A |  | F |  | C |
| Approach Delay |  | 17.6 |  |  | 28.1 |  |  | 120.1 |  | 34.8 |
| Approach LOS |  | B |  |  | C |  |  | F |  | C |

Intersection Summary
Area Type: Other
Cycle Length: 110
Actuated Cycle Length: 110
Offset: $0(0 \%)$, Referenced to phase 2:EBT and 6:WBT, Start of Green, Master Intersection
Natural Cycle: 90
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 1.03
Intersection Signal Delay: 28.2
Intersection LOS: C
Intersection Capacity Utilization 88.9\%
ICU Level of Service E
Analysis Period (min) 15
Splits and Phases: $\quad 3:$ Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26)


2029 With Project Trips Conditions - Weekday PM Peak Hour + With Right-Turn Lane

Queues
3: Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26)

|  | 4 | $\rightarrow$ | \% | 7 | - | 4 | $\uparrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBT | SBT |
| Lane Group Flow (vph) | 198 | 1374 | 168 | 6 | 1416 | 21 | 185 | 226 |
| v/c Ratio | 0.84 | 0.62 | 0.17 | 0.07 | 0.86 | 0.03 | 1.03 | 0.61 |
| Control Delay | 74.8 | 11.2 | 3.1 | 50.8 | 28.5 | 0.1 | 120.1 | 34.8 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Delay | 74.8 | 11.2 | 3.1 | 50.8 | 28.5 | 0.1 | 120.1 | 34.8 |
| Queue Length 50th (ft) | 135 | 201 | 12 | 4 | 418 | 0 | ~158 | 100 |
| Queue Length 95th (ft) | \#263 | 369 | 44 | 18 | 504 | 0 | \#304 | \#190 |
| Internal Link Dist (ft) |  | 388 |  |  | 222 |  | 285 | 130 |
| Turn Bay Length (ft) | 115 |  | 100 | 105 |  | 100 |  |  |
| Base Capacity (vph) | 245 | 2211 | 1007 | 163 | 1756 | 819 | 179 | 370 |
| Starvation Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Spillback Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Storage Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Reduced v/c Ratio | 0.81 | 0.62 | 0.17 | 0.04 | 0.81 | 0.03 | 1.03 | 0.61 |

## Intersection Summary

~ Volume exceeds capacity, queue is theoretically infinite. Queue shown is maximum after two cycles.
\# 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

HCM Signalized Intersection Capacity Analysis
3: Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26)

c Critical Lane Group

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HCM 6th Signalized Intersection Summary
3: Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26)


Notes

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Sandy Health Clinic - PBS Project 71524.000 Synchro 10 Report - by PBS Engineering and Environmental 2029 With Project Trips Conditions - Weekday PM Peak Hour + With Right-Turn Lane

SimTraffic Performance Report
2029 Without Project Conditions - Weekday AM Peak Hour
1: Ten Eyck Road \& Pleasant Street (W) Performance by movement

| Movement | EBL | EBR | NBL | NBT | SBT | SBR | All |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Vehicles Entered | 17 | 25 | 18 | 56 | 163 | 24 | 303 |
| Vehicles Exited | 17 | 25 | 18 | 56 | 163 | 24 | 303 |
| Hourly Exit Rate | 17 | 25 | 18 | 56 | 163 | 24 | 303 |
| Input Volume | 18 | 24 | 19 | 54 | 169 | 21 | 306 |
| \% of Volume | 94 | 105 | 94 | 103 | 97 | 113 | 99 |

## 2: Ten Eyck Road \& Pleasant Street (E) Performance by movement

| Movement | NBT | NBR | SBT | All |
| :--- | ---: | ---: | ---: | ---: |
| Vehicles Entered | 78 | 10 | 192 | 280 |
| Vehicles Exited | 78 | 10 | 192 | 280 |
| Hourly Exit Rate | 78 | 10 | 192 | 280 |
| Input Volume | 78 | 11 | 196 | 284 |
| \% of Volume | 101 | 93 | 98 | 99 |

3: Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26) Performance by movement

| Movement | EBL | EBT | EBR | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR | All |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Vehicles Entered | 63 | 958 | 46 | 1086 | 20 | 162 | 4 | 2 | 23 | 12 | 157 | 2533 |
| Vehicles Exited | 63 | 959 | 46 | 1087 | 20 | 162 | 4 | 2 | 23 | 12 | 157 | 2535 |
| Hourly Exit Rate | 63 | 959 | 46 | 1087 | 20 | 162 | 4 | 2 | 23 | 12 | 157 | 2535 |
| Input Volume | 64 | 982 | 39 | 1106 | 19 | 160 | 4 | 2 | 25 | 13 | 159 | 2572 |
| \% of Volume | 98 | 98 | 118 | 98 | 107 | 101 | 100 | 100 | 93 | 91 | 99 | 99 |

Total Network Performance

|  |  |
| :--- | ---: |
| Vehicles Entered | 2579 |
| Vehicles Exited | 2582 |
| Hourly Exit Rate | 2582 |
| Input Volume | 7900 |
| \% of Volume | 33 |

Queuing and Blocking Report 2029 Without Project Conditions - Weekday AM Peak Hour
Intersection: 1: Ten Eyck Road \& Pleasant Street (W)

| Movement | EB | NB | SB |
| :--- | ---: | ---: | ---: |
| Directions Served | LR | LT | TR |
| Maximum Queue (ft) | 70 | 63 | 40 |
| Average Queue (ft) | 25 | 7 | 1 |
| 95th Queue (ft) | 52 | 35 | 19 |
| Link Distance (ft) | 624 | 59 | 201 |
| Upstream Blk Time (\%) |  | 0 |  |
| Queuing Penalty (veh) |  | 0 |  |
| Storage Bay Dist (ft) |  |  |  |
| Storage Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  |  |  |

Intersection: 2: Ten Eyck Road \& Pleasant Street (E)

| Movement | NB | SB |
| :--- | ---: | ---: |
| Directions Served | TR | LT |
| Maximum Queue (ft) | 3 | 71 |
| Average Queue (ft) | 0 | 8 |
| 95th Queue (ft) | 3 | 41 |
| Link Distance (ft) | 105 | 59 |
| Upstream Blk Time (\%) |  | 1 |
| Queuing Penalty (veh) |  | 2 |
| Storage Bay Dist (ft) |  |  |
| Storage Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |

Intersection: 3: Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26)

| Movement | EB | EB | EB | EB | WB | WB | B8 | B8 | NB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | T | T | R | T | TR | T | T | LTR | LTR |
| Maximum Queue (ft) | 152 | 291 | 233 | 106 | 283 | 267 | 108 | 68 | 183 | 143 |
| Average Queue (ft) | 33 | 142 | 94 | 15 | 174 | 162 | 9 | 3 | 84 | 83 |
| 95th Queue (ft) | 94 | 249 | 189 | 59 | 267 | 252 | 55 | 30 | 151 | 140 |
| Link Distance (ft) |  | 349 | 349 |  | 199 | 199 | 1221 | 1221 | 285 | 105 |
| Upstream Blk Time (\%) |  | 0 |  |  | 4 | 3 |  |  | 6 |  |
| Queuing Penalty (veh) |  | 0 |  |  | 0 | 0 |  |  | 12 |  |
| Storage Bay Dist (ft) | 115 |  |  | 100 |  |  |  |  |  |  |
| Storage Blk Time (\%) | 0 | 7 | 3 |  | 19 |  |  |  |  |  |
| Queuing Penalty (veh) | 1 | 5 | 1 |  | 0 |  |  |  |  |  |

Queuing and Blocking Report 2029 Without Project Conditions - Weekday AM Peak Hour
Intersection: 8: Bend

| Movement | EB |
| :--- | :---: |
| Directions Served | T |
| Maximum Queue (ft) | 6 |
| Average Queue (ft) | 0 |
| 95th Queue (ft) | 6 |
| Link Distance (ft) | 199 |
| Upstream Blk Time (\%) |  |
| Queuing Penalty (veh) |  |
| Storage Bay Dist (ft) |  |
| Storage Blk Time (\%) |  |
| Queuing Penalty (veh) |  |

Network Summary
Network wide Queuing Penalty: 21

SimTraffic Performance Report
2029 Without Project Conditions - Weekday PM Peak Hour
1: Ten Eyck Road \& Pleasant Street (W) Performance by movement

| Movement | EBL | EBR | NBL | NBT | SBT | SBR | All |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Vehicles Entered | 52 | 48 | 14 | 194 | 140 | 15 | 463 |
| Vehicles Exited | 52 | 48 | 14 | 194 | 141 | 15 | 464 |
| Hourly Exit Rate | 52 | 48 | 14 | 194 | 141 | 15 | 464 |
| Input Volume | 53 | 48 | 17 | 198 | 138 | 13 | 466 |
| \% of Volume | 98 | 101 | 82 | 98 | 102 | 118 | 100 |

## 2: Ten Eyck Road \& Pleasant Street (E) Performance by movement

| Movement | WBL | WBR | NBT | NBR | SBL | SBT | All |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Vehicles Entered | 12 | 6 | 204 | 20 | 4 | 187 | 433 |
| Vehicles Exited | 12 | 6 | 205 | 21 | 4 | 187 | 435 |
| Hourly Exit Rate | 12 | 6 | 205 | 21 | 4 | 187 | 435 |
| Input Volume | 12 | 5 | 211 | 21 | 4 | 184 | 437 |
| \% of Volume | 102 | 114 | 97 | 100 | 94 | 102 | 100 |

## 3: Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26) Performance by movement

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| SBR |  |  |  |  |  |  |  |  |  |  |  |
| Vehicles Entered | 167 | 1289 | 160 | 6 | 1333 | 19 | 137 | 31 | 8 | 44 | 34 |
| Vehicles Exited | 165 | 1289 | 161 | 6 | 1338 | 19 | 139 | 32 | 8 | 44 | 34 |
| Hourly Exit Rate | 165 | 1289 | 161 | 6 | 1338 | 19 | 139 | 32 | 8 | 44 | 34 |
| Input Volume | 178 | 1292 | 158 | 6 | 1331 | 19 | 137 | 28 | 8 | 45 | 33 |
| \% of Volume | 93 | 100 | 102 | 100 | 101 | 99 | 101 | 115 | 97 | 98 | 103 |

## 3: Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26) Performance by movement

| Movement | All |
| :--- | ---: |
| Vehicles Entered | 3348 |
| Vehicles Exited | 3355 |
| Hourly Exit Rate | 3355 |
| Input Volume | 3354 |
| \% of Volume | 100 |

Total Network Performance

|  |  |
| :--- | ---: |
|  |  |
| Vehicles Entered | 3433 |
| Vehicles Exited | 3436 |
| Hourly Exit Rate | 3436 |
| Input Volume | 10386 |
| $\%$ of Volume | 33 |

Queuing and Blocking Report 2029 Without Project Conditions - Weekday PM Peak Hour
Intersection: 1: Ten Eyck Road \& Pleasant Street (W)

| Movement | EB | NB | SB |
| :--- | ---: | ---: | ---: |
| Directions Served | LR | LT | TR |
| Maximum Queue (ft) | 100 | 58 | 75 |
| Average Queue (ft) | 39 | 3 | 5 |
| 95th Queue (ft) | 72 | 26 | 36 |
| Link Distance (ft) | 624 | 59 | 201 |
| Upstream Blk Time (\%) |  | 0 |  |
| Queuing Penalty (veh) |  | 0 |  |
| Storage Bay Dist (ft) |  |  |  |
| Storage Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  |  |  |

Intersection: 2: Ten Eyck Road \& Pleasant Street (E)

| Movement | WB | NB | SB |
| :--- | ---: | ---: | ---: |
| Directions Served | LR | TR | LT |
| Maximum Queue (ft) | 44 | 9 | 91 |
| Average Queue (ft) | 15 | 0 | 25 |
| 95th Queue (ft) | 41 | 7 | 78 |
| Link Distance (ft) | 599 | 106 | 59 |
| Upstream Blk Time (\%) |  |  | 7 |
| Queuing Penalty (veh) |  |  | 12 |
| Storage Bay Dist (ft) |  |  |  |
| Storage Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  |  |  |

Intersection: 3: Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26)

| Movement | EB | EB | EB | EB | WB | WB | WB | B8 | B8 | NB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | T | T | R | L | T | TR | T | T | LTR | LTR |
| Maximum Queue (ft) | 214 | 437 | 402 | 200 | 67 | 292 | 289 | 365 | 342 | 297 | 142 |
| Average Queue (ft) | 127 | 215 | 186 | 52 | 7 | 257 | 247 | 111 | 84 | 153 | 109 |
| 95th Queue (ft) | 224 | 368 | 340 | 159 | 39 | 308 | 314 | 280 | 246 | 268 | 157 |
| Link Distance (ft) |  | 700 | 700 |  |  | 200 | 200 | 1221 | 1221 | 882 | 106 |
| Upstream Blk Time (\%) |  |  |  |  | 0 | 24 | 20 |  |  |  | 25 |
| Queuing Penalty (veh) |  |  |  |  | 0 | 0 | 0 |  |  |  | 49 |
| Storage Bay Dist (ft) | 115 |  |  | 100 | 105 |  |  |  |  |  |  |
| Storage Blk Time (\%) | 15 | 14 | 13 | 0 |  | 36 |  |  |  |  |  |
| Queuing Penalty (veh) | 98 | 25 | 20 | 0 |  | 2 |  |  |  |  |  |

Network Summary
Network wide Queuing Penalty: 208

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2029 Without Project Conditions - Weekday PM Peak Hour
Page 2

SimTraffic Performance Report
2029 With Project Conditions - Weekday AM Peak Hour
1: Ten Eyck Road \& Pleasant Street (W) Performance by movement

| Movement | EBL | EBR | NBL | NBT | SBT | SBR | All |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Vehicles Entered | 16 | 24 | 17 | 58 | 166 | 24 | 305 |
| Vehicles Exited | 16 | 24 | 17 | 58 | 167 | 25 | 307 |
| Hourly Exit Rate | 16 | 24 | 17 | 58 | 167 | 25 | 307 |
| Input Volume | 18 | 24 | 19 | 54 | 169 | 21 | 306 |
| \% of Volume | 89 | 101 | 88 | 106 | 99 | 118 | 100 |

## 2: Ten Eyck Road \& Pleasant Street (E) Performance by movement

| Movement | WBL | NBT | NBR | SBT | All |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Vehicles Entered | 7 | 79 | 38 | 195 | 319 |
| Vehicles Exited | 7 | 78 | 38 | 195 | 318 |
| Hourly Exit Rate | 7 | 78 | 38 | 195 | 318 |
| Input Volume | 8 | 78 | 38 | 196 | 320 |
| \% of Volume | 90 | 100 | 99 | 100 | 100 |

## 3: Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26) Performance by movement

| Movement | EBL | EBT | EBR | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Vehicles Entered | 87 | 974 | 40 | 1102 | 23 | 160 | 4 | 2 | 25 | 14 | 163 |
| Vehicles Exited | 87 | 977 | 40 | 1103 | 23 | 161 | 4 | 2 | 25 | 15 | 163 |
| Hourly Exit Rate | 87 | 977 | 40 | 1103 | 23 | 161 | 4 | 2 | 25 | 15 | 163 |
| Input Volume | 88 | 982 | 39 | 1106 | 22 | 160 | 4 | 2 | 26 | 14 | 166 |
| \% of Volume | 99 | 99 | 103 | 100 | 106 | 101 | 100 | 100 | 97 | 105 | 98 |

## Total Network Performance

|  |  |
| :--- | ---: |
| Vehicles Entered | 2639 |
| Vehicles Exited | 2650 |
| Hourly Exit Rate | 2650 |
| Input Volume | 8011 |
| $\%$ of Volume | 33 |

Queuing and Blocking Report
2029 With Project Conditions - Weekday AM Peak Hour
Intersection: 1: Ten Eyck Road \& Pleasant Street (W)

| Movement | EB | NB | SB |
| :--- | ---: | ---: | ---: |
| Directions Served | LR | LT | TR |
| Maximum Queue (ft) | 55 | 52 | 12 |
| Average Queue (ft) | 23 | 5 | 1 |
| 95th Queue (ft) | 49 | 28 | 8 |
| Link Distance (ft) | 624 | 59 | 201 |
| Upstream Blk Time (\%) |  | 0 |  |
| Queuing Penalty (veh) | 0 |  |  |
| Storage Bay Dist (ft) |  |  |  |
| Storage Blk Time (\%) |  |  |  |

Intersection: 2: Ten Eyck Road \& Pleasant Street (E)

| Movement | WB | NB | SB |
| :--- | ---: | ---: | ---: |
| Directions Served | LR | TR | LT |
| Maximum Queue (ft) | 33 | 9 | 75 |
| Average Queue (ft) | 7 | 0 | 8 |
| 95th Queue (ft) | 28 | 6 | 42 |
| Link Distance (ft) | 599 | 105 | 59 |
| Upstream Blk Time (\%) |  |  | 1 |
| Queuing Penalty (veh) |  |  | 2 |
| Storage Bay Dist (ft) |  |  |  |
| Storage Blk Time (\%) |  |  |  |

Intersection: 3: Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26)

| Movement | EB | EB | EB | EB | WB | WB | B8 | B8 | NB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | T | T | R | T | TR | T | T | LTR | LTR |
| Maximum Queue (ft) | 149 | 310 | 233 | 55 | 272 | 269 | 98 | 75 | 204 | 141 |
| Average Queue (ft) | 44 | 143 | 96 | 12 | 190 | 173 | 11 | 5 | 91 | 85 |
| 95th Queue (ft) | 109 | 251 | 193 | 40 | 283 | 267 | 56 | 37 | 164 | 143 |
| Link Distance (ft) |  | 349 | 349 |  | 199 | 199 | 1221 | 1221 | 285 | 105 |
| Upstream Blk Time (\%) |  | 0 |  |  | 6 | 4 |  |  | 0 | 7 |
| Queuing Penalty (veh) |  | 0 |  |  | 0 | 0 |  |  | 0 | 13 |
| Storage Bay Dist (ft) | 115 |  |  | 100 |  |  |  |  |  |  |
| Storage Blk Time (\%) | 0 | 7 | 3 | 0 | 22 |  |  |  |  |  |
| Queuing Penalty (veh) | 2 | 7 | 1 | 0 | 0 |  |  |  |  |  |

Queuing and Blocking Report 2029 With Project Conditions - Weekday AM Peak Hour
Intersection: 8: Bend

| Movement | EB |
| :--- | :---: |
| Directions Served | T |
| Maximum Queue (ft) | 6 |
| Average Queue (ft) | 0 |
| 95th Queue (ft) | 6 |
| Link Distance ( ft ) | 199 |
| Upstream Blk Time (\%) |  |
| Queuing Penalty (veh) |  |
| Storage Bay Dist (ft) |  |
| Storage Blk Time (\%) |  |
| Queuing Penalty (veh) |  |

Network Summary
Network wide Queuing Penalty: 25

SimTraffic Performance Report
2029 With Project Trips Conditions - Weekday PM Peak Hour
1: Ten Eyck Road \& Pleasant Street (W) Performance by movement

| Movement | EBL | EBR | NBL | NBT | SBT | SBR | All |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Vehicles Entered | 56 | 46 | 18 | 196 | 142 | 12 | 470 |
| Vehicles Exited | 56 | 46 | 18 | 196 | 143 | 12 | 471 |
| Hourly Exit Rate | 56 | 46 | 18 | 196 | 143 | 12 | 471 |
| Input Volume | 53 | 48 | 17 | 198 | 138 | 13 | 466 |
| \% of Volume | 106 | 96 | 106 | 99 | 103 | 94 | 101 |

## 2: Ten Eyck Road \& Pleasant Street (E) Performance by movement

| Movement | WBL | WBR | NBT | NBR | SBL | SBT | All |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Vehicles Entered | 33 | 7 | 208 | 27 | 4 | 186 | 465 |
| Vehicles Exited | 34 | 6 | 210 | 27 | 4 | 186 | 467 |
| Hourly Exit Rate | 34 | 6 | 210 | 27 | 4 | 186 | 467 |
| Input Volume | 34 | 5 | 212 | 30 | 4 | 184 | 468 |
| \% of Volume | 100 | 114 | 99 | 91 | 94 | 101 | 100 |

## 3: Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26) Performance by movement

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| SBR |  |  |  |  |  |  |  |  |  |  |  |
| Vehicles Entered | 180 | 1297 | 162 | 5 | 1350 | 20 | 136 | 29 | 8 | 45 | 35 |
| Vehicles Exited | 179 | 1298 | 163 | 5 | 1354 | 20 | 137 | 29 | 8 | 45 | 36 |
| Hourly Exit Rate | 179 | 1298 | 163 | 5 | 1354 | 20 | 137 | 29 | 8 | 45 | 36 |
| Input Volume | 186 | 1292 | 158 | 6 | 1331 | 20 | 137 | 28 | 8 | 47 | 34 |
| \% of Volume | 96 | 100 | 103 | 83 | 102 | 99 | 100 | 105 | 97 | 96 | 107 |

## 3: Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26) Performance by movement

| Movement | All |
| :--- | ---: |
| Vehicles Entered | 3408 |
| Vehicles Exited | 3415 |
| Hourly Exit Rate | 3415 |
| Input Volume | 3386 |
| $\%$ of Volume | 101 |

Total Network Performance

|  |  |
| :--- | ---: |
| Vehicles Entered | 3490 |
| Vehicles Exited | 3494 |
| Hourly Exit Rate | 3494 |
| Input Volume | 10484 |
| $\%$ of Volume | 33 |

Queuing and Blocking Report 2029 With Project Trips Conditions - Weekday PM Peak Hour
Intersection: 1: Ten Eyck Road \& Pleasant Street (W)

| Movement | EB | NB | SB |
| :--- | ---: | ---: | ---: |
| Directions Served | LR | LT | TR |
| Maximum Queue (ft) | 131 | 46 | 96 |
| Average Queue (ft) | 42 | 4 | 9 |
| 95th Queue (ft) | 97 | 28 | 56 |
| Link Distance (ft) | 624 | 59 | 201 |
| Upstream Blk Time (\%) |  | 0 | 0 |
| Queuing Penalty (veh) |  | 0 | 0 |

Storage Bay Dist (ft)
Storage BIk Time (\%)
Queuing Penalty (veh)
Intersection: 2: Ten Eyck Road \& Pleasant Street (E)

| Movement | WB | NB | SB |
| :--- | ---: | ---: | ---: |
| Directions Served | LR | TR | LT |
| Maximum Queue (ft) | 128 | 24 | 89 |
| Average Queue (ft) | 39 | 1 | 32 |
| 95th Queue (ft) | 102 | 16 | 88 |
| Link Distance (ft) | 599 | 106 | 59 |
| Upstream Blk Time (\%) |  | 0 | 10 |
| Queuing Penalty (veh) |  | 0 | 20 |
| Storage Bay Dist (ft) |  |  |  |
| Storage Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  |  |  |

Intersection: 3: Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26)

| Movement | EB | EB | EB | EB | WB | WB | WB | B8 | B8 | NB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | T | T | R | L | T | TR | T | T | LTR | LTR |
| Maximum Queue (ft) | 215 | 556 | 500 | 200 | 126 | 304 | 307 | 371 | 336 | 284 | 145 |
| Average Queue (ft) | 147 | 254 | 213 | 58 | 8 | 259 | 249 | 116 | 90 | 145 | 115 |
| 95th Queue (ft) | 243 | 470 | 419 | 170 | 54 | 316 | 314 | 284 | 251 | 248 | 158 |
| Link Distance (ft) |  | 700 | 700 |  |  | 200 | 200 | 1221 | 1221 | 882 | 106 |
| Upstream Blk Time (\%) |  | 0 | 0 |  | 0 | 24 | 20 |  |  |  | 34 |
| Queuing Penalty (veh) |  | 0 | 0 |  | 0 | 0 | 0 |  |  |  | 76 |
| Storage Bay Dist (ft) | 115 |  |  | 100 | 105 |  |  |  |  |  |  |
| Storage Blk Time (\%) | 23 | 15 | 14 | 0 | 0 | 35 |  |  |  |  |  |
| Queuing Penalty (veh) | 148 | 28 | 22 | 0 | 0 | 2 |  |  |  |  |  |

Queuing and Blocking Report 2029 With Project Trips Conditions - Weekday PM Peak Hour
Intersection: 8: Bend

| Movement | EB | EB |
| :--- | ---: | ---: |
| Directions Served | T | T |
| Maximum Queue (ft) | 6 | 11 |
| Average Queue (ft) | 0 | 0 |
| 95th Queue (ft) | 6 | 8 |
| Link Distance (ft) | 200 | 200 |
| Upstream Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |
| Storage Bay Dist (ft) |  |  |
| Storage Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |

Network Summary
Network wide Queuing Penalty: 297

SimTraffic Performance Report
2029 With Project Conditions - Weekday AM Peak Hour + SBLT Lane
1: Ten Eyck Road \& Pleasant Street (W) Performance by movement

| Movement | EBL | EBR | NBL | NBT | SBT | SBR | All |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Vehicles Entered | 18 | 26 | 17 | 56 | 172 | 24 | 313 |
| Vehicles Exited | 18 | 26 | 17 | 56 | 172 | 24 | 313 |
| Hourly Exit Rate | 18 | 26 | 17 | 56 | 172 | 24 | 313 |
| Input Volume | 18 | 24 | 19 | 54 | 169 | 21 | 306 |
| \% of Volume | 100 | 109 | 88 | 103 | 102 | 113 | 102 |

## 2: Ten Eyck Road \& Pleasant Street (E) Performance by movement

| Movement | WBL | NBT | NBR | SBT | All |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Vehicles Entered | 7 | 78 | 36 | 201 | 322 |
| Vehicles Exited | 7 | 78 | 37 | 201 | 323 |
| Hourly Exit Rate | 7 | 78 | 37 | 201 | 323 |
| Input Volume | 8 | 78 | 38 | 196 | 320 |
| \% of Volume | 90 | 100 | 97 | 103 | 101 |

## 3: Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26) Performance by movement

| Movement | EBL | EBT | EBR | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Vehicles Entered | 88 | 993 | 39 | 1119 | 21 | 165 | 4 | 4 | 24 | 13 | 173 |
| Vehicles Exited | 87 | 993 | 39 | 1123 | 21 | 165 | 4 | 4 | 24 | 13 | 173 |
| Hourly Exit Rate | 87 | 993 | 39 | 1123 | 21 | 165 | 4 | 4 | 24 | 13 | 173 |
| Input Volume | 88 | 982 | 39 | 1106 | 22 | 160 | 4 | 2 | 26 | 14 | 166 |
| \% of Volume | 99 | 101 | 100 | 102 | 97 | 103 | 100 | 200 | 93 | 91 | 104 |

Total Network Performance

|  |  |
| :--- | ---: |
| Vehicles Entered | 2691 |
| Vehicles Exited | 2693 |
| Hourly Exit Rate | 2693 |
| Input Volume | 8011 |
| \% of Volume | 34 |

Queuing and Blocking Report
2029 With Project Conditions - Weekday AM Peak Hour + SBLT Lane
Intersection: 1: Ten Eyck Road \& Pleasant Street (W)

| Movement | EB | NB | SB |
| :--- | ---: | ---: | ---: |
| Directions Served | LR | LT | TR |
| Maximum Queue (ft) | 58 | 46 | 19 |
| Average Queue (ft) | 24 | 5 | 1 |
| 95th Queue (ft) | 49 | 27 | 17 |
| Link Distance (ft) | 624 | 59 | 201 |
| Upstream Blk Time (\%) |  | 0 |  |
| Queuing Penalty (veh) |  | 0 |  |
| Storage Bay Dist (ft) |  |  |  |
| Storage Blk Time (\%) |  |  |  |

Intersection: 2: Ten Eyck Road \& Pleasant Street (E)

| Movement | WB | SB |
| :--- | ---: | ---: |
| Directions Served | LR | LT |
| Maximum Queue (ft) | 34 | 41 |
| Average Queue (ft) | 7 | 2 |
| 95th Queue (ft) | 28 | 20 |
| Link Distance (ft) | 601 | 59 |
| Upstream Blk Time (\%) |  | 0 |
| Queuing Penalty (veh) |  | 1 |
| Storage Bay Dist (ft) |  |  |
| Storage Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |

Intersection: 3: Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26)

| Movement | EB | EB | EB | EB | WB | WB | B8 | B8 | NB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | T | T | R | T | TR | T | T | LTR | L | TR |
| Maximum Queue (ft) | 193 | 301 | 242 | 80 | 275 | 262 | 143 | 113 | 202 | 81 | 130 |
| Average Queue (ft) | 47 | 139 | 92 | 12 | 190 | 176 | 16 | 10 | 90 | 22 | 67 |
| 95th Queue (ft) | 119 | 252 | 195 | 49 | 285 | 268 | 84 | 65 | 158 | 60 | 118 |
| Link Distance (ft) |  | 341 | 341 |  | 192 | 192 | 1221 | 1221 | 285 | 104 | 104 |
| Upstream Blk Time (\%) |  | 0 | 0 |  | 7 | 6 |  |  |  | 0 | 3 |
| Queuing Penalty (veh) |  | 0 | 0 |  | 0 | 0 |  |  |  | 0 | 3 |
| Storage Bay Dist (ft) | 115 |  |  | 100 |  |  |  |  |  |  |  |
| Storage Blk Time (\%) | 0 | 7 | 3 | 0 | 22 |  |  |  |  |  |  |
| Queuing Penalty (veh) | 2 | 6 | 1 | 0 | 0 |  |  |  |  |  |  |

Network Summary
Network wide Queuing Penalty: 14

SimTraffic Performance Report
2029 With Project Trips Conditions - Weekday PM Peak Hour + SBLT Lane
1: Ten Eyck Road \& Pleasant Street (W) Performance by movement

| Movement | EBL | EBR | NBL | NBT | SBT | SBR | All |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Vehicles Entered | 50 | 48 | 17 | 198 | 144 | 14 | 471 |
| Vehicles Exited | 51 | 48 | 17 | 198 | 145 | 14 | 473 |
| Hourly Exit Rate | 51 | 48 | 17 | 198 | 145 | 14 | 473 |
| Input Volume | 53 | 48 | 17 | 198 | 138 | 13 | 466 |
| \% of Volume | 96 | 101 | 100 | 100 | 105 | 110 | 101 |

## 2: Ten Eyck Road \& Pleasant Street (E) Performance by movement

| Movement | WBL | WBR | NBT | NBR | SBL | SBT | All |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Vehicles Entered | 31 | 5 | 212 | 29 | 4 | 191 | 472 |
| Vehicles Exited | 32 | 5 | 213 | 29 | 4 | 191 | 474 |
| Hourly Exit Rate | 32 | 5 | 213 | 29 | 4 | 191 | 474 |
| Input Volume | 34 | 5 | 212 | 30 | 4 | 184 | 468 |
| \% of Volume | 94 | 95 | 101 | 97 | 94 | 104 | 101 |

## 3: Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26) Performance by movement

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| SBR |  |  |  |  |  |  |  |  |  |  |  |
| Vehicles Entered | 190 | 1277 | 152 | 6 | 1336 | 20 | 136 | 23 | 8 | 45 | 38 |
| Vehicles Exited | 190 | 1279 | 153 | 5 | 1340 | 20 | 139 | 24 | 8 | 44 | 38 |
| Hourly Exit Rate | 190 | 1279 | 153 | 5 | 1340 | 20 | 139 | 24 | 8 | 44 | 38 |
| Input Volume | 186 | 1292 | 158 | 6 | 1331 | 20 | 137 | 28 | 8 | 47 | 34 |
| \% of Volume | 102 | 99 | 97 | 83 | 101 | 99 | 101 | 86 | 97 | 94 | 113 |

## 3: Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26) Performance by movement

| Movement | All |
| :--- | ---: |
| Vehicles Entered | 3372 |
| Vehicles Exited | 3380 |
| Hourly Exit Rate | 3380 |
| Input Volume | 3386 |
| $\%$ of Volume | 100 |

Total Network Performance

|  |  |
| :--- | ---: |
| Vehicles Entered | 3449 |
| Vehicles Exited | 3456 |
| Hourly Exit Rate | 3456 |
| Input Volume | 10484 |
| $\%$ of Volume | 33 |

Queuing and Blocking Report 2029 With Project Trips Conditions - Weekday PM Peak Hour + SBLT Lane
Intersection: 1: Ten Eyck Road \& Pleasant Street (W)

| Movement | EB | NB | SB |
| :--- | ---: | ---: | ---: |
| Directions Served | LR | LT | TR |
| Maximum Queue (ft) | 95 | 56 | 56 |
| Average Queue (ft) | 38 | 5 | 4 |
| 95th Queue (ft) | 70 | 33 | 32 |
| Link Distance (ft) | 624 | 59 | 201 |
| Upstream Blk Time (\%) |  | 0 |  |
| Queuing Penalty (veh) |  | 0 |  |
| Storage Bay Dist (ft) |  |  |  |
| Storage Blk Time (\%) |  |  |  |

Intersection: 2: Ten Eyck Road \& Pleasant Street (E)

| Movement | WB | NB | SB |
| :--- | ---: | ---: | ---: |
| Directions Served | LR | TR | LT |
| Maximum Queue (ft) | 96 | 40 | 86 |
| Average Queue (ft) | 28 | 2 | 14 |
| 95th Queue (ft) | 68 | 21 | 59 |
| Link Distance (ft) | 601 | 104 | 59 |
| Upstream Blk Time (\%) |  | 0 | 3 |
| Queuing Penalty (veh) |  | 0 | 6 |
| Storage Bay Dist (ft) |  |  |  |

Intersection: 3: Wolf Drive/Ten Eyck Road \& Mt. Hood Highway (US 26)

| Movement | EB | EB | EB | EB | WB | WB | WB | B8 | B8 | NB | SB | SB |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Directions Served | L | T | T | R | L | T | TR | T | T | LTR | L | TR |
| Maximum Queue (ft) | 215 | 550 | 490 | 200 | 128 | 302 | 289 | 376 | 353 | 280 | 102 | 144 |
| Average Queue (ft) | 145 | 240 | 197 | 50 | 10 | 253 | 242 | 121 | 94 | 147 | 36 | 86 |
| 95th Queue (ft) | 239 | 440 | 385 | 153 | 64 | 306 | 302 | 303 | 281 | 247 | 85 | 147 |
| Link Distance (ft) |  | 692 | 692 |  |  | 193 | 193 | 1221 | 1221 | 881 | 104 | 104 |
| Upstream Blk Time (\%) |  | 0 |  |  | 0 | 25 | 20 |  |  |  | 1 | 13 |
| Queuing Penalty (veh) |  | 0 |  |  | 0 | 0 | 0 |  |  |  | 1 | 15 |
| Storage Bay Dist (ft) | 115 |  |  | 100 | 105 |  |  |  |  |  |  |  |
| Storage Blk Time (\%) | 24 | 14 | 13 | 0 |  | 35 |  |  |  |  |  |  |
| Queuing Penalty (veh) | 153 | 26 | 20 | 0 |  | 2 |  |  |  |  |  |  |

Network Summary
Network wide Queuing Penalty: 223

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SimTraffic 10 Report - by PBS
2029 With Project Trips Conditions - Weekday PM Peak Hour + SBLT Lane
Page 2
$\underset{\text { Crash History }}{\text { Appendix D }}$



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| General \& Site Information |  |
| :--- | :--- |
| Analyst: | John Manix PE |
| Agency/Company: | PBS Engineering and Env. |
| Date: |  |
| Project Name: | Sandy Health Clinic |



| Intersection Population Type Crash Rate |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| Average Crash Rate per intersection type |  |  |  |  |
| Intersection Pop. Type | Sum of Crashes | Sum of 5year MEV | Avg Crash Rate for Ref Pop. | INT in Pop |
| Rural 3SG | 0 | 0 |  |  |
| Rural 3ST | 0 | 0 |  |  |
| Rural 4SG | 0 | 0 |  |  |
| Rural 4ST | 0 | 0 |  |  |
| Urban 3ST | 0 | 0 |  |  |
| Urban 3SG | 0 | 0 |  |  |
| Urban 4ST | 0 | 0 |  |  |
| Urban 4SG | 11 | 43 | 0.2567 | 1 |


| Critical Rate Calculation |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Intersection | AADT Entering Intersection | 5-year MEV | Crash Total | Intersection Population Type | Intersection Crash Rate | Reference Population Crash Rate | Critical Rate | Over Critical |
| Highway 26 and Ten Eyck | 23,480 | 42.9 | 11 | Urban 4SG | 0.26 | 0.26 | 0.40 | Under |
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Administration
Public Service Building
2051 Kaen Road, Suite 367 Oregon City, OR 97045-4035

503-742-5300
Beavercreek Health Center 110 Beavercreek Road Oregon City, OR 97045-4023

503-655-8471
Primary Care, Dental Services and Behavioral Health

Sunnyside Health Center 9775 SE Sunnyside Road Suite 200
Clackamas, OR 97015-5721 503-655-8471
Primary Care, Dental Services and Behavioral Health

Gladstone Health Clinic 18911 Portland Avenue Gladstone, OR 97027-1630 503-850-4472 Primary Care, Pediatrics,

Dental Services and Behavioral Health

Sandy Health Clinic 37400 SE Bell Street Sandy, OR 97055-7868 503-668-3493
Primary Care and Behavioral Health

Hilltop Behavioral Health Clinic 998 Library Court Oregon City, OR 97045-4041 503-655-8401

Stewart Behavioral Health Clinic
1002 Library Court Oregon City, OR 97045-4066 503-655-8264

Sandy Behavioral Health Clinic 38872 Proctor Boulevard PO Box 1390 Sandy, OR 97055-8035 503-722-6950

School Based Health Centers Oregon City High School 19761 S Beavercreek Road Oregon City, OR 97045-9557 503-785-8770

Rex Putnam High School 4950 SE Roethe Rd. Milwaukie, OR 97267-5746 503-722-6858

Sandy High School 37400 Bell Street Sandy, OR 97055-7868 503-668-3483

February 10, 2020

Police Chief Ernie Roberts
Sandy Police Department
39850 Pleasant Street
Sandy, Oregon 97055

## RE: SIX SHARED PARKING SPACES ALIGNED ON THE PROPERTY LINE BETWEEN OUR PROPERTIES

Dear Chief Roberts:

This letter is to communicate openly with your office regarding the six shared parking spaces, along your west property line. At the time of our interest in the vacant building (i.e. property) adjacent to your precinct, we were informed by the sellers) of the existing six shared parking spaces agreement that was established some time ago. The City of Sandy Design Review Process requires the County to maintain the use of the existing six shared parking spaces.

Our architectural firm, Ankrom Moisan Architects, Inc. on our behalf is working with the City of Sandy Planning Office to meet their requirements for our new building design. At this point, we are unaware of an exact start of our construction schedule for the New Sandy Health Clinic. However, the construction project will occur sometime in 2020-2021. As soon as our project is released to the public via the bidding process to contractors, we will alert your office of our planned steps and keep you apprised of an established construction schedule, once we receive bids. Once a general contractor is hired, we will need to use the parking spaces between our property lines for staging for the project. We are hopeful this is an allowable request.

In regards to the shared parking spaces, no employee will park there in the future, once the new building opens to the public. Only patients will be parking in our new lot and periodically using the shared parking spaces. Attached please find the proposed building site plan, which includes the shared parking spaces.

It is the County's full intention to be good neighbors of the Sandy Police Department. If you have questions, I can reach me at my office phone number 503-742-5495. The assigned Project Coordinator is Steve Kelly and his phone number is 503-650-5665. Thank you for your valuable time.


Deborah Cockrell, FQHC
Health Centers, Director

```
Cc: Steve Kelly, Project Coordinator
    Ankrom Moisan Architects, Inc.
                            Healthy Families. Strong Communities.
        2051 Kaen Road, Oregon City, OR 97045 • www.clackamas.us/healthcenters
                                Phone (503) 742-5300 \bullet Fax (503) 742-5979
```



## PARKING SPACE LEASE AGREEMENT

This agreement is made and entered into this $\qquad$ day of $\qquad$ , 2020, by and between Immanuel Lutheran Church ("Owner") and Clackamas County ("County") for the lease of Seventeen (17) parking spaces, in the south parking lot across Pleasant Street from property described as Lots One (1) to including Lot Twelve (12), Block Two (2), Minnie Meinig's Pine View Tracts and commonly known as 39901 Pleasant Street, Sandy OR 97055 (map attached).

Owner represents that it has the right to lease these parking spaces to the County for the County's exclusive use, subject to the following terms and conditions.

1. Term. The lease shall commence on $\qquad$ , 2020 and shall end on $\qquad$ ,
2. County shall be allowed use of the spaces between the hours of 6 am to 8 pm , Monday through Friday.
3. Hold Over. If County shall hold over and remain in possession of said premises after expiration of this Lease without any written lease actually being made, such holding over shall not be deemed to operate as a renewal or extension of this Lease but shall only create a tenancy which may be terminated at any time by Owner upon sixty (60) days written notice to County.
4. Renewal. This Lease may be renewed for successive seven (7) year terms ("Renewal Term") by mutual written agreement of the parties, executed not less than six (6) months prior to the expiration of the Initial Term or any Renewal Term, as applicable.
5. Use. County will use the leased parking spaces only for the parking of vehicles. Owner will provide Country with exclusive use of the leased parking spaces, and Owner will provide signage indicating that these spaces are reserved for County use during the hours of operation noted in 1. Above. County shall be entitled to full use and possession of the parking spaces for the entire lease term.
6. Fees and Payment. The annual fee will be $\$ 6,000$ per year. Payment is due on the first day of $\qquad$ and is payable by the $14^{\text {th }}$ day of $\qquad$ . A late fee of $\$ 100$ will be assessed on the $15^{\text {th }}$ day of $\qquad$ if payment has not been received by the $14^{\text {th }}$ day. No deposit is required. Checks for payment should be made payable to:

Immanuel Lutheran Church
39901 Pleasant Street

Sandy Parking Lease
Page 1 of $\mathbf{5}$

Sandy, Oregon 97055
6. Property Taxes and other expenses. Owner is solely responsible for property taxes or other expenses such as electric bill for lighting (if any) on the leased parking spaces.
7. Maintenance. County shall be responsible for any necessary trash collection, sweeping, or maintenance of the leased parking spaces. Owner shall be responsible for all other necessary maintenance of the leased parking spaces.

## 8. Termination.

a. Either party may terminate the Lease upon occurrence of an event of default. An event of default shall be deemed to occur should any of the following events happen:

1. Failure of County to pay lease fees within 30 days from written notice by Owner to County that lease fees are overdue;
2. Repeated failure of County or its employees to obey reasonable rules of the Owner concerning matters of security, safety, or preservation of the Owner's facilities, during the term of the Agreement; or
3. Failure of either party to comply with any term or condition of this Agreement.

In the event of default, the defaulting party shall be given notice of the default in writing by the other party. The party which has been given notice of default shall have 30 days to correct said default. If the default is not corrected within the 30 day notice period, the other party shall have the right to terminate this lease by giving written notice of uncorrected default and termination to the defaulting party. Any notice shall be given by in writing through certified mail, and shall be effective upon receipt. Notice shall be sent to the address for the receiving party as designated herein.
b. County may terminate this Lease in the event the County fails to receive expenditure authority sufficient to allow County, in the exercise of its reasonable administrative discretion, to continue to make payments for performance of this Lease, or if federal or state laws, regulations or guidelines are modified or interpreted in such a way that County is prohibited from performing under the Lease.
c. Any termination of this Lease shall not prejudice any rights or obligations accrued to the parties prior to termination.
9. Constitutional Debt Limitation. This Lease is expressly subject to the debt limitation of Oregon Counties set forth in Article XI, Section 10 of the Oregon Constitution, and is contingent
upon funds being appropriated therefor. Any provisions herein which would conflict with law are deemed inoperative to that extent.
10. No Attorney Fees: In the event any arbitration, action or proceeding, including any bankruptcy proceeding, is instituted to enforce any term of this Lease, each party shall be responsible for its own attorneys' fees and expenses.
11. Warrant of Authority: Owner warrants and represents that it is the sole owner of the leased premises subject to this Lease, and that Owner has full authority to execute this Lease. The undersigned warrants and represents that he/she has full authority to sign on behalf of Owner.
[SIGNATURES TO FOLLOW]

Lessee:
CLACKAMAS COUNTY

Commissioner Jim Bernard, Chair
Commissioner Sonya Fischer
Commissioner Ken Humberston
Commissioner Paul Savas
Commissioner Martha Schrader

Signing on Behalf of the Board.

Richard Swift, Director
Health, Housing, and Human Services
Department

Date

Lessor:
IMMANUEL LUTHERAN CHURCH

39901 Pleasant Street
Sandy, Oregon 97055

Dwight Reigert, Legal Signer

Date

## STATE OF OREGON, County of Clackamas) ss.

BE IT REMEMBERED, that on this $\qquad$ day of $\qquad$ 2020, before me, the undersigned, a Notary Public in and for said County and State, personally appeared DWIGHT REIGERT known to be to be the individual(s) described in and who executed the within Parking Space Lease Agreement, and acknowledged to me that said individual(s) executed the same freely and voluntarily.

IN TESTIMONY WHEREOF, I have hereunto sent my hand and affixed my official seal the day and year last about written.

NOTARY PUBLIC FOR OREGON
[PROPERTY MAP TO FOLLOW]


ENGINEERING

## MEMORANDUM

```
DATE: 03/23/2020
BY: Norm Scheg
SUBJECT: Stormwater Utility Narrative
PROJECT: Sandy Health Clinic
PROJECT NO.: A19188.10
```

This memorandum is to outline the stormwater requirements for the Sandy Health Clinic project located at 39831 Highway 26, Sandy, OR 97055. The project consists of construction of a 10,940 SF building, $5,291 \mathrm{SF}$ of pervious parking lot with associated pedestrian pathways and utilities. The total site is $20,204 \mathrm{SF}$ and is predominately impervious with an existing building and parking lot. Post construction we will be collecting roof runoff in downspouts that will be hard piped to a drywell. This drywell is "authorized by rule" by Oregon DEQ as it will infiltrate runoff from roofs and a parking lot of less than 50 vehicles. We will be constructing pervious pavement for the parking area as well. Both of these facilities have been modeled utilizing $2^{\prime \prime}$ per hour as a basis for storage sizing. Once the site has been cleared, and before construction of any utilities, a Geotechnical Engineer will be retained to test the actual infiltration rates of the native soils to assure they have at least a design infiltration rate of $2^{\prime \prime}$ per hour or more. The drywell sizing will be reviewed at that time to see it the annulus of rock can be reduced. The rock under the pervious pavement is the minimum thickness for structural integrity. Attached are HydroCAD calculations verifying the infiltration systems meets the stated requirements. A comprehensive stormwater plan will be submitted with the next submittal.


## Pervious Pavement



Time span=5.00-48.00 hrs, dt=0.05 hrs, 861 points
Runoff by SBUH method, Split Pervious/Imperv.
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method
Subcatchment 1S: Pervious Pavement Runoff Area=5,291 sf 100.00\% Impervious Runoff Depth>4.19" $\mathrm{Tc}=0.0 \mathrm{~min} \mathrm{CN}=0 / 98$ Runoff=0.15 cfs 0.042 af

Peak Elev=0.00' Storage=16 cf Inflow=0.15 cfs 0.042 af Outflow $=0.15$ cfs 0.042 af

Total Runoff Area $=0.121$ ac Runoff Volume $=0.042$ af Average Runoff Depth $=4.19 "$
$0.00 \%$ Pervious $=0.000$ ac $100.00 \%$ Impervious $=0.121 \mathrm{ac}$

## Summary for Subcatchment 1S: Pervious Pavement

Runoff $=0.15 \mathrm{cfs} @ 7.80 \mathrm{hrs}$, Volume= 0.042 af, Depth> 4.19"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 5.00-48.00 hrs, dt= 0.05 hrs Type IA 24-hr 25yr Rainfall=5.00"

|  | Area (sf) | CN | Description |
| :--- | ---: | :--- | :--- |
| 291 | 98 | Pervious Pavement |  |
|  | 5,291 | 98 | $100.00 \%$ Impervious Area |

Subcatchment 1S: Pervious Pavement


QRunoff

## Summary for Pond 1P: Pavement Aggregate

| Inflow Area $=$ | $0.121 \mathrm{ac}, 100.00 \%$ Impervious, Inflow Depth $>4.19 "$ | for 25 yr event |  |
| :--- | :--- | :--- | :--- |
| Inflow | $=$ | $0.15 \mathrm{cfs} @$ | 7.80 hrs, Volume= |
| Outflow | $=$ | $0.15 \mathrm{cfs} @$ | 7.83 hrs , Volume= |
| Discarded | $=$ | $0.15 \mathrm{cfs} @$ | 7.83 hrs , Volume= |

Routing by Stor-Ind method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Peak Elev=0.00' @ 7.83 hrs Surf.Area= 211,640 sf Storage= 16 cf
Plug-Flow detention time $=2.6 \mathrm{~min}$ calculated for 0.042 af ( $100 \%$ of inflow)
Center-of-Mass det. time $=1.8 \mathrm{~min}(714.7-712.9$ )

| Volume | Invert | Avail. Storage | Storage Description |
| :---: | ---: | ---: | ---: |
| $\# 1$ | $0.00^{\prime}$ | $105,820 \mathrm{cf}$ | Custom Stage Data (Prismatic) Listed below (Recalc) $\times 40$ |


| Elevation <br> (feet) | Surf.Area <br> (sq-ft) | Inc.Store <br> (cubic-feet) | Cum.Store <br> (cubic-feet) |
| ---: | ---: | ---: | ---: |
| 0.00 | 5,291 | 0 | 0 |
| 0.50 | 5,291 | 2,646 | 2,646 |


| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | :---: | :--- |
| $\# 1$ | Discarded | $0.00^{\prime}$ | $2.000 \mathrm{in} / \mathrm{hr}$ Exfiltration over Surface area |

Discarded OutFlow Max=9.80 cfs @ $7.83 \mathrm{hrs} H W=0.00^{\prime}$ (Free Discharge)
(1=Exfiltration (Exfiltration Controls 9.80 cfs )
Pond 1P: Pavement Aggregate


Time span=5.00-48.00 hrs, dt=0.05 hrs, 861 points
Runoff by SBUH method, Split Pervious/Imperv.
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method
Subcatchment 1S: Pervious Pavement Runoff Area=5,291 sf 100.00\% Impervious Runoff Depth>5.05" $\mathrm{Tc}=0.0 \mathrm{~min} \mathrm{CN}=0 / 98$ Runoff=$=0.18 \mathrm{cfs} 0.051$ af

Pond 1P: Pavement Aggregate
Peak Elev=0.00' Storage=19 cf Inflow=0.18 cfs 0.051 af Outflow $=0.18 \mathrm{cfs} \quad 0.051$ af

Total Runoff Area $=0.121$ ac Runoff Volume $=0.051$ af Average Runoff Depth $=5.05^{\prime \prime}$ $0.00 \%$ Pervious $=0.000$ ac $100.00 \%$ Impervious $=0.121$ ac

Summary for Subcatchment 1S: Pervious Pavement
Runoff $=0.18 \mathrm{cfs} @ 7.80 \mathrm{hrs}$, Volume= 0.051 af, Depth> $5.05^{\prime \prime}$
Runoff by SBUH method, Split Pervious/Imperv., Time Span= $5.00-48.00 \mathrm{hrs}$, $\mathrm{dt}=0.05 \mathrm{hrs}$ Type IA 24-hr 100yr Rainfall=6.00"

|  | Area (sf) | CN |
| :--- | ---: | :--- |
|  | Description |  |
|  | 98 | Pervious Pavement |
|  | 98 | $100.00 \%$ Impervious Area |

Subcatchment 1S: Pervious Pavement



## Summary for Pond 1P: Pavement Aggregate

| Inflow Area $=$ | $0.121 \mathrm{ac}, 100.00 \%$ | Impervious, Inflow Depth > $5.05 "$ | for 100 yr event |
| :--- | :--- | :--- | :--- | :--- |
| Inflow $=$ | $0.18 \mathrm{cfs} @$ | 7.80 hrs, Volume $=$ | 0.051 af |
| Outflow $=$ | $0.18 \mathrm{cfs} @$ | 7.83 hrs, Volume $=$ | 0.051 af, Atten= $=0 \%$, Lag= $=1.8 \mathrm{~min}$ |
| Discarded $=$ | $0.18 \mathrm{cfs} @$ | 7.83 hrs, Volume $=$ | 0.051 af |

Routing by Stor-Ind method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Peak Elev=0.00' @ 7.83 hrs Surf.Area=211,640 sf Storage= 19 cf
Plug-Flow detention time $=2.6$ min calculated for 0.051 af (100\% of inflow)
Center-of-Mass det. time $=1.8 \mathrm{~min}$ (714.1-712.3)

| Volume | Invert | Avail.Storage | Storage Description |
| :---: | ---: | ---: | ---: |
| $\# 1$ | $0.00^{\prime}$ | 105,820 cf | Custom Stage Data (Prismatic) Listed below (Recalc) $\times 40$ |


| Elevation <br> (feet) | Surf.Area <br> (sq-ft) | Inc.Store <br> (cubic-feet) | Cum.Store <br> (cubic-feet) |
| ---: | ---: | ---: | ---: |
| 0.00 | 5,291 | 0 | 0 |
| 0.50 | 5,291 | 2,646 | 2,646 |


| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | :---: | :--- |
| $\# 1$ | Discarded | $0.00^{\prime}$ | 2.000 in/hr Exfiltration over Surface area |

Discarded OutFlow Max=9.80 cfs @ $7.83 \mathrm{hrs} \mathrm{HW}=0.00^{\prime}$ (Free Discharge)
1=Exfiltration (Exfiltration Controls 9.80 cfs )
Pond 1P: Pavement Aggregate



Time span $=5.00-48.00 \mathrm{hrs}, \mathrm{dt}=0.05 \mathrm{hrs}, 861$ points Runoff by SBUH method, Split Pervious/Imperv.
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 2S: Roof

Pond 2P: Drywell

Runoff Area=10,940 sf $100.00 \%$ Impervious Runoff Depth>4.19" $\mathrm{Tc}=0.0 \mathrm{~min} \mathrm{CN}=0 / 98$ Runoff $=0.30 \mathrm{cfs} 0.088$ af

Peak Elev=12.83' Storage=2,396 cf Inflow=0.30 cfs 0.088 af Outflow $=0.02 \mathrm{cfs} \quad 0.074$ af

Total Runoff Area $=0.251$ ac Runoff Volume $=0.088$ af Average Runoff Depth $=4.19^{\prime \prime}$ $0.00 \%$ Pervious $=0.000$ ac $\quad 100.00 \%$ Impervious $=0.251$ ac

Summary for Subcatchment 2S: Roof
Runoff $=\quad 0.30 \mathrm{cfs} @ 7.80 \mathrm{hrs}$, Volume $=\quad 0.088 \mathrm{af}$, Depth> 4.19"

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 5.00-48.00 hrs, dt= 0.05 hrs Type IA 24-hr 25yr Rainfall=5.00"

|  | Area (sf) | CN |
| :--- | ---: | :--- | Description 10.

Subcatchment 2S: Roof


## $\square$ Runoff

Summary for Pond 2P: Drywell


Routing by Stor-Ind method, Time Span=5.00-48.00 hrs, dt= 0.05 hrs
Peak Elev= 12.83' @ 23.99 hrs Surf.Area= 452 sf Storage= 2,396 cf
Plug-Flow detention time= 982.6 min calculated for 0.074 af ( $85 \%$ of inflow)
Center-of-Mass det. time= 880.4 min (1,593.3-712.9)

| Volume | Invert | Avail.Storage | Storage Description |
| :--- | ---: | ---: | :--- |
| $\# 1$ | $0.00^{\prime}$ | $3,482 \mathrm{cf}$ | $24.00^{\prime} \mathrm{D} \times 20.00^{\prime} \mathrm{H}$ Vertical Cone/Cylinder |
| \#2 | $0.00^{\prime}$ | 251 cf | $9,048 \mathrm{cf}$ Overall -342 cf Embedded $=8,706 \mathrm{cf} \times 40.0 \%$ Voids |
|  |  | $3,734 \mathrm{cf}$ | Total Available Storage |


| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | :---: | :--- |
| \#1 | Discarded | $0.00^{\prime}$ | 2.000 in/hr Exfiltration over Surface area |

Discarded OutFlow Max=0.02 cfs @ 5.25 hrs HW=0.23' (Free Discharge)
-1=Exfiltration (Exfiltration Controls 0.02 cfs )

## Pond 2P: Drywell



Time span=5.00-48.00 hrs, dt=0.05 hrs, 861 points
Runoff by SBUH method, Split Pervious/Imperv.
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 2S: Roof

Pond 2P: Drywell

Runoff Area $=10,940$ sf $100.00 \%$ Impervious Runoff Depth $>5.05$ " $\mathrm{Tc}=0.0 \mathrm{~min} \mathrm{CN}=0 / 98$ Runoff $=0.36 \mathrm{cfs} 0.106$ af

Peak Elev=16.98' Storage=3,170 cf Inflow=0.36 cfs 0.106 af Outflow=0.02 cfs 0.074 af

Total Runoff Area $=0.251$ ac Runoff Volume $=0.106$ af Average Runoff Depth $=5.05^{\prime \prime}$ $0.00 \%$ Pervious $=0.000$ ac $100.00 \%$ Impervious $=0.251$ ac

## Summary for Subcatchment 2S: Roof

Runoff $=0.36 \mathrm{cfs} @ \quad 7.80 \mathrm{hrs}$, Volume $=\quad 0.106$ af, Depth> $5.05^{\prime \prime}$

Runoff by SBUH method, Split Pervious/Imperv., Time Span= 5.00-48.00 hrs, dt= 0.05 hrs Type IA 24-hr 100yr Rainfall=6.00"


Subcatchment 2S: Roof


Summary for Pond 2P: Drywell


Routing by Stor-Ind method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Peak Elev=16.98' @ 24.00 hrs Surf.Area= 452 sf Storage $=3,170 \mathrm{cf}$
Plug-Flow detention time= 1,057.8 min calculated for 0.074 af ( $70 \%$ of inflow)
Center-of-Mass det. time $=880.3 \mathrm{~min}(1,592.6-712.3)$

| Volume | Invert | Avail.Storage | Storage Description |
| :---: | :---: | :---: | :---: |
| \#1 | 0.00' | 3,482 cf | $24.00^{\prime} \mathrm{D} \times 20.00^{\prime} \mathrm{H}$ Vertical Cone/Cylinder <br> 9,048 cf Overall -342 cf Embedded $=8,706$ cf $\times 40.0 \%$ Voids |
| \#2 | 0.00' | 251 cf | 4.00'D x 20.00'H Vertical Cone/Cylinder Inside \#1 342 cf Overall - 4.0" Wall Thickness $=251$ cf |
|  |  | 3,734 cf | Total Available Storage |


| Device | Routing | Invert | Outlet Devices |
| :---: | :--- | :---: | :--- |
| \#1 | Discarded | $0.00^{\prime}$ | $2.000 \mathrm{in} / \mathrm{hr}$ Exfiltration over Surface area |

Discarded OutFlow Max=0.02 cfs @ 5.20 hrs HW=0.24' (Free Discharge)
-1=Exfiltration (Exfiltration Controls 0.02 cfs )
Pond 2P: Drywell


EXHIBIT M
A. Site Analysis: 500 FT Radius


500 ft radius

# CURRAN-McLEOD, INC. CONSULTING ENGINEERS <br> 6655 SW HAMPTON, SUITE 210 <br> PORTLAND, OR 97223 

May 29, 2020

Ms. Emily Meharg
City of Sandy
39250 Pioneer Blvd.
Sandy, OR 97055

## RE: CITY OF SANDY <br> SANDY HEALTH CLINIC (File \# 20-006 DR//VAR/DEV/ADJ) PRELIMINARY REVIEW

Dear Emily:
We have reviewed the preliminary submittal for the above noted development and have the following comments/ recommendations:

1. All earthwork activities shall follow the requirements of the most current edition of the Oregon Structural Specialty Code. Site grading shall not in any way impede or impound or inundate the surface drainage flow from the adjoining properties without a proper collection system. The earthwork activities shall be observed and documented under the supervision of the geotechnical Engineer.
2. The proposed driveway accesses on Pleasant Street shall be Concrete Commercial Driveway Approach constructed in conformance with the applicable City of Sandy driveway detail and meeting PROWAG requirements.
3. Where the existing driveway is removed on Pleasant Street, it shall be replaced with sidewalks and an ADA ramp shall be constructed at the intersection with Ten Eyck Road to current PROPWAG requirements.
4. Ten Eyck Road is a County road, we recommend the county requires sidewalks to be constructed along the entire site frontage to match the existing sidewalks on Hwy 26.
5. A demolition permit shall be required from the City prior to demoing the existing building.
6. We have reviewed the preliminary stormwater calculations that was provided with this submittal. The calculations were found not meeting the water quality/quantity criteria as stated in the City of Sandy Development Code (SDC) 13.18 Standards and the 2016 City of Portland Stormwater Management Manual (SWMM) Standards, that were adopted by reference into the Sandy Development Code. The water quality shall be designed based on $0.19 \mathrm{in} / \mathrm{hr}$ rate for 5 minutes time of concentration. While the water quality shall be designed for 2, 5, 10 and 25 year storm events and not 25 and 100 year storm events only. A detailed final report stamped by a licensed professional shall be resubmitted for review with the final construction plans.
7. The proposed 6 " sanitary sewer service should be adequate to serve this building, unless the City public works department determines a sanitary sewer service exists and a new one is not needed.
8. The final construction plans shall be submitted to Sandy Fire Department for review and approval to ensure that the proposed vault has adequate fire protection and also acceptable access is provided to the building.
9. The final construction plans shall verify the domestic 2 " meter size is adequate based on the meter flow and the building fixture counts found in the 2017 Oregon Plumbing Specialty Code, if larger than 2" meter size is needed shall be verified in the AWWA series 700 and the Oregon Plumbing Specialty Code.

We have no concerns about the proceedings with this project subject to the above stated comments.

Sincerely,
CURRAN-McLEOD, INC.


Hassan A. Ibrahim, P.E.
cc: Mr. Mike Walker, City of Sandy

## Clackamas County Health Clinic (File No. 20-006 DR/VAR/DEV/ADJ)

## Gary Boyles [fmboyles.sandyfire@gmail.com](mailto:fmboyles.sandyfire@gmail.com)

Wed, Jun 3, 2020 at 12:32 PM To: emeharg@ci.sandy.or.us
Cc: Don Patty [d.patty3710@gmail.com](mailto:d.patty3710@gmail.com)
Hi Emily,
The only comment I have regarding this application is that the new fire department connection (FDC) be relocated to the Mt. Hood Highway side of the proposed fire vault and to be as close as possible to the existing fire hydrant located in that area as possible.

Thank you,
Gary Boyles
Fire Marshal
Sandy Fire District No. 72
PO Box 518
17460 SE Bruns Ave.
Sandy, Oregon 97055

## Business line: 503-668-8093 <br> Cell number: 503-891-7042

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## EXHIBIT P

## REPLINGER \& ASSOCIATES LLC

TRANSPORTATION ENGINEERING

June 8, 2020

Ms. Emily Meharg
City of Sandy
39250 Pioneer Blvd.
Sandy, OR 97055

## SUBJECT: REVIEW OF TRANSPORTATION IMPACT ANALYSIS - SANDY HEALTH CLINIC

Dear Emily:
In response to your request, I have reviewed materials submitted in support of the Sandy Health Clinic in the northeast quadrant of Highway 26 and Ten Eyck Road in the east part of Sandy. The Transportation Impact Analysis (TIA), dated March 12, 2020, was prepared under the direction of John Manix, PE of PBS.

The site, currently occupied by a warehouse, is proposed to have a 9600-square foot health clinic. Access will be on Pleasant Street.

## Overall

I find the TIA addresses the city's requirements and provides an adequate basis to evaluate impacts of the proposed development.

## Comments

1. Study Area. The study addresses the appropriate intersections. It includes analyses of:

- Highway 26 at SE Ten Eyck Road;
- Ten Eyck Road at Pleasant Street.

2. Traffic Counts. The $A M$ and $P M$ peak hour traffic counts were conducted during February 2020. The engineer adjusted the traffic counts to account for seasonal variations. The engineer adjusted the February counts by 28 percent to estimate the $30^{\text {th }}$ highest hour traffic volumes. The methodology appears consistent with the procedures defined by the Oregon Department of Transportation (ODOT). The adjusted counts appear reasonable.

Ms. Emily Meharg
June 8, 2020
Page 2
3. Trip Generation. The TIA uses trip generation for a medical clinic and for a warehouse (land use code 630 and 150, respectively) from the Institute of Transportation Engineers' (ITE) Trip Generation Manual. The warehouse calculation was used to calculate traffic from the existing use. After accounting for the warehouse, the engineer calculates that the medical clinic would produce 34 net new AM peak hour trips; 30 net new PM peak hour trips; and 335 net new daily trips. The calculation of trips generated by the development appears reasonable.
4. Trip Distribution. The TIA provided information about trip distribution from the site. Among other resources, the engineer consulted with county medical staff about clients. The engineer assumed 90 percent of the traffic would travel to and from the west on US 26 and 10 percent would travel to and from the east on US 26. The trip distribution seems reasonable.
5. Traffic Growth. The TIA uses a 2.0 percent annual increase for Highway 26 based on projected volumes from the Transportation System Plan. No adjustments were made for in process developments. The future year background traffic volumes appear reasonable.
6. Analysis. Traffic volumes were calculated for the intersections cited in \#1, above. Intersection level-of-service (LOS) and the volume-to-capacity (v/c) ratio were provided. The intersection of US 26 with SE Ten Eyck Road is signalized; the intersection of Ten Eyck Road and Pleasant Street is stop-controlled. The analyses were conducted for existing 2020 conditions, 2022 background conditions, 2022 with the development, 2029 background conditions, and 2029 with the development.

The engineer calculated that the intersection of Highway 26 and Ten Eyck Road would operate at LOS B or better and a v/c ratio of 0.66 or better during the AM peak hour. For the PM peak hour, he calculated the intersection would operate at LOS C or better and a v/c ratio of 0.84 or better under all conditions. This meets ODOT's performance standard.

The engineer did not report the LOS or v/c ratio for the intersection of Ten Eyck Road with Pleasant Street. Both the east leg and west leg approaches of Pleasant Street with Ten Eyck Road were analyzed in a simulation that showed the anticipated queues and blockage time. Due to the low traffic volumes and short predicted queues on Pleasant Street, it is apparent that operations of the intersection will be good even with the proposed development. My own calculations using Synchro indicated the intersection will operate at LOS A, meeting city operational standards.

Ms. Emily Meharg
June 8, 2020
Page 3

The engineer provides a thorough discussion of queuing issues using traffic volumes for 2029. During the PM peak hour, eastbound left-turn and eastbound right-turn queues are calculated to exceed available storage both with and without the development. In addition, southbound queues on Ten Eyck Road are expected to block nearby driveways and intersections both with and without the development. The engineer notes that the addition of a southbound left-turn lane may shorten queues. He recommended tracking volumes and queues over time to assess queuing storage needs.

The engineer also evaluated the effect of a westbound right-turn lane on Highway 26 at the intersection with Ten Eyck Road. He determined the performance of the intersection is not significantly different with a turn lane and meets $\mathrm{v} / \mathrm{c}$ standards without it. He recommends against a westbound right-turn lane. He recommends retaining the existing configuration that features a slip lane.
7. Crash Information. The TIA provides information on crashes for the most recent available five-year period covering 2014 through 2018.

At the intersection of US 26 and SE Ten Eyck Road, there were eleven reported crashes. Rear-end crashes were the most common type. This is typical of signalized intersections in an urban area. The intersection has a relatively low crash rate of 0.26 crashes per million entering vehicles. The engineer concluded that no further investigation or mitigation is required. I concur.
8. Site Plan and Access. The site plan provides for a single access on Pleasant Street near the parcel's east boundary. The TIA indicates safety will be improved by the elimination of two existing driveways serving the site, including one on Ten Eyck Road. The site access is an improvement relative to existing conditions.
9. Sight Distance. The engineer analyzed sight distance at the proposed access and concludes stopping sight distance is met. He recommends maintaining 200 feet of sight distance at the access.
10. Conclusions and Recommendations. The engineer concludes that the intersections will meet ODOT and city operational standards for the study area intersections either with or without the development. He also indicates that queuing is not significantly different with or without the development, but that queue storage will be exceeded by 2029 for some movements.

He found crash rates at the intersection of Highway 26 and Ten Eyck Road to be low and did not recommend further investigation. He concluded that a right-turn lane for

Ms. Emily Meharg
June 8, 2020
Page 4

Highway 26 westbound was not warranted and recommended retaining the existing slip lane.

He concluded the reduction in accesses to the site, including the elimination of a driveway to Ten Eyck Road, would be beneficial and improve safety.

He recommended monitoring traffic volumes and queuing at the Highway 26 and Ten Eyck Road intersection and reevaluating the intersection in connection with a future Transportation System Plan update.

I concur with the engineer's conclusions.

## Conclusion and Recommendations

Based on the information provided by the applicant, I find the TIA meets city requirements. The engineer used appropriate methods and documents his procedures and conclusions.

The intersections of Highway 26 and Ten Eyck Road and Ten Eyck Road and Pleasant Street are calculated to meet ODOT and city performance standards. I do not find a need for mitigation measures to address traffic impacts of the development or to address safety issues.

To the extent that the developer may be required to implement projects or participate in projects involving facilities under the jurisdiction of ODOT, conditions of approval should be included requiring that the development comply with the requirements standards and procedures specified by ODOT. I recommend that that ODOT requirements and standards associated with frontage improvements where the development abuts Highway 26 be made conditions of approval for the development.

If you have any questions or need any further information concerning this review, please contact me at replinger-associates@comcast.net.

Sincerely,


John Replinger, PE
Principal
SandyMedicalTIS060820

## EXHIBIT Q



Oregon
Kate Brown, Governor

## Department of Transportation

Region 1 Headquarters 123 NW Flanders Street Portland, Oregon 97209
(503) 731.8200

FAX (503) 731.8259

## Updated ODOT Response

| Project Name: Sandy Health Clinic | Applicant: Steve Kelly |
| :--- | :--- |
| Jurisdiction: City of Sandy | Jurisdiction Case \#: 20-006 DR/VAR/DEV/ADJ |
| Site Address: 39831 US Hwy 26, Sandy, OR <br> 97055 | Legal Description: 02S 04E 13ad <br> Tax Lot(s): 01000 |
| State Highway: US 26 |  |

The site of this proposed land use action is adjacent to US 26. ODOT has permitting authority for this facility and an interest in ensuring that this proposed land use is compatible with its safe and efficient operation. ODOT has reviewed the Traffic Impact Analysis for the proposed medical clinic development and determined that a westbound right turn lane at the US 26/SE Ten Eyck Rd intersection is not warranted.
$\boxtimes \quad$ ODOT has determined there will be no significant impacts to state highway facilities and no additional state review is required.

Please send a copy of the Notice of Decision including conditions of approval to:
ODOT Region 1 Planning
Development Review
123 NW Flanders St
Portland, OR 97209
ODOT_R1_DevRev@odot.state.or.us

| Development Review Planner: Marah Danielson | 503.731 .8258, <br> marah.b.danielson@odot.state.or.us |
| :--- | :--- |

MEMORANDUM
TO: EMILY MEHARG, ASSOCIATE PLANNER
FROM: MIKE WALKER, PUBLIC WORKS DIRECTOR
RE: FILE 20-006 CLACKAMAS COUNTY HEALTH CLINIC
DATE: JUNE 18, 2020
The following are Public Works' comments on the above-referenced application:

## Transportation

The applicant shall improve the US 26 frontage of the site in compliance with ODOT requirements including modifying the existing slip/right-turn lane to reduce the curb radius at the corner to accommodate east-west and north-south ADA compliant ramps if required by ODOT. This may require right-of-way dedication at the NE corner of US 26 and Ten Eyck Rd.

The existing driveway approach onto Ten Eyck Rd. from the site does not meet the minimum spacing standards in 17.98 .80 (A) of the Sandy Municipal Code (SMC). The applicant shall abandon the existing driveway approach and improve the Ten Eyck Road frontage of the site including but not limited to: curbs, sidewalks, storm drainage, streetlighting, street trees per the requirements in sections 15.20 and 17.84 .30 of the SMC. This section of Ten Eyck Rd. is under the jurisdiction of Clackamas County. The applicant shall coordinate with Clackamas County DTD to determine the required section for Ten Eyck. This may include relocating the existing fire hydrant at the intersection of Ten Eyck Rd. and Pleasant Ave. to install ADA compliant access ramps. Ten Eyck Road is a minor arterial street. Both Clackamas County and City of Sandy require minimum 6 ft . wide sidewalks on arterial streets. The County standard and the proposed ultimate section for Ten Eyck Road will only permit a curb tight sidewalk in the available right-of-way.

The City recently designed a pedestrian improvement project on Ten Eyck Rd. at the subject site. The Clackamas County DTD plan review fee has been paid and the plans have been approved by DTD. The applicant will be responsible for these improvements and is welcome to use the approved planset for this work.

The applicant shall dedicate sufficient right-of-way at the SW corner of the site to accommodate the required street section and pedestrian improvements including ADAcompliant ramp(s) at the intersection with US 26. The exact dedication area shall be determined during construction plan review. The applicant shall be responsible for providing legal descriptions and sketch maps of the dedication area, dedicating the right-of-way using the City's standard documents, and pay all recording costs.

The applicant shall remove the existing west driveway approach onto Pleasant St. The existing driveway approach doesn't to meet the minimum spacing standard in 17.98.80(A) SMC or the maneuvering standard in 17.98.70(B). The applicant shall
improve the Pleasant St. frontage of the site including but not limited to: curbs, sidewalks, storm drainage, streetlighting, street trees per the requirements in sections 15.20 and 17.84 .30 of the SMC. The sidewalk shall be curb-tight, minimum 8 ft . in width with street trees specified by the City in tree wells on XX foot centers. The sidewalk shall be five feet wide separated from the curb with a five foot wide planter strip including street trees specified by the City on XX foot centers.

Street tree and landscaping placement shall conform to the sight distance recommendations in the traffic impact analysis submitted by the Applicant.

## Utilities

The site is served by the existing 16 " water main in US 26 and the existing 8 " sanitary sewer main in Pleasant St. The applicant is proposing a drywell for stormwater disposal. Typically, the soils in Sandy do not permit treatment and discharge of stormwater in this manner. Stormwater management shall conform to the requirements in the City of Portland Stormwater Management Manual and the requirements in section 13.18 and 13.20 SMC.

Utility and right-of-way improvement plans are submitted with the land use application solely for conformance with the submittal requirements in Section 17.100.60(D). Land use approval does note connote approval of public improvement plans.

Please let me know if you have any questions or need more information.

## Incompleteness Letter: 20-006 DR/VAR/DEV/ADJ

Scott Soukup [scotts@ankrommoisan.com](mailto:scotts@ankrommoisan.com)
Wed, Apr 1, 2020 at 9:21 AM
To: Emily Meharg [emeharg@ci.sandy.or.us](mailto:emeharg@ci.sandy.or.us)
Cc: Marisol Martinez [mmartinez@ci.sandy.or.us](mailto:mmartinez@ci.sandy.or.us), "Kelly O'Neill Jr." [koneill@ci.sandy.or.us](mailto:koneill@ci.sandy.or.us), "Kelly, Steve"
[SteveKel@clackamas.us](mailto:SteveKel@clackamas.us), Lori Kellow [lorik@ankrommoisan.com](mailto:lorik@ankrommoisan.com)

Hi Emily,

Here is the credit card authorization form.

For the vertical Nichiha siding, Section 17.90.110(B.3.d) allows composite-wood (concrete fiberboard, panels or shingles). The vertical ribbed Nichiha product is a panelized fiber cement product with $15 / 8$ " vertical slats and $3 / 8$ " reveals between each slat which add depth and rustic texture to the façade. The product is available in custom colors that can match any selected Miller or Sherwin Williams paint. The ribbed fiber cement is not the same as board-andbatten siding or T1-11 sheet siding.

The product can be installed either vertically or horizontally. In terms of design, the vertical siding is a darker earth tone and is the bulk of the siding. The vertical orientation was selected to distinguish it from the horizontal redwood Nichiha material pops of warm color at the bumpouts and recesses in the facade. Both siding products are Nichiha so all the siding can come from a single source manufacturer. This will make the construction process smoother and simplify the detailing. The vertical siding has been selected as the main siding, because the vertical orientation is better for cleaning and maintenance over time.
[Quoted text hidden]

Credit Card Authorization Form_signed 4.1.2020.pdf 337K





## Staff Report

Meeting Date: June 30, 2020
From Shelley Denison, Associate Planner
SUBJECT: 20-012 DCA 5G Small Cell Code Change

## Background:

File No. 20-012 DCA proposes an additional section to Title 12 (12.20) of Sandy's Municipal Code to regulate the siting and franchising of 5 G small cell "stealth" facilities. New cellular technology has developed with the addition of 5 G wireless technology. Rather than requiring cell towers, 5 G requires small cell "stealth" facilities. These facilities can be sited on existing utility poles within a city. Because of this, cities are given some degree of legal leeway to regulate the siting and franchising of small cell facilities.
The municipal code does not currently contain any regulatory language related to small cell facilities. The purpose of this proposed Chapter 12.20 is to establish reasonable and nondiscriminatory policies and procedures for the placement of small wireless facilities in the right-of-way within the City's jurisdiction, consistent with and to the extent permitted by federal and state law, in order to provide public benefit consistent with the preservation of the integrity, safe usage, and reasonable aesthetic qualities of the City rights-of-way and the City as a whole.

## Recommendation:

Staff recommends the Planning Commission forward a recommendation of approval for the proposed code addition to the City Council.

## Code Analysis:

N/A

## Budgetary Impact:

N/A

### 12.20 SMALL WIRELESS FACILITIES

### 12.20.010 - Purpose and Scope

(A) Purpose. The purpose of this Chapter is to establish reasonable and nondiscriminatory policies and procedures for the placement of small wireless facilities in the right-of-way within the City's jurisdiction, consistent with and to the extent permitted by federal and state law, in order to provide public benefit consistent with the preservation of the integrity, safe usage, and reasonable aesthetic qualities of the City rights-of-way and the City as a whole.
(B) Intent. In enacting this Chapter, the City is establishing uniform standards consistent with federal law to address the placement of small wireless facilities and associated poles in the rights-of-way, including without limitation, to manage the public right of way in order to:
(1) prevent interference with the use of streets, sidewalks, alleys, parkways and other public ways and places;
(2) prevent the creation of obstructions and other conditions that are hazardous to vehicular and pedestrian traffic;
(3) prevent interference with the facilities and operations of facilities lawfully located in rights-of-way or public property;
(4) protect against environmental damage, including damage to trees;
(5) preserve the character of the community, Historic Districts or areas with Decorative Poles; and
(6) facilitate technology advancements, such as deployment of small wireless facilities, to provide the benefits of wireless services.

### 12.20.020 - Definitions

(A) "Accessory equipment" means antenna equipment as defined in 47 C.F.R. § 1.6002(c), as may be amended or superseded, which defines the term to mean equipment, switches, wiring, cabling, power sources, shelters or cabinets associated with an antenna, located at the same fixed location as the antenna, and, when collocated on a structure, is mounted or installed at the same time as such antenna. ${ }^{1}$

[^13](B) "Antenna" means the same as defined in 47 C.F.R. § 1.6002 (b), as may be amended or superseded, which defines the term to mean an apparatus designed for the purpose of emitting radiofrequency (RF) emission, to be operated or operating from a fixed location pursuant to Federal Communication Commission authorization, for the provision of personal wireless service and any commingled information services. For purposes of this definition, the term antenna does not include an unintentional radiator, mobile station, or device authorized under 47 C.F.R. Part 15.
(C) "Antenna facility" means the same as defined in 47 C.F.R. § 1.6002(d), as may be amended or superseded, which defines the term to mean an antenna and associated accessory equipment. ${ }^{2}$
(D) "Applicable codes" means, without limitation, uniform building, fire, safety, electrical, plumbing, or mechanical codes adopted by a recognized national code organization or state or local amendments to those codes that are of general application and consistent with state and federal law.
(E) "Applicant" means any person duly authorized to submit an application as or on behalf of a wireless provider.
(F) "Application" or "applications" means a request(s) submitted by an applicant for permission to collocate small wireless facilities on an existing, modified, new or replacement structure.
(G) "City structure" means a structure in the rights of way owned, managed or operated by the City including, but not limited to streetlights, traffic signals, utility poles and other structures.
(H) "Collocate" or "collocation" means the same as defined in 47 C.F.R. § 1.6002(g), as may be amended or superseded, which defines that term to mean (1) mounting or installing an antenna facility on a preexisting structure, and/or (2) modifying a structure for the purpose of mounting or installing an antenna facility on that structure.
(I) "Day" means calendar day. For purposes of the FCC shot clock, a terminal day that falls on a holiday or weekend shall be deemed to be the next immediate business day.
(J) "Decorative pole" means a city structure that is specially designed and placed for aesthetic purposes.
(K) "Historic district" means a group of buildings, properties, or sites that are either: (1) listed in the National Register of Historic Places or formally determined eligible for listing by

[^14]the Keeper of the National Register in accordance with Section VI.D.1a.i-v of the Nationwide Programmatic Agreement codified at 47 C.F.R. Part 1, Appendix C; or, (2) a locally designated historic district.
(L) "Permissions" means those authorizations needed for deployment of Small Wireless Facilities.
(M) "Person" means an individual, corporation, limited liability company, partnership, association, trust, or other entity or organization, including the City.
(N) "Pole" means a type of structure in the rights-of-way that is or may be used in whole or in part by or for wireline communications, electric distribution, lighting, traffic control, signage, or similar function, or for collocation of small wireless facilities consistent with applicable codes; provided, such term does not include a tower, building or electric transmission structures.
(O) "Right-of-way" means the same as provided in Chapter 12.02.050.
(P) "Routine maintenance" means inspections, testing, repair, and modifications that maintain functional capacity, aesthetic and structural integrity of a small wireless facility and/or the associated pole or structure. Any work on a small wireless facility that would not require a permit (e.g., a traffic control permit, building permit, encroachment permit, etc.) qualifies as routine maintenance. As an illustration and not a limitation, routine maintenance would include, without limitation, one-for-one antenna or accessory equipment replacements but would not include technology upgrades that alter or add to the RF emissions from the antenna facility. Similarly, routine maintenance would include, without limitation, the installation of minor brackets or braces to harden an antenna facility but would not include the replacement or reinstallation of the underlying support structure.
(Q) "Small wireless facility" means a facility that meets each of the following conditions per the 47 C.F.R § 1.6002(1), as may be amended or superseded:
(1) The facilities (i) are mounted on structures 50 feet or less in height including the antennas, or (ii) are mounted on structures no more than 10 percent taller than other adjacent structures, or (iii) do not extend existing structures on which they are located to a height of more than 50 feet or by more than 10 percent, whichever is greater; and
(2) Each antenna associated with the deployment, excluding associated accessory equipment, is no more than three cubic feet in volume; and
(3) All other wireless equipment associated with the structure, including wireless equipment associated with the antenna and any pre-existing associated equipment on the structure, is no more than 28 cubic feet in volume; and
(4) The facilities do not result in human exposure to radio frequency in excess of the applicable safety standards specified in 47 C.F.R. § 1.1307(b).
(P) "Structure" means the same as defined in 47 C.F.R. § 1.6002(m), as may be amended or superseded, which defines that term as "a pole, tower, or base station, or other building, whether or not it has an existing antenna facility, that is used or to be used for the provision of personal wireless service (whether on its own or comingled with other types of service)."
(Q) "Technically feasible" means that the proposed placement, location or design for a small wireless facility can be implemented without a material reduction in the intended service objective of the small wireless facility.
(R) "Wireless Provider" means either (1) any person who provides "personal wireless services", as defined in 47 U.S.C. § 332(c)(7)(C)(i), as may be amended or superseded; or (2) any person, including a person authorized to provide communications service in the state, that builds or installs wireless communication transmission equipment, wireless facilities, but does not provide personal wireless services.

### 12.20.030 - Applications and Fees

(A) Application Required. Except as otherwise provided in this Chapter, no person shall place any small wireless facility in the right-of-way without first filing an application for the facility and obtaining all permits necessary for the installation.
(B) Application Requirements.

An application filed pursuant to this Chapter shall be on forms provided by the City and, at a minimum, contain the following:
(1) The applicant's name, address, telephone number, and e-mail address;
(2) The names, addresses, telephone numbers, and e-mail addresses of all duly authorized representatives and consultants, if any, acting on behalf of the applicant with respect to the filing of the application;
(3) A general description of the proposed small wireless facility and associated pole, if applicable. The scope and detail of such description shall be appropriate to the
nature and character of the work to be performed, with special emphasis on those matters likely to be affected or impacted by the physical work proposed;
(4) Site plans and engineering drawings to scale that identify the proposed small wireless facility;
(5) A statement that the small wireless facility shall comply with all applicable codes, regulations and standards, including a certification in a form acceptable to the City that the small wireless facility will comply with applicable FCC regulations for human exposure to RF emissions.
(6) The applicant shall not be required to provide more information to obtain a small wireless facility permit than is required of other entities who install small wireless facilities in the rights-of-way.
(C) Routine Maintenance. Routine maintenance in connection with an existing small wireless facility shall be permitted, subject only to any traffic control, encroachment or other regulatory authorizations as may be required for the specific scope of work. Notwithstanding anything to the contrary in this chapter, the applicant and/or permittee conducting routine maintenance shall not be relieved of its obligations to comply with all generally applicable health and safety regulations.
(D) Application Fees. Application fees in accordance with applicable state and federal law shall be set by resolution of the City Council.

### 12.20.040 - Decisions; Notice of Decision

(A) Findings for Denial. The City must process all applications on a nondiscriminatory basis and may deny an application subject to this Chapter if the proposed small wireless facility:
(1) Materially and demonstrably interferes with the safe operation of traffic control equipment;
(2) Materially and demonstrably interferes with sight lines or clear zones for transportation or pedestrians;
(3) Materially fails to comply with the Americans with Disabilities Act or similar federal, state, or local laws, standards and regulations regarding pedestrian access or movement;
(4) Fails to comply with applicable codes, standards and regulations, including the City's design standards; or
(5) Fails to comply with the provisions in this Chapter.
(B) Time for Final Action; Notice of Final Action. The City shall act on an application within the applicable shot clock and advise the applicant in writing of its final action. If the final action is to deny the application, ${ }^{3}$ The written notice shall state the reasons for denial, with reference to specific code provisions, ordinance, application instructions or otherwise publicly-stated procedures on which the denial was based, and send the notice to the applicant within five (5) days after the City denies the application or before the applicable shot clock expires, whichever occurs first.
(C) Batched Applications. Applicants may submit batched applications as provided under applicable law, and the City shall act on such applications as provided in 12.20.040 (B) and consistent with applicable law. A batched application that includes only collocations on existing structures shall be subject to a 60 -day timeline. A batched application that includes only new support structures shall be subject to a 90 -day timeline. A batched application that includes both collocations and new support structures shall be subject to a 90-day timeline

### 12.20.050 - Special Design Requirements

(A) Decorative Poles. Subject to the City's discretion, a wireless provider may be permitted to collocate on or replace a decorative pole when necessary to collocate a small wireless facility; provided that any such replacement pole shall, to the extent feasible, replicate the design of the pole being replaced.
(B) Underground District. [NOTE: ACCORDING TO THE FCC ORDER, UNDERGROUNDING REQUIREMENTS ARE SUBJECT TO THE SAME CRITERIA AS OTHER AESTHETIC STANDARDS. AESTHETIC REQUIREMENTS (INCLUDING UNDERGROUNDING) MUST BE (1) REASONABLE; (2) NO MORE BURDENSOME THAN REQUIREMENTS IMPOSED ON OTHER INFRASTRUCTURE DEPLOYMENTS; (3) OBJECTIVE; AND (4) PUBLISHED IN ADVANCE.

IN THIS REGARD, A REQUIREMENT THAT ALL ELEMENTS OF ALL WIRELESS FACILITIES BE DEPLOYED UNDERGROUND WOULD AMOUNT TO AN EFFECTIVE PROHIBITION GIVEN THE PROPAGATION CHARACTERISTICS OF WIRELESS SIGNALS EMITTED FROM ANTENNAS. CITIES ARE ENCOURAGED TO REVIEW CURRENT UNDERGROUNDING REQUIREMENTS AND WORK WITH THEIR ATTORNEYS/ROW SPECIALISTS TO MAKE SURE THOSE REQUIREMENTS ARE NOT IN CONFLICT WITH THE FCC ORDER.]
(C) Historic District. Small wireless facilities or poles to support collocation of small wireless facilities located in Historic Districts shall be designed to have a substantially

[^15]similar appearance, including coloring and design elements, if technically feasible, of other poles in the rights-of-way within 500 feet of the proposed installation.

### 12.20.060 - Effect of Construction/Work Permit

(A) Authority Granted. A permit from the City authorizes an applicant to undertake only certain activities in accordance with this Chapter and does not create a property right or grant authority to the applicant to impinge upon the rights of others who may already have an interest in the rights-of-way.
(B) Permit Duration.
(1) The build-out period for a permit for construction granted pursuant to this Section shall be valid for a period of one year after issuance.
(2) The permit authorizing the use shall be coterminous with the applicable right-ofway license, franchise or other agreement granting the applicant access to the rights-of-way.
(3) The installed facility is subject to applicable relocation requirements, termination for material non-compliance after notice and a reasonable opportunity to cure, and an applicant's right to terminate a permit at any time.

### 12.20.070 - Removal, Relocation or Modification of Small Wireless Facility in the ROW

(A) Notice. The City shall provide the permittee reasonable advance written notice whenever the City has determined that such removal, relocation, change or alteration, is reasonably necessary for the construction, repair, maintenance, or installation of any City improvement in or upon, or the operations of the City in or upon, the rights-of-way (collectively, "City work"). The City shall specify a reasonable time for such removal, relocation, change or alteration in its notice, taking into account the nature and scope of the work involved and the urgency of the City's need for such work to be performed. Within the time specified in the written notice from the City, the permittee shall, at its own expense, protect, support, temporarily or permanently disconnect, remove, relocate, change or alter the position of any small wireless facilities within the rights-of-way in order to accommodate the City work.
(B) Emergency Removal or Relocation of Facilities. The City retains the right and privilege to cut or move any small wireless facility located within the rights-of-way of the City in the event of an emergency, as the City may determine to be necessary, appropriate or useful in response to any imminent danger to public health, safety, or property. If practicable under the circumstances, the City shall notify the permittee and provide the permittee an opportunity to move its own facilities prior to cutting or removing a facility and shall notify the permittee promptly when practicable after cutting or removing a small wireless facility.
(C) Abandonment of Facilities. Within 90 days after a small wireless facility is abandoned, the permittee shall completely remove the small wireless facility and all related improvements and shall restore all affected areas to a condition compliant with all applicable codes. In the event that the permittee does not complete the obligations under this condition, or cause them to be completed, within said 90-day period, the City shall have the right (but not the obligation) to perform such removal and restoration with or without notice, and the permittee shall be liable for all costs and expenses incurred by the City in connection with such removal and/or restoration activities.
(D) Damage and Repair. The City may require a permittee to repair all damage to the rights-of-way directly caused by the activities of the permittee or third parties acting under the permittee's direction and restore the rights-of-way to its the condition that existed before the damage occurred. All such repair work shall be performed in accordance with applicable laws and to the City Public Works Director's satisfaction. If the permittee fails to make the repairs within 30 days after written notice, the City shall have the right (but not the obligation) to affect those repairs, and the permittee shall be liable for all costs and expenses incurred by the City in connection with such repairs.

### 12.20.080 - Collocation on City Structures in the ROW

(A) Collocation on City Structures. Small wireless facilities may be collocated on city structures in the rights-of-way pursuant to this Chapter. No person will be permitted an exclusive arrangement or an arrangement which excludes otherwise qualified applicants to attach to city structures in the rights-of-way. A person who purchases or otherwise acquires a City structure is subject to the requirements of this section.
(B) Make-Ready. The rates, fees, terms and conditions for the make-ready work to collocate a small wireless facility on a pole owned or controlled by the City must be nondiscriminatory, competitively neutral and be subject to the following:
(1) The City or any person owning, managing, or controlling the poles owned by the City will provide a good faith estimate for any make-ready work reasonably necessary to make a specific city pole suitable for attachment of the requested small wireless facility, including pole replacement if necessary, within 60 days after receipt of a completed request. Make-ready work including any pole replacement shall be completed within 60 days of written acceptance of the good faith estimate by the applicant.
(2) The City or any person owning, managing, or controlling the poles owned by the city shall not require more make-ready work than required to meet applicable codes or may be reasonably necessary to avoid interference with other attachments on the pole. Fees for make-ready work shall not include costs related
to pre-existing or prior damage and non-compliance, unless such fees are necessary to accommodate the proposed attachment on the pole. Fees for makeready work including any pole replacement shall not exceed actual and direct costs, or the amount charged to others for similar work and shall not include any contingency based consultant fees or expenses of any kind.

### 12.20.090 - Rates for ROW and Collocation on City Structures in the ROW

(A) The recurring rate for use of the ROW and attachment of small wireless facilities to a City structure in the ROW shall be subject to the following requirements:
(1) Annual Rate. A person authorized to place small wireless facilities and any related pole in the rights-of-way will pay to the City compensation for use of the rights-of-way and collocation on city structures in the ROW a rate in accordance with applicable state and federal law and set by resolution of the City Council.
(2) Cease Payment. A person authorized to use the ROW and/or city structures for a small wireless facility may remove its facilities at any time from the rights-of-way and city structures in the ROW with the required permits and cease paying the City compensation as of the date of the complete removal of the facilities and restoration of the site to the condition that existed prior to the deployment.


[^0]:    Targetti USA - A Targetti Group Company - 750-A W. 17th St. Costa Mesa, CA 92627 - Phone (714) 513-1991 - Email: targettiusa@targetticom - www.Targetti.us - 12.16 .19 - Page 4 of 6

[^1]:    415 W 6TH STREET, SUITE 601 • VANCOUVER, WA 98660 • 360.695.3488 MAIN • 866.727.0140 FAX • PBSUSA.COM

[^2]:    ${ }^{1}$ Noting that Highway 26 is a primary route connecting between the Portland metro area and the Mount Hood winter recreation areas, the recreational summer-winter seasonal trend was also evaluated. Averaging the commuter and recreational summer-winter trends, while not an approved blend per ODOT APMv2, also yields a $28 \%$ SAF.

[^3]:    71524.000

[^4]:    << Over $30 \%$, so too high to be used
    $\ll$ This is an approved blend of trends, so this one is used.
    << Over 30\%, so too high to be used
    << This is not an approved blend of trends, but it validates the number used above

[^5]:    Sandy Health Clinic - PBS Project 71524.000
    2020 Existing Conditions - Weekday PM Peak Hour

[^6]:    Sandy Health Clinic - PBS Project 71524.000
    2022 Without Project Conditions - Weekday PM Peak Hour

[^7]:    Sandy Health Clinic - PBS Project 71524.000

[^8]:    Sandy Health Clinic - PBS Project 71524.000
    2029 Without Project Conditions - Weekday AM Peak Hour

[^9]:    Sandy Health Clinic - PBS Project 71524.000
    2029 Without Project Conditions - Weekday PM Peak Hour

[^10]:    Sandy Health Clinic - PBS Project 71524.000
    2029 With Project Trips Conditions - Weekday PM Peak Hour

[^11]:    Sandy Health Clinic - PBS Project 71524.000
    Synchro 10 Report - by PBS Engineering and Environmental
    2029 With Project Conditions - Weekday AM Peak Hour + With WB Right-Turn Lane
    Page 1

[^12]:    Sandy Health Clinic - PBS Project 71524.000
    Synchro 10 Report - by PBS Engineering and Environmental

[^13]:    ${ }^{1}$ The FCC uses the term "antenna equipment" to mean the non-antenna accessory equipment associated with a small cell. The City finds this term confusing because using "antenna equipment" to describe equipment that is not antenna appears contrary to term on its face. In fact, the FCC's full definition of "antenna equipment" refers to 47 C.F.R. § $1320(\mathrm{~d})$, which includes a definition for "antenna" that is essentially a combination of "antenna" and "antenna equipment" as defined in 47 C.F.R. $\S 1.6002$. The reference to $\S 1320(\mathrm{~d})$ has been omitted from the definition and the City uses "accessory equipment" in this Chapter 12.20 to avoid confusion.

[^14]:    ${ }^{2}$ As written, the definition uses the term "accessory equipment" in-lieu of "antenna equipment" for the reasons stated in the previous footnote.

[^15]:    ${ }^{3}$ Note that a "final action" for the purposes of federal regulations is not the same as a "final decision" for the purposes of Oregon state law.

