

Exhibit E

Traffic Impact Study

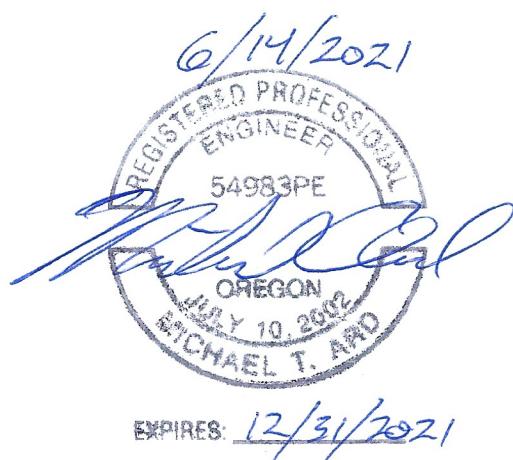


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DEER MEADOWS SUBDIVISION TRAFFIC IMPACT STUDY

SANDY, OREGON



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DATE:

June 14, 2021



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EXECUTIVE SUMMARY

1. A property located on the south side of US Highway 26 opposite SE Vista Loop Drive in Sandy, Oregon is proposed for a 32-lot subdivision which will support up to 32 single-family homes and 120 apartment units. The site will take access via extensions of Dubarko Road and Fawn Street into the site.
2. Upon completion of residential development within the R-1, R-2, and C-3 zones, the subject property is projected to generate up to 79 site trips during the morning peak hour, 99 trips during the evening peak hour, and 1,180 daily site trips.
3. With conversion to all-way stop control, the intersection of Highway 211 at Dubarko Road is projected to operate acceptably under year 2023 traffic conditions. All other study intersections are projected to operate acceptably through year 2023 either with or without the addition of site trips from the proposed development. No other operational mitigations are necessary or recommended in conjunction with the proposed subdivision.
4. Based on the crash data, the majority of the study intersections are currently operating acceptably with respect to safety. The intersection of Highway 211 at Dubarko Road has a high historical crash rate which recent safety improvements have not significantly improved. This intersection meets all-way stop control warrants based on crash history, and conversion to all-way stop control would be expected to reduce the frequency and severity of right-angle and turning-movement collisions. It is therefore recommended that all-way stop control be installed at the intersection of Highway 211 and Dubarko Road. No other safety improvements are recommended.
5. Based on the warrant analysis, no new turn lanes or traffic signals are recommended in conjunction with the proposed subdivision.



PROJECT DESCRIPTION & LOCATION

INTRODUCTION

A property located on the south side of US Highway 26 opposite SE Vista Loop Drive is proposed for development with 32 lots across R-1, R-2, and C-3 zoning. The site can support up to 30 single-family homes, 2 duplex units, and 120 apartment units. The portion of the site zoned C-3 is expected to ultimately include some form of commercial development; however, the nature of this future use has not yet been determined. Accordingly, a future traffic study will be required as part of the design review application for the future commercial site use. The site will take access via extensions of Dubarko Road and Fawn Street into the site. Dubarko Road will be extended to intersect a new north/south collector street within the site, which will stub to the south side of the property.

This report addresses the impacts of the proposed development on the surrounding street system. An operational and safety analysis was conducted for the proposed site access as well as the intersections of:

- Highway 26 at SE Ten Eyck Road;
- Highway 26 at SE Langensand Road;
- Highway 211 at Dubarko Road; and
- Dubarko Road at SE Langensand Road.

The purpose of this analysis is to determine whether the surrounding transportation system is capable of safely and efficiently supporting the proposed use and to identify any necessary improvements and mitigations.

SITE LOCATION AND STUDY AREA DESCRIPTION

The project site has an area of approximately 16 acres, which is currently undeveloped. The property is surrounded by a mixture of residential development, agricultural uses and undeveloped forested land.

The proposed development will include an extension of Dubarko Road into the site to intersect a new north/south collector roadway. The proposed development will connect to the existing street system via extensions of Dubarko Road and Fawn Street into the project site.

US Highway 26 (Mt. Hood Highway) is classified by the Oregon Department of Transportation as a Statewide Highway and a Freight Route. It has two through lanes in each direction and added turn lanes at intersections. Between SE Langensand Road and SE Vista Loop Drive it has a center two-way left-turn lane. It has a posted speed limit of 25 mph at SE Ten Eyck Road, 40 mph at SE Langensand Road, and 55 mph at SE Vista Loop Drive. West of SE Ten Eyck Road the highway divides into a couplet, with westbound traffic traveling on Proctor Boulevard and eastbound traffic traveling on Pioneer Boulevard.



SE Ten Eyck Road has one through lane in each direction and is striped to prohibit passing in the site vicinity. It has a basic rule speed limit of 55 mph and is classified by the City of Sandy as a Minor Arterial.

SE Langensand Road is also classified by the City of Sandy as a Minor Arterial. It has a two-lane cross-section with one through lane in each direction and a posted speed limit of 25 mph. Partial sidewalks are in place on both sides of the roadway, and on-street parking is available where sufficient paved width is provided.

Oregon Highway 211 (Eagle Creek Sandy Highway) is classified by the Oregon Department of Transportation as a District Highway. It has a two-lane cross-section with one through lane in each direction and added turn lanes at major intersections. It has a posted speed limit of 45 mph in the vicinity of Dubarko Road.

Dubarko Road is classified by the City of Sandy as a Minor Arterial. It generally has a two-lane cross-section with some added turn lanes at major intersections and bike lanes on each side of the roadway. Partial sidewalks are in place on each side of the roadway adjacent to developed properties. It has a posted residential speed limit of 25 mph.



EXISTING CONDITIONS

The intersection of US Highway 26 at SE Ten Eyck Road/Wolf Drive is controlled by a traffic signal. The northbound and southbound approaches each have a single, shared lane for all turning movements. The westbound approach has a left-turn lane, two through lanes, and a short right-turn pocket. The eastbound approach has a left-turn lane, a dedicated through lane and a shared through/right lane. The northbound and southbound approaches operate with concurrent signal phasing. Protected phasing is provided for the eastbound and westbound left-turn movements. Bike lanes are provided along Highway 26 to the right of the through lanes.

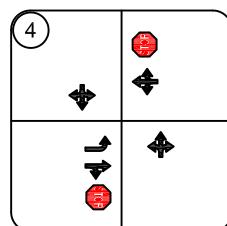
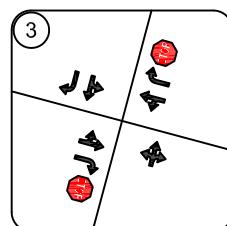
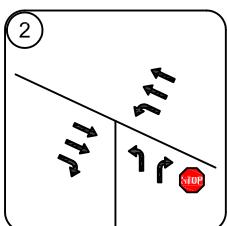
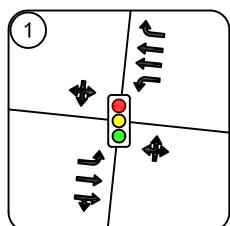
The intersection of US Highway 26 at SE Langensand Road is a T- intersection controlled by a stop sign on the northbound Langensand Road approach. Through traffic traveling along Highway 26 does not stop. The northbound approach has a left-turn lane and a right-turn lane. The eastbound approach has two through lanes and a right-turn lane. The westbound approach has a left-turn lane and two through lanes. Bike lanes are provided along Highway 26 to the right of the through lanes.

The intersection of Oregon Highway 211 at Dubarko Road is a four-way intersection controlled by stop signs on the eastbound and westbound Dubarko Road approaches. The southbound, eastbound and westbound approaches each have a shared through/left lane, a bike lane, and a dedicated right-turn lane. The northbound approach has a single, shared lane for all motorized turning movements and a bike lane.

The intersection of Dubarko Road at SE Langensand Road is a four-way intersection currently controlled by stop signs on the eastbound and westbound Dubarko Road approaches. Through traffic traveling along SE Langensand Road does not stop. The northbound and southbound approaches each have a single, shared lane for all turning movements. The westbound approach has a single, shared lane for all motor vehicle turning movements and a bike lane. The eastbound approach has a left-turn lane, a shared through/right lane and a bike lane.

A vicinity map displaying the project site, vicinity streets, and the study intersections including lane configurations is provided in Figure 1 on page 7.

FIGURE 1



LEGEND

- Study Intersection
- Traffic Signal
- Stop Sign



VICINITY MAP
Study Intersections
Lane Configurations and Traffic Control

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TRAFFIC COUNT DATA

Traffic counts were conducted at the study intersections on Tuesday March 19th, 2019 from 4:00 to 6:00 PM and on Wednesday March 20th, 2019 from 7:00 to 9:00 AM. Data was used from the highest-volume hour during each analysis period. This historical data was used since it predates the impacts of the current COVID-19 pandemic, allowing conservative projections of future peak-hour traffic conditions once conditions return to normal.

Since the count data was collected during a non-peak period of the year, the observed traffic volumes were adjusted to account for seasonal traffic variations to represent the 30th-highest hour design volumes.

US Highway 26 serves local and commuter traffic as well as trips to and from Mt. Hood and beyond. These trip types would be expected to exhibit very different seasonal variations in travel demands over the course of the year, since local and commuter traffic volumes are relatively stable regardless of season, while travel volumes to and from Mt. Hood vary significantly based on the season.

To determine the portion of traffic attributable to each of the two primary travel types, data from ODOT's 2017 Highway Volume Tables was utilized. Specifically, the data used was collected at ODOT's Automatic Count Data station 03-006, located 0.30 miles east of Camp Creek Road in Rhododendron, Oregon. This site is located on Highway 26 approximately 21 miles east of SE Vista Loop Drive. Although the distance to the ATR station means the data cannot be used directly, the ATR data provides useful information regarding the variation in traffic volumes traveling to Mt. Hood and beyond during the time of the count data collection as well as during the peak season of the year. Accordingly, this data allows determination of the likely portion of highway traffic that falls into each of the two seasonal variation categories ("commuter" and "recreational summer/winter"), as well as providing information regarding the most appropriate seasonal adjustment factor for the recreational summer/winter traffic.

Based on the data, 6,763 vehicles per day (approximately 676 per hour during the peak hour) travel along Highway 26 to and from Mt. Hood at the Rhododendron permanent count station location during the month of March. This volume represents 45.3 percent of the through traffic volumes measured on Highway 26 east of SE Vista Loop Drive. Accordingly, it is expected that no more than 45.3 percent of the trips traveling along Highway 26 in the project vicinity are traveling to and from destinations beyond the Rhododendron count station. Since the remaining 54.7 percent of through traffic volumes on the Highway 26 at the study intersections never reach Mt. Hood, it was assumed that these traffic volumes represent more typical commuter and local trips.

The ODOT data also showed that 11,738 vehicles were measured per day (approximately 1174 per hour during the peak hour) during the peak-season month of August at the ATR station near Rhododendron. This indicates that the seasonal recreational traffic volumes along the Highway 26 corridor increased by no more than 4,975 vehicles per day (11,738 vehicles per day in August - 6,763 vehicles per day in March). This equates to roughly 498 additional vehicles per hour during the peak hour of the peak recreational season. It is expected that the increased recreational traffic flows will be somewhat directional, with approximately 55% traveling westbound during the evening peak hour.



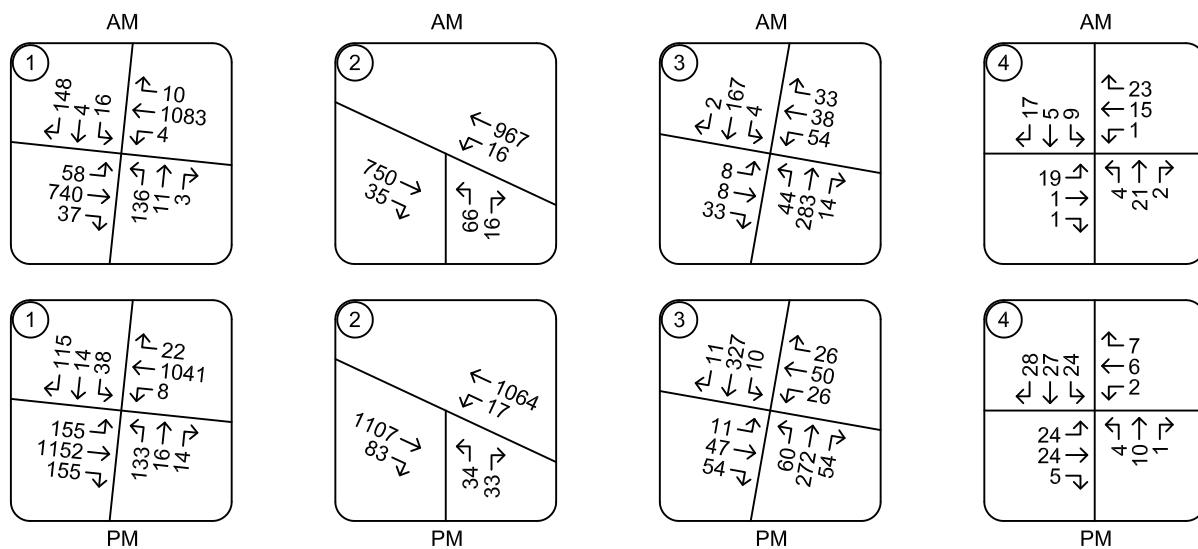
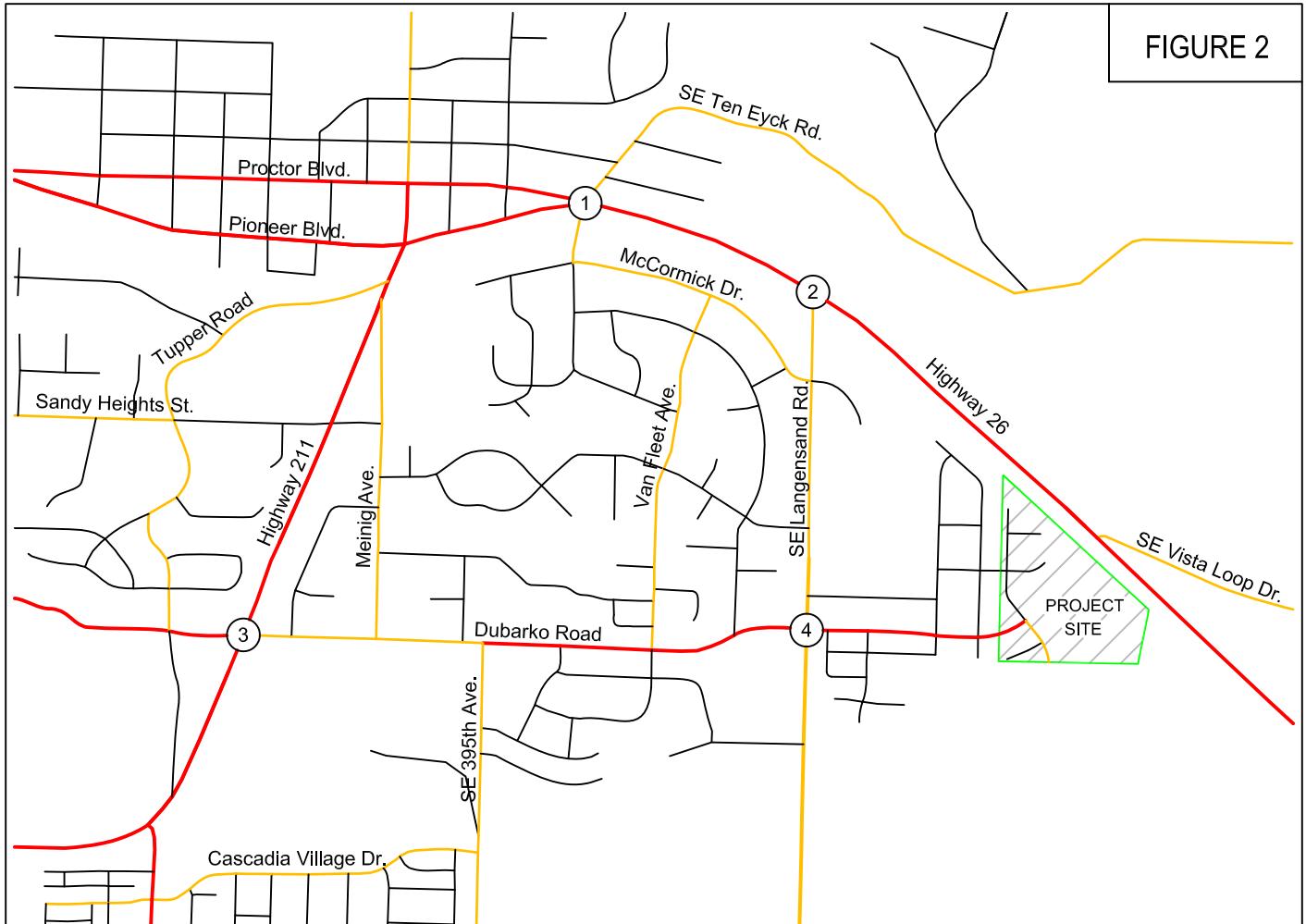
In order to seasonally adjust the local and commuter traffic volumes, the through traffic volumes were reduced by the amount of the assumed seasonal traffic (676 vehicles per hour during the evening peak hour), and a seasonal adjustment of 1.08 was applied to the remaining local and commuter traffic volumes. Following this adjustment, the 676 March recreational trips and the 498 peak-season through trips were added to determine the total peak-season traffic volumes. These calculated through traffic volumes represent the anticipated traffic levels for the intersections along Highway 26 during the 30th-highest hour in August. The morning peak hour traffic volumes along the highway were then increased by the same overall percentage as the evening peak hour volumes.

The observed traffic volumes along Highway 211 also had a seasonal adjustment of 1.08 applied to represent peak-season traffic volumes.

Following application of the seasonal adjustments, two years of growth was added to the year 2019 traffic count data to represent the expected year 2021 seasonal peak traffic conditions absent the impacts of the current COVID-19 pandemic. Based on data from ODOT's Future Volume Tables, the growth rate for traffic volumes on Highway 26 in the site vicinity was calculated to be 1.96 percent per year. The growth rate for traffic volumes on Highway 211 was calculated to be 3.13 percent per year. These growth rates were applied to the through traffic volumes on the highways. All other turning movements had a growth factor of 2 percent per year applied. The respective growth rates were applied over a period of two years to generate the year 2021 seasonal peak traffic volumes.

Figure 2 on page 10 shows the existing year 2021 30th-highest hour traffic volumes for the morning and evening peak hours at the study intersections.

FIGURE 2



TRAFFIC VOLUMES
2021 Existing 30th-Highest Hour (August) Conditions
Morning and Evening Peak Hours

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OPERATIONAL ANALYSIS

An operational analysis was conducted for the study intersections using Synchro 10 software, with outputs calculated based on the *HIGHWAY CAPACITY MANUAL, 6th Edition*. The analysis was conducted for the weekday morning and evening peak hours.

The purpose of the existing conditions analysis is to establish how the study area intersections operate currently and allow for calibration of the operational analysis if required.

The results of the operational analysis are reported based on delay, Level of Service (LOS), and volume-to-capacity ratio (v/c). Delays are reported in seconds. Level of service is reported as a letter grade and can range from A to F, with level of service A representing nearly free-flow conditions and level of service F representing high delays and severe congestion. A report of level of service D generally indicates moderately high but tolerable delays, and typically occurs prior to reaching intersection capacity. For unsignalized intersections, the v/c represents the portion of the available intersection capacity that is being utilized on the worst intersection approach. For signalized intersections, it indicates the portion of the overall intersection's capacity that is being used. A v/c ratio of 1.0 would indicate that the intersection is operating at capacity.

The Oregon Department of Transportation requires that the signalized intersection of Highway 26 at SE Ten Eyck Road operate with a v/c ratio of 0.85 or less during the peak hours. The intersection of Highway 26 at SE Langensand Road is required to operate with a v/c ratio of 0.80 or less on the major-street approaches and a v/c ratio of 0.90 or less on the minor-street approaches.

Intersections operating under the jurisdiction of the City of Sandy are required to operate at level of service D or better. This operational standard applies to the intersections of Dubarko Road at Langensand Road and Highway 211 at Dubarko Road.

A summary of the existing conditions operational analysis is provided in Table 1 on the following page. For the unsignalized intersections the reported delays and levels-of-service represent the approach lane which experiences the highest delays. The reported v/c ratios represent the highest ratio for the major-street and minor-street movements. For the signalized intersection of Highway 26 at SE Ten Eyck Road, the reported delays, levels-of-service and v/c ratios represent the operation of the overall intersection.

Based on the analysis, the study intersections are currently operating acceptably per the respective ODOT and City of Sandy standards. Detailed capacity analysis worksheets are provided in the technical appendix.



Table 1 - Operational Analysis Summary: Year 2021 30th-Highest Hour Conditions

Intersection	AM Peak Hour			PM Peak Hour		
	Delay	LOS	v/c*	Delay	LOS	v/c*
Highway 26 at Ten Eyck Road	24.0	C	0.66	27.3	C	0.71
Highway 26 at Langensand Road	56.1	F	0.29 / 0.51	96.7	F	0.36 / 0.50
Highway 211 at Dubarko Road	18.9	C	0.22 / 0.28	27.0	D	0.23 / 0.33
Dubarko Road at Langensand Road	9.4	A	0.05	9.8	A	0.04

*(major street v/c) / (minor-street v/c) is shown for unsignalized ODOT intersections.



SITE TRIPS

Proposed Development

The proposed subdivision will support development of 32 single-family homes as well as up to 120 apartment units. Although some commercial development is expected to occur within the C-3 zoned portion of the property in the longer-range future, a separate design review application and analysis will be required for future commercial development. To estimate the number of trips that will be generated by the potential residential development within the proposed subdivision, trip rates from the *TRIP GENERATION MANUAL, 10th EDITION* were used. Data from land-use code 210, *Single-Family Detached Housing*, and 220, *Multi-Family Housing*, were used. The trip estimates are based on the number of dwelling units.

A summary of the trip generation calculations is provided in Table 2 below. Detailed trip generation worksheets are also included in the technical appendix.

Table 2 - Proposed Development Trip Generation Summary

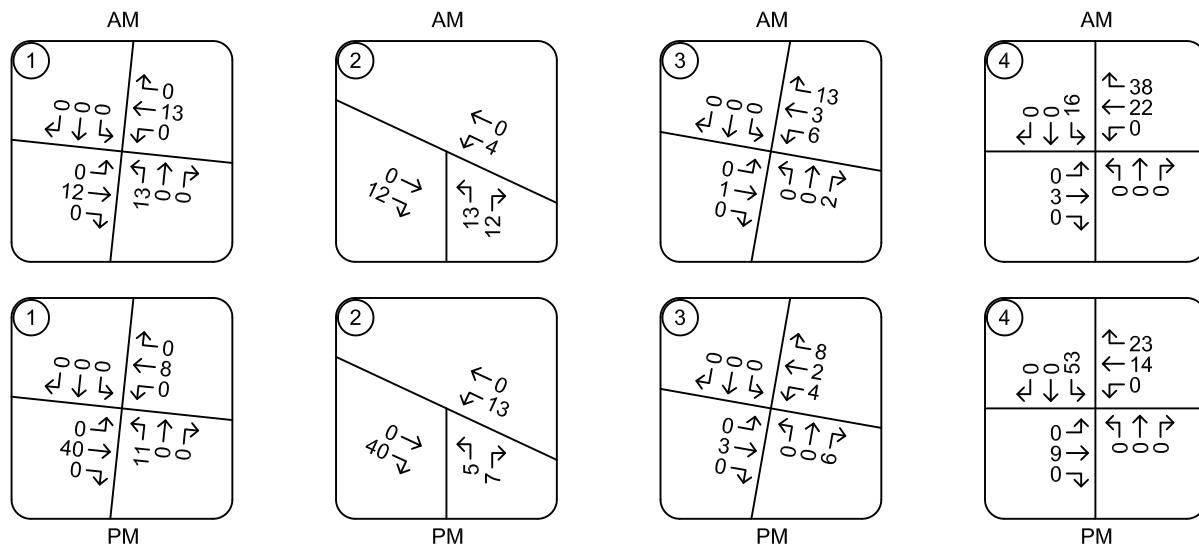
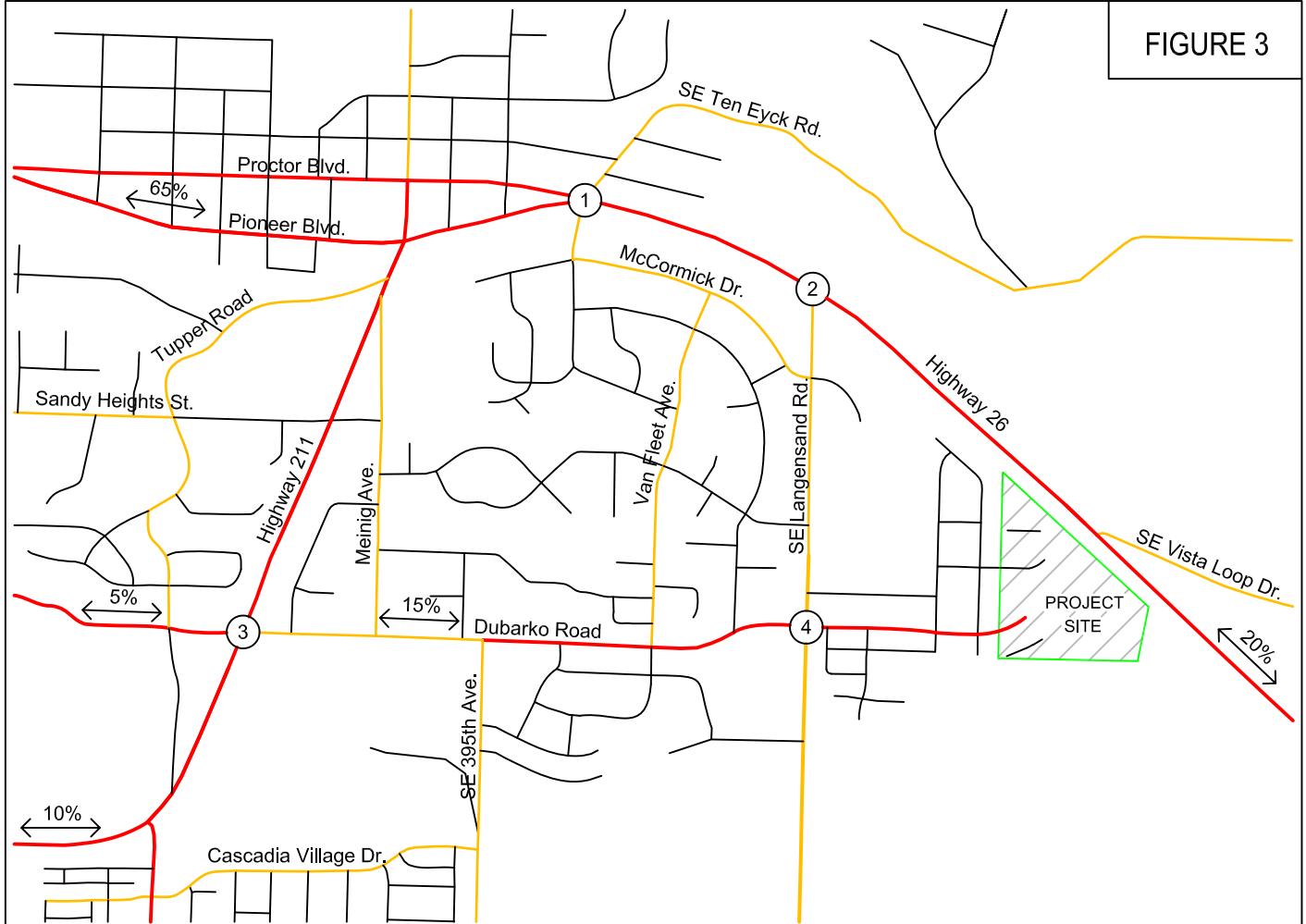
	AM Peak Hour			PM Peak Hour			Daily Total
	In	Out	Total	In	Out	Total	
32 Single-Family Homes	6	18	24	20	12	32	302
120 Multi-Family Dwelling Units	13	42	55	42	25	67	878
Total Site Trips	19	60	79	62	37	99	1,180

TRIP DISTRIBUTION

The directional distribution of site trips to and from the project site was estimated based the existing travel patterns in the site vicinity, as well as the locations of likely trip destinations and major transportation routes. Overall, 65 percent of the anticipated site trips are projected to travel to and from the northwest on Highway 26, 20 percent are projected to travel to and from the southeast on Highway 26, and the remaining 15 percent of site trips are projected to travel to and from the west on Dubarko Road.

The trip distribution percentages and trip assignment for residential development within the proposed subdivision are shown in Figure 3 on page 14.

FIGURE 3



TRAFFIC VOLUMES
Proposed Development - Primary Site Trips
Morning and Evening Peak Hours

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FUTURE CONDITIONS ANALYSIS

BACKGROUND VOLUMES

In order to determine the expected impact of site trips on the study area intersections, it is necessary to compare traffic conditions both with and without the addition of the projected traffic from the proposed development. This comparison is made for future traffic conditions at the time of project completion. It is anticipated that the proposed use will be completed and occupied within two years. Accordingly, the analysis was conducted for year 2023 traffic conditions.

Prior to adding the projected site trips to the study intersections, the existing traffic volumes were adjusted to account for background traffic growth over time. Based on data from ODOT's Future Volume Tables, the growth rate for traffic volumes on Highway 26 in the site vicinity was calculated to be 1.96 percent per year (linear). The growth rate for traffic volumes on Highway 211 was calculated to be 3.13 percent per year (linear). These growth rates were applied to the through traffic volumes on the highways. All other turning movements had a growth factor of 2 percent per year (exponential) applied.

In addition to the background growth, future site trips associated with other anticipated developments within the City of Sandy were added to the background traffic volumes. These projects included the Clackamas County Health Clinic, Mt. Hood Senior Living, The Pad, The Views, Shaylee Meadows, Mt. View Ridge, Marshall Ridge, Jacoby Heights, Trimble PD, and Bornstedt Views. The projected site trips for these residential developments are shown in Figure 6 in the attached technical appendix.

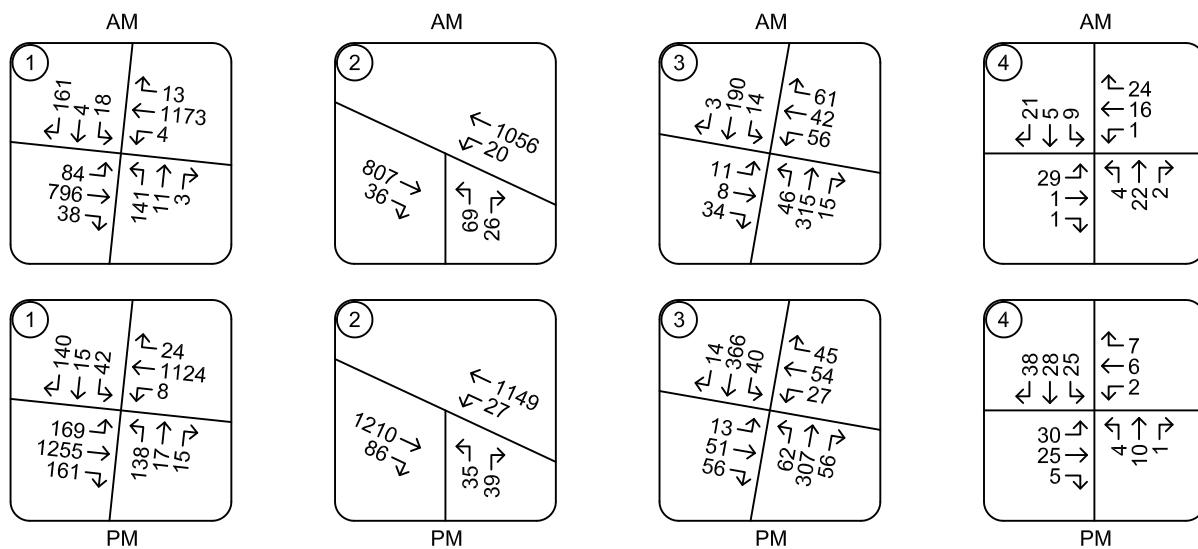
Figure 4 on page 16 shows the projected year 2023 background traffic volumes at the study intersections during the morning and evening peak hours.

BACKGROUND VOLUMES PLUS SITE TRIPS

Peak hour trips calculated to be generated by the proposed development were added to the projected year 2023 background traffic volumes to obtain the year 2023 total traffic volumes following completion of the proposed residential development.

Figure 5 on page 17 shows the projected year 2023 peak hour volumes including background growth, and site trips from the proposed development for the morning and evening peak hours.

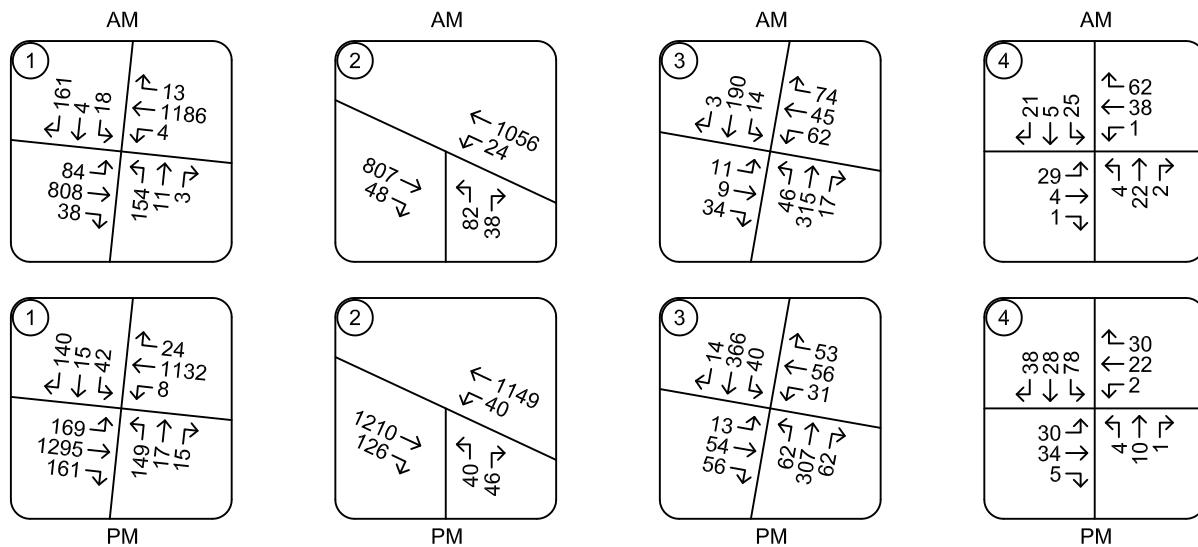
FIGURE 4



TRAFFIC VOLUMES
2023 Background Conditions
Morning and Evening Peak Hours

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FIGURE 5



TRAFFIC VOLUMES
2023 Background Plus Site Trips
Morning and Evening Peak Hours

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OPERATIONAL ANALYSIS

The operational analysis for future traffic conditions was again conducted using Synchro analysis software, with outputs based on the analysis methodologies contained in the *HIGHWAY CAPACITY MANUAL*. The analysis was prepared for the intersections' morning and evening peak hours.

The results of the operational analysis are summarized in Table 4 below. Detailed analysis worksheets are also included in the technical appendix.

Table 4 - Operational Analysis Summary: Year 2023 Future Conditions

Intersection	AM Peak Hour			PM Peak Hour		
	Delay	LOS	v/c*	Delay	LOS	v/c*
Highway 26 at Ten Eyck Road						
2023 Background Conditions	25.5	C	0.72	29.2	C	0.78
2023 Background plus Site	25.8	C	0.75	29.8	C	0.81
Highway 26 at Langensand Road						
2023 Background Conditions	76.4	F	0.32 / 0.62	160.1	F	0.39 / 0.68
2023 Background plus Site	97.3	F	0.32 / 0.75	210.4	F	0.40 / 0.84
Highway 211 at Dubarko Road						
2023 Background Conditions	22.8	C	0.35	39.4	E	0.46
2023 Background plus Site	23.9	C	0.39	43.3	E	0.50
2023 Background plus Site AWSC	19.5	C	0.67	29.6	D	0.79
Dubarko Road at Langensand Road						
2023 Background Conditions	9.5	A	0.05	9.9	A	0.04
2023 Background plus Site	10.5	B	0.13	11.3	B	0.08

*(major street v/c) / (minor-street v/c) is shown for the unsignalized ODOT intersection.

AWSC = Mitigated conditions analysis with conversion to all-way stop control

The intersection of Oregon Highway 211 at Dubarko Road was previously under the jurisdiction of the Oregon Department of Transportation and subject to a volume-to-capacity ratio standard rather than level of service. The intersection would have met ODOT standards for operation, but with conversion to a city intersection it is projected to operate at level of service "E" either with or without the addition of site trips from the proposed development. If the intersection is converted to all-way stop control (as recommended in the safety analysis section of this report on page 20), the intersection is projected to operate at level of service D, thereby meeting the city's operational standard.

All other intersections are projected to operate acceptably per the appropriate jurisdictional standards. No other operational mitigations are recommended in conjunction with the proposed development.



SAFETY ANALYSIS

CRASH DATA ANALYSIS

Using data obtained from the Oregon Department of Transportation, a review of the five most recent years of available crash history (from January 2015 through December 2019) was performed for the study intersections. The crash data was evaluated based on the number, type, and severity of collisions, as well as the intersection crash rate. Crash rates allow comparison of relative safety risks at intersections with different lane configurations, volumes, and traffic control devices by accounting for both the number of crashes that occur during the study period and the number of vehicles that traveled through the intersection during that period. Crash rates are calculated using the standard assumption that evening peak hour volumes are approximately 10 percent of the average daily traffic volume at an intersection. The crash rates were compared to statewide crash rates for similar intersection types to identify any locations with crash rates in excess of the 90th percentile.

The intersection of Highway 26 at SE Ten Eyck Road had eight reported collisions during the five-year analysis period. These included four rear-end collisions, three turning movement collisions, and one angle collision. The crashes resulted in no serious injuries or fatalities and six reports of a “possible injury/complaint of pain”. The crash rate for the intersection was calculated to be 0.15 crashes per million entering vehicles. This is well below the 90th percentile crash rate of 0.86 crashes per million entering vehicles for signalized, four-way urban intersections in Oregon.

The intersection of Highway 26 at SE Langensand Road had seven reported collisions during the five-year analysis period. These included five turning-movement collisions, one backing collision and one pedestrian collision. The pedestrian collision occurred when a pedestrian walking along the south side of Highway 26 crossing Langensand Road was struck by a driver making an eastbound right turn from the highway onto Langensand Road. The collision resulted in a report of a “possible injury/complaint of pain” by the pedestrian. Overall, the crashes resulted in one non-incapacitating injury and five reports of a “possible injury/complaint of pain”. The crash rate for the intersection was calculated to be 0.16 crashes per million entering vehicles. This is well below the 90th percentile crash rate of 0.29 crashes per million entering vehicles for stop-controlled, three-way urban intersections in Oregon.

The intersection of Highway 211 at Dubarko Road had 27 reported crashes during the five-year analysis period. These included 16 angle collisions, 4 turning-movement collisions, 4 rear-end collisions, 1 backing collision, 1 sideswipe-overtaking collision, and 1 pedestrian collision. The crashes resulted in one incapacitating injury and no fatalities. There were 10 “non-incapacitating” injuries reported and 19 reports of a “possible injury/complaint of pain”. The incapacitating injury occurred when a westbound driver failed to yield to a southbound vehicle and was struck in the intersection. The pedestrian collision occurred when a southbound pedestrian was struck by a westbound driver that failed to yield right-of-way to the pedestrian crossing, resulting in a report of a possible injury/complaint of pain by the pedestrian. The crash rate for the intersection was calculated to be 1.56 crashes per million entering vehicles. This is above the 90th percentile crash rate of 1.08 crashes per million entering vehicles for rural unsignalized four-way intersections in the state of Oregon.

The Oregon Department of Transportation recently undertook safety improvements at this intersection, including re-alignment of the minor-street approaches to intersect at a 90-degree angle and the addition



of some striping and speed feedback signs along the major-street to increase driver awareness of speed. However, the crash data for subsequent years has shown no significant improvement in the crash frequency at this intersection. An examination of the current intersection configuration revealed no significant apparent hazards and adequate sight distance from the minor-street approaches, allowing drivers approaching the highway to select safe gaps when turning onto or crossing the highway.

As described in the Warrant Analysis section of this report below, the intersection currently meets all-way stop control warrants based on crash history. Accordingly, it is recommended that all-way stop control be installed at this intersection. No other safety mitigations are recommended at this time.

The intersection of Dubarko Road at SE Langensand Road had one reported collision during the five-year analysis period. It was an angle collision that resulted in property damage only. The crash rate for the intersection was calculated to be 0.34 crashes per million entering vehicles. This is well below the 90th percentile crash rate of 0.408 crashes per million entering vehicles for stop-controlled, four-way urban intersections in Oregon.

Based on the crash data, the majority of the study intersections are currently operating acceptably with respect to safety. The intersection of Highway 211 at Dubarko Road has a high historical crash rate which recent safety improvements have not significantly improved. It is recommended that consideration be given to installing all-way stop control at this intersection. No other safety improvements are recommended for the study area intersections at this time.

TRAFFIC SIGNAL AND ALL-WAY STOP CONTROL WARRANT ANALYSIS

Traffic signal warrants were examined for the unsignalized study intersections. Based on the projected traffic volumes, traffic signal warrants are not projected to be met for any of the unsignalized study intersections under any of the analysis scenarios.

All-way stop control can be installed where there are “Five or more crashes in a 12-month period that are susceptible to correction by a multi-way stop installation. Such crashes include right-turn and left-turn collisions as well as right-angle collisions.” Examination of the crash data shows that there were six angle collisions at the intersection in the most recent year for which complete data is available (2019). Accordingly, installation of all-way stop control is warranted based on crash history.

Consideration was also given to installing a roundabout at the intersection of Highway 211 and Dubarko Road. Installation of a roundabout would result in operation well within capacity and at level of service A. However, according to *Roundabouts: An Informational Guide*, published by the Federal Highway Administration, “It is generally not desirable to locate roundabouts in locations where grades through the intersection are greater than four percent. The installation of roundabouts on roadways with grades lower than three percent is generally not problematic.” In this instance, Highway 211 has a constant grade of approximately 6 percent through its intersection with Dubarko Road. Accordingly, installation of a roundabout would not be recommended absent significant re-grading of the approach roadways. The potential for snow and ice at the intersection compound this concern.



TURN LANE WARRANT ANALYSIS

Turn lane warrants were also examined for the major-street approaches to the unsignalized study intersections. Left-turn lane warrants are intended to evaluate whether a meaningful safety benefit may be expected if the turning vehicles are provided with turn lane within the street, allowing left-turning drivers to move out of the through travel lane so that following vehicles may pass without conflicts.

The intersection of Highway 26 at Langensand Road already has left and right turn lanes in place.

The intersection of Highway 211 at Dubarko Road currently meets ODOT warrants for a northbound left-turn lane and a northbound right-turn lane. However, the need for these turn lanes is not meaningfully related to the proposed development. Further, if all-way stop control is installed at the intersection as recommended based on the safety analysis, additional turn lanes will not be required for either safety or operations.

The intersection of Dubarko Road at Langensand Road is not projected to meet turn lane warrants under any analysis scenarios.



CONCLUSIONS

With conversion to all-way stop control, the intersection of Highway 211 at Dubarko Road is projected to operate acceptably under year 2023 traffic conditions. All other study intersections are projected to operate acceptably through year 2023 either with or without the addition of site trips from the proposed development. No other operational mitigations are necessary or recommended in conjunction with the proposed subdivision.

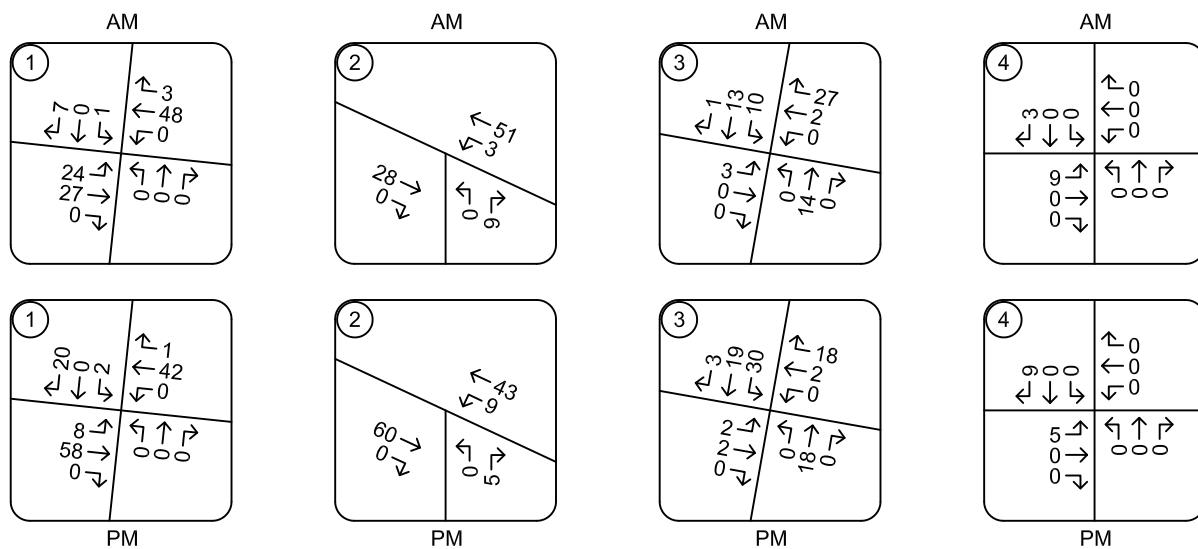
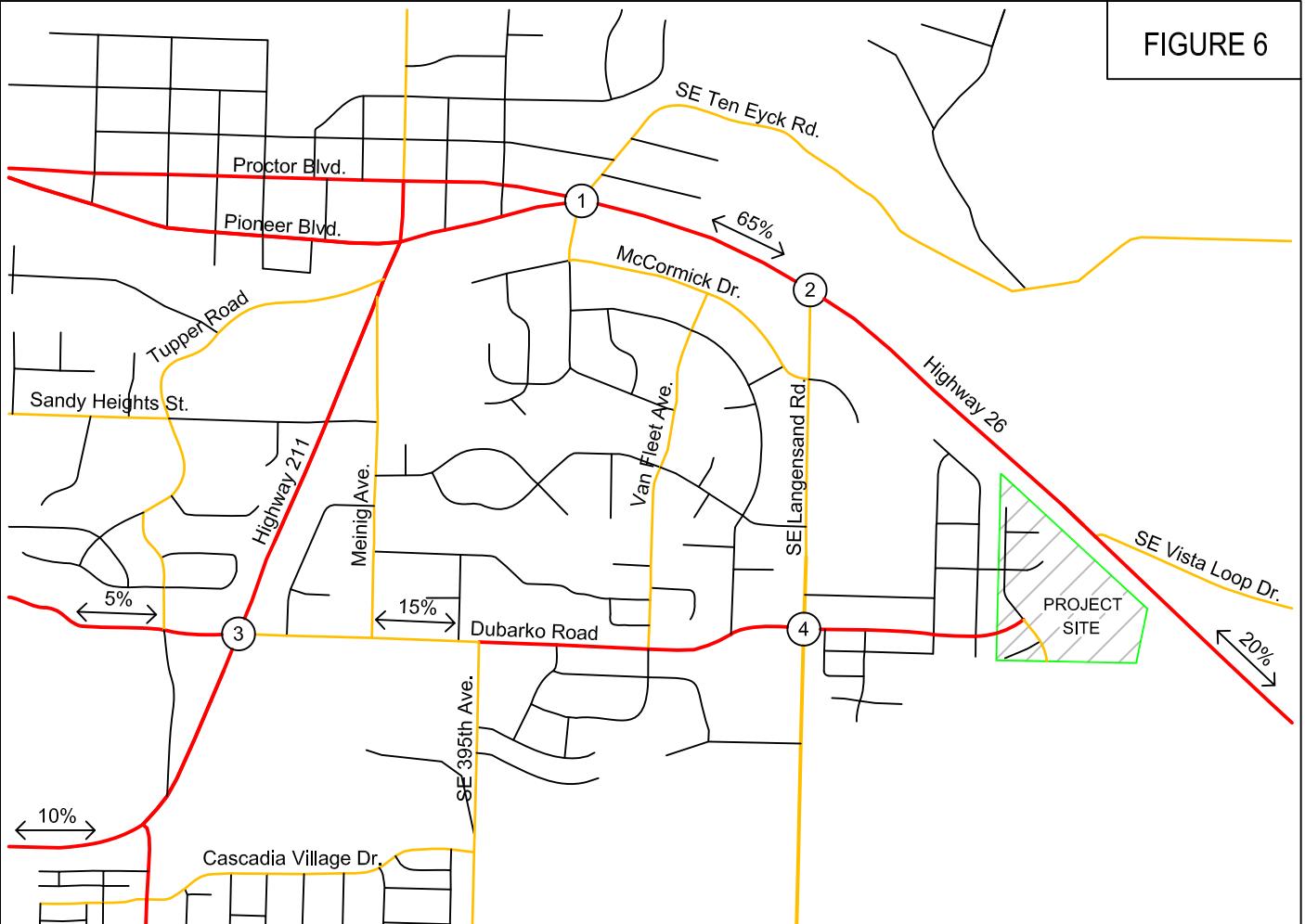
Based on the crash data, the majority of the study intersections are currently operating acceptably with respect to safety. The intersection of Highway 211 at Dubarko Road has a high historical crash rate which recent safety improvements have not significantly improved. This intersection meets all-way stop control warrants based on crash history, and conversion to all-way stop control would be expected to reduce the frequency and severity of right-angle and turning-movement collisions. It is therefore recommended that all-way stop control be installed at the intersection of Highway 211 and Dubarko Road. No other safety improvements are recommended.

Based on the warrant analysis, no new turn lanes or traffic signals are recommended in conjunction with the proposed subdivision.



APPENDIX

FIGURE 6



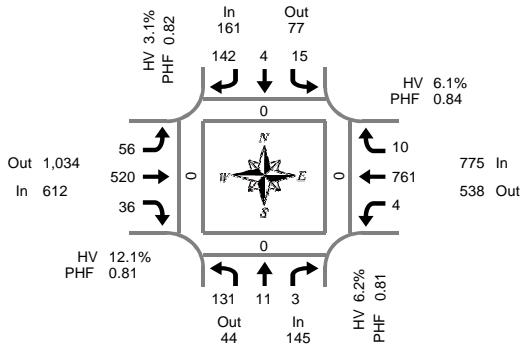
TRAFFIC VOLUMES
In-Process Development - Site Trips
Morning and Evening Peak Hours

PAGE
APP1

Total Vehicle Summary



Clay Carney
(503) 833-2740



SE Ten Eyck Rd & Hwy 26

Wednesday, March 20, 2019

7:00 AM to 9:00 AM

**Peak Hour Summary
7:00 AM to 8:00 AM**

5-Minute Interval Summary

7:00 AM to 9:00 AM

Interval Start Time	Northbound SE Ten Eyck Rd				Southbound SE Ten Eyck Rd				Eastbound Hwy 26				Westbound Hwy 26				Interval Total
	L	T	R	Bikes	L	T	R	Bikes	L	T	R	Bikes	L	T	R	Bikes	
7:00 AM	16	0	0	0	0	0	17	0	5	26	2	0	0	74	0	0	140
7:05 AM	10	0	1	0	1	0	10	0	2	18	3	0	1	65	2	0	113
7:10 AM	17	1	0	0	2	0	11	0	7	36	2	0	2	74	1	0	153
7:15 AM	12	0	0	0	1	2	9	0	9	40	2	0	1	84	1	0	161
7:20 AM	15	0	0	0	3	0	11	0	3	40	1	0	0	68	0	0	141
7:25 AM	14	1	0	0	1	1	16	0	2	40	4	0	0	70	1	0	150
7:30 AM	7	1	1	0	0	0	16	0	8	43	2	0	0	67	0	0	145
7:35 AM	12	2	0	0	3	0	12	0	0	56	5	0	0	57	1	0	148
7:40 AM	8	2	0	0	0	0	11	0	4	59	3	0	0	53	0	0	140
7:45 AM	12	1	1	0	2	0	11	0	4	53	3	0	0	45	2	0	134
7:50 AM	4	2	0	0	1	0	10	0	9	47	4	0	0	62	0	0	139
7:55 AM	4	1	0	0	1	1	8	0	3	62	5	0	0	42	2	0	129
8:00 AM	5	0	1	0	2	1	13	0	2	46	2	0	0	41	0	0	113
8:05 AM	6	0	0	0	1	1	5	0	8	50	2	0	0	42	2	0	117
8:10 AM	3	0	0	0	2	1	10	0	5	45	4	0	0	53	1	0	124
8:15 AM	12	0	0	0	2	0	7	0	3	38	1	0	0	34	1	0	98
8:20 AM	6	2	0	0	2	0	9	0	5	38	1	0	1	49	0	0	113
8:25 AM	8	0	0	0	1	0	11	0	4	44	3	0	0	39	2	0	112
8:30 AM	5	0	0	0	2	1	10	0	4	66	2	0	0	47	0	0	137
8:35 AM	10	0	0	0	3	0	13	0	6	59	5	0	0	45	1	0	142
8:40 AM	7	0	0	0	5	1	15	0	10	62	3	0	1	43	1	0	148
8:45 AM	5	0	0	0	1	0	12	0	5	69	5	0	0	63	0	0	160
8:50 AM	9	2	0	0	3	0	12	0	7	56	8	0	1	46	1	0	145
8:55 AM	8	1	0	0	2	0	13	0	6	51	8	0	2	44	1	0	136
Total Survey	215	16	4	0	41	9	272	0	121	1,144	80	0	9	1,307	20	0	3,238

15-Minute Interval Summary

7:00 AM to 9:00 AM

Interval Start Time	Northbound SE Ten Eyck Rd				Southbound SE Ten Eyck Rd				Eastbound Hwy 26				Westbound Hwy 26				Interval Total
	L	T	R	Bikes	L	T	R	Bikes	L	T	R	Bikes	L	T	R	Bikes	
7:00 AM	43	1	1	0	3	0	38	0	14	80	7	0	3	213	3	0	406
7:15 AM	41	1	0	0	5	3	36	0	14	120	7	0	1	222	2	0	452
7:30 AM	27	5	1	0	3	0	39	0	12	158	10	0	0	177	1	0	433
7:45 AM	20	4	1	0	4	1	29	0	16	162	12	0	0	149	4	0	402
8:00 AM	14	0	1	0	5	3	28	0	15	141	8	0	0	136	3	0	354
8:15 AM	26	2	0	0	5	0	27	0	12	120	5	0	1	122	3	0	323
8:30 AM	22	0	0	0	10	2	38	0	20	187	10	0	1	135	2	0	427
8:45 AM	22	3	0	0	6	0	37	0	18	176	21	0	3	153	2	0	441
Total Survey	215	16	4	0	41	9	272	0	121	1,144	80	0	9	1,307	20	0	3,238

Pedestrians Crosswalk			
North	South	East	West
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	1
0	0	0	1
1	0	0	0
0	0	0	0
1	0	0	2

Peak Hour Summary

7:00 AM to 8:00 AM

By Approach	Northbound				Southbound				Eastbound				Westbound				Total	
	SE Ten Eyck Rd				SE Ten Eyck Rd				Hwy 26				Hwy 26					
	In	Out	Total	Bikes	In	Out	Total	Bikes	In	Out	Total	Bikes	In	Out	Total	Bikes		
Volume	145	44	189	0	161	77	238	0	612	1,034	1,646	0	775	538	1,313	0	1,693	
%HV	6.2%				3.1%				12.1%				6.1%				8.0%	
PHF	0.81				0.82				0.81				0.84				0.93	

Pedestrians Crosswalk

By Movement	Northbound SE Ten Eyck Rd				Southbound SE Ten Eyck Rd				Eastbound Hwy 26				Westbound Hwy 26				Total
	L	T	R	Total	L	T	R	Total	L	T	R	Total	L	T	R	Total	
Volume	131	11	3	145	15	4	142	161	56	520	36	612	4	761	10	775	1,693
%HV	6.9%	0.0%	0.0%	6.2%	13.3%	25.0%	1.4%	3.1%	8.9%	12.7%	8.3%	12.1%	75.0%	5.5%	20.0%	6.1%	8.0%
PHF	0.74	0.55	0.75	0.81	0.63	0.33	0.81	0.82	0.74	0.77	0.75	0.81	0.25	0.84	0.63	0.84	0.93

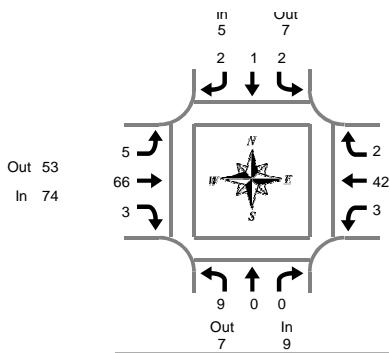
Rolling Hour Summary

7:00 AM to 9:00 AM

Interval Start Time	Northbound SE Ten Eyck Rd				Southbound SE Ten Eyck Rd				Eastbound Hwy 26				Westbound Hwy 26				Interval Total
	L	T	R	Bikes	L	T	R	Bikes	L	T	R	Bikes	L	T	R	Bikes	
7:00 AM	131	11	3	0	15	4	142	0	56	520	36	0	4	761	10	0	1,693
7:15 AM	102	10	3	0	17	7	132	0	57	581	37	0	1	684	10	0	1,641
7:30 AM	87	11	3	0	17	4	123	0	55	581	35	0	1	584	11	0	1,512
7:45 AM	82	6	2	0	24	6	122	0	63	610	35	0	2	542	12	0	1,506
8:00 AM	84	5	1	0	26	5	130	0	65	624	44	0	5	546	10	0	1,545

Pedestrians Crosswalk			
North	South	East	West
0	0	0	0
0	0	0	1
0	0	0	2
1	0	0	0
1	0	0	2

Heavy Vehicle Summary



SE Ten Eyck Rd & Hwy 26

Wednesday, March 20, 2019

7:00 AM to 9:00 AM

Peak Hour Summary
7:00 AM to 8:00 AM

Heavy Vehicle 5-Minute Interval Summary

7:00 AM to 9:00 AM

Interval Start Time	Northbound SE Ten Eyck Rd				Southbound SE Ten Eyck Rd				Eastbound Hwy 26				Westbound Hwy 26				Interval Total
	L	T	R	Total	L	T	R	Total	L	T	R	Total	L	T	R	Total	
7:00 AM	1	0	0	1	0	0	0	0	1	6	1	8	0	6	0	6	15
7:05 AM	0	0	0	0	0	0	0	0	5	0	5	0	5	0	5	0	10
7:10 AM	3	0	0	3	0	0	0	0	3	0	3	2	2	1	5	11	
7:15 AM	1	0	0	1	0	1	0	1	2	6	0	8	1	1	0	2	12
7:20 AM	2	0	0	2	1	0	0	1	0	5	0	5	0	1	0	1	9
7:25 AM	0	0	0	0	0	0	0	0	6	1	7	0	1	0	1	0	8
7:30 AM	0	0	0	0	0	0	0	0	7	0	7	0	7	0	7	0	14
7:35 AM	0	0	0	0	1	0	0	1	0	7	0	7	0	6	0	6	14
7:40 AM	0	0	0	0	0	0	0	0	1	8	0	9	0	1	0	1	10
7:45 AM	0	0	0	0	0	0	1	1	0	6	0	6	0	4	0	4	11
7:50 AM	0	0	0	0	0	0	1	1	0	3	0	3	0	7	0	7	11
7:55 AM	2	0	0	2	0	0	0	0	1	4	1	6	0	1	1	2	10
8:00 AM	1	0	0	1	0	0	1	1	0	10	1	11	0	2	0	2	15
8:05 AM	0	0	0	0	1	0	1	2	0	9	0	9	0	7	1	8	19
8:10 AM	0	0	0	0	0	0	0	0	0	2	0	2	0	6	0	6	8
8:15 AM	0	0	0	0	0	0	0	0	0	4	0	4	0	3	0	3	7
8:20 AM	0	0	0	0	0	0	0	1	1	0	5	0	5	1	2	0	9
8:25 AM	0	0	0	0	0	0	0	0	0	6	1	7	0	3	0	3	10
8:30 AM	0	0	0	0	1	0	0	1	2	6	0	8	0	3	0	3	12
8:35 AM	0	0	0	0	0	0	0	0	0	5	0	6	0	8	0	8	14
8:40 AM	0	0	0	0	0	0	0	1	1	0	5	0	5	0	1	0	7
8:45 AM	0	0	0	0	0	0	0	0	0	9	0	9	0	3	0	3	12
8:50 AM	0	0	0	0	0	0	0	0	0	1	4	0	5	1	8	0	9
8:55 AM	0	0	0	0	0	0	3	3	0	0	2	2	0	3	0	3	8
Total Survey	10	0	0	10	4	1	9	14	9	131	7	147	5	91	3	99	270

Heavy Vehicle 15-Minute Interval Summary

7:00 AM to 9:00 AM

Interval Start Time	Northbound SE Ten Eyck Rd				Southbound SE Ten Eyck Rd				Eastbound Hwy 26				Westbound Hwy 26				Interval Total
	L	T	R	Total	L	T	R	Total	L	T	R	Total	L	T	R	Total	
7:00 AM	4	0	0	4	0	0	0	0	1	14	1	16	2	13	1	16	36
7:15 AM	3	0	0	3	1	1	0	2	2	17	1	20	1	3	0	4	29
7:30 AM	0	0	0	0	1	0	0	1	1	22	0	23	0	14	0	14	38
7:45 AM	2	0	0	2	0	0	2	2	1	13	1	15	0	12	1	13	32
8:00 AM	1	0	0	1	1	0	2	3	0	21	1	22	0	15	1	16	42
8:15 AM	0	0	0	0	0	0	1	1	0	15	1	16	1	8	0	9	26
8:30 AM	0	0	0	0	1	0	1	2	3	16	0	19	0	12	0	12	33
8:45 AM	0	0	0	0	0	0	3	3	1	13	2	16	1	14	0	15	34
Total Survey	10	0	0	10	4	1	9	14	9	131	7	147	5	91	3	99	270

Heavy Vehicle Peak Hour Summary

7:00 AM to 8:00 AM

By Approach	Northbound SE Ten Eyck Rd			Southbound SE Ten Eyck Rd			Eastbound Hwy 26			Westbound Hwy 26			Total
	In	Out	Total	In	Out	Total	In	Out	Total	In	Out	Total	
Volume	9	7	16	5	7	12	74	53	127	47	68	115	135
PHF	0.38		0.63			0.80			0.73			0.89	

By Movement	Northbound SE Ten Eyck Rd				Southbound SE Ten Eyck Rd				Eastbound Hwy 26				Westbound Hwy 26				Total
	L	T	R	Total	L	T	R	Total	L	T	R	Total	L	T	R	Total	
Volume	9	0	0	9	2	1	2	5	5	66	3	74	3	42	2	47	135
PHF	0.38	0.00	0.00	0.38	0.50	0.25	0.25	0.63	0.63	0.75	0.75	0.80	0.25	0.75	0.50	0.73	0.89

Heavy Vehicle Rolling Hour Summary

7:00 AM to 9:00 AM

Interval Start Time	Northbound SE Ten Eyck Rd				Southbound SE Ten Eyck Rd				Eastbound Hwy 26				Westbound Hwy 26				Interval Total
	L	T	R	Total	L	T	R	Total	L	T	R	Total	L	T	R	Total	
7:00 AM	9	0	0	9	2	1	2	5	5	66	3	74	3	42	2	47	135
7:15 AM	6	0	0	6	3	1	4	8	4	73	3	80	1	44	2	47	141
7:30 AM	3	0	0	3	2	0	5	7	2	71	3	76	1	49	2	52	138
7:45 AM	3	0	0	3	2	0	6	8	4	65	3	72	1	47	2	50	133
8:00 AM	1	0	0	1	2	0	7	9	4	65	4	73	2	49	1	52	135

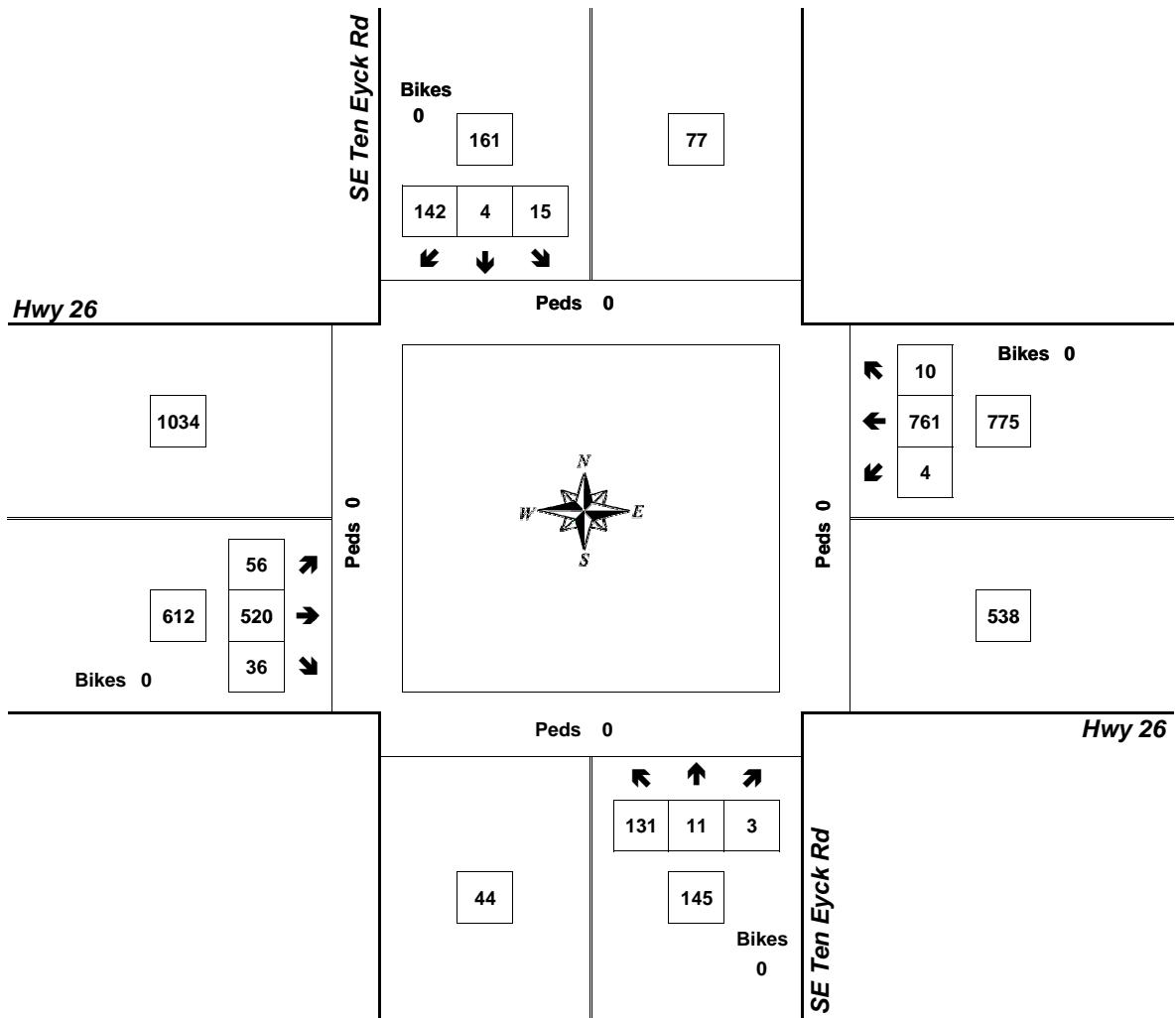
Peak Hour Summary



Clay Carney
(503) 833-2740

SE Ten Eyck Rd & Hwy 26

7:00 AM to 8:00 AM
Wednesday, March 20, 2019



Count Period: 7:00 AM to 9:00 AM

Total Vehicle Summary

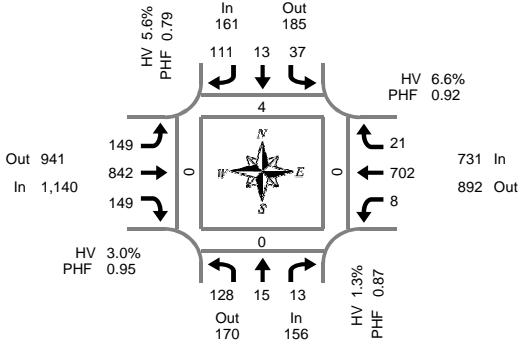


Clay Carney
(503) 833-2740

SE Ten Eyck Rd & Hwy 26

Tuesday, March 19, 2019

4:00 PM to 6:00 PM



Peak Hour Summary
4:10 PM to 5:10 PM

5-Minute Interval Summary 4:00 PM to 6:00 PM

Interval Start Time	Northbound SE Ten Eyck Rd				Southbound SE Ten Eyck Rd				Eastbound Hwy 26				Westbound Hwy 26				Interval Total
	L	T	R	Bikes	L	T	R	Bikes	L	T	R	Bikes	L	T	R	Bikes	
4:00 PM	4	0	2	0	4	3	11	0	8	58	12	0	1	49	2	0	154
4:05 PM	10	1	0	0	7	1	5	0	12	63	8	0	1	53	3	0	164
4:10 PM	7	2	3	0	1	0	17	0	12	76	11	0	0	65	1	0	195
4:15 PM	14	0	1	0	7	1	9	0	18	71	15	0	0	62	1	0	199
4:20 PM	9	0	1	0	4	1	11	0	9	75	10	0	0	62	7	0	189
4:25 PM	12	2	0	0	5	0	10	0	12	61	14	0	0	52	0	0	168
4:30 PM	11	1	4	0	3	2	12	0	17	87	16	1	1	58	1	0	213
4:35 PM	15	0	0	0	2	2	6	0	6	59	14	0	0	65	3	0	172
4:40 PM	7	1	1	0	3	0	7	0	7	54	9	0	1	57	0	0	147
4:45 PM	8	1	0	0	4	1	3	0	13	71	15	1	3	51	3	0	173
4:50 PM	13	2	1	0	1	1	6	0	19	74	8	0	0	56	0	0	181
4:55 PM	7	1	0	0	1	0	12	0	10	67	14	0	3	57	1	0	173
5:00 PM	13	3	1	0	2	2	14	0	12	81	12	0	0	49	1	0	190
5:05 PM	12	2	1	0	4	3	4	0	14	66	11	0	0	68	3	1	188
5:10 PM	8	0	0	0	6	2	10	0	13	60	12	0	0	68	2	0	181
5:15 PM	8	2	1	0	6	2	8	0	9	70	11	0	0	57	1	0	175
5:20 PM	8	1	1	1	1	4	10	0	15	73	10	0	0	43	1	0	167
5:25 PM	9	1	0	0	4	2	8	0	14	74	11	0	0	43	0	0	166
5:30 PM	5	0	1	0	4	0	5	0	15	64	10	0	0	44	0	0	148
5:35 PM	5	1	0	0	7	0	9	0	17	50	4	1	0	39	0	0	132
5:40 PM	4	0	0	0	2	1	5	0	11	56	7	0	0	30	1	0	117
5:45 PM	4	1	0	0	3	2	8	0	14	76	6	0	3	41	1	0	159
5:50 PM	7	1	0	0	0	1	6	0	14	69	8	0	0	42	0	0	148
5:55 PM	10	1	0	0	0	2	3	0	16	65	10	0	0	51	1	0	159
Total Survey	210	24	18	1	81	33	199	0	307	1,620	258	3	13	1,262	33	1	4,058

Pedestrians Crosswalk			
North	South	East	West
0	1	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
1	0	0	0
2	0	0	0
0	0	0	0
0	1	0	0
0	0	0	0
1	0	0	0
0	0	0	0
2	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
9	2	0	2

15-Minute Interval Summary 4:00 PM to 6:00 PM

Interval Start Time	Northbound SE Ten Eyck Rd				Southbound SE Ten Eyck Rd				Eastbound Hwy 26				Westbound Hwy 26				Interval Total
	L	T	R	Bikes	L	T	R	Bikes	L	T	R	Bikes	L	T	R	Bikes	
4:00 PM	21	3	5	0	12	4	33	0	32	197	31	0	2	167	6	0	513
4:15 PM	35	2	2	0	16	2	30	0	39	207	39	0	0	176	8	0	556
4:30 PM	33	2	5	0	8	4	25	0	30	200	39	1	2	180	4	0	532
4:45 PM	28	4	1	0	6	2	21	0	42	212	37	1	6	164	4	0	527
5:00 PM	33	5	2	0	12	7	28	0	39	207	35	0	0	185	6	1	559
5:15 PM	25	4	2	1	11	8	26	0	38	217	32	0	0	143	2	0	508
5:30 PM	14	1	1	0	13	1	19	0	43	170	21	1	0	113	1	0	397
5:45 PM	21	3	0	0	3	5	17	0	44	210	24	0	3	134	2	0	466
Total Survey	210	24	18	1	81	33	199	0	307	1,620	258	3	13	1,262	33	1	4,058

Pedestrians Crosswalk			
North	South	East	West
0	1	0	0
0	0	0	0
1	0	0	0
1	0	0	0
4	0	0	0
0	1	0	0
3	0	0	2
0	0	0	0
9	2	0	2

Peak Hour Summary 4:10 PM to 5:10 PM

By Approach	Northbound SE Ten Eyck Rd				Southbound SE Ten Eyck Rd				Eastbound Hwy 26				Westbound Hwy 26				Total
	In	Out	Total	Bikes	In	Out	Total	Bikes	In	Out	Total	Bikes	In	Out	Total	Bikes	
Volume	156	170	326	0	161	185	346	0	1,140	941	2,081	2	731	892	1,623	1	2,188
%HV	1.3%				5.6%				3.0%				6.6%			4.3%	
PHF	0.87				0.79				0.95				0.92			0.94	

By Movement	Northbound SE Ten Eyck Rd				Southbound SE Ten Eyck Rd				Eastbound Hwy 26				Westbound Hwy 26				Total
	L	T	R	Total	L	T	R	Total	L	T	R	Total	L	T	R	Total	
Volume	128	15	13	156	37	13	111	161	149	842	149	1,140	8	702	21	731	2,188
%HV	1.6%	0.0%	0.0%	1.3%	0.0%	0.0%	8.1%	5.6%	4.0%	3.0%	2.0%	3.0%	0.0%	6.7%	4.8%	6.6%	4.3%
PHF	0.84	0.63	0.65	0.87	0.58	0.65	0.75	0.79	0.69	0.94	0.85	0.95	0.33	0.93	0.58	0.92	0.94

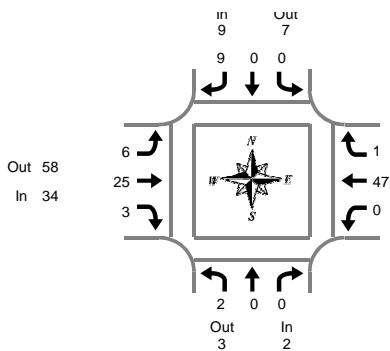
Pedestrians Crosswalk			
North	South	East	West
2	1	0	0
6	0	0	0
6	1	0	0
8	1	0	2
7	1	0	2

Heavy Vehicle Summary

All Traffic Data

Services Inc.

Clay Carney
(503) 833-2740



SE Ten Eyck Rd & Hwy 26

Tuesday, March 19, 2019

4:00 PM to 6:00 PM

Peak Hour Summary
4:10 PM to 5:10 PM

Heavy Vehicle 5-Minute Interval Summary

4:00 PM to 6:00 PM

Interval Start Time	Northbound SE Ten Eyck Rd				Southbound SE Ten Eyck Rd				Eastbound Hwy 26				Westbound Hwy 26				Interval Total
	L	T	R	Total	L	T	R	Total	L	T	R	Total	L	T	R	Total	
4:00 PM	0	0	0	0	0	0	0	0	0	4	0	4	0	10	1	11	15
4:05 PM	0	0	0	0	1	0	0	1	0	6	0	6	0	3	1	4	11
4:10 PM	0	0	0	0	0	0	0	0	2	0	2	0	8	0	8	10	
4:15 PM	2	0	0	2	0	0	2	2	3	0	5	0	3	0	3	12	
4:20 PM	0	0	0	0	0	0	2	2	1	3	0	4	0	5	1	6	12
4:25 PM	0	0	0	0	0	0	1	1	0	5	1	6	0	4	0	4	11
4:30 PM	0	0	0	0	0	0	2	2	1	0	0	1	0	3	0	3	6
4:35 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	5	0	5	5
4:40 PM	0	0	0	0	0	0	1	1	0	3	0	3	0	2	0	2	6
4:45 PM	0	0	0	0	0	0	0	0	1	1	0	2	0	4	0	4	6
4:50 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	7	0	7	7
4:55 PM	0	0	0	0	0	0	0	1	1	2	1	4	0	0	0	0	5
5:00 PM	0	0	0	0	0	0	0	0	0	4	1	5	0	1	0	1	6
5:05 PM	0	0	0	0	0	0	0	0	0	2	0	2	0	5	0	5	7
5:10 PM	0	0	0	0	0	0	0	0	0	1	3	0	4	0	4	0	8
5:15 PM	0	0	0	0	0	0	0	0	0	1	1	0	2	0	2	4	
5:20 PM	0	0	0	0	0	0	0	0	0	1	0	1	0	5	0	5	6
5:25 PM	0	0	0	0	0	0	0	0	0	2	0	2	0	1	0	1	3
5:30 PM	0	0	0	0	0	0	0	0	0	3	1	4	0	3	0	3	7
5:35 PM	0	0	0	0	0	0	0	0	0	1	1	0	2	0	4	0	6
5:40 PM	0	0	0	0	0	0	0	0	0	0	2	0	2	0	0	0	2
5:45 PM	1	0	0	1	0	0	0	0	0	2	0	2	0	3	0	3	6
5:50 PM	1	0	0	1	0	0	0	0	0	1	1	2	0	4	0	4	7
5:55 PM	0	0	0	0	0	0	0	0	1	2	0	3	0	5	0	5	8
Total Survey	4	0	0	4	1	0	9	10	10	53	5	68	0	91	3	94	176

Heavy Vehicle 15-Minute Interval Summary

4:00 PM to 6:00 PM

Interval Start Time	Northbound SE Ten Eyck Rd				Southbound SE Ten Eyck Rd				Eastbound Hwy 26				Westbound Hwy 26				Interval Total
	L	T	R	Total	L	T	R	Total	L	T	R	Total	L	T	R	Total	
4:00 PM	0	0	0	0	1	0	0	1	0	12	0	12	0	21	2	23	36
4:15 PM	2	0	0	2	0	0	5	5	3	11	1	15	0	12	1	13	35
4:30 PM	0	0	0	0	0	0	3	3	1	3	0	4	0	10	0	10	17
4:45 PM	0	0	0	0	0	0	1	1	2	3	1	6	0	11	0	11	18
5:00 PM	0	0	0	0	0	0	0	0	1	9	1	11	0	10	0	10	21
5:15 PM	0	0	0	0	0	0	0	0	1	4	0	5	0	8	0	8	13
5:30 PM	0	0	0	0	0	0	0	0	1	6	1	8	0	7	0	7	15
5:45 PM	2	0	0	2	0	0	0	0	1	5	1	7	0	12	0	12	21
Total Survey	4	0	0	4	1	0	9	10	10	53	5	68	0	91	3	94	176

Heavy Vehicle Peak Hour Summary

4:10 PM to 5:10 PM

By Approach	Northbound SE Ten Eyck Rd			Southbound SE Ten Eyck Rd			Eastbound Hwy 26			Westbound Hwy 26			Total
	In	Out	Total	In	Out	Total	In	Out	Total	In	Out	Total	
Volume	2	3	5	9	7	16	34	58	92	48	25	73	93
PHF	0.25		0.45			0.57			0.71			0.66	

By Movement	Northbound SE Ten Eyck Rd				Southbound SE Ten Eyck Rd				Eastbound Hwy 26				Westbound Hwy 26				Total
	L	T	R	Total	L	T	R	Total	L	T	R	Total	L	T	R	Total	
Volume	2	0	0	2	0	0	9	9	6	25	3	34	0	47	1	48	93
PHF	0.25	0.00	0.00	0.25	0.00	0.00	0.45	0.45	0.50	0.57	0.38	0.57	0.00	0.73	0.25	0.71	0.66

Heavy Vehicle Rolling Hour Summary

4:00 PM to 6:00 PM

Interval Start Time	Northbound SE Ten Eyck Rd				Southbound SE Ten Eyck Rd				Eastbound Hwy 26				Westbound Hwy 26				Interval Total
	L	T	R	Total	L	T	R	Total	L	T	R	Total	L	T	R	Total	
4:00 PM	2	0	0	2	1	0	9	10	6	29	2	37	0	54	3	57	106
4:15 PM	2	0	0	2	0	0	9	9	7	26	3	36	0	43	1	44	91
4:30 PM	0	0	0	0	0	0	4	4	5	19	2	26	0	39	0	39	69
4:45 PM	0	0	0	0	0	0	1	1	5	22	3	30	0	36	0	36	67
5:00 PM	2	0	0	2	0	0	0	0	4	24	3	31	0	37	0	37	70

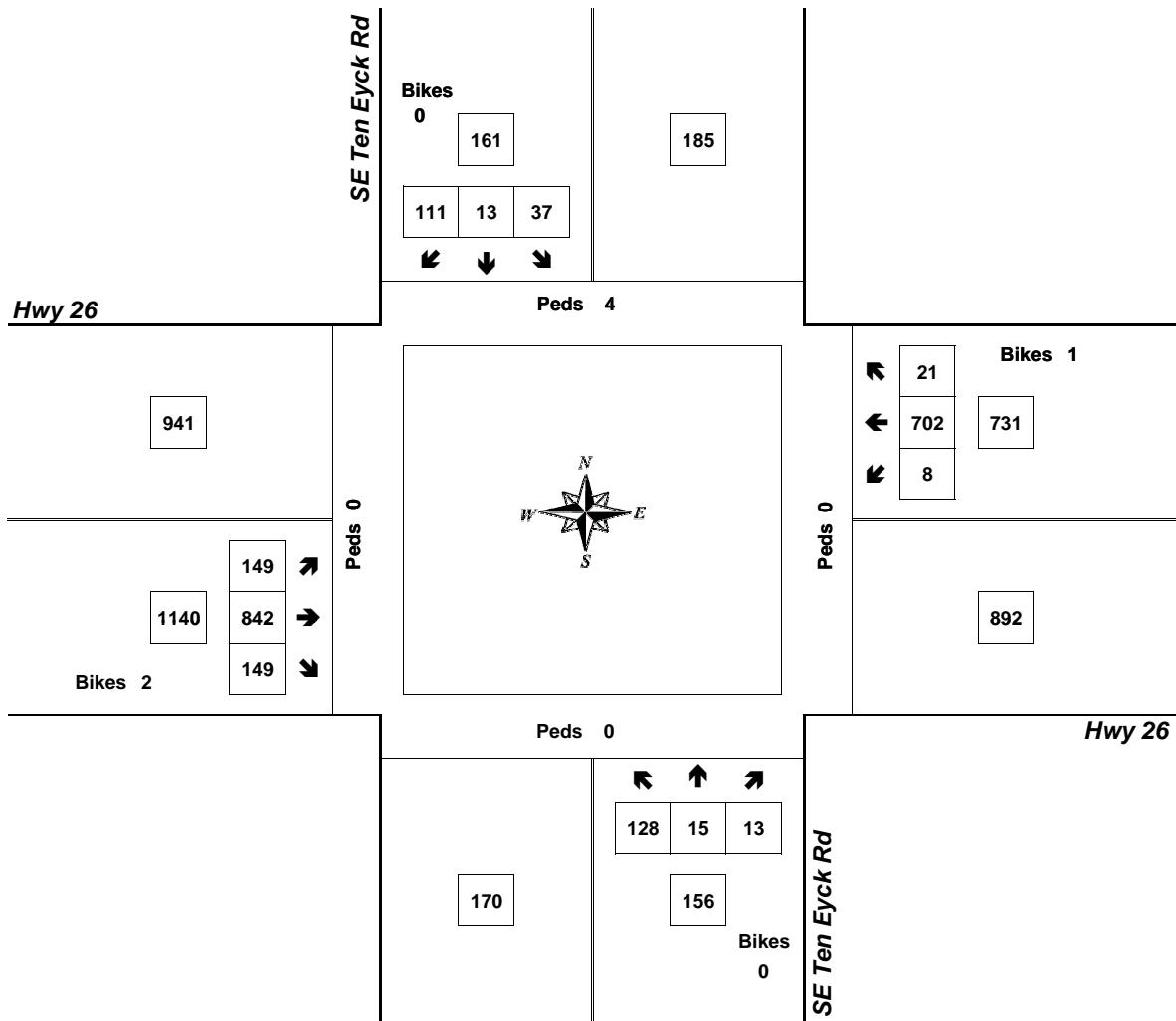
Peak Hour Summary



Clay Carney
(503) 833-2740

SE Ten Eyck Rd & Hwy 26

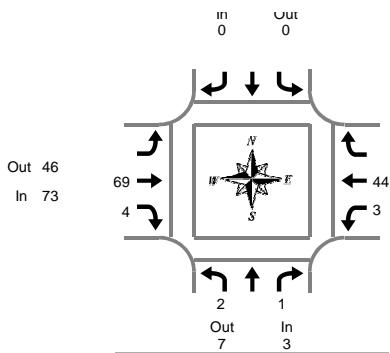
4:10 PM to 5:10 PM
Tuesday, March 19, 2019



Approach	PHF	HV%	Volume
EB	0.95	3.0%	1,140
WB	0.92	6.6%	731
NB	0.87	1.3%	156
SB	0.79	5.6%	161
Intersection	0.94	4.3%	2,188

Count Period: 4:00 PM to 6:00 PM

Heavy Vehicle Summary



SE Langensand Rd & Hwy 26

Wednesday, March 20, 2019

7:00 AM to 9:00 AM

Peak Hour Summary
7:05 AM to 8:05 AM

Heavy Vehicle 5-Minute Interval Summary

7:00 AM to 9:00 AM

Interval Start Time	Northbound SE Langensand Rd			Southbound SE Langensand Rd			Eastbound Hwy 26			Westbound Hwy 26			Interval Total
	L	R	Total			Total	T	R	Total	L	T	Total	
7:00 AM	0	0	0			0	6	1	7	0	6	6	13
7:05 AM	0	0	0			0	4	1	5	0	6	6	11
7:10 AM	0	0	0			0	2	0	2	0	3	3	5
7:15 AM	0	0	0			0	6	0	6	0	3	3	9
7:20 AM	0	0	0			0	7	0	7	0	0	0	7
7:25 AM	0	0	0			0	5	1	6	1	2	3	9
7:30 AM	0	0	0			0	6	0	6	0	6	6	12
7:35 AM	0	0	0			0	5	0	5	1	7	8	13
7:40 AM	1	0	1			0	7	0	7	0	2	2	10
7:45 AM	0	0	0			0	11	1	12	1	3	4	16
7:50 AM	0	1	1			0	4	1	5	0	5	5	11
7:55 AM	1	0	1			0	3	0	3	0	5	5	9
8:00 AM	0	0	0			0	9	0	9	0	2	2	11
8:05 AM	1	0	1			0	11	1	12	0	7	7	20
8:10 AM	0	0	0			0	2	0	2	0	5	5	7
8:15 AM	0	0	0			0	3	0	3	0	4	4	7
8:20 AM	0	0	0			0	4	1	5	0	2	2	7
8:25 AM	0	1	1			0	4	1	5	0	3	3	9
8:30 AM	0	2	2			0	9	0	9	1	3	4	15
8:35 AM	1	1	2			0	5	0	5	0	6	6	13
8:40 AM	0	0	0			0	5	0	5	0	3	3	8
8:45 AM	0	0	0			0	7	0	7	0	1	1	8
8:50 AM	0	0	0			0	3	0	3	0	9	9	12
8:55 AM	0	0	0			0	4	0	4	0	4	4	8
Total Survey	4		5	9		0	132	8	140	4	97	101	250

Heavy Vehicle 15-Minute Interval Summary

7:00 AM to 9:00 AM

Interval Start Time	Northbound SE Langensand Rd			Southbound SE Langensand Rd			Eastbound Hwy 26			Westbound Hwy 26			Interval Total
	L	R	Total			Total	T	R	Total	L	T	Total	
7:00 AM	0	0	0			0	12	2	14	0	15	15	29
7:15 AM	0	0	0			0	18	1	19	1	5	6	25
7:30 AM	1	0	1			0	18	0	18	1	15	16	35
7:45 AM	1	1	2			0	18	2	20	1	13	14	36
8:00 AM	1	0	1			0	22	1	23	0	14	14	38
8:15 AM	0	1	1			0	11	2	13	0	9	9	23
8:30 AM	1	3	4			0	19	0	19	1	12	13	36
8:45 AM	0	0	0			0	14	0	14	0	14	14	28
Total Survey	4		5	9		0	132	8	140	4	97	101	250

Heavy Vehicle Peak Hour Summary

7:05 AM to 8:05 AM

By Approach	Northbound SE Langensand Rd			Southbound SE Langensand Rd			Eastbound Hwy 26			Westbound Hwy 26			Total
	In	Out	Total	In	Out	Total	In	Out	Total	In	Out	Total	
Volume	3	7	10	0	0	0	73	46	119	47	70	117	123
PHF	0.38		0.00			0.76				0.69		0.79	

By Movement	Northbound SE Langensand Rd			Southbound SE Langensand Rd			Eastbound Hwy 26			Westbound Hwy 26			Total
	L	R	Total			Total	T	R	Total	L	T	Total	
Volume	2	1	3	0		0	69	4	73	3	44	47	123
PHF	0.50		0.25	0.38		0.00	0.75	0.50	0.76	0.38	0.73	0.69	0.79

Heavy Vehicle Rolling Hour Summary

7:00 AM to 9:00 AM

Interval Start Time	Northbound SE Langensand Rd			Southbound SE Langensand Rd			Eastbound Hwy 26			Westbound Hwy 26			Interval Total
	L	R	Total			Total	T	R	Total	L	T	Total	
7:00 AM	2	1	3	0		0	66	5	71	3	48	51	125
7:15 AM	3	1	4	0		0	76	4	80	3	47	50	134
7:30 AM	3	2	5	0		0	69	5	74	2	51	53	132
7:45 AM	3	5	8	0		0	70	5	75	2	48	50	133
8:00 AM	2	4	6	0		0	66	3	69	1	49	50	125

Peak Hour Summary



Clay Carney
(503) 833-2740

SE Langensand Rd & Hwy 26

7:05 AM to 8:05 AM
Wednesday, March 20, 2019

Bikes
0

Hwy 26

Peds 0

Bikes 0

Bikes 0

743

561

527

34



542

Peds 0

Hwy 26

SE Langensand
Rd

49

63

Bikes
0

15

78

680

695

15

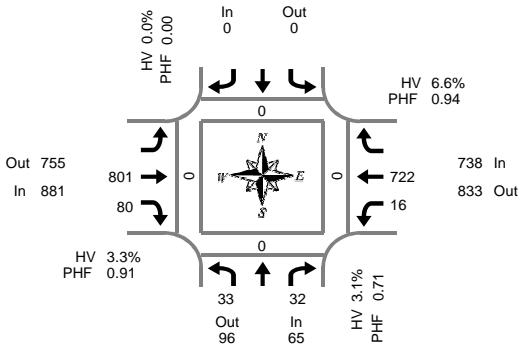
Approach	PHF	HV%	Volume
EB	0.80	13.0%	561
WB	0.80	6.8%	695
NB	0.85	3.8%	78
SB	0.00	0.0%	0
Intersection	0.93	9.2%	1,334

Count Period: 7:00 AM to 9:00 AM

Total Vehicle Summary



Clay Carney
(503) 833-2740



SE Langensand Rd & Hwy 26

Tuesday, March 19, 2019

4:00 PM to 6:00 PM

**Peak Hour Summary
4:10 PM to 5:10 PM**

5-Minute Interval Summary

4:00 PM to 6:00 PM

Interval Start Time	Northbound SE Langensand Rd			Southbound SE Langensand Rd			Eastbound Hwy 26			Westbound Hwy 26			Interval Total
	L	R	Bikes			Bikes	T	R	Bikes	L	T		
4:00 PM	2	4	0			0	62	9	0	5	50	0	132
4:05 PM	1	2	0			0	69	6	0	3	52	0	133
4:10 PM	1	3	0			0	61	3	0	1	74	0	143
4:15 PM	6	1	0			0	76	5	0	1	50	0	139
4:20 PM	5	5	0			0	79	9	0	1	70	0	169
4:25 PM	6	0	1			0	58	8	0	1	49	0	122
4:30 PM	0	3	0			0	75	12	0	1	56	0	147
4:35 PM	2	5	0			0	61	7	0	1	64	0	140
4:40 PM	0	1	0			0	59	1	0	1	55	0	117
4:45 PM	1	1	0			0	64	3	0	2	63	0	134
4:50 PM	6	5	0			0	62	6	0	0	54	0	133
4:55 PM	3	0	0			0	72	5	0	2	56	0	138
5:00 PM	1	5	0			0	62	10	0	1	55	0	134
5:05 PM	2	3	0			0	72	11	0	4	76	0	168
5:10 PM	2	3	0			0	58	14	0	1	65	0	143
5:15 PM	1	2	0			0	51	8	0	2	59	0	123
5:20 PM	2	4	0			0	78	7	0	2	43	0	136
5:25 PM	3	1	0			0	71	5	0	1	42	0	123
5:30 PM	2	2	0			0	67	7	0	3	38	0	119
5:35 PM	1	1	0			0	60	5	0	1	38	0	106
5:40 PM	0	4	0			0	49	7	0	0	34	0	94
5:45 PM	2	1	0			0	69	7	0	1	45	0	125
5:50 PM	0	3	0			0	60	4	0	0	43	0	110
5:55 PM	4	1	0			0	65	8	0	3	52	0	133
Total Survey	53	60	1			0	1,560	167	0	38	1,283	0	3,161

15-Minute Interval Summary

4:00 PM to 6:00 PM

Interval Start Time	Northbound SE Langensand Rd			Southbound SE Langensand Rd			Eastbound Hwy 26			Westbound Hwy 26			Interval Total
	L	R	Bikes			Bikes	T	R	Bikes	L	T	Bikes	
4:00 PM	4	9	0			0	192	18	0	9	176	0	408
4:15 PM	17	6	1			0	213	22	0	3	169	0	430
4:30 PM	2	9	0			0	195	20	0	3	175	0	404
4:45 PM	10	6	0			0	198	14	0	4	173	0	405
5:00 PM	5	11	0			0	192	35	0	6	196	0	445
5:15 PM	6	7	0			0	200	20	0	5	144	0	382
5:30 PM	3	7	0			0	176	19	0	4	110	0	319
5:45 PM	6	5	0			0	194	19	0	4	140	0	368
Total Survey	53		60	1		0	1,560	167	0	38	1,283	0	3,161

Peak Hour Summary

4:10 PM to 5:10 PM

By Approach	Northbound				Southbound				Eastbound				Westbound				Total	
	SE Langensand Rd				SE Langensand Rd				Hwy 26				Hwy 26					
	In	Out	Total	Bikes	In	Out	Total	Bikes	In	Out	Total	Bikes	In	Out	Total	Bikes		
Volume	65	96	161	1	0	0	0	0	881	755	1,636	0	738	833	1,571	0	1,684	
%HV	3.1%				0.0%				3.3%				6.6%				4.8%	
PHF	0.71				0.00				0.91				0.94				0.93	

Pedestrians			
Crosswalk			
North	South	East	West
0	0	0	0

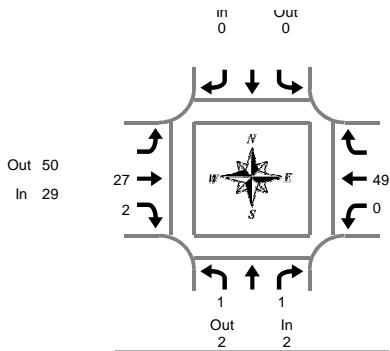
By Movement	Northbound			Southbound			Eastbound			Westbound			Total		
	SE Langensand Rd	L	R	SE Langensand Rd	Total		Hwy 26	T	R	Total	L	T	Hwy 26		
Volume	33		32	65			0	801	80	881	16	722	738	1,684	
%HV	3.0%	NA	3.1%	3.1%	NA	NA	0.0%	NA	3.4%	2.5%	3.3%	0.0%	6.8%	NA	6.6%
PHF	0.49		0.80	0.71			0.00	0.93	0.69	0.91	0.57	0.93	0.94	0.93	

Rolling Hour Summary

4:00 PM to 6:00 PM

Interval Start Time	Northbound			Southbound			Eastbound			Westbound			Interval Total		
	SE Langensand Rd		Bikes	SE Langensand Rd		Bikes	Hwy 26		T	R	Bikes	L	T	R	Bikes
	L	R	Bikes			Bikes		T	R	Bikes	L	T	R	Bikes	
4:00 PM	33	30	1			0		798	74	0	19	693	0	1,647	
4:15 PM	34	32	1			0		798	91	0	16	713	0	1,684	
4:30 PM	23	33	0			0		785	89	0	18	688	0	1,636	
4:45 PM	24	31	0			0		766	88	0	19	623	0	1,551	
5:00 PM	20	30	0			0		762	93	0	19	590	0	1,514	

Heavy Vehicle Summary



SE Langensand Rd & Hwy 26

Tuesday, March 19, 2019

4:00 PM to 6:00 PM

Peak Hour Summary
4:10 PM to 5:10 PM

Heavy Vehicle 5-Minute Interval Summary

4:00 PM to 6:00 PM

Interval Start Time	Northbound SE Langensand Rd			Southbound SE Langensand Rd			Eastbound Hwy 26			Westbound Hwy 26			Interval Total
	L	R	Total			Total	T	R	Total	L	T	Total	
4:00 PM	0	0	0			0	3	0	3	0	11	11	14
4:05 PM	0	0	0			0	8	0	8	0	5	5	13
4:10 PM	0	0	0			0	2	0	2	0	7	7	9
4:15 PM	0	0	0			0	5	0	5	0	4	4	9
4:20 PM	1	0	1			0	4	1	5	0	4	4	10
4:25 PM	0	0	0			0	3	0	3	0	5	5	8
4:30 PM	0	1	1			0	1	1	2	0	3	3	6
4:35 PM	0	0	0			0	1	0	1	0	4	4	5
4:40 PM	0	0	0			0	2	0	2	0	3	3	5
4:45 PM	0	0	0			0	1	0	1	0	4	4	5
4:50 PM	0	0	0			0	2	0	2	0	6	6	8
4:55 PM	0	0	0			0	1	0	1	0	2	2	3
5:00 PM	0	0	0			0	3	0	3	0	1	1	4
5:05 PM	0	0	0			0	2	0	2	0	6	6	8
5:10 PM	0	0	0			0	0	1	1	0	4	4	5
5:15 PM	0	0	0			0	2	0	2	0	3	3	5
5:20 PM	0	0	0			0	0	0	0	0	5	5	5
5:25 PM	0	0	0			0	1	0	1	0	1	1	2
5:30 PM	0	0	0			0	4	0	4	0	2	2	6
5:35 PM	0	0	0			0	0	0	0	1	2	3	3
5:40 PM	0	0	0			0	1	0	1	0	3	3	4
5:45 PM	0	0	0			0	4	0	4	0	3	3	7
5:50 PM	0	0	0			0	1	0	1	0	2	2	3
5:55 PM	0	0	0			0	2	0	2	0	7	7	9
Total Survey	1		1	2		0		53	3	56	1	97	156

Heavy Vehicle 15-Minute Interval Summary

4:00 PM to 6:00 PM

Interval Start Time	Northbound SE Langensand Rd			Southbound SE Langensand Rd			Eastbound Hwy 26			Westbound Hwy 26			Interval Total
	L	R	Total			Total	T	R	Total	L	T	Total	
4:00 PM	0	0	0			0	13	0	13	0	23	23	36
4:15 PM	1	0	1			0	12	1	13	0	13	13	27
4:30 PM	0	1	1			0	4	1	5	0	10	10	16
4:45 PM	0	0	0			0	4	0	4	0	12	12	16
5:00 PM	0	0	0			0	5	1	6	0	11	11	17
5:15 PM	0	0	0			0	3	0	3	0	9	9	12
5:30 PM	0	0	0			0	5	0	5	1	7	8	13
5:45 PM	0	0	0			0	7	0	7	0	12	12	19
Total Survey	1		1	2		0		53	3	56	1	97	156

Heavy Vehicle Peak Hour Summary

4:10 PM to 5:10 PM

By Approach	Northbound SE Langensand Rd			Southbound SE Langensand Rd			Eastbound Hwy 26			Westbound Hwy 26			Total
	In	Out	Total	In	Out	Total	In	Out	Total	In	Out	Total	
Volume	2	2	4	0	0	0	29	50	79	49	28	77	80
PHF	0.25		0.00			0.56			0.56			0.82	0.71

By Movement	Northbound SE Langensand Rd			Southbound SE Langensand Rd			Eastbound Hwy 26			Westbound Hwy 26			Total
	L	R	Total			Total	T	R	Total	L	T	Total	
Volume	1	1	2	0		0	27	2	29	0	49	49	80
PHF	0.25		0.25	0.25		0.56			0.56			0.82	0.71

Heavy Vehicle Rolling Hour Summary

4:00 PM to 6:00 PM

Interval Start Time	Northbound SE Langensand Rd			Southbound SE Langensand Rd			Eastbound Hwy 26			Westbound Hwy 26			Interval Total
	L	R	Total			Total	T	R	Total	L	T	Total	
4:00 PM	1	1	2	0		0	33	2	35	0	58	58	95
4:15 PM	1	1	2	0		0	25	3	28	0	46	46	76
4:30 PM	0	1	1	0		0	16	2	18	0	42	42	61
4:45 PM	0	0	0	0		0	17	1	18	1	39	40	58
5:00 PM	0	0	0	0		0	20	1	21	1	39	40	61

Peak Hour Summary



Clay Carney
(503) 833-2740

SE Langensand Rd & Hwy 26

4:10 PM to 5:10 PM
Tuesday, March 19, 2019

Bikes
0

Hwy 26

Peds 0

Bikes 0

Bikes 0

755

881

801

80



722

16

738

833

Peds 0

Hwy 26

SE Langensand
Rd

33 32
65
Bikes 1

96

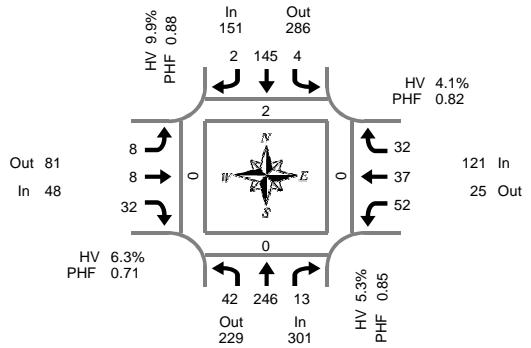
Approach	PHF	HV%	Volume
EB	0.91	3.3%	881
WB	0.94	6.6%	738
NB	0.71	3.1%	65
SB	0.00	0.0%	0
Intersection	0.93	4.8%	1,684

Count Period: 4:00 PM to 6:00 PM

Total Vehicle Summary



Clay Carney
(503) 833-2740



Hwy 211 & Dubarko Rd

Wednesday, March 20, 2019

7:00 AM to 9:00 AM

Peak Hour Summary
7:05 AM to 8:05 AM

5-Minute Interval Summary

7:00 AM to 9:00 AM

Interval Start Time	Northbound Hwy 211				Southbound Hwy 211				Eastbound Dubarko Rd				Westbound Dubarko Rd				Interval Total
	L	T	R	Bikes	L	T	R	Bikes	L	T	R	Bikes	L	T	R	Bikes	
7:00 AM	2	18	1	0	0	8	0	0	0	0	0	0	4	5	0	0	38
7:05 AM	3	20	1	0	0	12	0	0	0	0	0	0	3	1	5	0	45
7:10 AM	5	23	0	0	0	12	0	0	2	2	4	0	4	3	9	0	64
7:15 AM	5	32	0	0	0	9	0	0	1	0	2	0	4	2	2	0	57
7:20 AM	8	13	0	0	2	13	1	0	0	0	2	0	5	3	5	0	52
7:25 AM	1	23	2	0	0	13	0	0	1	1	5	0	4	3	3	0	56
7:30 AM	3	17	0	0	1	12	0	0	0	0	3	0	4	9	1	0	50
7:35 AM	2	23	0	0	0	17	0	0	0	0	7	0	6	5	1	0	61
7:40 AM	2	23	1	0	0	6	1	0	1	2	4	0	6	4	1	0	51
7:45 AM	4	20	3	0	0	14	0	0	0	1	0	0	3	1	0	0	46
7:50 AM	5	15	3	0	0	10	0	0	1	1	1	0	5	4	2	0	47
7:55 AM	1	21	2	0	1	15	0	0	1	0	3	0	3	1	1	0	49
8:00 AM	3	16	1	0	0	12	0	0	1	1	1	0	5	1	2	0	43
8:05 AM	2	15	0	0	0	7	0	0	1	1	2	0	4	0	3	0	35
8:10 AM	2	19	1	0	1	8	0	0	3	1	2	0	3	4	1	0	45
8:15 AM	3	27	1	0	0	8	0	0	0	0	1	0	1	3	2	0	46
8:20 AM	0	19	0	0	0	10	0	0	0	1	0	0	1	3	0	0	34
8:25 AM	6	8	1	0	0	8	0	0	0	1	1	0	1	1	2	0	29
8:30 AM	3	27	2	0	0	10	0	0	0	1	1	0	2	2	5	0	53
8:35 AM	1	14	0	0	0	16	0	0	0	1	0	0	2	2	0	0	36
8:40 AM	0	19	1	0	0	15	0	0	0	1	1	0	1	3	1	0	42
8:45 AM	1	21	1	0	0	15	1	0	0	2	3	0	1	2	4	0	51
8:50 AM	0	21	0	0	0	9	0	0	0	2	0	0	3	3	2	0	40
8:55 AM	4	20	1	0	1	10	0	0	1	3	2	0	3	3	0	0	51
Total Survey	66	474	22	0	6	269	3	0	13	22	45	0	78	68	55	0	1,121

Pedestrians Crosswalk			
North	South	East	West
0	1	0	0
0	0	0	0
0	0	0	0
1	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
1	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
3	1	0	0

15-Minute Interval Summary

7:00 AM to 9:00 AM

Interval Start Time	Northbound Hwy 211				Southbound Hwy 211				Eastbound Dubarko Rd				Westbound Dubarko Rd				Interval Total
	L	T	R	Bikes	L	T	R	Bikes	L	T	R	Bikes	L	T	R	Bikes	
7:00 AM	10	61	2	0	0	32	0	0	2	4	0	0	11	9	14	0	147
7:15 AM	14	68	2	0	2	35	1	0	2	1	9	0	13	8	10	0	165
7:30 AM	7	63	1	0	1	35	1	0	1	2	14	0	16	18	3	0	162
7:45 AM	10	56	8	0	1	39	0	0	2	2	4	0	11	6	3	0	142
8:00 AM	7	50	2	0	1	27	0	0	5	3	5	0	12	5	6	0	123
8:15 AM	9	54	2	0	0	26	0	0	0	2	2	0	3	7	4	0	109
8:30 AM	4	60	3	0	0	41	0	0	3	2	0	0	5	7	6	0	131
8:45 AM	5	62	2	0	1	34	1	0	1	7	5	0	7	8	9	0	142
Total Survey	66	474	22	0	6	269	3	0	13	22	45	0	78	68	55	0	1,121

Pedestrians Crosswalk			
North	South	East	West
0	1	0	0
1	0	0	0
1	0	0	0
0	0	0	0
1	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
3	1	0	0

Peak Hour Summary

7:05 AM to 8:05 AM

By Approach	Northbound Hwy 211				Southbound Hwy 211				Eastbound Dubarko Rd				Westbound Dubarko Rd				Total
	In	Out	Total	Bikes	In	Out	Total	Bikes	In	Out	Total	Bikes	In	Out	Total	Bikes	
Volume	301	229	530	0	151	286	437	0	48	81	129	0	121	25	146	0	621
%HV	5.3%				9.9%				6.3%				4.1%				6.3%
PHF	0.85				0.88				0.71				0.82				0.90

Pedestrians Crosswalk			
North	South	East	West
2	1	0	0
3	0	0	0
2	0	0	0
1	0	0	0
1	0	0	0

Rolling Hour Summary

7:00 AM to 9:00 AM

Interval Start Time	Northbound Hwy 211				Southbound Hwy 211				Eastbound Dubarko Rd				Westbound Dubarko Rd				Total
	L	T	R	Bikes	L	T	R	Bikes	L	T	R	Bikes	L	T	R	Bikes	
7:00 AM	41	248	13	0	4	141	2	0	7	7	31	0	51	41	30	0	616
7:15 AM	38	237	13	0	5	136	2	0	10	8	32	0	52	37	22	0	592
7:30 AM	33	223	13	0	3	127	1	0	8	9	25	0	42	36	16	0	536
7:45 AM	30	220	15	0	2	133	0	0	7	10	13	0	31	25	19	0	505
8:00 AM	25	226	9	0	2	128	1	0	6	15	14	0	27	27	25	0	505

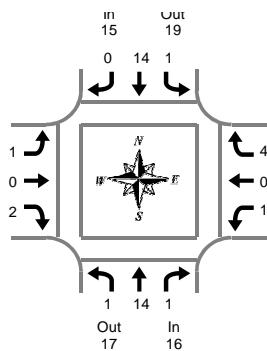
Pedestrians Crosswalk	
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Heavy Vehicle Summary

All Traffic Data

Services Inc.

Clay Carney
(503) 833-2740



Hwy 211 & Dubarko Rd

Wednesday, March 20, 2019

7:00 AM to 9:00 AM

Peak Hour Summary
7:05 AM to 8:05 AM

Heavy Vehicle 5-Minute Interval Summary

7:00 AM to 9:00 AM

Interval Start Time	Northbound Hwy 211				Southbound Hwy 211				Eastbound Dubarko Rd				Westbound Dubarko Rd				Interval Total
	L	T	R	Total	L	T	R	Total	L	T	R	Total	L	T	R	Total	
7:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	1
7:05 AM	0	1	0	1	0	1	0	1	0	0	0	0	0	0	0	0	2
7:10 AM	0	1	0	1	0	0	0	0	0	1	1	1	0	0	1	1	3
7:15 AM	0	1	0	1	0	0	0	0	0	1	1	1	0	0	0	0	2
7:20 AM	0	0	0	0	1	1	0	2	0	0	0	0	1	0	0	0	3
7:25 AM	0	0	0	0	0	0	0	0	1	0	0	1	0	0	2	2	3
7:30 AM	0	1	0	1	0	2	0	2	0	0	0	0	0	0	0	0	3
7:35 AM	0	1	0	1	0	2	0	2	0	0	0	0	0	0	0	0	3
7:40 AM	0	3	1	4	0	0	0	0	0	0	0	0	0	0	1	1	5
7:45 AM	0	0	0	0	0	3	0	3	0	0	0	0	0	0	0	0	3
7:50 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:55 AM	1	0	0	1	0	3	0	3	0	0	0	0	0	0	0	0	4
8:00 AM	0	6	0	6	0	2	0	2	0	0	0	0	0	0	0	0	8
8:05 AM	0	0	0	0	0	3	0	3	0	0	0	0	1	0	0	1	4
8:10 AM	0	2	0	2	0	0	0	0	0	0	0	0	1	1	0	2	4
8:15 AM	1	2	0	3	0	1	0	1	0	0	0	0	0	0	0	0	4
8:20 AM	0	2	0	2	0	2	0	2	0	1	0	1	0	0	0	0	5
8:25 AM	0	2	0	2	0	1	0	1	0	0	0	0	0	0	0	0	3
8:30 AM	0	3	0	3	0	2	0	2	0	0	0	0	0	0	0	0	5
8:35 AM	0	3	0	3	0	4	0	4	0	0	0	0	0	0	0	0	7
8:40 AM	0	1	0	1	0	1	0	1	0	0	0	0	0	0	0	0	2
8:45 AM	0	2	0	2	0	0	0	0	0	0	0	0	0	0	0	0	2
8:50 AM	0	0	0	0	0	1	0	1	0	0	0	0	0	0	0	0	1
8:55 AM	0	0	0	0	0	2	0	2	0	0	0	0	0	1	0	1	3
Total Survey	2	31	1	34	1	31	0	32	1	1	2	4	3	3	4	10	80

Heavy Vehicle 15-Minute Interval Summary

7:00 AM to 9:00 AM

Interval Start Time	Northbound Hwy 211				Southbound Hwy 211				Eastbound Dubarko Rd				Westbound Dubarko Rd				Interval Total
	L	T	R	Total	L	T	R	Total	L	T	R	Total	L	T	R	Total	
7:00 AM	0	2	0	2	0	1	0	1	0	0	1	1	0	1	1	2	6
7:15 AM	0	1	0	1	1	1	0	2	1	0	1	2	1	0	2	3	8
7:30 AM	0	5	1	6	0	4	0	4	0	0	0	0	0	1	1	11	11
7:45 AM	1	0	0	1	0	6	0	6	0	0	0	0	0	0	0	0	7
8:00 AM	0	8	0	8	0	5	0	5	0	0	0	0	2	1	0	3	16
8:15 AM	1	6	0	7	0	4	0	4	0	1	0	1	0	0	0	0	12
8:30 AM	0	7	0	7	0	7	0	7	0	0	0	0	0	0	0	0	14
8:45 AM	0	2	0	2	0	3	0	3	0	0	0	0	0	1	0	1	6
Total Survey	2	31	1	34	1	31	0	32	1	1	2	4	3	3	4	10	80

Heavy Vehicle Peak Hour Summary

7:05 AM to 8:05 AM

By Approach	Northbound Hwy 211			Southbound Hwy 211			Eastbound Dubarko Rd			Westbound Dubarko Rd			Total
	In	Out	Total	In	Out	Total	In	Out	Total	In	Out	Total	
Volume	16	17	33	15	19	34	3	1	4	5	2	7	39
PHF	0.57		0.63			0.38			0.42			0.81	

By Movement	Northbound Hwy 211				Southbound Hwy 211				Eastbound Dubarko Rd				Westbound Dubarko Rd				Total
	L	T	R	Total	L	T	R	Total	L	T	R	Total	L	T	R	Total	
Volume	1	14	1	16	1	14	0	15	1	0	2	3	1	0	4	5	39
PHF	0.25	0.58	0.25	0.57	0.25	0.58	0.00	0.63	0.25	0.00	0.25	0.38	0.25	0.00	0.50	0.42	0.81

Heavy Vehicle Rolling Hour Summary

7:00 AM to 9:00 AM

Interval Start Time	Northbound Hwy 211				Southbound Hwy 211				Eastbound Dubarko Rd				Westbound Dubarko Rd				Interval Total
	L	T	R	Total	L	T	R	Total	L	T	R	Total	L	T	R	Total	
7:00 AM	1	8	1	10	1	12	0	13	1	0	2	3	1	1	4	6	32
7:15 AM	1	14	1	16	1	16	0	17	1	0	1	2	3	1	3	7	42
7:30 AM	2	19	1	22	0	19	0	19	0	1	0	1	2	1	1	4	46
7:45 AM	2	21	0	23	0	22	0	22	0	1	0	1	2	1	0	3	49
8:00 AM	1	23	0	24	0	19	0	19	0	1	0	1	2	2	0	4	48

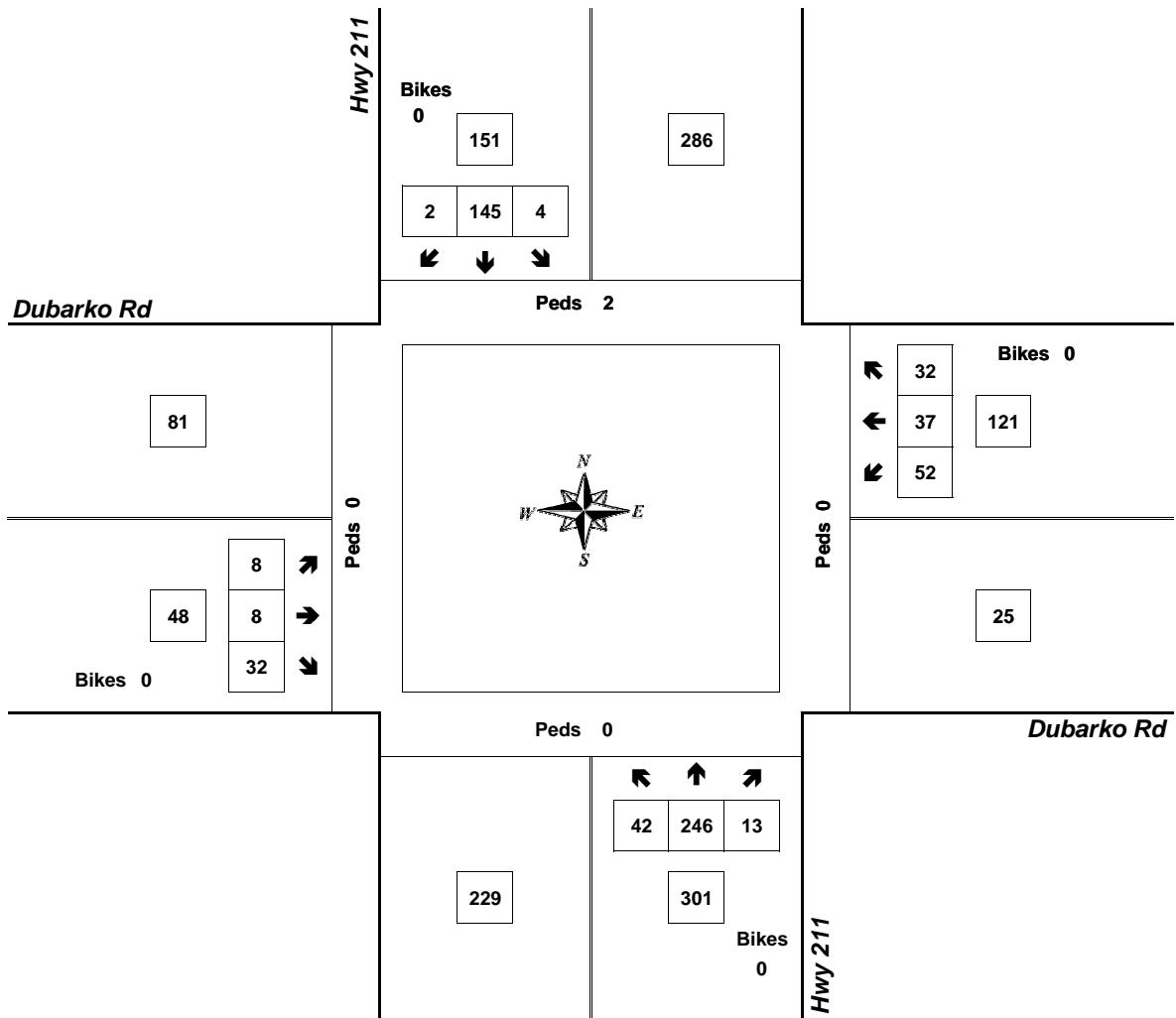
Peak Hour Summary



Clay Carney
(503) 833-2740

Hwy 211 & Dubarko Rd

7:05 AM to 8:05 AM
Wednesday, March 20, 2019



Count Period: 7:00 AM to 9:00 AM

Total Vehicle Summary



Clay Carney
(503) 833-2740

Clay Carney

Clay Canney
(503) 833-2740

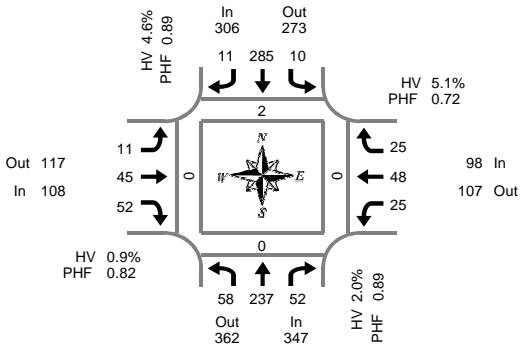
Clay Carney

(503) 833-2740

Hwy 211 & Dubarko Rd

Tuesday, March 19, 2019

4:00 PM to 6:00 PM



Peak Hour Summary
4:05 PM to 5:05 PM

5-Minute Interval Summary

4:00 PM to 6:00 PM

Interval Start Time	Northbound Hwy 211				Southbound Hwy 211				Eastbound Dubarko Rd				Westbound Dubarko Rd				Interval Total
	L	T	R	Bikes	L	T	R	Bikes	L	T	R	Bikes	L	T	R	Bikes	
4:00 PM	4	14	0	0	2	25	1	0	0	3	3	0	2	3	3	0	60
4:05 PM	4	28	3	0	1	31	0	0	1	7	6	0	2	6	2	0	91
4:10 PM	10	17	2	0	1	19	0	0	0	4	3	0	3	4	3	0	66
4:15 PM	4	20	6	0	2	20	1	0	2	7	3	1	1	5	1	0	72
4:20 PM	6	12	1	0	1	14	1	0	2	3	4	0	5	7	4	0	60
4:25 PM	5	16	4	0	1	21	1	0	3	3	4	0	2	4	1	0	65
4:30 PM	4	22	3	0	0	19	3	0	1	2	2	0	5	5	1	0	67
4:35 PM	2	23	7	0	0	29	1	0	1	2	1	0	0	1	3	0	70
4:40 PM	2	17	4	0	0	22	0	0	0	2	1	0	1	3	3	0	55
4:45 PM	10	23	7	0	2	29	1	0	0	6	8	0	3	2	0	0	91
4:50 PM	3	22	6	0	1	19	1	0	1	0	4	0	1	1	2	0	61
4:55 PM	4	20	3	0	0	20	2	0	0	6	2	0	1	6	1	0	65
5:00 PM	4	17	6	0	1	42	0	0	0	3	14	0	1	4	4	0	96
5:05 PM	2	24	5	0	0	20	0	0	0	4	5	0	1	2	3	0	66
5:10 PM	8	24	4	0	1	13	1	0	1	8	2	0	2	1	3	0	68
5:15 PM	4	13	4	0	1	19	1	0	0	4	3	0	5	3	0	0	57
5:20 PM	1	19	6	0	1	29	1	0	1	2	2	0	1	4	0	0	67
5:25 PM	5	14	6	0	0	17	1	0	1	3	9	0	2	4	3	0	65
5:30 PM	5	19	6	0	0	19	1	0	1	5	5	0	0	2	3	0	66
5:35 PM	5	15	1	0	2	24	0	0	1	5	6	0	1	2	1	0	63
5:40 PM	5	19	7	0	0	29	1	0	0	8	3	0	1	2	0	1	75
5:45 PM	4	15	8	0	0	16	1	0	0	7	3	0	3	0	0	0	57
5:50 PM	4	13	2	0	0	20	3	0	2	5	3	0	0	5	3	0	60
5:55 PM	5	13	2	0	1	18	0	0	0	2	3	0	2	1	1	0	48
Total Survey	110	439	103	0	18	534	22	0	18	101	99	1	45	77	45	1	1,611

Pedestrians Crosswalk				
North	South	East	West	
0	0	1	0	
0	0	0	0	
0	0	0	0	
0	0	0	0	
1	0	0	0	
0	0	0	0	
1	0	0	0	
0	0	0	0	
0	0	0	0	
0	0	0	0	
0	0	0	0	
0	0	0	0	
0	0	0	0	
0	0	0	0	
0	0	0	0	
0	0	0	0	
0	0	0	0	
0	0	0	0	
0	0	0	0	
0	0	0	0	
0	0	0	0	
0	0	0	0	
0	0	0	0	
0	0	0	0	
0	0	0	0	
0	0	0	0	
0	0	0	0	
2	0	1	0	

15-Minute Interval Summary

4:00 PM to 6:00 PM

Interval Start Time	Northbound Hwy 211				Southbound Hwy 211				Eastbound Dubarko Rd				Westbound Dubarko Rd				Interval Total
	L	T	R	Bikes	L	T	R	Bikes	L	T	R	Bikes	L	T	R	Bikes	
4:00 PM	18	59	5	0	4	75	1	0	1	14	12	0	7	13	8	0	217
4:15 PM	15	48	11	0	4	55	3	0	7	13	11	1	8	16	6	0	197
4:30 PM	8	62	14	0	0	70	4	0	2	6	4	0	6	9	7	0	192
4:45 PM	17	65	16	0	3	68	4	0	1	12	14	0	5	9	3	0	217
5:00 PM	14	65	15	0	2	75	1	0	1	15	21	0	4	7	10	0	230
5:15 PM	10	46	16	0	2	65	3	0	2	9	14	0	8	11	3	0	189
5:30 PM	15	53	14	0	2	72	2	0	2	18	14	0	2	6	4	1	204
5:45 PM	13	41	12	0	1	54	4	0	2	14	9	0	5	6	4	0	165
Total Survey	110	439	103	0	18	534	22	0	18	101	99	1	45	77	45	1	1,611

Pedestrians Crosswalk				
North	South	East	West	
0	0	1	0	
1	0	0	0	
1	0	0	0	
0	0	0	0	
0	0	0	0	
0	0	0	0	
0	0	0	0	
2	0	1	0	

Peak Hour Summary

Year-End Summary
4:05 PM to 5:05 PM

By Approach	Northbound Hwy 211				Southbound Hwy 211				Eastbound Dubarko Rd				Westbound Dubarko Rd				Total
	In	Out	Total	Bikes	In	Out	Total	Bikes	In	Out	Total	Bikes	In	Out	Total	Bikes	
	Volume	347	362	709	0	306	273	579	0	108	117	225	1	98	107	205	0
%HV	2.0%				4.6%				0.9%				5.1%				3.1%
PHF	0.89				0.89				0.82				0.72				0.94

Pedestrians

Crosswalk

By Movement	Northbound Hwy 211				Southbound Hwy 211				Eastbound Dubarko Rd				Westbound Dubarko Rd				Total
	L	T	R	Total	L	T	R	Total	L	T	R	Total	L	T	R	Total	
Volume	58	237	52	347	10	285	11	306	11	45	52	108	25	48	25	98	859
%HV	3.4%	1.7%	1.9%	2.0%	0.0%	4.9%	0.0%	4.6%	0.0%	0.0%	1.9%	0.9%	4.0%	2.1%	12.0%	5.1%	3.1%
PHF	0.73	0.91	0.72	0.89	0.63	0.88	0.55	0.89	0.39	0.63	0.65	0.82	0.52	0.75	0.78	0.72	0.94

Rolling Hour Summary

**Rolling Head, Summer
4:00 PM to 6:00 PM**

Interval Start Time	Northbound Hwy 211			Southbound Hwy 211			Eastbound Dubarko Rd			Westbound Dubarko Rd			Interval Total				
	L	T	R	Bikes	L	T	R	Bikes	L	T	R	Bikes	L	T	R	Bikes	
4:00 PM	58	234	46	0	11	268	12	0	11	45	41	1	26	47	24	0	823
4:15 PM	54	240	56	0	9	268	12	0	11	46	50	1	23	41	26	0	836
4:30 PM	49	238	61	0	7	278	12	0	6	42	53	0	23	36	23	0	828
4:45 PM	56	229	61	0	9	280	10	0	6	54	63	0	19	33	20	1	840
5:00 PM	52	205	57	0	7	266	10	0	7	56	58	0	19	30	21	1	788

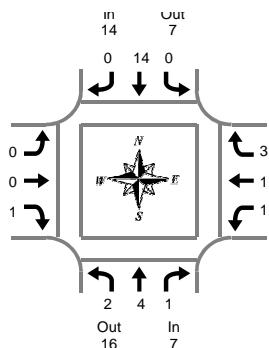
Pedestrians Crosswalk			
North	South	East	West
2	0	1	0
2	0	0	0
1	0	0	0
0	0	0	0
0	0	0	0

Heavy Vehicle Summary

All Traffic Data

Services Inc.

Clay Carney
(503) 833-2740



Hwy 211 & Dubarko Rd

Tuesday, March 19, 2019

4:00 PM to 6:00 PM

Peak Hour Summary
4:05 PM to 5:05 PM

Heavy Vehicle 5-Minute Interval Summary

4:00 PM to 6:00 PM

Interval Start Time	Northbound Hwy 211				Southbound Hwy 211				Eastbound Dubarko Rd				Westbound Dubarko Rd				Interval Total
	L	T	R	Total	L	T	R	Total	L	T	R	Total	L	T	R	Total	
4:00 PM	0	1	0	1	0	4	0	4	0	0	1	1	1	0	0	1	7
4:05 PM	0	0	0	0	0	1	0	1	0	0	0	0	0	0	0	0	1
4:10 PM	2	0	0	2	0	0	0	0	0	0	0	0	0	0	1	1	3
4:15 PM	0	1	0	1	0	4	0	4	0	0	0	0	0	0	0	0	5
4:20 PM	0	0	0	0	0	0	0	0	0	0	0	0	1	1	0	2	2
4:25 PM	0	0	0	0	0	2	0	2	0	0	0	0	0	0	0	0	2
4:30 PM	0	0	0	0	0	2	0	2	0	0	0	0	0	0	1	1	3
4:35 PM	0	1	0	1	0	1	0	1	0	0	0	0	0	0	1	1	3
4:40 PM	0	0	0	0	0	1	0	1	0	0	0	0	0	0	0	0	1
4:45 PM	0	0	0	0	0	0	0	0	0	0	1	1	0	0	0	0	1
4:50 PM	0	1	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
4:55 PM	0	0	1	1	0	1	0	1	0	0	0	0	0	0	0	0	2
5:00 PM	0	1	0	1	0	2	0	2	0	0	0	0	0	0	0	0	3
5:05 PM	0	1	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
5:10 PM	0	0	0	0	0	1	0	1	0	0	0	0	0	0	0	0	1
5:15 PM	1	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
5:20 PM	0	0	0	1	1	0	0	0	0	0	0	0	0	0	0	0	1
5:25 PM	0	2	0	2	0	0	0	0	0	0	0	0	0	0	0	0	2
5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:35 PM	0	0	0	0	0	1	0	1	0	0	0	0	0	0	0	0	1
5:40 PM	0	1	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:50 PM	0	0	0	0	0	1	0	1	0	0	0	0	0	0	0	0	1
5:55 PM	0	0	0	0	0	2	0	2	0	0	1	1	1	0	0	1	4
Total Survey	3	9	2	14	0	23	0	23	0	0	3	3	3	1	3	7	47

Heavy Vehicle 15-Minute Interval Summary

4:00 PM to 6:00 PM

Interval Start Time	Northbound Hwy 211				Southbound Hwy 211				Eastbound Dubarko Rd				Westbound Dubarko Rd				Interval Total
	L	T	R	Total	L	T	R	Total	L	T	R	Total	L	T	R	Total	
4:00 PM	2	1	0	3	0	5	0	5	0	0	1	1	1	0	1	2	11
4:15 PM	0	1	0	1	0	6	0	6	0	0	0	0	1	1	0	2	9
4:30 PM	0	1	0	1	0	4	0	4	0	0	0	0	0	0	2	2	7
4:45 PM	0	1	1	2	0	1	0	1	0	0	1	1	0	0	0	0	4
5:00 PM	0	2	0	2	0	3	0	3	0	0	0	0	0	0	0	0	5
5:15 PM	1	2	1	4	0	0	0	0	0	0	0	0	0	0	0	0	4
5:30 PM	0	1	0	1	0	1	0	1	0	0	0	0	0	0	0	0	2
5:45 PM	0	0	0	0	0	3	0	3	0	0	1	1	1	0	0	1	5
Total Survey	3	9	2	14	0	23	0	23	0	0	3	3	3	1	3	7	47

Heavy Vehicle Peak Hour Summary

4:05 PM to 5:05 PM

By Approach	Northbound Hwy 211			Southbound Hwy 211			Eastbound Dubarko Rd			Westbound Dubarko Rd			Total
	In	Out	Total	In	Out	Total	In	Out	Total	In	Out	Total	
Volume	7	16	23	14	7	21	1	3	4	5	1	6	27
PHF	0.58		0.58			0.25				0.42		0.68	

By Movement	Northbound Hwy 211				Southbound Hwy 211				Eastbound Dubarko Rd				Westbound Dubarko Rd				Total
	L	T	R	Total	L	T	R	Total	L	T	R	Total	L	T	R	Total	
Volume	2	4	1	7	0	14	0	14	0	0	1	1	1	1	3	5	27
PHF	0.25	0.50	0.25	0.58	0.00	0.58	0.00	0.58	0.00	0.00	0.25	0.25	0.25	0.25	0.38	0.42	0.68

Heavy Vehicle Rolling Hour Summary

4:00 PM to 6:00 PM

Interval Start Time	Northbound Hwy 211				Southbound Hwy 211				Eastbound Dubarko Rd				Westbound Dubarko Rd				Interval Total
	L	T	R	Total	L	T	R	Total	L	T	R	Total	L	T	R	Total	
4:00 PM	2	4	1	7	0	16	0	16	0	0	2	2	2	1	3	6	31
4:15 PM	0	5	1	6	0	14	0	14	0	0	1	1	1	1	2	4	25
4:30 PM	1	6	2	9	0	8	0	8	0	0	1	1	0	0	2	2	20
4:45 PM	1	6	2	9	0	5	0	5	0	0	1	1	0	0	0	0	15
5:00 PM	1	5	1	7	0	7	0	7	0	0	1	1	1	0	0	1	16

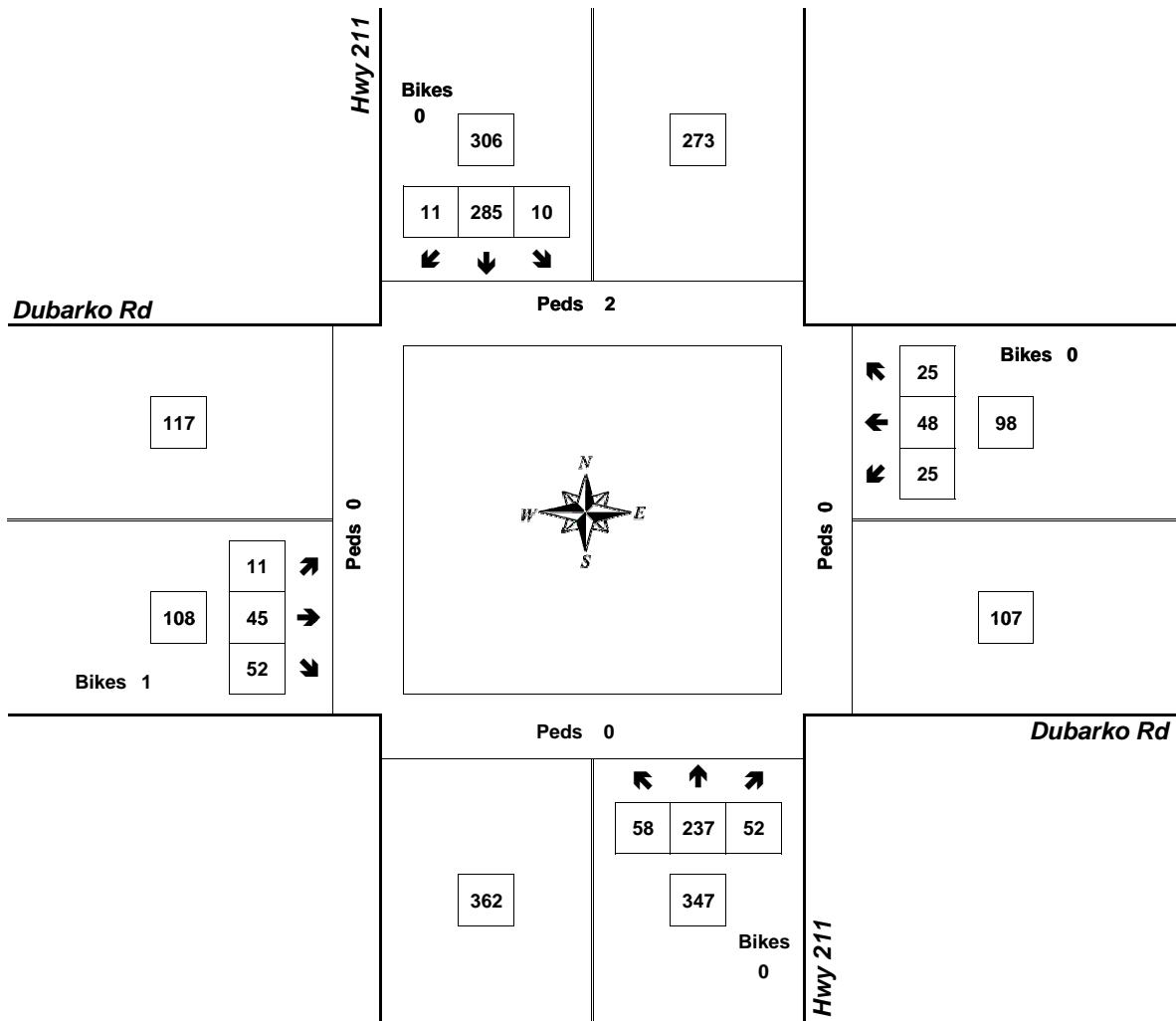
Peak Hour Summary



Clay Carney
(503) 833-2740

Hwy 211 & Dubarko Rd

4:05 PM to 5:05 PM
Tuesday, March 19, 2019

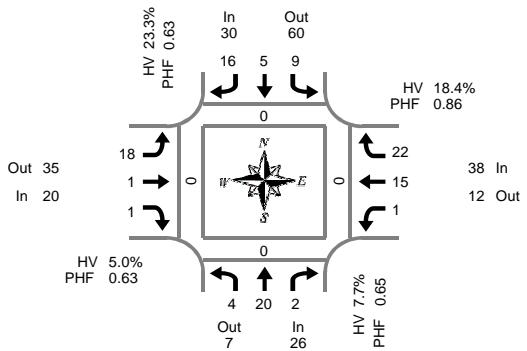


Count Period: 4:00 PM to 6:00 PM

Total Vehicle Summary



Clay Carney
(503) 833-2740



SE Langensand Rd & Dubarko Rd

Wednesday, March 20, 2019

7:00 AM to 9:00 AM

**Peak Hour Summary
7:05 AM to 8:05 AM**

5-Minute Interval Summary

7:00 AM to 9:00 AM

Interval Start Time	Northbound SE Langensand Rd				Southbound SE Langensand Rd				Eastbound Dubarko Rd				Westbound Dubarko Rd				Interval Total
	L	T	R	Bikes	L	T	R	Bikes	L	T	R	Bikes	L	T	R	Bikes	
7:00 AM	1	1	0	0	0	1	1	0	0	0	0	0	0	2	0	0	6
7:05 AM	2	1	0	0	1	0	3	0	1	1	0	0	0	2	3	0	14
7:10 AM	0	0	0	0	0	1	0	0	0	0	0	0	0	1	2	0	5
7:15 AM	0	2	1	0	0	1	1	0	2	0	0	0	0	0	1	0	8
7:20 AM	0	0	0	0	0	0	0	3	0	0	0	1	3	2	0	0	9
7:25 AM	0	0	0	0	0	2	2	3	0	1	0	0	0	1	2	0	11
7:30 AM	0	6	0	0	0	0	3	0	0	0	0	0	0	1	1	0	11
7:35 AM	1	2	0	0	0	0	0	0	0	0	1	0	0	2	0	0	6
7:40 AM	0	0	1	0	2	1	3	0	0	0	0	0	0	2	2	0	11
7:45 AM	0	1	0	0	2	0	1	0	2	0	0	0	0	0	3	0	9
7:50 AM	1	1	0	0	1	0	2	0	3	0	0	0	0	1	3	0	12
7:55 AM	0	4	0	0	0	0	0	0	3	0	0	0	0	0	2	0	9
8:00 AM	0	3	0	0	0	1	0	0	2	0	0	0	0	2	1	0	9
8:05 AM	0	1	0	0	0	1	1	0	3	0	0	0	0	3	1	0	10
8:10 AM	0	1	0	0	1	0	1	0	0	0	0	0	0	0	0	0	3
8:15 AM	0	2	0	0	0	0	0	1	0	3	0	0	0	1	1	0	8
8:20 AM	1	0	0	0	0	0	1	1	0	1	1	0	0	0	0	0	5
8:25 AM	1	0	0	0	0	1	1	0	3	0	1	0	0	0	1	0	8
8:30 AM	0	0	0	0	0	0	0	0	2	2	0	0	1	2	1	0	8
8:35 AM	1	0	0	0	0	1	0	0	0	1	1	0	1	2	0	0	8
8:40 AM	1	1	0	0	0	3	2	0	1	0	0	0	0	1	0	0	9
8:45 AM	1	3	0	0	0	1	2	0	3	0	2	0	1	2	1	0	16
8:50 AM	1	4	1	0	0	1	2	0	2	0	0	0	0	1	3	0	15
8:55 AM	1	2	1	0	0	0	1	0	1	0	0	0	0	2	1	0	9
Total Survey	12	35	4	0	11	14	29	0	38	5	5	0	4	31	31	0	219

15-Minute Interval Summary

7:00 AM to 9:00 AM

Interval Start Time	Northbound SE Langensand Rd				Southbound SE Langensand Rd				Eastbound Dubarko Rd				Westbound Dubarko Rd				Interval Total
	L	T	R	Bikes	L	T	R	Bikes	L	T	R	Bikes	L	T	R	Bikes	
7:00 AM	3	2	0	0	2	1	4	0	2	1	0	0	0	5	5	0	25
7:15 AM	0	2	1	0	2	3	4	0	6	0	0	0	1	4	5	0	28
7:30 AM	1	8	1	0	2	1	6	0	0	0	1	0	0	5	3	0	28
7:45 AM	1	6	0	0	3	0	3	0	8	0	0	0	0	1	8	0	30
8:00 AM	0	5	0	0	1	2	2	0	5	0	0	0	0	5	2	0	22
8:15 AM	2	2	0	0	0	2	3	0	7	1	1	0	0	1	2	0	21
8:30 AM	2	1	0	0	1	3	2	0	4	3	1	0	2	5	1	0	25
8:45 AM	3	9	2	0	0	2	5	0	6	0	2	0	1	5	5	0	40
Total Survey	12	35	4	0	11	14	29	0	38	5	5	0	4	31	31	0	219

Pedestrians Crosswalk			
North	South	East	West
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
1	0	0	0
0	0	1	0
0	0	0	0
0	0	0	0
1	0	1	0

Peak Hour Summary

7:05 AM to 8:05 AM

By Approach	Northbound				Southbound				Eastbound				Westbound				Total
	SE Langensand Rd		SE Langensand Rd		SE Langensand Rd		Dubarko Rd		Dubarko Rd		Dubarko Rd		Dubarko Rd		Dubarko Rd		
In	Out	Total	Bikes	In	Out	Total	Bikes	In	Out	Total	Bikes	In	Out	Total	Bikes		
Volume	26	7	33	0	30	60	90	0	20	35	55	0	38	12	50	0	114
%HV	7.7%			23.3%			5.0%			18.4%			14.9%				
PHF	0.65			0.63			0.63			0.86			0.89				

Pedestrians

Crosswalk

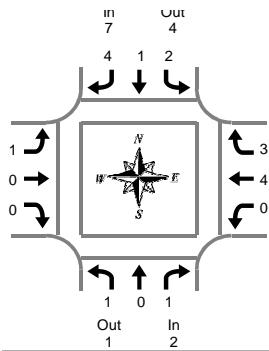
By Movement	Northbound SE Langensand Rd				Southbound SE Langensand Rd				Eastbound Dubarko Rd				Westbound Dubarko Rd				Total
	L	T	R	Total	L	T	R	Total	L	T	R	Total	L	T	R	Total	
Volume	4	20	2	26	9	5	16	30	18	1	1	20	1	15	22	38	114
%HV	25.0%	0.0%	50.0%	7.7%	22.2%	20.0%	25.0%	23.3%	5.6%	0.0%	0.0%	5.0%	0.0%	26.7%	13.6%	18.4%	14.9%
PHF	0.50	0.63	0.50	0.65	0.45	0.42	0.67	0.63	0.56	0.25	0.25	0.63	0.25	0.75	0.63	0.86	0.89

Pedestrians Crosswalk			
North	South	East	West
0	0	0	0
1	0	0	0
1	0	1	0
1	0	1	0
1	0	1	0

Rolling Hour Summary

7:00 AM to 9:00 AM

Heavy Vehicle Summary



SE Langensand Rd & Dubarko Rd

Wednesday, March 20, 2019

7:00 AM to 9:00 AM

Peak Hour Summary
7:05 AM to 8:05 AM

Heavy Vehicle 5-Minute Interval Summary

7:00 AM to 9:00 AM

Interval Start Time	Northbound SE Langensand Rd				Southbound SE Langensand Rd				Eastbound Dubarko Rd				Westbound Dubarko Rd				Interval Total
	L	T	R	Total	L	T	R	Total	L	T	R	Total	L	T	R	Total	
7:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:05 AM	1	0	0	1	0	0	1	1	0	0	0	0	0	0	0	0	2
7:10 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	1	1
7:15 AM	0	0	1	1	0	0	0	0	0	0	0	0	0	0	0	0	1
7:20 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0	2
7:25 AM	0	0	0	0	0	0	1	1	0	0	0	0	0	1	0	1	2
7:30 AM	0	0	0	0	0	0	1	1	0	0	0	0	0	0	0	0	1
7:35 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:40 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:45 AM	0	0	0	1	0	1	2	0	0	0	0	0	0	0	0	0	2
7:50 AM	0	0	0	1	0	0	1	1	0	0	1	1	0	1	1	2	4
7:55 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:00 AM	0	0	0	0	1	0	1	0	0	0	0	0	0	0	1	1	2
8:05 AM	0	1	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1
8:10 AM	0	0	0	0	0	0	1	1	0	0	0	0	0	0	0	0	1
8:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:20 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:25 AM	0	0	0	0	0	0	0	0	1	0	0	1	0	0	0	0	1
8:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	1
8:35 AM	1	0	0	1	0	0	0	0	1	0	0	1	1	0	0	1	3
8:40 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:50 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:55 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total Survey	2	1	1	4	2	1	5	8	3	0	0	3	2	4	3	9	24

Heavy Vehicle 15-Minute Interval Summary

7:00 AM to 9:00 AM

Interval Start Time	Northbound SE Langensand Rd				Southbound SE Langensand Rd				Eastbound Dubarco Rd				Westbound Dubarco Rd				Interval Total
	L	T	R	Total	L	T	R	Total	L	T	R	Total	L	T	R	Total	
7:00 AM	1	0	0	1	0	0	1	1	0	0	0	0	0	0	1	1	3
7:15 AM	0	0	1	1	0	0	1	1	0	0	0	0	0	3	0	3	5
7:30 AM	0	0	0	0	0	0	1	1	0	0	0	0	0	0	0	0	1
7:45 AM	0	0	0	0	2	0	1	3	1	0	0	1	0	1	1	2	6
8:00 AM	0	1	0	1	0	1	1	2	0	0	0	0	0	0	1	1	4
8:15 AM	0	0	0	0	0	0	0	0	1	0	0	1	0	0	0	0	1
8:30 AM	1	0	0	1	0	0	0	0	1	0	0	1	1	2	0	0	4
8:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total Survey	2	1	1	4	2	1	5	8	3	0	0	3	2	4	3	9	24

Heavy Vehicle Peak Hour Summary

**Heavy Vehicle Peak
7:05 AM to 8:05 AM**

By Approach	Northbound			Southbound			Eastbound			Westbound			Total	
	SE Langensand Rd			SE Langensand Rd			Dubarko Rd			Dubarko Rd				
	In	Out	Total	In	Out	Total	In	Out	Total	In	Out	Total		
Volume	2	1	3	7	4	11	1	9	10	7	3	10	17	
PHF	0.25		0.58			0.25			0.58			0.71		

By Movement	Northbound SE Langensand Rd				Southbound SE Langensand Rd				Eastbound Dubarko Rd				Westbound Dubarko Rd				Total	
	L	T	R	Total	L	T	R	Total	L	T	R	Total	L	T	R	Total		
Volume	1	0	1	2	2	1	4	7	1	0	0	0	1	0	4	3	7	17
PHF	0.25	0.00	0.25	0.25	0.25	0.25	0.50	0.58	0.25	0.00	0.00	0.25	0.00	0.33	0.38	0.58	0.71	

Heavy Vehicle Rolling Hour Summary

7:00 AM to 9:00 AM

Interval Start Time	Northbound SE Langensand Rd					Southbound SE Langensand Rd					Eastbound Dubarko Rd					Westbound Dubarko Rd					Interval Total
	L	T	R	Total	L	T	R	Total	L	T	R	Total	L	T	R	Total	L	T	R	Total	
7:00 AM	1	0	1	2	2	0	4	6	1	0	0	1	0	4	2	6					15
7:15 AM	0	1	1	2	2	1	4	7	1	0	0	1	0	4	2	6					16
7:30 AM	0	1	0	1	2	1	3	6	2	0	0	2	0	1	2	3					12
7:45 AM	1	1	0	2	2	1	2	5	3	0	0	3	2	1	2	5					15
8:00 AM	1	1	0	2	0	1	1	2	2	0	0	2	2	0	1	3					9

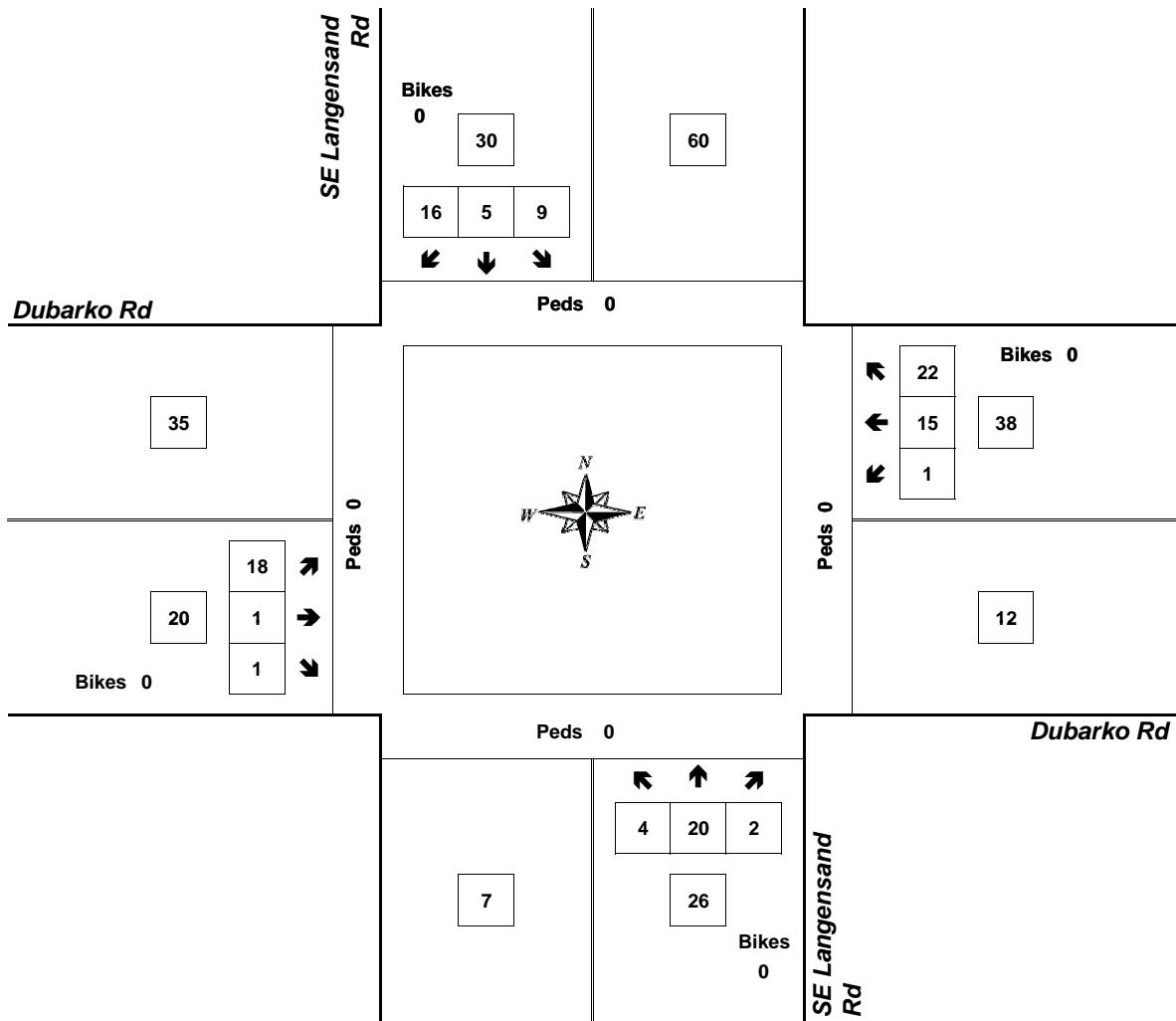
Peak Hour Summary



Clay Carney
(503) 833-2740

SE Langensand Rd & Dubarko Rd

7:05 AM to 8:05 AM
Wednesday, March 20, 2019

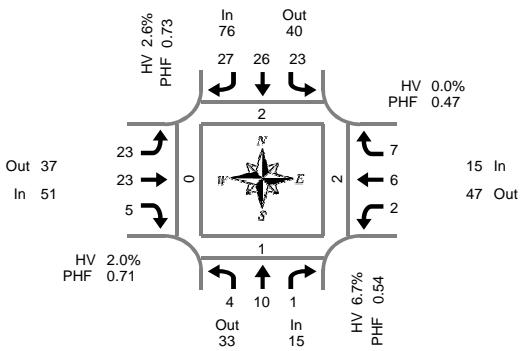


Count Period: 7:00 AM to 9:00 AM

Total Vehicle Summary



Clay Carney
(503) 833-2740



SE Langensand Rd & Dubarko Rd

Tuesday, March 19, 2019

4:00 PM to 6:00 PM

5-Minute Interval Summary

4:00 PM to 6:00 PM

Interval Start Time	Northbound SE Langensand Rd				Southbound SE Langensand Rd				Eastbound Dubarko Rd				Westbound Dubarko Rd				Interval Total	
	L	T	R	Bikes	L	T	R	Bikes	L	T	R	Bikes	L	T	R	Bikes		
4:00 PM	1	2	1	0	1	0	5	0	3	2	0	0	0	0	0	0	15	
4:05 PM	2	1	1	0	0	2	2	0	1	1	0	0	0	2	0	0	12	
4:10 PM	2	0	0	0	2	0	0	0	1	1	2	0	0	0	1	0	9	
4:15 PM	3	2	0	0	1	1	3	0	4	0	0	0	0	1	1	0	16	
4:20 PM	0	0	0	0	1	3	2	0	3	2	0	0	0	2	1	0	14	
4:25 PM	0	3	0	0	1	2	1	0	1	0	0	1	0	3	1	0	12	
4:30 PM	0	2	0	0	0	1	3	2	4	0	0	0	0	2	0	0	12	
4:35 PM	0	1	0	0	0	2	0	0	1	0	1	0	0	0	0	0	5	
4:40 PM	0	2	0	0	0	2	1	0	1	0	1	0	0	1	1	0	9	
4:45 PM	0	2	0	0	0	2	1	0	3	2	1	0	0	1	1	0	13	
4:50 PM	0	0	0	0	0	2	4	0	0	1	2	0	0	0	1	2	0	12
4:55 PM	1	2	0	0	1	2	2	0	2	0	0	0	0	0	0	0	10	
5:00 PM	1	3	0	0	3	3	1	0	1	2	0	0	0	0	0	0	14	
5:05 PM	0	0	0	0	1	4	4	0	4	2	0	0	0	0	0	0	15	
5:10 PM	0	0	0	0	2	2	4	0	1	2	0	0	0	0	0	0	11	
5:15 PM	0	1	0	0	3	3	3	0	3	1	0	0	0	1	1	0	16	
5:20 PM	1	1	0	0	1	1	4	0	0	1	1	0	0	0	2	0	12	
5:25 PM	0	0	0	0	0	3	0	2	0	2	2	0	2	0	1	0	14	
5:30 PM	0	0	0	0	0	1	2	3	0	0	3	0	0	3	0	0	12	
5:35 PM	0	0	0	0	1	3	1	0	0	3	1	0	0	1	1	0	10	
5:40 PM	0	1	1	0	1	1	1	1	2	4	0	0	0	1	0	0	12	
5:45 PM	1	0	0	0	2	3	2	0	4	2	1	0	0	0	0	0	15	
5:50 PM	0	2	0	0	0	2	4	1	0	1	3	1	0	0	2	0	16	
5:55 PM	1	0	0	0	1	0	3	0	1	1	0	0	0	1	0	0	8	
Total Survey	13	25	3	1	32	45	48	3	47	34	10	1	2	20	15	0	294	

Pedestrians Crosswalk			
North	South	East	West
0	0	0	0
0	0	0	0
0	0	1	0
1	0	0	0
0	0	1	0
0	0	0	0
2	0	3	0
0	0	1	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	1	0	0
0	0	1	0
0	0	0	0
1	0	0	0
0	0	0	0
1	0	0	0
0	0	1	0
0	0	0	0
0	0	0	0
5	1	8	0

15-Minute Interval Summary

4:00 PM to 6:00 PM

Interval Start Time	Northbound SE Langensand Rd				Southbound SE Langensand Rd				Eastbound Dubarko Rd				Westbound Dubarko Rd				Interval Total
	L	T	R	Bikes	L	T	R	Bikes	L	T	R	Bikes	L	T	R	Bikes	
4:00 PM	5	3	2	0	3	2	7	0	5	4	2	0	0	2	1	0	36
4:15 PM	3	5	0	0	3	6	6	0	8	2	0	1	0	6	3	0	42
4:30 PM	0	5	0	0	0	5	4	2	6	0	2	0	0	3	1	0	26
4:45 PM	1	4	0	0	3	8	3	0	6	4	1	0	0	2	3	0	35
5:00 PM	1	3	0	0	6	9	9	0	6	6	0	0	0	0	0	0	40
5:15 PM	1	2	0	0	7	4	9	0	5	4	3	0	2	1	4	0	42
5:30 PM	0	1	1	1	5	4	4	1	5	8	0	0	0	5	1	0	34
5:45 PM	2	2	0	0	5	7	6	0	6	6	2	0	0	1	2	0	39
Total Survey	13	25	3	1	32	45	48	3	47	34	10	1	2	20	15	0	294

Pedestrians Crosswalk				
North	South	East	West	
0	0	1	1	0
1	0	1	0	0
2	0	4	0	0
0	0	0	0	0
0	1	0	0	0
1	0	1	0	0
1	0	1	0	0
0	0	0	0	0
5	1	8	0	0

Peak Hour Summary

4:55 PM to 5:55 PM

By Approach	Northbound				Southbound				Eastbound				Westbound				Total	
	SE Langensand Rd				SE Langensand Rd				Dubarko Rd				Dubarko Rd					
	In	Out	Total	Bikes	In	Out	Total	Bikes	In	Out	Total	Bikes	In	Out	Total	Bikes		
Volume	15	33	48	1	76	40	116	1	51	37	88	0	15	47	62	0	157	
%HV	6.7%				2.6%				2.0%				0.0%				2.5%	
PHF	0.54				0.73				0.71				0.47				0.91	

Pedestrians
Crosswalk

By Movement	Northbound				Southbound				Eastbound				Westbound				Total
	SE Langensand Rd		SE Langensand Rd		SE Langensand Rd		Dubarko Rd		Dubarko Rd		Dubarko Rd		Dubarko Rd		Dubarko Rd		
	L	T	R	Total	L	T	R	Total	L	T	R	Total	L	T	R	Total	
Volume	4	10	1	15	23	26	27	76	23	23	5	51	2	6	7	15	157
%HV	25.0%	0.0%	0.0%	6.7%	0.0%	0.0%	7.4%	2.6%	4.3%	0.0%	0.0%	2.0%	0.0%	0.0%	0.0%	0.0%	2.5%
PHF	0.50	0.50	0.25	0.54	0.82	0.72	0.61	0.73	0.64	0.64	0.42	0.71	0.25	0.30	0.44	0.47	0.91

Rolling Hour Summary

4:00 PM to 6:00 PM

Interval Start Time	Northbound SE Langensand Rd				Southbound SE Langensand Rd				Eastbound Dubarko Rd				Westbound Dubarko Rd				Interval Total
	L	T	R	Bikes	L	T	R	Bikes	L	T	R	Bikes	L	T	R	Bikes	
4:00 PM	9	17	2	0	9	21	20	2	25	10	5	1	0	13	8	0	139
4:15 PM	5	17	0	0	12	28	22	2	26	12	3	1	0	11	7	0	143
4:30 PM	3	14	0	0	16	26	25	2	23	14	6	0	2	6	8	0	143
4:45 PM	3	10	1	1	21	25	25	1	22	22	4	0	2	8	8	0	151
5:00 PM	4	8	1	1	23	24	28	1	22	24	5	0	2	7	7	0	155

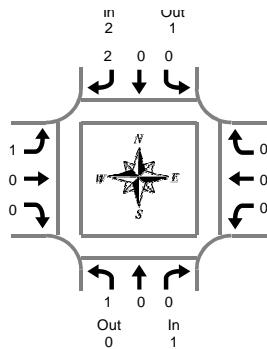
Pedestrians Crosswalk			
North	South	East	West
3	0	6	0
3	1	5	0
3	1	5	0
2	1	2	0
2	1	2	0

Heavy Vehicle Summary

All Traffic Data

Services Inc.

Clay Carney
(503) 833-2740



SE Langensand Rd & Dubarko Rd

Tuesday, March 19, 2019

4:00 PM to 6:00 PM

Peak Hour Summary
4:55 PM to 5:55 PM

Heavy Vehicle 5-Minute Interval Summary

4:00 PM to 6:00 PM

Interval Start Time	Northbound SE Langensand Rd				Southbound SE Langensand Rd				Eastbound Dubarko Rd				Westbound Dubarko Rd				Interval Total
	L	T	R	Total	L	T	R	Total	L	T	R	Total	L	T	R	Total	
4:00 PM	0	0	1	1	0	0	1	1	0	0	0	0	0	0	0	0	2
4:05 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:10 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:15 PM	0	0	0	0	0	0	0	0	1	0	0	1	0	0	0	0	1
4:20 PM	0	0	0	0	1	1	0	2	0	0	0	0	0	0	0	0	2
4:25 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	1
4:35 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:40 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:50 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:55 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:05 PM	0	0	0	0	0	0	0	1	1	0	0	0	0	0	0	0	1
5:10 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:20 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:25 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:35 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:40 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:45 PM	1	0	0	1	0	0	0	0	0	1	0	0	1	0	0	0	2
5:50 PM	0	0	0	0	0	0	0	1	1	0	0	0	0	0	0	0	1
5:55 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total Survey	1	0	1	2	1	1	3	5	2	0	0	2	0	1	0	1	10

Heavy Vehicle 15-Minute Interval Summary

4:00 PM to 6:00 PM

Interval Start Time	Northbound SE Langensand Rd				Southbound SE Langensand Rd				Eastbound Dubarko Rd				Westbound Dubarko Rd				Interval Total
	L	T	R	Total	L	T	R	Total	L	T	R	Total	L	T	R	Total	
4:00 PM	0	0	1	1	0	0	1	1	0	0	0	0	0	0	0	0	2
4:15 PM	0	0	0	0	1	1	0	2	1	0	0	1	0	0	0	0	3
4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	1	1
4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:00 PM	0	0	0	0	0	0	0	1	1	0	0	0	0	0	0	0	1
5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:45 PM	1	0	0	1	0	0	1	1	0	0	0	1	0	0	0	0	3
Total Survey	1	0	1	2	1	1	3	5	2	0	0	2	0	1	0	1	10

Heavy Vehicle Peak Hour Summary

4:55 PM to 5:55 PM

By Approach	Northbound SE Langensand Rd			Southbound SE Langensand Rd			Eastbound Dubarko Rd			Westbound Dubarko Rd			Total
	In	Out	Total	In	Out	Total	In	Out	Total	In	Out	Total	
Volume	1	0	1	2	1	3	1	3	4	0	0	0	4
PHF	0.25			0.50			0.25			0.00			0.33

By Movement	Northbound SE Langensand Rd				Southbound SE Langensand Rd				Eastbound Dubarko Rd				Westbound Dubarko Rd				Total
	L	T	R	Total	L	T	R	Total	L	T	R	Total	L	T	R	Total	
Volume	1	0	0	1	0	0	2	2	1	0	0	1	0	0	0	0	4
PHF	0.25	0.00	0.00	0.25	0.00	0.00	0.50	0.50	0.25	0.00	0.00	0.25	0.00	0.00	0.00	0.00	0.33

Heavy Vehicle Rolling Hour Summary

4:00 PM to 6:00 PM

Interval Start Time	Northbound SE Langensand Rd				Southbound SE Langensand Rd				Eastbound Dubarko Rd				Westbound Dubarko Rd				Interval Total
	L	T	R	Total	L	T	R	Total	L	T	R	Total	L	T	R	Total	
4:00 PM	0	0	1	1	1	1	1	3	1	0	0	1	0	1	0	1	6
4:15 PM	0	0	0	0	1	1	1	3	1	0	0	1	0	1	0	1	5
4:30 PM	0	0	0	0	0	0	0	0	1	0	0	0	0	1	0	1	2
4:45 PM	0	0	0	0	0	0	0	1	1	0	0	0	0	0	0	0	1
5:00 PM	1	0	0	1	0	0	2	2	1	0	0	1	0	0	0	0	4

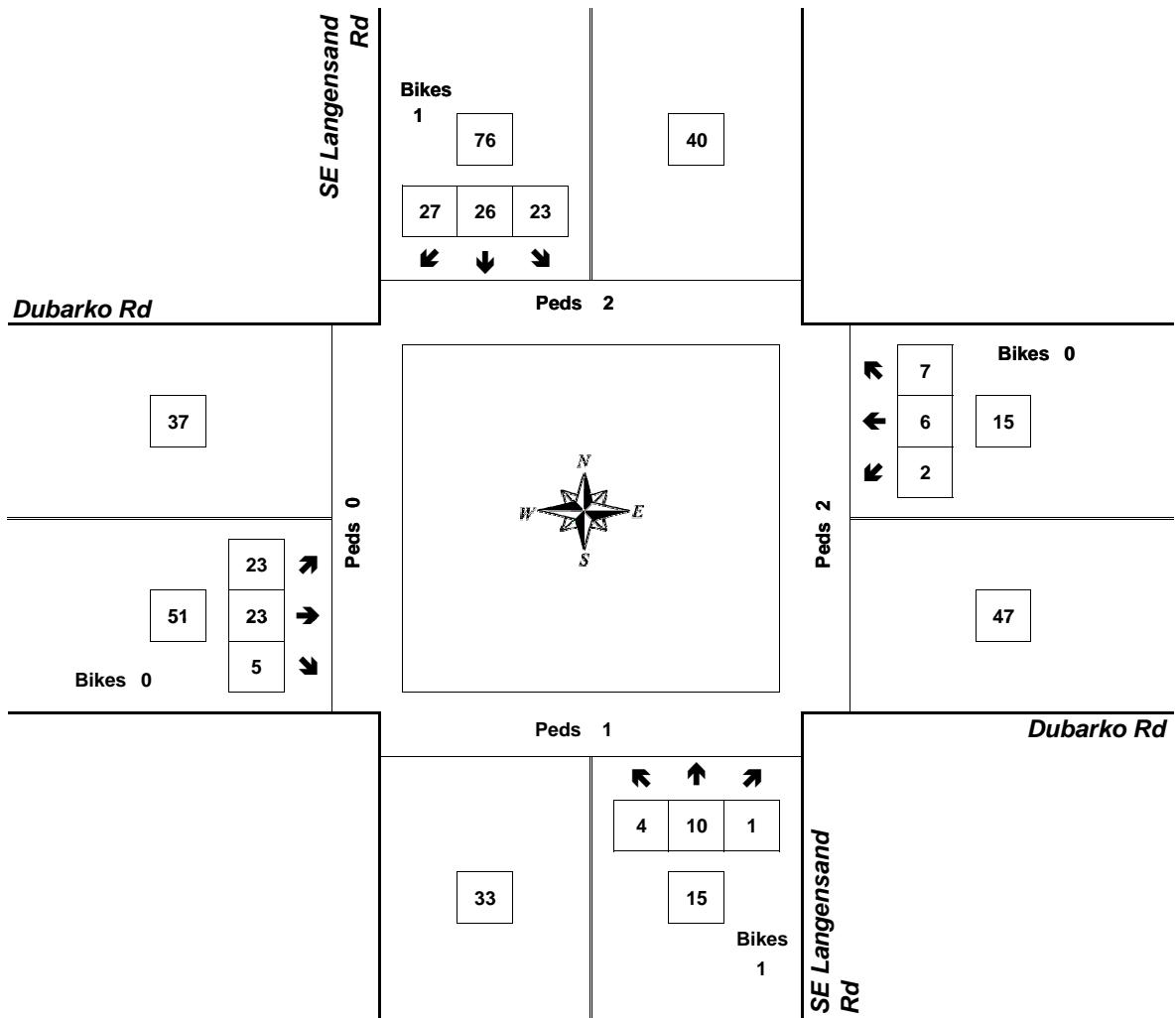
Peak Hour Summary



Clay Carney
(503) 833-2740

SE Langensand Rd & Dubarko Rd

4:55 PM to 5:55 PM
Tuesday, March 19, 2019



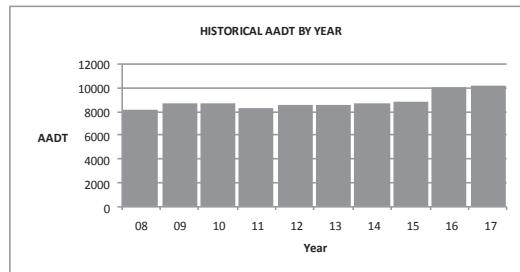
Approach	PHF	HV%	Volume
EB	0.71	2.0%	51
WB	0.47	0.0%	15
NB	0.54	6.7%	15
SB	0.73	2.6%	76
Intersection	0.91	2.5%	157

Count Period: 4:00 PM to 6:00 PM

Location:	US26; MP 46.38; MT. HOOD HIGHWAY NO. 26; 0.30 mile east of Camp Creek Rd (USFS 28)	Site Name:	Rhododendron (03-006)
		Installed:	August, 1995

HISTORICAL TRAFFIC DATA

Year	AADT	Percent of AADT				
		Max Day	Max Hour	10TH Hour	20TH Hour	30TH Hour
2008	8162	233	22.9	20.1	19.1	18.2
2009	8737	197	22.3	19.6	18.4	17.8
2010	8714	207	21.6	19.8	18.9	18.5
2011	8330	214	24.7	20.0	18.6	18.1
2012	8480	227	24.0	21.0	20.2	19.4
2013	8527	213	23.4	21.1	20.3	19.1
2014	8652	216	23.2	21.1	20.3	19.2
2015	8861	242	21.4	20.3	19.4	18.7
2016	10071	208	22.9	19.6	18.8	17.9
2017	10223	200	19.9	19.1	18.1	17.5



2017 TRAFFIC DATA

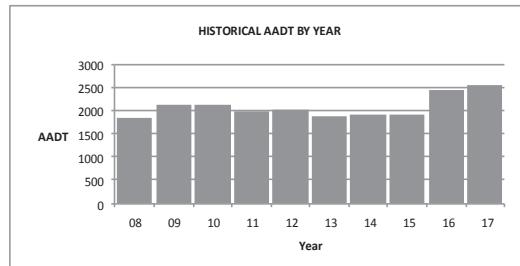
	Average Weekday Traffic	Percent of AADT	Average Daily Traffic	Percent of AADT
January	6744	66	9080	89
February	6533	64	9496	93
March	6763	66	9337	91
April	6166	60	8675	85
May	7675	75	9598	94
June	8568	84	10695	105
July	11291	110	13874	136
August	11738	115	13623	133
September	11300	111	12734	125
October	6589	64	8087	79
November	5493	54	7313	72
December	8753	86	10161	99

For Vehicle Classification data near your project, please go to the following web page:
https://www.oregon.gov/ODOT/Data/Documents/TVT_2017.xlsx

Location:	OR35; MP 57.79; MT. HOOD HIGHWAY NO. 26; 0.02 mile east of Warm Springs Highway No. 53 (US26)	Site Name:	Mt. Hood Meadows (03-007)
		Installed:	September, 1995

HISTORICAL TRAFFIC DATA

Year	AADT	Percent of AADT				
		Max Day	Max Hour	10TH Hour	20TH Hour	30TH Hour
2008	1854	398	56.8	44.2	39.9	36.1
2009	2130	***	***	***	***	***
2010	2145	374	49.2	39.5	34.8	33.2
2011	1976	476	79.2	49.1	45.0	39.1
2012	2023	452	65.4	43.4	40.3	37.7
2013	1868	427	68.1	48.7	42.0	37.1
2014	1908	400	60.0	41.9	37.4	33.6
2015	1931	393	50.4	38.6	34.4	32.6
2016	2455	366	55.9	38.3	33.1	31.2
2017	2565	340	52.1	37.7	32.5	31.3



2017 TRAFFIC DATA

	Average Weekday Traffic	Percent of AADT	Average Daily Traffic	Percent of AADT
January	2449	95	3616	141
February	1978	77	3362	131
March	1781	69	2833	110
April	1116	44	2050	80
May	1202	47	1609	63
June	1794	70	2070	81
July	2405	94	2837	111
August	2302	90	2614	102
September	3956	154	3993	156
October	1387	54	1614	63
November	768	30	1156	45
December	2499	97	2966	116

For Vehicle Classification data near your project, please go to the following web page:
https://www.oregon.gov/ODOT/Data/Documents/TVT_2017.xlsx

Site id	HWY	MP	DIR	HS	Description	2017	2018	2019	2039	RSQ
1778	026	22.72	1		0.02 mile northwest of SE 362nd Drive, west city limits of Sandy		33700		47300	MODEL
1779	026	23.85	1		0.02 mile west of Bluff Road		33300		47100	MODEL
1780	026	23.89	1		0.02 mile east of Bluff Road		15700		22400	MODEL
1781	026	24.02	1		0.02 mile west of Beers Avenue		16200		23100	MODEL
1782	026	24.35	1		0.05 mile west of Eagle Creek-Sandy Highway (OR211)		16000		23400	MODEL
1783	026	24.42	1		0.02 mile east of Eagle Creek-Sandy Highway (OR211)		12400		17700	MODEL
1784	026	24.59	1		0.02 mile west of Ten Eyck Road		12500		17800	MODEL
1785	026	23.89	2		0.02 mile east of Bluff Road		16600		23300	MODEL
1786	026	24.04	2		0.02 mile west of Beers Avenue		18300		25600	MODEL
1787	026	24.36	2		0.05 mile west of Eagle Creek-Sandy Highway (OR211)		15900		22700	MODEL
1788	026	24.40	2		0.02 mile east of Eagle Creek-Sandy Highway (OR211)		13700		19200	MODEL
1789	026	24.61	2		0.02 mile west of Ten Eyck Road		12600		17600	MODEL
1790	026	25.10	1		0.02 mile west of Langensand Road		20700		29200	MODEL
1791	026	25.66	1		0.10 mile east of Vista Loop Drive		23500		32900	MODEL

Site id	HWY	MP	DIR	HS	Description	2017	2018	2019	2039	RSQ
3563	172	-0.13	1		0.10 mile east of Clackamas Highway (OR224)			6000	9400	MODEL
3564	172	1.45	1		0.10 mile southwest of Judd Road		7100	11200	MODEL	
3565	172	1.65	1		0.10 mile northeast of Judd Road		7400	11400	MODEL	
3566	172	3.65	1		0.05 mile west of 362nd Drive		8000	12200	MODEL	
3567	172	3.75	1		0.05 mile east of 362nd Drive		5900	8800	MODEL	
3568	172	5.07	1		0.10 mile west of Bornstedt Road		4600	7600	MODEL	
3569	172	5.29	1		0.10 mile south of Dubarko Road		6300	10300	MODEL	
3570	172	5.50	1		0.11 mile north of Dubarko Road		5700	9200	MODEL	
					0.05 mile south of Mt. Hood Highway (US26-EB)					
3571	172	5.83	1			7500		12100	MODEL	
3572	172	5.92	1		0.02 mile south of Mt. Hood Highway (US26-WB)		4400	7100	MODEL	

HCM Signalized Intersection Capacity Analysis

1: Wolf Drive/Ten Eyck Road & Highway 26

05/31/2021

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑↑↓		↑	↑↑	↑	↔	↔			↔	
Traffic Volume (vph)	58	740	37	4	1083	10	136	11	3	16	4	148
Future Volume (vph)	58	740	37	4	1083	10	136	11	3	16	4	148
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Total Lost time (s)	4.5	4.5		4.5	4.5	4.5					4.5	
Lane Util. Factor	1.00	0.95		1.00	0.95	1.00					1.00	
Frpb, ped/bikes	1.00	1.00		1.00	1.00	0.97					0.98	
Flpb, ped/bikes	1.00	1.00		1.00	1.00	1.00					1.00	
Fr _t	1.00	0.99		1.00	1.00	0.85					0.88	
Flt Protected	0.95	1.00		0.95	1.00	1.00					1.00	
Satd. Flow (prot)	1484	2945		1568	3137	1356					1464	
Flt Permitted	0.95	1.00		0.95	1.00	1.00					0.97	
Satd. Flow (perm)	1484	2945		1568	3137	1356					1423	
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Adj. Flow (vph)	62	787	39	4	1152	11	145	12	3	17	4	157
RTOR Reduction (vph)	0	3	0	0	0	5	0	1	0	0	109	0
Lane Group Flow (vph)	62	823	0	4	1152	6	0	159	0	0	69	0
Confl. Peds. (#/hr)						4					4	
Confl. Bikes (#/hr)			2			1						
Heavy Vehicles (%)	12%	12%	12%	6%	6%	6%	6%	6%	6%	3%	3%	3%
Turn Type	Prot	NA		Prot	NA	Perm	Perm	NA		Perm	NA	
Protected Phases	5	2		1	6			4			8	
Permitted Phases						6	4				8	
Actuated Green, G (s)	8.4	68.9		1.1	61.6	61.6		36.5			36.5	
Effective Green, g (s)	8.4	68.9		1.1	61.6	61.6		36.5			36.5	
Actuated g/C Ratio	0.07	0.57		0.01	0.51	0.51		0.30			0.30	
Clearance Time (s)	4.5	4.5		4.5	4.5	4.5		4.5			4.5	
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0		3.0			3.0	
Lane Grp Cap (vph)	103	1690		14	1610	696		274			432	
v/s Ratio Prot	c0.04	0.28		0.00	c0.37							
v/s Ratio Perm						0.00		c0.18			0.05	
v/c Ratio	0.60	0.49		0.29	0.72	0.01		0.58			0.16	
Uniform Delay, d1	54.2	15.1		59.1	22.5	14.3		35.3			30.5	
Progression Factor	1.00	1.00		1.00	1.00	1.00		1.00			1.00	
Incremental Delay, d2	9.5	1.0		10.9	2.8	0.0		8.7			0.2	
Delay (s)	63.7	16.1		70.0	25.2	14.3		44.0			30.7	
Level of Service	E	B		E	C	B		D			C	
Approach Delay (s)		19.4			25.3			44.0			30.7	
Approach LOS		B			C			D			C	
Intersection Summary												
HCM 2000 Control Delay		24.8			HCM 2000 Level of Service				C			
HCM 2000 Volume to Capacity ratio		0.66										
Actuated Cycle Length (s)		120.0			Sum of lost time (s)				13.5			
Intersection Capacity Utilization		72.6%			ICU Level of Service				C			
Analysis Period (min)		15										
c Critical Lane Group												

HCM 6th Signalized Intersection Summary
1: Wolf Drive/Ten Eyck Road & Highway 26

05/31/2021

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑↑		↑	↑↑	↑	↔	↔		↔	↔	↔
Traffic Volume (veh/h)	58	740	37	4	1083	10	136	11	3	16	4	148
Future Volume (veh/h)	58	740	37	4	1083	10	136	11	3	16	4	148
Initial Q (Q _b), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00			1.00		0.98	1.00			1.00	1.00	1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No			No			No		
Adj Sat Flow, veh/h/ln	1586	1586	1586	1668	1668	1668	1668	1668	1668	1709	1709	1709
Adj Flow Rate, veh/h	62	787	39	4	1152	11	145	12	3	17	4	157
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Percent Heavy Veh, %	12	12	12	6	6	6	6	6	6	3	3	3
Cap, veh/h	76	1687	84	8	1689	735	323	25	6	57	29	396
Arrive On Green	0.05	0.58	0.58	0.01	0.53	0.53	0.30	0.30	0.30	0.30	0.30	0.30
Sat Flow, veh/h	1511	2919	145	1589	3169	1379	874	82	18	80	94	1300
Grp Volume(v), veh/h	62	406	420	4	1152	11	160	0	0	178	0	0
Grp Sat Flow(s), veh/h/ln	1511	1507	1556	1589	1585	1379	974	0	0	1474	0	0
Q Serve(g_s), s	4.9	18.7	18.7	0.3	32.0	0.5	8.1	0.0	0.0	0.0	0.0	0.0
Cycle Q Clear(g_c), s	4.9	18.7	18.7	0.3	32.0	0.5	19.6	0.0	0.0	11.5	0.0	0.0
Prop In Lane	1.00			1.00		1.00	0.91			0.02	0.10	0.88
Lane Grp Cap(c), veh/h	76	871	900	8	1689	735	353	0	0	481	0	0
V/C Ratio(X)	0.82	0.47	0.47	0.48	0.68	0.01	0.45	0.00	0.00	0.37	0.00	0.00
Avail Cap(c_a), veh/h	145	871	900	73	1689	735	353	0	0	481	0	0
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	0.00	1.00	0.00	0.00
Uniform Delay (d), s/veh	56.4	14.6	14.6	59.5	20.6	13.2	37.0	0.0	0.0	33.1	0.0	0.0
Incr Delay (d2), s/veh	18.5	1.8	1.7	38.0	2.2	0.0	4.1	0.0	0.0	0.5	0.0	0.0
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln	2.3	6.8	7.0	0.2	12.2	0.2	4.5	0.0	0.0	4.2	0.0	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	74.9	16.4	16.4	97.5	22.8	13.2	41.1	0.0	0.0	33.5	0.0	0.0
LnGrp LOS	E	B	B	F	C	B	D	A	A	C	A	A
Approach Vol, veh/h					1167				160			178
Approach Delay, s/veh	20.5				23.0				41.1			33.5
Approach LOS		C			C			D		C		
Timer - Assigned Phs	1	2		4	5	6		8				
Phs Duration (G+Y+R _c), s	5.1	73.9		41.0	10.5	68.5		41.0				
Change Period (Y+R _c), s	4.5	4.5		4.5	4.5	4.5		4.5				
Max Green Setting (Gmax), s	5.5	64.5		36.5	11.5	58.5		36.5				
Max Q Clear Time (g_c+l1), s	2.3	20.7		21.6	6.9	34.0		13.5				
Green Ext Time (p_c), s	0.0	6.8		0.8	0.0	10.0		1.1				
Intersection Summary												
HCM 6th Ctrl Delay				24.0								
HCM 6th LOS				C								

Intersection						
Int Delay, s/veh	2.2					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑	↗	↖	↑↑	↖	↗
Traffic Vol, veh/h	750	35	16	967	66	16
Future Vol, veh/h	750	35	16	967	66	16
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	160	215	-	120	0
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	94	94	94	94	94	94
Heavy Vehicles, %	13	13	7	7	4	4
Mvmt Flow	798	37	17	1029	70	17
Major/Minor	Major1	Major2	Minor1			
Conflicting Flow All	0	0	835	0	1347	399
Stage 1	-	-	-	-	798	-
Stage 2	-	-	-	-	549	-
Critical Hdwy	-	-	4.24	-	6.88	6.98
Critical Hdwy Stg 1	-	-	-	-	5.88	-
Critical Hdwy Stg 2	-	-	-	-	5.88	-
Follow-up Hdwy	-	-	2.27	-	3.54	3.34
Pot Cap-1 Maneuver	-	-	763	-	140	595
Stage 1	-	-	-	-	399	-
Stage 2	-	-	-	-	537	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	763	-	137	595
Mov Cap-2 Maneuver	-	-	-	-	137	-
Stage 1	-	-	-	-	399	-
Stage 2	-	-	-	-	525	-
Approach	EB	WB	NB			
HCM Control Delay, s	0	0.2	47.3			
HCM LOS			E			
Minor Lane/Major Mvmt	NBLn1	NBLn2	EBT	EBR	WBL	WBT
Capacity (veh/h)	137	595	-	-	763	-
HCM Lane V/C Ratio	0.513	0.029	-	-	0.022	-
HCM Control Delay (s)	56.1	11.2	-	-	9.8	-
HCM Lane LOS	F	B	-	-	A	-
HCM 95th %tile Q(veh)	2.4	0.1	-	-	0.1	-

Intersection

Int Delay, s/veh 4.4

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	8	8	33	54	38	33	44	283	14	4	167	2
Future Vol, veh/h	8	8	33	54	38	33	44	283	14	4	167	2
Conflicting Peds, #/hr	2	0	0	0	0	2	0	0	0	2	0	2
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None									
Storage Length	-	-	90	-	-	125	-	-	-	-	-	330
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	1	1	1	5	5	5	2	2	2	5	5	5
Mvmt Flow	9	9	37	60	42	37	49	314	16	4	186	2

Major/Minor	Minor2	Minor1			Major1			Major2				
Conflicting Flow All	658	626	188	640	620	326	190	0	0	332	0	0
Stage 1	196	196	-	422	422	-	-	-	-	-	-	-
Stage 2	462	430	-	218	198	-	-	-	-	-	-	-
Critical Hdwy	7.11	6.51	6.21	7.15	6.55	6.25	4.12	-	-	4.15	-	-
Critical Hdwy Stg 1	6.11	5.51	-	6.15	5.55	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.11	5.51	-	6.15	5.55	-	-	-	-	-	-	-
Follow-up Hdwy	3.509	4.009	3.309	3.545	4.045	3.345	2.218	-	-	2.245	-	-
Pot Cap-1 Maneuver	379	402	857	384	400	708	1384	-	-	1211	-	-
Stage 1	808	740	-	604	583	-	-	-	-	-	-	-
Stage 2	582	585	-	778	732	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	316	381	855	347	379	705	1381	-	-	1209	-	-
Mov Cap-2 Maneuver	316	381	-	347	379	-	-	-	-	-	-	-
Stage 1	771	736	-	576	556	-	-	-	-	-	-	-
Stage 2	486	558	-	733	728	-	-	-	-	-	-	-

Approach	EB	WB			NB			SB		
HCM Control Delay, s	11.6	16.7			1			0.2		
HCM LOS	B	C								
Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	EBLn2	WBLn1	WBLn2	SBL	SBT	SBR
Capacity (veh/h)	1381	-	-	345	855	360	705	1209	-	-
HCM Lane V/C Ratio	0.035	-	-	0.052	0.043	0.284	0.052	0.004	-	-
HCM Control Delay (s)	7.7	0	-	16	9.4	18.9	10.4	8	0	-
HCM Lane LOS	A	A	-	C	A	C	B	A	A	-
HCM 95th %tile Q(veh)	0.1	-	-	0.2	0.1	1.1	0.2	0	-	-

Intersection

Int Delay, s/veh 5.5

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑		↔		↔	↔	↔		↔		↔
Traffic Vol, veh/h	19	1	1	1	15	23	4	21	2	9	5	17
Future Vol, veh/h	19	1	1	1	15	23	4	21	2	9	5	17
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None									
Storage Length	115	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	89	89	89	89	89	89	89	89	89	89	89	89
Heavy Vehicles, %	5	5	5	18	18	18	8	8	8	23	23	23
Mvmt Flow	21	1	1	1	17	26	4	24	2	10	6	19

Major/Minor	Minor2	Minor1			Major1			Major2				
Conflicting Flow All	91	70	16	70	78	25	25	0	0	26	0	0
Stage 1	36	36	-	33	33	-	-	-	-	-	-	-
Stage 2	55	34	-	37	45	-	-	-	-	-	-	-
Critical Hdwy	7.15	6.55	6.25	7.28	6.68	6.38	4.18	-	-	4.33	-	-
Critical Hdwy Stg 1	6.15	5.55	-	6.28	5.68	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.15	5.55	-	6.28	5.68	-	-	-	-	-	-	-
Follow-up Hdwy	3.545	4.045	3.345	3.662	4.162	3.462	2.272	-	-	2.407	-	-
Pot Cap-1 Maneuver	886	815	1055	884	783	1007	1551	-	-	1462	-	-
Stage 1	972	859	-	944	837	-	-	-	-	-	-	-
Stage 2	950	861	-	939	827	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	843	807	1055	875	775	1007	1551	-	-	1462	-	-
Mov Cap-2 Maneuver	843	807	-	875	775	-	-	-	-	-	-	-
Stage 1	969	853	-	941	834	-	-	-	-	-	-	-
Stage 2	904	858	-	930	821	-	-	-	-	-	-	-

Approach	EB	WB			NB			SB				
HCM Control Delay, s	9.4	9.2			1.1			2.2				
HCM LOS	A	A			A			A				
Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	EBLn2	WBLn1	SBL	SBT	SBR			
Capacity (veh/h)	1551	-	-	843	914	900	1462	-	-			
HCM Lane V/C Ratio	0.003	-	-	0.025	0.002	0.049	0.007	-	-			
HCM Control Delay (s)	7.3	0	-	9.4	8.9	9.2	7.5	0	-			
HCM Lane LOS	A	A	-	A	A	A	A	A	A			
HCM 95th %tile Q(veh)	0	-	-	0.1	0	0.2	0	-	-			

HCM Signalized Intersection Capacity Analysis

1: Wolf Drive/Ten Eyck Road & Highway 26

05/31/2021

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑↑		↑	↑↑	↑		↔			↔	
Traffic Volume (vph)	155	1152	155	8	1041	22	133	16	14	38	14	115
Future Volume (vph)	155	1152	155	8	1041	22	133	16	14	38	14	115
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Total Lost time (s)	4.5	4.5		4.5	4.5	4.5		4.5			4.5	
Lane Util. Factor	1.00	0.95		1.00	0.95	1.00		1.00			1.00	
Frpb, ped/bikes	1.00	1.00		1.00	1.00	0.97		1.00			0.99	
Flpb, ped/bikes	1.00	1.00		1.00	1.00	1.00		1.00			1.00	
Fr _t	1.00	0.98		1.00	1.00	0.85		0.99			0.91	
Flt Protected	0.95	1.00		0.95	1.00	1.00		0.96			0.99	
Satd. Flow (prot)	1614	3163		1554	3107	1343		1645			1461	
Flt Permitted	0.95	1.00		0.95	1.00	1.00		0.57			0.91	
Satd. Flow (perm)	1614	3163		1554	3107	1343		983			1340	
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	163	1213	163	8	1096	23	140	17	15	40	15	121
RTOR Reduction (vph)	0	8	0	0	0	12	0	3	0	0	66	0
Lane Group Flow (vph)	163	1368	0	8	1096	11	0	169	0	0	110	0
Confl. Peds. (#/hr)						4					4	
Confl. Bikes (#/hr)			2			1						
Heavy Vehicles (%)	3%	3%	3%	7%	7%	7%	1%	1%	1%	6%	6%	6%
Turn Type	Prot	NA		Prot	NA	Perm	Perm	NA		Perm	NA	
Protected Phases	5	2		1	6			4			8	
Permitted Phases						6	4				8	
Actuated Green, G (s)	16.3	73.0		1.0	57.7	57.7		32.5			32.5	
Effective Green, g (s)	16.3	73.0		1.0	57.7	57.7		32.5			32.5	
Actuated g/C Ratio	0.14	0.61		0.01	0.48	0.48		0.27			0.27	
Clearance Time (s)	4.5	4.5		4.5	4.5	4.5		4.5			4.5	
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0		3.0			3.0	
Lane Grp Cap (vph)	219	1924		12	1493	645		266			362	
v/s Ratio Prot	c0.10	c0.43		0.01	0.35							
v/s Ratio Perm						0.01		c0.17			0.08	
v/c Ratio	0.74	0.71		0.67	0.73	0.02		0.64			0.30	
Uniform Delay, d1	49.8	16.2		59.3	25.0	16.3		38.5			34.8	
Progression Factor	1.00	1.00		1.00	1.00	1.00		1.00			1.00	
Incremental Delay, d2	12.8	2.3		89.5	3.2	0.0		11.1			0.5	
Delay (s)	62.7	18.5		148.8	28.2	16.4		49.6			35.2	
Level of Service	E	B		F	C	B		D			D	
Approach Delay (s)		23.2			28.8			49.6			35.2	
Approach LOS		C			C			D			D	
Intersection Summary												
HCM 2000 Control Delay		27.5									C	
HCM 2000 Volume to Capacity ratio		0.71										
Actuated Cycle Length (s)		120.0									13.5	
Intersection Capacity Utilization		80.5%									D	
Analysis Period (min)		15										
c Critical Lane Group												

HCM 6th Signalized Intersection Summary
1: Wolf Drive/Ten Eyck Road & Highway 26

05/31/2021

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑↑		↑	↑↑	↑	↔	↔		↔	↔	↔
Traffic Volume (veh/h)	155	1152	155	8	1041	22	133	16	14	38	14	115
Future Volume (veh/h)	155	1152	155	8	1041	22	133	16	14	38	14	115
Initial Q (Q _b), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00			1.00		0.98	1.00			1.00	1.00	1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No			No			No		
Adj Sat Flow, veh/h/ln	1709	1709	1709	1654	1654	1654	1736	1736	1736	1668	1668	1668
Adj Flow Rate, veh/h	163	1213	163	8	1096	23	140	17	15	40	15	121
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Percent Heavy Veh, %	3	3	3	7	7	7	1	1	1	6	6	6
Cap, veh/h	189	1742	233	15	1573	684	290	35	26	109	53	275
Arrive On Green	0.12	0.61	0.61	0.01	0.50	0.50	0.27	0.27	0.27	0.27	0.27	0.27
Sat Flow, veh/h	1628	2870	384	1576	3143	1368	869	128	95	266	195	1014
Grp Volume(v), veh/h	163	684	692	8	1096	23	172	0	0	176	0	0
Grp Sat Flow(s), veh/h/ln	1628	1624	1630	1576	1572	1368	1092	0	0	1475	0	0
Q Serve(g_s), s	11.8	34.3	34.8	0.6	32.1	1.0	7.0	0.0	0.0	0.0	0.0	0.0
Cycle Q Clear(g_c), s	11.8	34.3	34.8	0.6	32.1	1.0	18.4	0.0	0.0	11.4	0.0	0.0
Prop In Lane	1.00			1.00		1.00	0.81		0.09	0.23		0.69
Lane Grp Cap(c), veh/h	189	985	989	15	1573	684	350	0	0	436	0	0
V/C Ratio(X)	0.86	0.69	0.70	0.52	0.70	0.03	0.49	0.00	0.00	0.40	0.00	0.00
Avail Cap(c_a), veh/h	264	985	989	67	1573	684	350	0	0	436	0	0
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	0.00	1.00	0.00	0.00
Uniform Delay (d), s/veh	52.1	16.0	16.1	59.1	23.0	15.2	39.2	0.0	0.0	36.1	0.0	0.0
Incr Delay (d2), s/veh	18.3	4.0	4.1	24.7	2.6	0.1	4.9	0.0	0.0	0.6	0.0	0.0
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln	5.8	13.4	13.7	0.3	12.3	0.3	5.0	0.0	0.0	4.4	0.0	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	70.4	20.0	20.2	83.8	25.6	15.3	44.1	0.0	0.0	36.7	0.0	0.0
LnGrp LOS	E	C	C	F	C	B	D	A	A	D	A	A
Approach Vol, veh/h	1539				1127			172			176	
Approach Delay, s/veh	25.5				25.8			44.1			36.7	
Approach LOS	C				C			D			D	
Timer - Assigned Phs	1	2		4	5	6		8				
Phs Duration (G+Y+R _c), s	5.7	77.3		37.0	18.4	64.6		37.0				
Change Period (Y+R _c), s	4.5	4.5		4.5	4.5	4.5		4.5				
Max Green Setting (Gmax), s	5.1	68.9		32.5	19.5	54.5		32.5				
Max Q Clear Time (g_c+l1), s	2.6	36.8		20.4	13.8	34.1		13.4				
Green Ext Time (p_c), s	0.0	13.3		0.8	0.2	8.7		1.0				
Intersection Summary												
HCM 6th Ctrl Delay				27.3								
HCM 6th LOS				C								

Intersection

Int Delay, s/veh 1.7

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑	↗	↖	↑↑	↖	↗
Traffic Vol, veh/h	1107	83	17	1064	34	33
Future Vol, veh/h	1107	83	17	1064	34	33
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	160	215	-	120	0
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	3	3	7	7	3	3
Mvmt Flow	1165	87	18	1120	36	35

Major/Minor	Major1	Major2	Minor1		
Conflicting Flow All	0	0	1252	0	1761 583
Stage 1	-	-	-	-	1165 -
Stage 2	-	-	-	-	596 -
Critical Hdwy	-	-	4.24	-	6.86 6.96
Critical Hdwy Stg 1	-	-	-	-	5.86 -
Critical Hdwy Stg 2	-	-	-	-	5.86 -
Follow-up Hdwy	-	-	2.27	-	3.53 3.33
Pot Cap-1 Maneuver	-	-	525	-	75 453
Stage 1	-	-	-	-	257 -
Stage 2	-	-	-	-	510 -
Platoon blocked, %	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	525	-	72 453
Mov Cap-2 Maneuver	-	-	-	-	72 -
Stage 1	-	-	-	-	257 -
Stage 2	-	-	-	-	493 -

Approach	EB	WB	NB
HCM Control Delay, s	0	0.2	55.8
HCM LOS		F	

Minor Lane/Major Mvmt	NBLn1	NBLn2	EBT	EBR	WBL	WBT
Capacity (veh/h)	72	453	-	-	525	-
HCM Lane V/C Ratio	0.497	0.077	-	-	0.034	-
HCM Control Delay (s)	96.7	13.6	-	-	12.1	-
HCM Lane LOS	F	B	-	-	B	-
HCM 95th %tile Q(veh)	2	0.2	-	-	0.1	-

Intersection

Int Delay, s/veh 5.1

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	11	47	54	26	50	26	60	272	54	10	327	11
Future Vol, veh/h	11	47	54	26	50	26	60	272	54	10	327	11
Conflicting Peds, #/hr	2	0	0	0	0	2	0	0	0	2	0	2
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None									
Storage Length	-	-	90	-	-	125	-	-	-	-	-	330
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	94	94	94	94	94	94	94	94	94	94	94	94
Heavy Vehicles, %	1	1	1	5	5	5	2	2	2	5	5	5
Mvmt Flow	12	50	57	28	53	28	64	289	57	11	348	12

Major/Minor	Minor2	Minor1			Major1			Major2				
Conflicting Flow All	860	848	350	878	832	322	362	0	0	348	0	0
Stage 1	372	372	-	448	448	-	-	-	-	-	-	-
Stage 2	488	476	-	430	384	-	-	-	-	-	-	-
Critical Hdwy	7.11	6.51	6.21	7.15	6.55	6.25	4.12	-	-	4.15	-	-
Critical Hdwy Stg 1	6.11	5.51	-	6.15	5.55	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.11	5.51	-	6.15	5.55	-	-	-	-	-	-	-
Follow-up Hdwy	3.509	4.009	3.309	3.545	4.045	3.345	2.218	-	-	2.245	-	-
Pot Cap-1 Maneuver	277	299	696	265	301	712	1197	-	-	1194	-	-
Stage 1	651	621	-	584	568	-	-	-	-	-	-	-
Stage 2	563	558	-	598	606	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	213	274	695	197	276	709	1195	-	-	1192	-	-
Mov Cap-2 Maneuver	213	274	-	197	276	-	-	-	-	-	-	-
Stage 1	606	612	-	544	529	-	-	-	-	-	-	-
Stage 2	453	519	-	498	598	-	-	-	-	-	-	-

Approach	EB	WB			NB			SB				
HCM Control Delay, s	17.1	22.7			1.3			0.2				
HCM LOS	C	C										
<hr/>												
Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	EBLn2	WBLn1	WBLn2	SBL	SBT	SBR		
Capacity (veh/h)	1195	-	-	260	695	243	709	1192	-	-		
HCM Lane V/C Ratio	0.053	-	-	0.237	0.083	0.333	0.039	0.009	-	-		
HCM Control Delay (s)	8.2	0	-	23.1	10.6	27	10.3	8	0	-		
HCM Lane LOS	A	A	-	C	B	D	B	A	A	-		
HCM 95th %tile Q(veh)	0.2	-	-	0.9	0.3	1.4	0.1	0	-	-		

Intersection

Int Delay, s/veh 5.3

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑		↔	↔		↔	↔		↔	↔	
Traffic Vol, veh/h	24	24	5	2	6	7	4	10	1	24	27	28
Future Vol, veh/h	24	24	5	2	6	7	4	10	1	24	27	28
Conflicting Peds, #/hr	2	0	1	3	0	4	1	0	3	4	0	2
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None									
Storage Length	115	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	91	91	91	91	91	91	91	91	91	91	91	91
Heavy Vehicles, %	2	2	2	2	2	2	7	7	7	3	3	3
Mvmt Flow	26	26	5	2	7	8	4	11	1	26	30	31

Major/Minor	Minor2	Minor1			Major1			Major2				
Conflicting Flow All	131	124	51	140	139	20	63	0	0	16	0	0
Stage 1	100	100	-	24	24	-	-	-	-	-	-	-
Stage 2	31	24	-	116	115	-	-	-	-	-	-	-
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.17	-	-	4.13	-	-
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.263	-	-	2.227	-	-
Pot Cap-1 Maneuver	841	766	1017	830	752	1058	1508	-	-	1595	-	-
Stage 1	906	812	-	994	875	-	-	-	-	-	-	-
Stage 2	986	875	-	889	800	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	812	746	1012	786	732	1050	1505	-	-	1589	-	-
Mov Cap-2 Maneuver	812	746	-	786	732	-	-	-	-	-	-	-
Stage 1	901	797	-	987	869	-	-	-	-	-	-	-
Stage 2	965	869	-	838	785	-	-	-	-	-	-	-

Approach	EB	WB			NB			SB				
HCM Control Delay, s	9.7	9.3			2			2.2				
HCM LOS	A	A			A			A				
Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	EBLn2	WBLn1	SBL	SBT	SBR			
Capacity (veh/h)	1505	-	-	812	781	862	1589	-	-			
HCM Lane V/C Ratio	0.003	-	-	0.032	0.041	0.019	0.017	-	-			
HCM Control Delay (s)	7.4	0	-	9.6	9.8	9.3	7.3	0	-			
HCM Lane LOS	A	A	-	A	A	A	A	A	A			
HCM 95th %tile Q(veh)	0	-	-	0.1	0.1	0.1	0.1	-	-			

Trip Generation Calculation Worksheet



Land Use Description: Single-Family Detached Housing

ITE Land Use Code: 210

Independent Variable: Dwelling Units

Quantity: 32 Dwelling Units

Summary of ITE Trip Generation Data

AM Peak Hour of Adjacent Street Traffic

Trip Rate: 0.74 trips per dwelling unit

Directional Distribution: 25% Entering 75% Exiting

PM Peak Hour of Adjacent Street Traffic

Trip Rate: 0.99 trips per dwelling unit

Directional Distribution: 63% Entering 37% Exiting

Total Weekday Traffic

Trip Rate: 9.44 trips per dwelling unit

Directional Distribution: 50% Entering 50% Exiting

Site Trip Generation Calculations

32 Dwelling Units

	Entering	Exiting	Total
AM Peak Hour	6	18	24
PM Peak Hour	20	12	32
Weekday	151	151	302

Trip Generation Calculation Worksheet



Land Use Description: Multi-Family Housing (Low-Rise)

ITE Land Use Code: 220

Independent Variable: Dwelling Units

Quantity: 120 Dwelling Units

Summary of ITE Trip Generation Data

AM Peak Hour of Adjacent Street Traffic

Trip Rate: 0.46 trips per dwelling unit

Directional Distribution: 23% Entering 77% Exiting

PM Peak Hour of Adjacent Street Traffic

Trip Rate: 0.56 trips per dwelling unit

Directional Distribution: 63% Entering 37% Exiting

Total Weekday Traffic

Trip Rate: 7.32 trips per dwelling unit

Directional Distribution: 50% Entering 50% Exiting

Site Trip Generation Calculations

120 Dwelling Units

	Entering	Exiting	Total
AM Peak Hour	13	42	55
PM Peak Hour	42	25	67
Weekday	439	439	878

HCM Signalized Intersection Capacity Analysis

1: Wolf Drive/Ten Eyck Road & Highway 26

06/01/2021

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	84	796	38	4	1173	13	141	11	3	18	4	161
Future Volume (vph)	84	796	38	4	1173	13	141	11	3	18	4	161
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Total Lost time (s)	4.5	4.5		4.5	4.5	4.5		4.5			4.5	
Lane Util. Factor	1.00	0.95		1.00	0.95	1.00		1.00			1.00	
Frpb, ped/bikes	1.00	1.00		1.00	1.00	0.97		1.00			0.98	
Flpb, ped/bikes	1.00	1.00		1.00	1.00	1.00		1.00			1.00	
Fr _t	1.00	0.99		1.00	1.00	0.85		1.00			0.88	
Flt Protected	0.95	1.00		0.95	1.00	1.00		0.96			1.00	
Satd. Flow (prot)	1484	2946		1568	3137	1356		1575			1464	
Flt Permitted	0.95	1.00		0.95	1.00	1.00		0.52			0.96	
Satd. Flow (perm)	1484	2946		1568	3137	1356		854			1418	
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	88	838	40	4	1235	14	148	12	3	19	4	169
RTOR Reduction (vph)	0	2	0	0	0	7	0	1	0	0	115	0
Lane Group Flow (vph)	88	876	0	4	1235	7	0	162	0	0	77	0
Confl. Peds. (#/hr)						4					4	
Confl. Bikes (#/hr)			2			1						
Heavy Vehicles (%)	12%	12%	12%	6%	6%	6%	6%	6%	6%	3%	3%	3%
Turn Type	Prot	NA		Prot	NA	Perm	Perm	NA		Perm	NA	
Protected Phases	5	2		1	6			4			8	
Permitted Phases						6	4				8	
Actuated Green, G (s)	11.0	71.0		1.0	61.0	61.0		34.5			34.5	
Effective Green, g (s)	11.0	71.0		1.0	61.0	61.0		34.5			34.5	
Actuated g/C Ratio	0.09	0.59		0.01	0.51	0.51		0.29			0.29	
Clearance Time (s)	4.5	4.5		4.5	4.5	4.5		4.5			4.5	
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0		3.0			3.0	
Lane Grp Cap (vph)	136	1743		13	1594	689		245			407	
v/s Ratio Prot	c0.06	0.30		0.00	c0.39							
v/s Ratio Perm						0.01		c0.19			0.05	
v/c Ratio	0.65	0.50		0.31	0.77	0.01		0.66			0.19	
Uniform Delay, d1	52.6	14.2		59.2	23.9	14.6		37.6			32.2	
Progression Factor	1.00	1.00		1.00	1.00	1.00		1.00			1.00	
Incremental Delay, d2	10.1	1.0		13.0	3.7	0.0		13.3			0.2	
Delay (s)	62.8	15.3		72.2	27.7	14.6		50.9			32.4	
Level of Service	E	B		E	C	B		D			C	
Approach Delay (s)		19.6			27.7			50.9			32.4	
Approach LOS		B			C			D			C	
Intersection Summary												
HCM 2000 Control Delay		26.5			HCM 2000 Level of Service				C			
HCM 2000 Volume to Capacity ratio		0.72										
Actuated Cycle Length (s)		120.0			Sum of lost time (s)				13.5			
Intersection Capacity Utilization		77.4%			ICU Level of Service				D			
Analysis Period (min)		15										
c Critical Lane Group												

HCM 6th Signalized Intersection Summary
1: Wolf Drive/Ten Eyck Road & Highway 26

06/01/2021

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑↑		↑	↑↑	↑		↔			↔	
Traffic Volume (veh/h)	84	796	38	4	1173	13	141	11	3	18	4	161
Future Volume (veh/h)	84	796	38	4	1173	13	141	11	3	18	4	161
Initial Q (Q _b), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00			1.00		0.98	1.00			1.00	1.00	1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No			No			No		
Adj Sat Flow, veh/h/ln	1586	1586	1586	1668	1668	1668	1668	1668	1668	1709	1709	1709
Adj Flow Rate, veh/h	88	838	40	4	1235	14	148	12	3	19	4	169
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Percent Heavy Veh, %	12	12	12	6	6	6	6	6	6	3	3	3
Cap, veh/h	107	1740	83	8	1678	730	295	22	5	57	27	379
Arrive On Green	0.07	0.59	0.59	0.01	0.53	0.53	0.29	0.29	0.29	0.29	0.29	0.29
Sat Flow, veh/h	1511	2925	140	1589	3169	1379	825	77	17	84	95	1318
Grp Volume(v), veh/h	88	432	446	4	1235	14	163	0	0	192	0	0
Grp Sat Flow(s), veh/h/ln	1511	1507	1557	1589	1585	1379	919	0	0	1497	0	0
Q Serve(g_s), s	6.9	19.5	19.5	0.3	36.1	0.6	8.9	0.0	0.0	0.0	0.0	0.0
Cycle Q Clear(g_c), s	6.9	19.5	19.5	0.3	36.1	0.6	21.7	0.0	0.0	12.8	0.0	0.0
Prop In Lane	1.00			1.00		1.00	0.91			0.02	0.10	0.88
Lane Grp Cap(c), veh/h	107	896	926	8	1678	730	321	0	0	463	0	0
V/C Ratio(X)	0.82	0.48	0.48	0.48	0.74	0.02	0.51	0.00	0.00	0.41	0.00	0.00
Avail Cap(c_a), veh/h	157	896	926	68	1678	730	321	0	0	463	0	0
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	0.00	1.00	0.00	0.00
Uniform Delay (d), s/veh	55.0	13.8	13.8	59.5	21.8	13.4	39.7	0.0	0.0	35.0	0.0	0.0
Incr Delay (d2), s/veh	19.6	1.9	1.8	38.0	2.9	0.0	5.6	0.0	0.0	0.6	0.0	0.0
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln	3.2	7.0	7.3	0.2	13.9	0.2	4.8	0.0	0.0	4.7	0.0	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	74.7	15.7	15.6	97.5	24.7	13.5	45.3	0.0	0.0	35.6	0.0	0.0
LnGrp LOS	E	B	B	F	C	B	D	A	A	D	A	A
Approach Vol, veh/h	966				1253			163			192	
Approach Delay, s/veh	21.0				24.8			45.3			35.6	
Approach LOS	C				C			D			D	
Timer - Assigned Phs	1	2		4	5	6		8				
Phs Duration (G+Y+R _c), s	5.1	75.9		39.0	13.0	68.0		39.0				
Change Period (Y+R _c), s	4.5	4.5		4.5	4.5	4.5		4.5				
Max Green Setting (Gmax), s	5.1	66.9		34.5	12.5	59.5		34.5				
Max Q Clear Time (g_c+l1), s	2.3	21.5		23.7	8.9	38.1		14.8				
Green Ext Time (p_c), s	0.0	7.4		0.7	0.1	10.2		1.1				
Intersection Summary												
HCM 6th Ctrl Delay				25.5								
HCM 6th LOS				C								

Intersection

Int Delay, s/veh 2.9

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑	↗	↖	↑↑	↖	↗
Traffic Vol, veh/h	807	36	20	1056	69	26
Future Vol, veh/h	807	36	20	1056	69	26
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	160	215	-	120	0
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	13	13	7	7	4	4
Mvmt Flow	849	38	21	1112	73	27

Major/Minor	Major1	Major2	Minor1		
Conflicting Flow All	0	0	887	0	1447 425
Stage 1	-	-	-	-	849 -
Stage 2	-	-	-	-	598 -
Critical Hdwy	-	-	4.24	-	6.88 6.98
Critical Hdwy Stg 1	-	-	-	-	5.88 -
Critical Hdwy Stg 2	-	-	-	-	5.88 -
Follow-up Hdwy	-	-	2.27	-	3.54 3.34
Pot Cap-1 Maneuver	-	-	728	-	120 572
Stage 1	-	-	-	-	375 -
Stage 2	-	-	-	-	506 -
Platoon blocked, %	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	728	-	117 572
Mov Cap-2 Maneuver	-	-	-	-	117 -
Stage 1	-	-	-	-	375 -
Stage 2	-	-	-	-	491 -

Approach	EB	WB	NB
HCM Control Delay, s	0	0.2	58.7
HCM LOS		F	

Minor Lane/Major Mvmt	NBLn1	NBLn2	EBT	EBR	WBL	WBT
Capacity (veh/h)	117	572	-	-	728	-
HCM Lane V/C Ratio	0.621	0.048	-	-	0.029	-
HCM Control Delay (s)	76.4	11.6	-	-	10.1	-
HCM Lane LOS	F	B	-	-	B	-
HCM 95th %tile Q(veh)	3.1	0.1	-	-	0.1	-

Intersection

Int Delay, s/veh 5.1

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖ ↗	↖ ↗	↖ ↗	↖ ↗	↖ ↗	↖ ↗	↖ ↗	↖ ↗	↖ ↗	↖ ↗	↖ ↗	↖ ↗
Traffic Vol, veh/h	11	8	34	56	42	61	46	315	15	14	190	3
Future Vol, veh/h	11	8	34	56	42	61	46	315	15	14	190	3
Conflicting Peds, #/hr	2	0	0	0	0	2	0	0	0	2	0	2
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None									
Storage Length	-	-	90	-	-	125	-	-	-	-	-	330
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	1	1	1	5	5	5	2	2	2	5	5	5
Mvmt Flow	12	9	38	62	47	68	51	350	17	16	211	3

Major/Minor	Minor2	Minor1			Major1			Major2			
Conflicting Flow All	765	716	213	731	711	363	216	0	0	369	0
Stage 1	245	245	-	463	463	-	-	-	-	-	-
Stage 2	520	471	-	268	248	-	-	-	-	-	-
Critical Hdwy	7.11	6.51	6.21	7.15	6.55	6.25	4.12	-	-	4.15	-
Critical Hdwy Stg 1	6.11	5.51	-	6.15	5.55	-	-	-	-	-	-
Critical Hdwy Stg 2	6.11	5.51	-	6.15	5.55	-	-	-	-	-	-
Follow-up Hdwy	3.509	4.009	3.309	3.545	4.045	3.345	2.218	-	-	2.245	-
Pot Cap-1 Maneuver	321	357	830	334	354	675	1354	-	-	1173	-
Stage 1	761	705	-	573	559	-	-	-	-	-	-
Stage 2	541	561	-	731	696	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-
Mov Cap-1 Maneuver	244	333	828	297	330	672	1351	-	-	1171	-
Mov Cap-2 Maneuver	244	333	-	297	330	-	-	-	-	-	-
Stage 1	723	692	-	544	531	-	-	-	-	-	-
Stage 2	422	533	-	678	683	-	-	-	-	-	-

Approach	EB	WB			NB			SB		
HCM Control Delay, s	13	18.3			1			0.5		
HCM LOS	B	C								
Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	EBLn2	WBLn1	WBLn2	SBL	SBT	SBR
Capacity (veh/h)	1351	-	-	275	828	310	672	1171	-	-
HCM Lane V/C Ratio	0.038	-	-	0.077	0.046	0.351	0.101	0.013	-	-
HCM Control Delay (s)	7.8	0	-	19.2	9.6	22.8	11	8.1	0	-
HCM Lane LOS	A	A	-	C	A	C	B	A	A	-
HCM 95th %tile Q(veh)	0.1	-	-	0.2	0.1	1.5	0.3	0	-	-

Intersection

Int Delay, s/veh 5.7

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑		↔		↔	↔	↔		↔		↔
Traffic Vol, veh/h	29	1	1	1	16	24	4	22	2	9	5	21
Future Vol, veh/h	29	1	1	1	16	24	4	22	2	9	5	21
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None									
Storage Length	115	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	89	89	89	89	89	89	89	89	89	89	89	89
Heavy Vehicles, %	5	5	5	18	18	18	8	8	8	23	23	23
Mvmt Flow	33	1	1	1	18	27	4	25	2	10	6	24

Major/Minor	Minor2	Minor1			Major1			Major2				
Conflicting Flow All	95	73	18	73	84	26	30	0	0	27	0	0
Stage 1	38	38	-	34	34	-	-	-	-	-	-	-
Stage 2	57	35	-	39	50	-	-	-	-	-	-	-
Critical Hdwy	7.15	6.55	6.25	7.28	6.68	6.38	4.18	-	-	4.33	-	-
Critical Hdwy Stg 1	6.15	5.55	-	6.28	5.68	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.15	5.55	-	6.28	5.68	-	-	-	-	-	-	-
Follow-up Hdwy	3.545	4.045	3.345	3.662	4.162	3.462	2.272	-	-	2.407	-	-
Pot Cap-1 Maneuver	881	812	1052	880	777	1006	1545	-	-	1461	-	-
Stage 1	970	857	-	943	836	-	-	-	-	-	-	-
Stage 2	947	860	-	937	823	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	836	804	1052	871	769	1006	1545	-	-	1461	-	-
Mov Cap-2 Maneuver	836	804	-	871	769	-	-	-	-	-	-	-
Stage 1	967	851	-	940	833	-	-	-	-	-	-	-
Stage 2	899	857	-	928	817	-	-	-	-	-	-	-

Approach	EB	WB			NB			SB				
HCM Control Delay, s	9.5	9.2			1			1.9				
HCM LOS	A	A			A			A				
Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	EBLn2	WBLn1	SBL	SBT	SBR			
Capacity (veh/h)	1545	-	-	836	911	895	1461	-	-			
HCM Lane V/C Ratio	0.003	-	-	0.039	0.002	0.051	0.007	-	-			
HCM Control Delay (s)	7.3	0	-	9.5	9	9.2	7.5	0	-			
HCM Lane LOS	A	A	-	A	A	A	A	A	A			
HCM 95th %tile Q(veh)	0	-	-	0.1	0	0.2	0	-	-			

HCM Signalized Intersection Capacity Analysis

1: Wolf Drive/Ten Eyck Road & Highway 26

06/01/2021

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑↑↓		↑	↑↑	↑	↓	↔			↔	
Traffic Volume (vph)	169	1255	161	8	1124	24	138	17	15	42	15	140
Future Volume (vph)	169	1255	161	8	1124	24	138	17	15	42	15	140
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Total Lost time (s)	4.5	4.5		4.5	4.5	4.5					4.5	
Lane Util. Factor	1.00	0.95		1.00	0.95	1.00		1.00			1.00	
Frpb, ped/bikes	1.00	1.00		1.00	1.00	0.97		1.00			0.99	
Flpb, ped/bikes	1.00	1.00		1.00	1.00	1.00		1.00			1.00	
Fr _t	1.00	0.98		1.00	1.00	0.85		0.99			0.90	
Flt Protected	0.95	1.00		0.95	1.00	1.00		0.96			0.99	
Satd. Flow (prot)	1614	3166		1554	3107	1343		1645			1456	
Flt Permitted	0.95	1.00		0.95	1.00	1.00		0.53			0.91	
Satd. Flow (perm)	1614	3166		1554	3107	1343		906			1339	
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	178	1321	169	8	1183	25	145	18	16	44	16	147
RTOR Reduction (vph)	0	8	0	0	0	13	0	3	0	0	74	0
Lane Group Flow (vph)	178	1482	0	8	1183	12	0	176	0	0	133	0
Confl. Peds. (#/hr)						4					4	
Confl. Bikes (#/hr)			2			1						
Heavy Vehicles (%)	3%	3%	3%	7%	7%	7%	1%	1%	1%	6%	6%	6%
Turn Type	Prot	NA		Prot	NA	Perm	Perm	NA		Perm	NA	
Protected Phases	5	2		1	6			4			8	
Permitted Phases						6	4				8	
Actuated Green, G (s)	16.7	74.0		1.0	58.3	58.3		31.5			31.5	
Effective Green, g (s)	16.7	74.0		1.0	58.3	58.3		31.5			31.5	
Actuated g/C Ratio	0.14	0.62		0.01	0.49	0.49		0.26			0.26	
Clearance Time (s)	4.5	4.5		4.5	4.5	4.5		4.5			4.5	
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0		3.0			3.0	
Lane Grp Cap (vph)	224	1952		12	1509	652		237			351	
v/s Ratio Prot	c0.11	c0.47		0.01	0.38							
v/s Ratio Perm						0.01		c0.19			0.10	
v/c Ratio	0.79	0.76		0.67	0.78	0.02		0.74			0.38	
Uniform Delay, d1	50.0	16.6		59.3	25.6	16.0		40.5			36.2	
Progression Factor	1.00	1.00		1.00	1.00	1.00		1.00			1.00	
Incremental Delay, d2	17.4	2.8		89.5	4.2	0.1		18.9			0.7	
Delay (s)	67.4	19.4		148.8	29.8	16.1		59.4			36.9	
Level of Service	E	B		F	C	B		E			D	
Approach Delay (s)		24.5			30.3			59.4			36.9	
Approach LOS		C			C			E			D	
Intersection Summary												
HCM 2000 Control Delay		29.4			HCM 2000 Level of Service				C			
HCM 2000 Volume to Capacity ratio		0.78										
Actuated Cycle Length (s)		120.0			Sum of lost time (s)				13.5			
Intersection Capacity Utilization		85.9%			ICU Level of Service				E			
Analysis Period (min)		15										
c Critical Lane Group												

HCM 6th Signalized Intersection Summary
1: Wolf Drive/Ten Eyck Road & Highway 26

06/01/2021

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑↑		↑	↑↑	↑	↔	↔		↔	↔	↔
Traffic Volume (veh/h)	169	1255	161	8	1124	24	138	17	15	42	15	140
Future Volume (veh/h)	169	1255	161	8	1124	24	138	17	15	42	15	140
Initial Q (Q _b), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00			1.00		0.98	1.00			1.00	1.00	1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No			No			No		
Adj Sat Flow, veh/h/ln	1709	1709	1709	1654	1654	1654	1736	1736	1736	1668	1668	1668
Adj Flow Rate, veh/h	178	1321	169	8	1183	25	145	18	16	44	16	147
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Percent Heavy Veh, %	3	3	3	7	7	7	1	1	1	6	6	6
Cap, veh/h	204	1778	226	15	1571	684	262	32	23	101	49	279
Arrive On Green	0.13	0.62	0.62	0.01	0.50	0.50	0.26	0.26	0.26	0.26	0.26	0.26
Sat Flow, veh/h	1628	2890	367	1576	3143	1368	790	121	89	246	188	1064
Grp Volume(v), veh/h	178	738	752	8	1183	25	179	0	0	207	0	0
Grp Sat Flow(s), veh/h/ln	1628	1624	1634	1576	1572	1368	1001	0	0	1498	0	0
Q Serve(g_s), s	12.9	38.5	39.4	0.6	36.2	1.1	7.8	0.0	0.0	0.0	0.0	0.0
Cycle Q Clear(g_c), s	12.9	38.5	39.4	0.6	36.2	1.1	21.7	0.0	0.0	13.9	0.0	0.0
Prop In Lane	1.00			1.00		1.00	0.81		0.09	0.21		0.71
Lane Grp Cap(c), veh/h	204	999	1005	15	1571	684	317	0	0	430	0	0
V/C Ratio(X)	0.87	0.74	0.75	0.52	0.75	0.04	0.56	0.00	0.00	0.48	0.00	0.00
Avail Cap(c_a), veh/h	251	999	1005	66	1571	684	317	0	0	430	0	0
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	0.00	1.00	0.00	0.00
Uniform Delay (d), s/veh	51.6	16.3	16.5	59.1	24.1	15.3	41.6	0.0	0.0	37.9	0.0	0.0
Incr Delay (d2), s/veh	23.6	4.9	5.1	24.7	3.4	0.1	7.1	0.0	0.0	0.8	0.0	0.0
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln	6.6	15.1	15.6	0.3	14.0	0.4	5.5	0.0	0.0	5.3	0.0	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	75.2	21.2	21.5	83.8	27.4	15.4	48.7	0.0	0.0	38.7	0.0	0.0
LnGrp LOS	E	C	C	F	C	B	D	A	A	D	A	A
Approach Vol, veh/h		1668			1216			179			207	
Approach Delay, s/veh		27.1			27.6			48.7			38.7	
Approach LOS		C			C			D			D	
Timer - Assigned Phs	1	2		4	5	6		8				
Phs Duration (G+Y+R _c), s	5.7	78.3		36.0	19.5	64.5		36.0				
Change Period (Y+R _c), s	4.5	4.5		4.5	4.5	4.5		4.5				
Max Green Setting (Gmax), s	5.0	70.0		31.5	18.5	56.5		31.5				
Max Q Clear Time (g_c+l1), s	2.6	41.4		23.7	14.9	38.2		15.9				
Green Ext Time (p_c), s	0.0	14.2		0.6	0.2	9.0		1.1				
Intersection Summary												
HCM 6th Ctrl Delay			29.2									
HCM 6th LOS			C									

Intersection

Int Delay, s/veh 2.6

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑	↗	↖	↑↑	↖	↗
Traffic Vol, veh/h	1210	86	27	1149	35	39
Future Vol, veh/h	1210	86	27	1149	35	39
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	160	215	-	120	0
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	3	3	7	7	3	3
Mvmt Flow	1274	91	28	1209	37	41

Major/Minor	Major1	Major2	Minor1		
Conflicting Flow All	0	0	1365	0	1935 637
Stage 1	-	-	-	-	1274 -
Stage 2	-	-	-	-	661 -
Critical Hdwy	-	-	4.24	-	6.86 6.96
Critical Hdwy Stg 1	-	-	-	-	5.86 -
Critical Hdwy Stg 2	-	-	-	-	5.86 -
Follow-up Hdwy	-	-	2.27	-	3.53 3.33
Pot Cap-1 Maneuver	-	-	474	-	57 418
Stage 1	-	-	-	-	225 -
Stage 2	-	-	-	-	473 -
Platoon blocked, %	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	474	-	54 418
Mov Cap-2 Maneuver	-	-	-	-	54 -
Stage 1	-	-	-	-	225 -
Stage 2	-	-	-	-	445 -

Approach	EB	WB	NB
HCM Control Delay, s	0	0.3	83.4
HCM LOS		F	

Minor Lane/Major Mvmt	NBLn1	NBLn2	EBT	EBR	WBL	WBT
Capacity (veh/h)	54	418	-	-	474	-
HCM Lane V/C Ratio	0.682	0.098	-	-	0.06	-
HCM Control Delay (s)	160.1	14.5	-	-	13.1	-
HCM Lane LOS	F	B	-	-	B	-
HCM 95th %tile Q(veh)	2.8	0.3	-	-	0.2	-

Intersection

Int Delay, s/veh 6.6

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	13	51	56	27	54	45	62	307	56	40	366	14
Future Vol, veh/h	13	51	56	27	54	45	62	307	56	40	366	14
Conflicting Peds, #/hr	2	0	0	0	0	2	0	0	0	2	0	2
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None									
Storage Length	-	-	90	-	-	125	-	-	-	-	-	330
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	95	95	95	95	95	95	95	95	95	95	95	95
Heavy Vehicles, %	1	1	1	5	5	5	2	2	2	5	5	5
Mvmt Flow	14	54	59	28	57	47	65	323	59	42	385	15

Major/Minor	Minor2	Minor1			Major1			Major2				
Conflicting Flow All	1008	985	387	1018	971	357	402	0	0	384	0	0
Stage 1	471	471	-	485	485	-	-	-	-	-	-	-
Stage 2	537	514	-	533	486	-	-	-	-	-	-	-
Critical Hdwy	7.11	6.51	6.21	7.15	6.55	6.25	4.12	-	-	4.15	-	-
Critical Hdwy Stg 1	6.11	5.51	-	6.15	5.55	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.11	5.51	-	6.15	5.55	-	-	-	-	-	-	-
Follow-up Hdwy	3.509	4.009	3.309	3.545	4.045	3.345	2.218	-	-	2.245	-	-
Pot Cap-1 Maneuver	220	249	663	213	250	680	1157	-	-	1158	-	-
Stage 1	575	561	-	558	547	-	-	-	-	-	-	-
Stage 2	530	537	-	525	546	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	149	219	662	144	220	677	1155	-	-	1156	-	-
Mov Cap-2 Maneuver	149	219	-	144	220	-	-	-	-	-	-	-
Stage 1	532	534	-	517	507	-	-	-	-	-	-	-
Stage 2	405	497	-	410	519	-	-	-	-	-	-	-

Approach	EB	WB			NB			SB				
HCM Control Delay, s	22.1	29.2			1.2			0.8				
HCM LOS	C	D										
<hr/>												
Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	EBLn2	WBLn1	WBLn2	SBL	SBT	SBR		
Capacity (veh/h)	1155	-	-	200	662	187	677	1156	-	-		
HCM Lane V/C Ratio	0.057	-	-	0.337	0.089	0.456	0.07	0.036	-	-		
HCM Control Delay (s)	8.3	0	-	31.9	11	39.4	10.7	8.2	0	-		
HCM Lane LOS	A	A	-	D	B	E	B	A	A	-		
HCM 95th %tile Q(veh)	0.2	-	-	1.4	0.3	2.2	0.2	0.1	-	-		

Intersection

Int Delay, s/veh 5.2

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑		↔		↔	↔	↔		↔		↔
Traffic Vol, veh/h	30	25	5	2	6	7	4	10	1	25	28	38
Future Vol, veh/h	30	25	5	2	6	7	4	10	1	25	28	38
Conflicting Peds, #/hr	2	0	1	3	0	4	1	0	3	4	0	2
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None									
Storage Length	115	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	91	91	91	91	91	91	91	91	91	91	91	91
Heavy Vehicles, %	2	2	2	2	2	2	7	7	7	3	3	3
Mvmt Flow	33	27	5	2	7	8	4	11	1	27	31	42

Major/Minor	Minor2	Minor1			Major1			Major2				
Conflicting Flow All	139	132	57	149	153	20	75	0	0	16	0	0
Stage 1	108	108	-	24	24	-	-	-	-	-	-	-
Stage 2	31	24	-	125	129	-	-	-	-	-	-	-
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.17	-	-	4.13	-	-
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.263	-	-	2.227	-	-
Pot Cap-1 Maneuver	831	759	1009	819	739	1058	1493	-	-	1595	-	-
Stage 1	897	806	-	994	875	-	-	-	-	-	-	-
Stage 2	986	875	-	879	789	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	801	739	1004	774	719	1050	1490	-	-	1589	-	-
Mov Cap-2 Maneuver	801	739	-	774	719	-	-	-	-	-	-	-
Stage 1	893	790	-	987	869	-	-	-	-	-	-	-
Stage 2	965	869	-	826	773	-	-	-	-	-	-	-

Approach	EB	WB			NB			SB				
HCM Control Delay, s	9.8	9.3			2			2				
HCM LOS	A	A			A			A				
Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	EBLn2	WBLn1	SBL	SBT	SBR			
Capacity (veh/h)	1490	-	-	801	773	852	1589	-	-			
HCM Lane V/C Ratio	0.003	-	-	0.041	0.043	0.019	0.017	-	-			
HCM Control Delay (s)	7.4	0	-	9.7	9.9	9.3	7.3	0	-			
HCM Lane LOS	A	A	-	A	A	A	A	A	A	-		
HCM 95th %tile Q(veh)	0	-	-	0.1	0.1	0.1	0.1	-	-			

HCM Signalized Intersection Capacity Analysis

1: Wolf Drive/Ten Eyck Road & Highway 26

06/01/2021

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑↑		↑	↑↑	↑	↔	↔		↔	↔	↔
Traffic Volume (vph)	84	808	38	4	1186	13	154	11	3	18	4	161
Future Volume (vph)	84	808	38	4	1186	13	154	11	3	18	4	161
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Total Lost time (s)	4.5	4.5		4.5	4.5	4.5				4.5		
Lane Util. Factor	1.00	0.95		1.00	0.95	1.00				1.00		
Frpb, ped/bikes	1.00	1.00		1.00	1.00	0.97				0.98		
Flpb, ped/bikes	1.00	1.00		1.00	1.00	1.00				1.00		
Fr _t	1.00	0.99		1.00	1.00	0.85				0.88		
Flt Protected	0.95	1.00		0.95	1.00	1.00				0.96		1.00
Satd. Flow (prot)	1484	2946		1568	3137	1356				1575		1464
Flt Permitted	0.95	1.00		0.95	1.00	1.00				0.52		0.96
Satd. Flow (perm)	1484	2946		1568	3137	1356				852		1416
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	88	851	40	4	1248	14	162	12	3	19	4	169
RTOR Reduction (vph)	0	2	0	0	0	7	0	1	0	0	115	0
Lane Group Flow (vph)	88	889	0	4	1248	7	0	176	0	0	77	0
Confl. Peds. (#/hr)						4						4
Confl. Bikes (#/hr)			2			1						
Heavy Vehicles (%)	12%	12%	12%	6%	6%	6%	6%	6%	6%	3%	3%	3%
Turn Type	Prot	NA		Prot	NA	Perm	Perm	NA		Perm	NA	
Protected Phases	5	2		1	6			4			8	
Permitted Phases						6	4			8		
Actuated Green, G (s)	11.0	71.0		1.0	61.0	61.0		34.5			34.5	
Effective Green, g (s)	11.0	71.0		1.0	61.0	61.0		34.5			34.5	
Actuated g/C Ratio	0.09	0.59		0.01	0.51	0.51		0.29			0.29	
Clearance Time (s)	4.5	4.5		4.5	4.5	4.5		4.5			4.5	
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0		3.0			3.0	
Lane Grp Cap (vph)	136	1743		13	1594	689		244			407	
v/s Ratio Prot	c0.06	0.30		0.00	c0.40							
v/s Ratio Perm						0.01		c0.21			0.05	
v/c Ratio	0.65	0.51		0.31	0.78	0.01		0.72			0.19	
Uniform Delay, d1	52.6	14.3		59.2	24.1	14.6		38.4			32.2	
Progression Factor	1.00	1.00		1.00	1.00	1.00		1.00			1.00	
Incremental Delay, d2	10.1	1.1		13.0	3.9	0.0		16.9			0.2	
Delay (s)	62.8	15.4		72.2	28.0	14.6		55.4			32.4	
Level of Service	E	B		E	C	B		E			C	
Approach Delay (s)		19.7			28.0			55.4			32.4	
Approach LOS		B			C			E			C	
Intersection Summary												
HCM 2000 Control Delay		27.1									C	
HCM 2000 Volume to Capacity ratio		0.75										
Actuated Cycle Length (s)		120.0									13.5	
Intersection Capacity Utilization		78.5%									D	
Analysis Period (min)		15										
c Critical Lane Group												

HCM 6th Signalized Intersection Summary
1: Wolf Drive/Ten Eyck Road & Highway 26

06/01/2021

Movement	EBL	EBT	EBC	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑↑		↑	↑↑	↑	↔	↔		↔	↔	↔
Traffic Volume (veh/h)	84	808	38	4	1186	13	154	11	3	18	4	161
Future Volume (veh/h)	84	808	38	4	1186	13	154	11	3	18	4	161
Initial Q (Q _b), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00			1.00		0.98	1.00			1.00	1.00	1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No			No			No		
Adj Sat Flow, veh/h/ln	1586	1586	1586	1668	1668	1668	1668	1668	1668	1709	1709	1709
Adj Flow Rate, veh/h	88	851	40	4	1248	14	162	12	3	19	4	169
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Percent Heavy Veh, %	12	12	12	6	6	6	6	6	6	3	3	3
Cap, veh/h	107	1741	82	8	1678	730	296	20	4	58	27	382
Arrive On Green	0.07	0.59	0.59	0.01	0.53	0.53	0.29	0.29	0.29	0.29	0.29	0.29
Sat Flow, veh/h	1511	2927	138	1589	3169	1379	830	70	16	85	96	1329
Grp Volume(v), veh/h	88	438	453	4	1248	14	177	0	0	192	0	0
Grp Sat Flow(s), veh/h/ln	1511	1507	1558	1589	1585	1379	915	0	0	1510	0	0
Q Serve(g_s), s	6.9	19.9	19.9	0.3	36.7	0.6	10.6	0.0	0.0	0.0	0.0	0.0
Cycle Q Clear(g_c), s	6.9	19.9	19.9	0.3	36.7	0.6	23.4	0.0	0.0	12.8	0.0	0.0
Prop In Lane	1.00			1.00		1.00	0.92			0.02	0.10	0.88
Lane Grp Cap(c), veh/h	107	896	927	8	1678	730	321	0	0	467	0	0
V/C Ratio(X)	0.82	0.49	0.49	0.48	0.74	0.02	0.55	0.00	0.00	0.41	0.00	0.00
Avail Cap(c_a), veh/h	157	896	927	68	1678	730	321	0	0	467	0	0
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	0.00	1.00	0.00	0.00
Uniform Delay (d), s/veh	55.0	13.9	13.9	59.5	21.9	13.4	40.4	0.0	0.0	35.0	0.0	0.0
Incr Delay (d2), s/veh	19.6	1.9	1.8	38.0	3.0	0.0	6.7	0.0	0.0	0.6	0.0	0.0
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln	3.2	7.2	7.4	0.2	14.1	0.2	5.4	0.0	0.0	4.7	0.0	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	74.7	15.8	15.7	97.5	25.0	13.5	47.1	0.0	0.0	35.6	0.0	0.0
LnGrp LOS	E	B	B	F	C	B	D	A	A	D	A	A
Approach Vol, veh/h	979				1266			177			192	
Approach Delay, s/veh	21.1				25.1			47.1			35.6	
Approach LOS	C				C			D			D	
Timer - Assigned Phs	1	2		4	5	6		8				
Phs Duration (G+Y+R _c), s	5.1	75.9		39.0	13.0	68.0		39.0				
Change Period (Y+R _c), s	4.5	4.5		4.5	4.5	4.5		4.5				
Max Green Setting (Gmax), s	5.1	66.9		34.5	12.5	59.5		34.5				
Max Q Clear Time (g_c+l1), s	2.3	21.9		25.4	8.9	38.7		14.8				
Green Ext Time (p_c), s	0.0	7.5		0.7	0.1	10.2		1.1				
Intersection Summary												
HCM 6th Ctrl Delay				25.8								
HCM 6th LOS				C								

Intersection

Int Delay, s/veh 4.2

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑	↗	↖	↑↑	↖	↗
Traffic Vol, veh/h	807	48	24	1056	82	38
Future Vol, veh/h	807	48	24	1056	82	38
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	160	215	-	120	0
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	13	13	7	7	4	4
Mvmt Flow	849	51	25	1112	86	40

Major/Minor	Major1	Major2	Minor1		
Conflicting Flow All	0	0	900	0	1455 425
Stage 1	-	-	-	-	849 -
Stage 2	-	-	-	-	606 -
Critical Hdwy	-	-	4.24	-	6.88 6.98
Critical Hdwy Stg 1	-	-	-	-	5.88 -
Critical Hdwy Stg 2	-	-	-	-	5.88 -
Follow-up Hdwy	-	-	2.27	-	3.54 3.34
Pot Cap-1 Maneuver	-	-	720	-	119 572
Stage 1	-	-	-	-	375 -
Stage 2	-	-	-	-	502 -
Platoon blocked, %	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	720	-	115 572
Mov Cap-2 Maneuver	-	-	-	-	115 -
Stage 1	-	-	-	-	375 -
Stage 2	-	-	-	-	484 -

Approach	EB	WB	NB
HCM Control Delay, s	0	0.2	70.2
HCM LOS		F	

Minor Lane/Major Mvmt	NBLn1	NBLn2	EBT	EBR	WBL	WBT
Capacity (veh/h)	115	572	-	-	720	-
HCM Lane V/C Ratio	0.751	0.07	-	-	0.035	-
HCM Control Delay (s)	97.3	11.8	-	-	10.2	-
HCM Lane LOS	F	B	-	-	B	-
HCM 95th %tile Q(veh)	4.2	0.2	-	-	0.1	-

Intersection

Int Delay, s/veh 5.5

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖ ↗	↖ ↗	↖ ↗	↖ ↗	↖ ↗	↖ ↗	↖ ↗	↖ ↗	↖ ↗	↖ ↗	↖ ↗	↖ ↗
Traffic Vol, veh/h	11	9	34	62	45	74	46	315	17	14	190	3
Future Vol, veh/h	11	9	34	62	45	74	46	315	17	14	190	3
Conflicting Peds, #/hr	2	0	0	0	0	2	0	0	0	2	0	2
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None									
Storage Length	-	-	90	-	-	125	-	-	-	-	-	330
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	90	90	90	90	90	90	90	90	90	90	90	90
Heavy Vehicles, %	1	1	1	5	5	5	2	2	2	5	5	5
Mvmt Flow	12	10	38	69	50	82	51	350	19	16	211	3

Major/Minor	Minor2	Minor1			Major1			Major2				
Conflicting Flow All	775	718	213	733	712	364	216	0	0	371	0	0
Stage 1	245	245	-	464	464	-	-	-	-	-	-	-
Stage 2	530	473	-	269	248	-	-	-	-	-	-	-
Critical Hdwy	7.11	6.51	6.21	7.15	6.55	6.25	4.12	-	-	4.15	-	-
Critical Hdwy Stg 1	6.11	5.51	-	6.15	5.55	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.11	5.51	-	6.15	5.55	-	-	-	-	-	-	-
Follow-up Hdwy	3.509	4.009	3.309	3.545	4.045	3.345	2.218	-	-	2.245	-	-
Pot Cap-1 Maneuver	316	356	830	332	354	674	1354	-	-	1171	-	-
Stage 1	761	705	-	573	558	-	-	-	-	-	-	-
Stage 2	534	560	-	730	696	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	233	332	828	294	330	671	1351	-	-	1169	-	-
Mov Cap-2 Maneuver	233	332	-	294	330	-	-	-	-	-	-	-
Stage 1	723	692	-	544	530	-	-	-	-	-	-	-
Stage 2	403	532	-	676	683	-	-	-	-	-	-	-

Approach	EB	WB			NB			SB				
HCM Control Delay, s	13.3	18.7			0.9			0.5				
HCM LOS	B	C										
Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	EBLn2	WBLn1	WBLn2	SBL	SBT	SBR		
Capacity (veh/h)	1351	-	-	269	828	308	671	1169	-	-		
HCM Lane V/C Ratio	0.038	-	-	0.083	0.046	0.386	0.123	0.013	-	-		
HCM Control Delay (s)	7.8	0	-	19.6	9.6	23.9	11.1	8.1	0	-		
HCM Lane LOS	A	A	-	C	A	C	B	A	A	-		
HCM 95th %tile Q(veh)	0.1	-	-	0.3	0.1	1.8	0.4	0	-	-		

Intersection

Int Delay, s/veh 7.2

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑		↔	↔		↔	↔		↔	↔	
Traffic Vol, veh/h	29	4	1	1	38	62	4	22	2	25	5	21
Future Vol, veh/h	29	4	1	1	38	62	4	22	2	25	5	21
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None									
Storage Length	115	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	89	89	89	89	89	89	89	89	89	89	89	89
Heavy Vehicles, %	5	5	5	18	18	18	8	8	8	23	23	23
Mvmt Flow	33	4	1	1	43	70	4	25	2	28	6	24

Major/Minor	Minor2	Minor1			Major1			Major2				
Conflicting Flow All	165	109	18	111	120	26	30	0	0	27	0	0
Stage 1	74	74	-	34	34	-	-	-	-	-	-	-
Stage 2	91	35	-	77	86	-	-	-	-	-	-	-
Critical Hdwy	7.15	6.55	6.25	7.28	6.68	6.38	4.18	-	-	4.33	-	-
Critical Hdwy Stg 1	6.15	5.55	-	6.28	5.68	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.15	5.55	-	6.28	5.68	-	-	-	-	-	-	-
Follow-up Hdwy	3.545	4.045	3.345	3.662	4.162	3.462	2.272	-	-	2.407	-	-
Pot Cap-1 Maneuver	793	775	1052	831	741	1006	1545	-	-	1461	-	-
Stage 1	928	828	-	943	836	-	-	-	-	-	-	-
Stage 2	909	860	-	894	793	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	692	757	1052	812	724	1006	1545	-	-	1461	-	-
Mov Cap-2 Maneuver	692	757	-	812	724	-	-	-	-	-	-	-
Stage 1	925	811	-	940	833	-	-	-	-	-	-	-
Stage 2	800	857	-	870	777	-	-	-	-	-	-	-

Approach	EB	WB			NB			SB				
HCM Control Delay, s	10.4	9.7			1			3.7				
HCM LOS	B	A			A			A				
Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	EBLn2	WBLn1	SBL	SBT	SBR			
Capacity (veh/h)	1545	-	-	692	802	876	1461	-	-			
HCM Lane V/C Ratio	0.003	-	-	0.047	0.007	0.13	0.019	-	-			
HCM Control Delay (s)	7.3	0	-	10.5	9.5	9.7	7.5	0	-			
HCM Lane LOS	A	A	-	B	A	A	A	A	A			
HCM 95th %tile Q(veh)	0	-	-	0.1	0	0.4	0.1	-	-			

HCM Signalized Intersection Capacity Analysis

1: Wolf Drive/Ten Eyck Road & Highway 26

06/01/2021

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	169	1295	161	8	1132	24	149	17	15	42	15	140
Future Volume (vph)	169	1295	161	8	1132	24	149	17	15	42	15	140
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Total Lost time (s)	4.5	4.5		4.5	4.5	4.5					4.5	
Lane Util. Factor	1.00	0.95		1.00	0.95	1.00		1.00			1.00	
Frpb, ped/bikes	1.00	1.00		1.00	1.00	0.97		1.00			0.99	
Flpb, ped/bikes	1.00	1.00		1.00	1.00	1.00		1.00			1.00	
Fr _t	1.00	0.98		1.00	1.00	0.85		0.99			0.90	
Flt Protected	0.95	1.00		0.95	1.00	1.00		0.96			0.99	
Satd. Flow (prot)	1614	3167		1554	3107	1343		1645			1456	
Flt Permitted	0.95	1.00		0.95	1.00	1.00		0.53			0.91	
Satd. Flow (perm)	1614	3167		1554	3107	1343		901			1338	
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	178	1363	169	8	1192	25	157	18	16	44	16	147
RTOR Reduction (vph)	0	7	0	0	0	13	0	3	0	0	74	0
Lane Group Flow (vph)	178	1525	0	8	1192	12	0	188	0	0	133	0
Confl. Peds. (#/hr)						4					4	
Confl. Bikes (#/hr)			2			1						
Heavy Vehicles (%)	3%	3%	3%	7%	7%	7%	1%	1%	1%	6%	6%	6%
Turn Type	Prot	NA		Prot	NA	Perm	Perm	NA		Perm	NA	
Protected Phases	5	2		1	6			4			8	
Permitted Phases						6	4				8	
Actuated Green, G (s)	16.7	74.0		1.0	58.3	58.3		31.5			31.5	
Effective Green, g (s)	16.7	74.0		1.0	58.3	58.3		31.5			31.5	
Actuated g/C Ratio	0.14	0.62		0.01	0.49	0.49		0.26			0.26	
Clearance Time (s)	4.5	4.5		4.5	4.5	4.5		4.5			4.5	
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0		3.0			3.0	
Lane Grp Cap (vph)	224	1952		12	1509	652		236			351	
v/s Ratio Prot	c0.11	c0.48		0.01	0.38							
v/s Ratio Perm						0.01		c0.21			0.10	
v/c Ratio	0.79	0.78		0.67	0.79	0.02		0.80			0.38	
Uniform Delay, d1	50.0	17.0		59.3	25.7	16.0		41.3			36.2	
Progression Factor	1.00	1.00		1.00	1.00	1.00		1.00			1.00	
Incremental Delay, d2	17.4	3.2		89.5	4.3	0.1		23.7			0.7	
Delay (s)	67.4	20.2		148.8	30.0	16.1		65.0			36.9	
Level of Service	E	C		F	C	B		E			D	
Approach Delay (s)		25.1			30.5			65.0			36.9	
Approach LOS		C			C			E			D	
Intersection Summary												
HCM 2000 Control Delay		30.1					HCM 2000 Level of Service		C			
HCM 2000 Volume to Capacity ratio		0.81										
Actuated Cycle Length (s)		120.0					Sum of lost time (s)		13.5			
Intersection Capacity Utilization		87.8%					ICU Level of Service		E			
Analysis Period (min)		15										
c Critical Lane Group												

HCM 6th Signalized Intersection Summary
1: Wolf Drive/Ten Eyck Road & Highway 26

06/01/2021

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑↑		↑	↑↑	↑	↔	↔		↔	↔	↔
Traffic Volume (veh/h)	169	1295	161	8	1132	24	149	17	15	42	15	140
Future Volume (veh/h)	169	1295	161	8	1132	24	149	17	15	42	15	140
Initial Q (Q _b), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00			1.00		0.98	1.00			1.00	1.00	1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No			No			No			No		No
Adj Sat Flow, veh/h/ln	1709	1709	1709	1654	1654	1654	1736	1736	1736	1668	1668	1668
Adj Flow Rate, veh/h	178	1363	169	8	1192	25	157	18	16	44	16	147
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Percent Heavy Veh, %	3	3	3	7	7	7	1	1	1	6	6	6
Cap, veh/h	204	1786	220	15	1571	684	264	30	22	102	50	282
Arrive On Green	0.13	0.62	0.62	0.01	0.50	0.50	0.26	0.26	0.26	0.26	0.26	0.26
Sat Flow, veh/h	1628	2902	357	1576	3143	1368	799	113	83	249	189	1072
Grp Volume(v), veh/h	178	758	774	8	1192	25	191	0	0	207	0	0
Grp Sat Flow(s), veh/h/ln	1628	1624	1636	1576	1572	1368	995	0	0	1510	0	0
Q Serve(g_s), s	12.9	40.4	41.5	0.6	36.7	1.1	9.3	0.0	0.0	0.0	0.0	0.0
Cycle Q Clear(g_c), s	12.9	40.4	41.5	0.6	36.7	1.1	23.1	0.0	0.0	13.8	0.0	0.0
Prop In Lane	1.00			1.00		1.00	0.82			0.08	0.21	0.71
Lane Grp Cap(c), veh/h	204	999	1006	15	1571	684	316	0	0	433	0	0
V/C Ratio(X)	0.87	0.76	0.77	0.52	0.76	0.04	0.60	0.00	0.00	0.48	0.00	0.00
Avail Cap(c_a), veh/h	251	999	1006	66	1571	684	316	0	0	433	0	0
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(l)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	0.00	1.00	0.00	0.00
Uniform Delay (d), s/veh	51.6	16.7	16.9	59.1	24.2	15.3	42.3	0.0	0.0	37.9	0.0	0.0
Incr Delay (d2), s/veh	23.6	5.4	5.7	24.7	3.5	0.1	8.3	0.0	0.0	0.8	0.0	0.0
Initial Q Delay(d3), s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%), veh/ln	6.6	15.9	16.5	0.3	14.2	0.4	6.0	0.0	0.0	5.3	0.0	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d), s/veh	75.2	22.0	22.5	83.8	27.7	15.4	50.6	0.0	0.0	38.7	0.0	0.0
LnGrp LOS	E	C	C	F	C	B	D	A	A	D	A	A
Approach Vol, veh/h	1710				1225			191			207	
Approach Delay, s/veh	27.8				27.8			50.6			38.7	
Approach LOS	C				C			D			D	
Timer - Assigned Phs	1	2		4	5	6		8				
Phs Duration (G+Y+R _c), s	5.7	78.3		36.0	19.5	64.5		36.0				
Change Period (Y+R _c), s	4.5	4.5		4.5	4.5	4.5		4.5				
Max Green Setting (Gmax), s	5.0	70.0		31.5	18.5	56.5		31.5				
Max Q Clear Time (g_c+l1), s	2.6	43.5		25.1	14.9	38.7		15.8				
Green Ext Time (p_c), s	0.0	14.2		0.6	0.2	8.9		1.1				
Intersection Summary												
HCM 6th Ctrl Delay				29.8								
HCM 6th LOS				C								

Intersection

Int Delay, s/veh 3.7

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑↑	↗	↖	↑↑	↖	↗
Traffic Vol, veh/h	1210	126	40	1149	40	46
Future Vol, veh/h	1210	126	40	1149	40	46
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	160	215	-	120	0
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	3	3	7	7	3	3
Mvmt Flow	1274	133	42	1209	42	48

Major/Minor	Major1	Major2	Minor1	
Conflicting Flow All	0	0	1407	0 1963 637
Stage 1	-	-	-	- 1274 -
Stage 2	-	-	-	- 689 -
Critical Hdwy	-	-	4.24	- 6.86 6.96
Critical Hdwy Stg 1	-	-	-	- 5.86 -
Critical Hdwy Stg 2	-	-	-	- 5.86 -
Follow-up Hdwy	-	-	2.27	- 3.53 3.33
Pot Cap-1 Maneuver	-	-	456	- 55 418
Stage 1	-	-	-	- 225 -
Stage 2	-	-	-	- 457 -
Platoon blocked, %	-	-	-	-
Mov Cap-1 Maneuver	-	-	456	- 50 418
Mov Cap-2 Maneuver	-	-	-	- 50 -
Stage 1	-	-	-	- 225 -
Stage 2	-	-	-	- 415 -

Approach	EB	WB	NB
HCM Control Delay, s	0	0.5	105.7
HCM LOS		F	

Minor Lane/Major Mvmt	NBLn1	NBLn2	EBT	EBR	WBL	WBT
Capacity (veh/h)	50	418	-	-	456	-
HCM Lane V/C Ratio	0.842	0.116	-	-	0.092	-
HCM Control Delay (s)	210.4	14.7	-	-	13.7	-
HCM Lane LOS	F	B	-	-	B	-
HCM 95th %tile Q(veh)	3.5	0.4	-	-	0.3	-

Intersection

Int Delay, s/veh 7.2

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	13	54	56	31	56	53	62	307	62	40	366	14
Future Vol, veh/h	13	54	56	31	56	53	62	307	62	40	366	14
Conflicting Peds, #/hr	2	0	0	0	0	2	0	0	0	2	0	2
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None									
Storage Length	-	-	90	-	-	125	-	-	-	-	-	330
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	95	95	95	95	95	95	95	95	95	95	95	95
Heavy Vehicles, %	1	1	1	5	5	5	2	2	2	5	5	5
Mvmt Flow	14	57	59	33	59	56	65	323	65	42	385	15

Major/Minor	Minor2	Minor1			Major1			Major2				
Conflicting Flow All	1016	991	387	1023	974	360	402	0	0	390	0	0
Stage 1	471	471	-	488	488	-	-	-	-	-	-	-
Stage 2	545	520	-	535	486	-	-	-	-	-	-	-
Critical Hdwy	7.11	6.51	6.21	7.15	6.55	6.25	4.12	-	-	4.15	-	-
Critical Hdwy Stg 1	6.11	5.51	-	6.15	5.55	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.11	5.51	-	6.15	5.55	-	-	-	-	-	-	-
Follow-up Hdwy	3.509	4.009	3.309	3.545	4.045	3.345	2.218	-	-	2.245	-	-
Pot Cap-1 Maneuver	217	247	663	211	249	678	1157	-	-	1152	-	-
Stage 1	575	561	-	556	545	-	-	-	-	-	-	-
Stage 2	524	534	-	524	546	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	144	218	662	140	219	675	1155	-	-	1150	-	-
Mov Cap-2 Maneuver	144	218	-	140	219	-	-	-	-	-	-	-
Stage 1	532	534	-	515	505	-	-	-	-	-	-	-
Stage 2	393	494	-	406	519	-	-	-	-	-	-	-

Approach	EB	WB			NB			SB				
HCM Control Delay, s	22.9	31			1.2			0.8				
HCM LOS	C	D										
<hr/>												
Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	EBLn2	WBLn1	WBLn2	SBL	SBT	SBR		
Capacity (veh/h)	1155	-	-	198	662	182	675	1150	-	-		
HCM Lane V/C Ratio	0.057	-	-	0.356	0.089	0.503	0.083	0.037	-	-		
HCM Control Delay (s)	8.3	0	-	32.9	11	43.3	10.8	8.2	0	-		
HCM Lane LOS	A	A	-	D	B	E	B	A	A	-		
HCM 95th %tile Q(veh)	0.2	-	-	1.5	0.3	2.5	0.3	0.1	-	-		

Intersection

Int Delay, s/veh 6.8

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑		↔	↔		↔	↔		↔	↔	
Traffic Vol, veh/h	30	34	5	2	22	30	4	10	1	78	28	38
Future Vol, veh/h	30	34	5	2	22	30	4	10	1	78	28	38
Conflicting Peds, #/hr	2	0	1	3	0	4	1	0	3	4	0	2
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None									
Storage Length	115	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	91	91	91	91	91	91	91	91	91	91	91	91
Heavy Vehicles, %	2	2	2	2	2	2	7	7	7	3	3	3
Mvmt Flow	33	37	5	2	24	33	4	11	1	86	31	42

Major/Minor	Minor2	Minor1			Major1			Major2				
Conflicting Flow All	278	250	57	272	271	20	75	0	0	16	0	0
Stage 1	226	226	-	24	24	-	-	-	-	-	-	-
Stage 2	52	24	-	248	247	-	-	-	-	-	-	-
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.17	-	-	4.13	-	-
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.263	-	-	2.227	-	-
Pot Cap-1 Maneuver	674	653	1009	680	636	1058	1493	-	-	1595	-	-
Stage 1	777	717	-	994	875	-	-	-	-	-	-	-
Stage 2	961	875	-	756	702	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	601	611	1004	611	595	1050	1490	-	-	1589	-	-
Mov Cap-2 Maneuver	601	611	-	611	595	-	-	-	-	-	-	-
Stage 1	773	675	-	987	869	-	-	-	-	-	-	-
Stage 2	899	869	-	668	661	-	-	-	-	-	-	-

Approach	EB	WB			NB			SB				
HCM Control Delay, s	11.1	10				2				4		
HCM LOS	B	B										
Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	EBLn2	WBLn1	SBL	SBT	SBR			
Capacity (veh/h)	1490	-	-	601	643	785	1589	-	-			
HCM Lane V/C Ratio	0.003	-	-	0.055	0.067	0.076	0.054	-	-			
HCM Control Delay (s)	7.4	0	-	11.3	11	10	7.4	0	-			
HCM Lane LOS	A	A	-	B	B	B	A	A	-			
HCM 95th %tile Q(veh)	0	-	-	0.2	0.2	0.2	0.2	-	-			

Intersection

Intersection Delay, s/veh 15.1

Intersection LOS C

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	11	9	34	62	45	74	46	315	17	14	190	3
Future Vol, veh/h	11	9	34	62	45	74	46	315	17	14	190	3
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles, %	1	1	1	5	5	5	2	2	2	5	5	5
Mvmt Flow	12	10	38	69	50	82	51	350	19	16	211	3
Number of Lanes	0	1	1	0	1	1	0	1	0	0	1	1
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	2			2			2			1		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	2			1			2			2		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	1			2			2			2		
HCM Control Delay	9.6			10.7			19.5			12.2		
HCM LOS	A			B			C			B		

Lane	NBLn1	EBLn1	EBLn2	WBLn1	WBLn2	SBLn1	SBLn2
Vol Left, %	12%	55%	0%	58%	0%	7%	0%
Vol Thru, %	83%	45%	0%	42%	0%	93%	0%
Vol Right, %	4%	0%	100%	0%	100%	0%	100%
Sign Control	Stop						
Traffic Vol by Lane	378	20	34	107	74	204	3
LT Vol	46	11	0	62	0	14	0
Through Vol	315	9	0	45	0	190	0
RT Vol	17	0	34	0	74	0	3
Lane Flow Rate	420	22	38	119	82	227	3
Geometry Grp	6	7	7	7	7	7	7
Degree of Util (X)	0.667	0.044	0.064	0.227	0.134	0.377	0.005
Departure Headway (Hd)	5.72	7.087	6.09	6.862	5.854	5.992	5.248
Convergence, Y/N	Yes						
Cap	630	504	586	523	612	601	681
Service Time	3.754	4.844	3.847	4.609	3.6	3.734	2.989
HCM Lane V/C Ratio	0.667	0.044	0.065	0.228	0.134	0.378	0.004
HCM Control Delay	19.5	10.2	9.3	11.6	9.5	12.3	8
HCM Lane LOS	C	B	A	B	A	B	A
HCM 95th-tile Q	5	0.1	0.2	0.9	0.5	1.7	0

Intersection

Intersection Delay, s/veh 23.9

Intersection LOS C

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔	↑		↔	↑		↔			↔	↑
Traffic Vol, veh/h	13	54	56	31	56	53	62	307	62	40	366	14
Future Vol, veh/h	13	54	56	31	56	53	62	307	62	40	366	14
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles, %	1	1	1	5	5	5	2	2	2	5	5	5
Mvmt Flow	14	57	59	33	59	56	65	323	65	42	385	15
Number of Lanes	0	1	1	0	1	1	0	1	0	0	1	1
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	2			2			2			1		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	2			1			2			2		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	1			2			2			2		
HCM Control Delay	11.3			11.7			29.6			25.7		
HCM LOS	B			B			D			D		

Lane	NBLn1	EBLn1	EBLn2	WBLn1	WBLn2	SBLn1	SBLn2
Vol Left, %	14%	19%	0%	36%	0%	10%	0%
Vol Thru, %	71%	81%	0%	64%	0%	90%	0%
Vol Right, %	14%	0%	100%	0%	100%	0%	100%
Sign Control	Stop						
Traffic Vol by Lane	431	67	56	87	53	406	14
LT Vol	62	13	0	31	0	40	0
Through Vol	307	54	0	56	0	366	0
RT Vol	62	0	56	0	53	0	14
Lane Flow Rate	454	71	59	92	56	427	15
Geometry Grp	6	7	7	7	7	7	7
Degree of Util (X)	0.796	0.152	0.113	0.199	0.107	0.757	0.023
Departure Headway (Hd)	6.315	7.741	6.919	7.828	6.923	6.376	5.626
Convergence, Y/N	Yes						
Cap	576	463	518	459	517	568	640
Service Time	4.329	5.491	4.668	5.577	4.671	4.088	3.326
HCM Lane V/C Ratio	0.788	0.153	0.114	0.2	0.108	0.752	0.023
HCM Control Delay	29.6	11.9	10.6	12.5	10.5	26.3	8.5
HCM Lane LOS	D	B	B	B	B	D	A
HCM 95th-tile Q	7.7	0.5	0.4	0.7	0.4	6.7	0.1

CITY OF SANDY, CLACKAMAS COUNTY

TEN EYCK RD at PROCTOR BLVD, CITY of Sandy, Clackamas County, 01/01/2015 to 12/31/2019
1 - 4 of 5 Crash records shown.

S	D	M	CLASS	CITY STREET	RD CHAR	OFFRD	WTHR	CRASH	MOVE	A	S	PED	ACT	EVENT	CAUSE			
SER#	P	R J S W	DATE	DIST	INT- TYPE (MEDIAN)	INT-REL	RNDBT	SURF	FROM	FRTC	INJ	G E	LICNS	LOC	ERROR			
INVEST	E A U	I C O	DAY	FROM	LEGS	TRAF-	DRWY	LIGHT	TO	P#	TYPE	SURVY	E X	RES				
RD DFT	B L G N H R	TIME		UNLOCK?	DCS V L K LAT	#LANES)	CONTL											
03911	N	N	N	10/27/2018	17	PROCTOR BLVD	INTER	5-LLEG	N	N	S-1STOP	0	STRGHT		29			
NONE	SA	0	SE TEN EYCK RD	NE	TRF SIGNAL	N	UNK	REAR	PRVTE	NE-SW	01 DRVR	NONE	70 M	OR-Y OR<25	000			
N	5P	45 23 49.25	-122.15	002600200500	06	0	DUSK	INJ	PSNGR CAR		01 DRVR	NONE	70 M	OR-Y OR<25	026			
N	5P	45 23 49.25	-122.15	19.74					02 NONE	0	STOP	NE-SW	01 DRVR	INJ/C	55 F	OR-Y OR<25		
03089	N	N	N	09/03/2018	14	PROCTOR BLVD	INTER	5-LLEG	N	CLR	S-1STOP	0	STRGHT		29			
NONE	MO	SE TEN EYCK RD	SE	TRF SIGNAL	N	DRY	REAR	UNKN	PSNGR CAR	SE-NW	01 DRVR	NONE	00 F	UNK	000			
N	3P	45 23 49.25	-122.15	002600200500	06	0	DAY	INJ	PSNGR CAR		01 DRVR	NONE	00 F	UNK	026			
N	3P	45 23 49.25	-122.15	19.75					02 NONE	0	STOP	SE-NW	01 DRVR	INJ/C	25 F	OR-Y OR<25		
03213	N	N	N	09/17/2019	14	PROCTOR BLVD	INTER	5-LLEG	N	CLR	S-1STOP	0	STRGHT		29			
NONE	TU	SE TEN EYCK RD	SE	TRF SIGNAL	N	DRY	REAR	PRVTE	PSNGR CAR	SE-NW	01 DRVR	NONE	41 F	OTH-Y N/RES	000			
N	3P	45 23 49.26	-122.15	002600200500	06	0	DAY	INJ	PSNGR CAR		02 NONE	0	STOP	SE-NW	01 DRVR	INJ/C	48 F	OR-Y OR<25
N	3P	45 23 49.26	-122.15	19.69					02 NONE	0	STOP	SE-NW	01 DRVR	INJ/C	48 F	OR-Y OR<25		
05173	N	N	N	11/08/2016	14	PROCTOR BLVD	INTER	5-LLEG	N	CLR	ANGL-OTH	0	NON	9	U-TURN	06		
NONE	TU	SE TEN EYCK RD	W	TRF SIGNAL	N	DRY	TURN	N/A	PSNGR CAR	W-W	01 DRVR	NONE	00 UNK	UNK	000			
N	5P	45 23 49.25	-122.15	002600200500	05	0	DUSK	PDO	PSNGR CAR		02 NONE	9	TURN-R	NE-W	01 DRVR	NONE	00 UNK	
N	5P	45 23 49.25	-122.15	19.74					N/A	PSNGR CAR								
04335	N	N	N	11/06/2018	14	PROCTOR BLVD	INTER	5-LLEG	N	CLR	ANGL-OTH	0	NON	9	STRGHT	02		
NONE	TU	SE TEN EYCK RD	CN	TRF SIGNAL	N	DRY	TURN	N/A	PSNGR CAR	SE-NW	01 DRVR	NONE	00 UNK	UNK	000			
N	2P	45 23 49.25	-122.15	002600200500	01	0	DAY	PDO	SEMI TOW		01 DRVR	NONE	00 UNK	UNK	000			
N	2P	45 23 49.25	-122.15	19.72														

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CITY OF SANDY, CLACKAMAS COUNTY

TEN EYCK RD at PROCTOR BLVD, CITY of Sandy, Clackamas County, 01/01/2015 to 12/31/2019

5 - 5 of 5 Crash records shown.

S	D	M	CLASS	CITY STREET	RD CHAR	INT-TYPE (MEDIAN)	OFFRD	WTHR	CRASH	SPCL USE TRLR QTY	MOVE	A	S	FRTC	INJ	G	E	LICNS	PED	ACT	EVENT	CAUSE
SER#	P	R J S W DATE	DI ST	FIRST STREET	DIRECT	INT-REL LEGS	RNDBT	SURF	COLL	FROM	FROM											
INVEST	B A U I C O DAY	FROM	SECOND STREET	LOCTN	CONTL	TRAF- (#LANES)	DRWY	LIGHT	SURVY	V# TYP E	TO	P# TYPE	SURVY	E	X	RES	LOC	ERROR	ACT	EVENT		
RD DFT	B L G N H R TIME	LONG	LRS							02	NONE	9										
UNLOCK?	D C S V L K LAT									N/A	TURN-R NE-NW											
										PSNGR CAR	01 DRVR	NONE	00	Unk UNK	000	000	000	000	000	000	000	

CITY OF SANDY, CLACKAMAS COUNTY

TEN EYCK RD at PIONEER BLVD, City of Sandy, Clackamas County, 01/01/2015 to 12/31/2019
1 - 1 of 1 Crash records shown.

S	D	M	P	R	J	S	W	DATE	CLASS	CITY STREET	RD CHAR	INT- TYPE (MEDIAN)	OFFRD	WTHR	CRASH	SPCL USE	A	S	
INVEST	E	A	U	I	C	O	DAY	DIST	FIRST STREET	DIRECT	LEGS	INT-REL	RNDBT	SURF	TRLR QTY	MOVE	G	B	
RD DFT	B	L	G	N	H	R	TIME	FROM	SECOND STREET	LOCN	(#LANES)	TRAF-	COLL	DRWY	SVRTY	OWNER	LICNS	PED	
03787	N	N	N	N	N	N	N	09/15/2015	14	PIONEER BLVD	INTER	5 -LEG	N	CLR	S-1STOP	0	X	RES	
NONE	TU								SE TEN EYCK RD	E	TRF SIGNAL	N	DRY	REAR	PRVTE	E -W			
N	1P	45	23	49	24	-122	15	002600010800	19.74	06	0		N	DAY	INT	PSNGR CAR	01	DRVR	
N																OR<25	026	000	
												0.2	NONE	0	STOP				
															PRVTE	E -W			
															PSNGR CAR	01	DRVR	INT/C	0.00
																0TH-Y	3.8	F	0.00
																N-RES			0.11
																			0.0

CITY OF SANDY, CLACKAMAS COUNTY

WOLF DR at PIONEER BLVD, City of Sandy, Clackamas County, 01/01/2015 to 12/31/2019
1 - 2 of 2 Crash records shown.

SER#	D	M	P	R	J	S	W	DATE	CLASS	CITY STREET	RD CHAR	INT- TYPE (MEDIAN)	OFFRD	WTHR	CRASH	SPCL USE	MOVE	A	S
INVEST	E	A	U	I	C	O	DAY	DI ST	FIRST STREET	DIRECT	RDCTN	TRAF- LEGS	RNDBT	SURF	TRLR QTY	FROM	FRTC	INJ	G
RD DFT	B	L	G	N	H	R	TIME	FROM	SECOND STREET	LOC TN	(#LANES)	DRWY	LIGHT	SVRTY	V# TYP E	TO	P# TYPE	ACT	EVNT
UNLOCK?	D	C	S	V	L	K	LAT	LONG	LRS	INTER	5-LLEG	N	CLR	ANGL-OTH	01 NONE	STRGHT		CAUSE	
01741	N	N	N	N	05/09/2015	14			PIONEER BLVD	CN	TRF SIGNAL	N	DRY	ANGL	PRVTE	S -N		000	00
NONE			SA					WOLF DR	04	0							01 DRV R	NONE	020
N			6A	45	23	49	.25	-122.15	0026000100800	19.74								M OTH-Y OR<25	020
																		04	
00512	N	N	N	N	02/07/2017	14			PIONEER BLVD	INTER	5-LLEG	N	RAIN	ANGL-OTH	01 NONE	STRGHT			
CITY			TU					WOLF DR	CN	TRF SIGNAL	N	WET	TURN	PRVTE	S -W			000	00
N			4P	45	23	49	.25	-122.15	0026000100800	19.74	04	0						000	00
																		04	

CITY OF SANDY, CLACKAMAS COUNTY

MT HOOD HY at LANGENSAND RD, City of Sandy, Clackamas County, 01/01/2015 to 12/31/2019
 1 - 4 of 7 Crash records shown.

SER#	R	D	M	CLASS	CITY STREET	RD CHAR	OFFRD	WTHR	CRASH	MOVE	FRTC	INJ	A	S	CAUSE		
INVEST	P	R	J	S	W DATE	FIRST STREET	RNDBT	SURF	COLL	FROM	G	E	LICNS	PED			
RD DFT	B	L	G	N	H	TIME	LEGS	DRWY	LIGHT	TO	P#	TYPE	SVRTY	E	LOC	ERROR	
UNLOC?	D	C	S	V	L	K	LAT	(#LANES)	CONTL	01	NONC	0	TURN-R			19, 02	
05355	N	N	N	11/18/2016	14	LANGENSAND RD	INTER	3 -LEG	N	RAIN	PED	PRVTE	S -SE			015	
CITY	FR			MT HOOD HY		S	UNKNOWN	N	WET	PED	PSNGR CAR	01 DRVR	NONE	59 M	OR-Y OR>25	00	
N	6P	45	23	44.19	-122.15	0.03	002600100800	06	O	DUSK	INT			029	000	02	
																19	
05056	N	N	N	12/01/2017	14	LANGENSAND RD	INTER	3 -LEG	N	RAIN	ANGL-STP	01	NONE	0	TURN-R	08	
NONE	FR			MT HOOD HY		S	STOP	SIGN	N	WET	TURN	PRVTE	NW-S			000	
N	9P	45	23	44.19	-122.15	0.03	002600100800	06	O	DLTT	INJ	PSNGR CAR	01 DRVR	NONE	27 F	OR-Y OR<25	001
N																08	
01431	N	N	N	04/14/2017	14	LANGENSAND RD	INTER	3 -LEG	N	CLR	O-1STOP	01	NONE	9	BACK	1.0	
NONE	FR			MT HOOD HY		S	STOP	SIGN	N	DRY	BACK	N/A	N -S			000	
N	3P	45	23	44.19	-122.15	0.03	002600100800	06	O	DAY	PDO	PSNGR CAR					000
N																00	
00297	N	N	N	01/24/2015	14	LANGENSAND RD	INTER	3 -LEG	N	CLR	O-1 L- TURN	01	NONE	0	STRGHT	02, 08	
CITY	SA			MT HOOD HY		CN	UNKNOWN	N	DRY	TURN	PRVTE	NW-SE					000
N	4P	45	23	44.19	-122.15	0.03	002600100800	03	O	DAY	INJ	PSNGR CAR	01 DRVR	NONE	21 M	OR-Y OR<25	000
N																00	

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CITY OF SANDY, CLACKAMAS COUNTY

MT HOOD HY at LANGENSAND RD, City of Sandy, Clackamas County, 01/01/2015 to 12/31/2019

5 - 7 of 7 Crash records shown.

SER#	D	M	CLASS	CITY STREET	RD CHAR	INT- TYPE (MEDIAN)	OFFRD	WTHR	CRASH	MOVE	FRTC	INJ	A	S	PED	ACT	EVENT	CAUSE	
INVEST	P	R J S W DATE	DI ST	FIRST STREET	DIRECT	TRAF- LEGS	RNDBT	SURF	COLL	FROM	P#	TYPE	SYRTY	E	X	RES	LOC	ERROR	
RD DFT	B L G N H R TIME	FROM	SECOND STREET	LOCTN	CONTL	DRWY	LIGHT	SVRTY	01	NON E	TURN-L								
UNLOCK?	D C S V L K LAT	LONG	LRS	INTER	3 -LEG	N	RAIN	ANGL-OTH	0										
04571	N N Y	N N 10/05/2016	14	LANGENSAND RD	CN	STOP SIGN	N	WET	TURN	PRVTE	S -W	01	DRV R	NONE	21	M	OR-Y OR<25	013	02,08
CITY	WE	MT HOOD HY			04	0			DUSK	INJ	PSNGR CAR								00
N	6P	45 23 44 .19	-122 15	002600100800							02	NONE	0	STRGHT					00
											PRVTE	PNNGR CAR							00
											03	NONE	0	STRGHT					00
											PRVTE	PSNGR CAR							00
											03	NONE	0	STRGHT					00
											PRVTE	PSNGR CAR							00
03612	N N N	N N 10/16/2019	14	LANGENSAND RD	INTER	3 -LEG	N	RAIN	ANGL-OTH	01	NONE	9	TURN-L						02
CITY	WE	MT HOOD HY			CN	STOP SIGN	N	WET	TURN	N/A		S -NW							00
N	2P	45 23 44 .19	-122 15	002600100800	02	0			DAY	PDO	PSNGR CAR								00
											02	NONE	9	STRGHT					00
											N/A	PSNGR CAR							00
04040	N N N	11/14/2019	14	LANGENSAND RD	INTER	3 -LEG	N	CLR	ANGL-OTH	01	NONE	9	STRGHT						02
NONE	TH	MT HOOD HY			CN	STOP SIGN	N	DRY	TURN	N/A		E -W							00
N	8A	45 23 44 .2	-122 15	002600100800	02	0			DAWN	PDO	SEMI TOW								00
											02	NONE	9	TURN-L					00
											N/A	PSNGR CAR							00

CITY OF SANDY, CLACKAMAS COUNTY

DUBARKO RD at EAGLE CRK-SANDY HY, City of Sandy, Clackamas County, 01/01/2015 to 12/31/2019
1 - 4 of 27 Crash records shown.

S	D	M		CITY STREET	RD CHAR	INT-TYPE (MEDIAN)	OFFRD	WTHR	CRASH	MOVE	FRTC	INJ	A	S	CAUSE					
SER#	P	R	J	STREET	DIRECT	TRAF- LEGS	RNDBT	SURF	COLL	FROM	P# TYPE	G	E	LICNS	PED					
INVEST	B	A	U	FIRST STREET	DIRECT	(#LANES)	DRWY	LIGHT	SVRY	TO	P# TYPE	SVRY	E	X	RES	LOC	ERROR	ACT	EVENT	
RD DFT	B	L	G	SECOND STREET	LOCTN	CONT'L													27,29,32	
UNLOCK?	D	C	S	V L	K LAT	LONG														
02296	N	N	N	N	07/06/2019	16	DUBARKO RD	INTER	CROSS	N	CLL	S-1STOP	0	NONE	9	STRGHT				
CITY									NONE	N	DRY	REAR	N/A			NE-SW				
	SA									N	DAY	PDO	PSNGR CAR				01 DRVR	NONE	0	Unk UNK UNK
N		11A							0	N	DAY						00	000	000	00
N		45	23	22	.65	-122	15	017200100S00	48	.74										
01165	N	N	N	N	03/10/2016	16	DUBARKO RD	INTER	CROSS	N	CLL	O-1STOP	0	NONE	0	BACK				10
NONE									STOP SIGN	N	DRY	BACK	PRVTE			W-E				000
N		TH								N	DAY	INJ	PSNGR CAR				01 DRVR	NONE	22	M OR-Y OR<25
N		6P							0	N	DAY						01	000	000	10
N		45	23	22	.76	-122	15	017200100S00	48	.39										
04008	N	N	N	N	11/02/2018	16	DUBARKO RD	INTER	CROSS	N	CLL	PED	0	NONE	0	STRGHT				02
CITY									STOP SIGN	N	WET	PED	PRVTE			E-W				000
N		FR								N	DLIT	INJ	PSNGR CAR				01 DRVR	NONE	74	M OR-Y OR<25
N		7P							0	N	DLIT						029	000	000	02
N		45	23	22	.54	-122	15	017200100S00	48	.5										
03026	N	N	N	N	07/27/2015	16	DUBARKO RD	INTER	CROSS	N	CLL	S-1STOP	0	NONE	0	STRGHT				07,29
CITY									STOP SIGN	N	DRY	REAR	PRVTE			SW-NE				00
N		MO								N	DUSK	INJ	PSNGR CAR				01 DRVR	INJTC	19	M OR-Y OR<25
N		8P							0	N							043	026	000	07,29
N		45	23	22	.76	-122	15	017200100S00	48	.39										
01095	N	N	N	N	03/04/2016	16	DUBARKO RD	INTER	CROSS	N	CLL	S-1STOP	0	NONE	0	STRGHT				27,07,32
CITY									STOP SIGN	N	SS-O	PRVTE				NE-SW				000
N		FR								N	DAY	INJ	PSNGR CAR				01 DRVR	NONE	30	M OR-Y OR<25
N		4P							0	N							016	043	010	27,07,32
N		45	23	22	.76	-122	15	017200100S00	48	.39										

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CITY OF SANDY, CLACKAMAS COUNTY

DUBARKO RD at EAGLE CRK-SANDY HY, City of Sandy, Clackamas County, 01/01/2015 to 12/31/2019
5 - 8 cf 27 Crash records shown.

S	D	M	P	R	J	S	W	DATE	CLASS	CITY STREET	RD CHAR	INT- TYPE (MEDIAN)	OFFRD	WTHR	CRASH	MOVE	FROM	FRTC	INJ	A	S	CAUSE					
SER#	P	R	A	U	I	C	O	DAY	CLASS	FIRST STREET	DIRECT	TRAF- LEGS	RNDBT	SURF	COLL	OWNER	G	E	LICNS	PED	ACT	EVENT					
INVEST	E	B	A	U	I	C	O	TIME	FROM	SECOND STREET	LOCNTN	(#LANES)	DRWY	LIGHT	SURVY	V# TYP	P# TYPE	SYRVTY	E	X	RES	LOC	ERROR				
RD DFT	B	L	G	N	H	R	TIME	LONG	LONG	LONG	LOCTN	CONTNL	DRWY	LIGHT	SURVY	02	NONE	0	STOP	NE-SW	OR-Y	000	012	00			
UNLOCK?	D	C	S	V	L	K	LAT									PRVTE	PSNGR	CAR	01	DRV	INJB	26	M	OR-Y	000	000	00
00763	N	N	N	N	02/17/2016	16	DUBARKO RD	INTER	CROSS	N	N	RAIN	S-1STOP	01	NONE	9	STRGHT							07	000	00	
CITY	WE						EAGLE CRK-SANDY HY	SW		NONE	N	WET	REAR	N/A		S-N								000	000	00	
N	5P							06	0		N	DLIT	PDO		PSNGR	CAR	01	DRV	NONE	00	Unk UNK	UNK	000	000	00		
N	45	23	22	76	-122	15		017200100500									02	NONE	9	STOP	S-N				012	000	00
01324	N	N	N	N	04/19/2018	16	DUBARKO RD	INTER	CROSS	N	N	CLR	S-1STOP	01	NONE	0	STRGHT							29	000	00	
CITY	TH						EAGLE CRK-SANDY HY	SW		UNKNOWN	N	DRY	REAR		PRVTE		SW-NE							026	000	29	
N	6P							06	0		N	DAY	INJ		PSNGR	CAR	01	DRV	NONE	19	M	OR-Y	000	000	00		
N	45	23	22	55	-122	15		017200100500									02	NONE	0	STOP	SW-NE				012	000	00
04952	N	N	N	N	11/22/2015	16	DUBARKO RD	INTER	CROSS	N	N	CLD	ANGL-OTH	01	NONE	0	TURN-L							03	000	00	
CITY	SU						EAGLE CRK-SANDY HY	CN		STOP SIGN	N	DRY	TURN		PRVTE		W-NE							000	000	03	
N	4P							03	0		N	DAY	INJ		PSNGR	CAR	01	DRV	INJB	53	F	OTH-Y	021	000	03		
N	45	23	22	76	-122	15		017200100500									02	NONE	0	STRGHT	NE-SW				000	000	00

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CITY OF SANDY, CLACKAMAS COUNTY

DUBARKO RD at EAGLE CRK-SANDY HY, City of Sandy, Clackamas County, 01/01/2015 to 12/31/2019
9 - 12 of 27 Crash records shown.

S	D	M		CITY STREET	RD CHAR	RD TYPE	OFFRD	WTHR	CRASH	MOVE	FRTC	INJ	A	S									
SER#	P	R	J	S	W	DATE	INT- DIST	(MEDIAN)	INT-REL	FROM	G	E	LICNS	PED	ACT	EVENT	CAUSE						
INVEST	E	A	U	I	O	DAY	FIRST STREET	LEGS	RNDBT	COLL	P#	TYPE	SVRTY	E	X	RES	LOC	ERROR					
RD DFT	B	L	G	N	H	R	FROM	(#LANES)	CONT'L	DRWY	TO	P#	TYPE	SVRTY	E	X	RES	LOC	ERROR				
UNLOCK?	D	C	S	V	L	K	LAT	LRS	LOCTN	DRWY	STRTGHT	01	NON	0	STRTGHT					02			
05614	N	N	N	N	N	12/25/2015	16	DUBARKO RD	INTER	CROSS	N	CLD	ANGL-OTH	0	STRTGHT					00			
CITY		FR						EAGLE CRK-SANDY HY	CN	STOP SIGN	N	WET	ANGL		PRVTE	N	-S			00			
N		6P	45	23	22	.76	-122	15	017200100S00	01	0	DLIT	INJ	PSNGR CAR	01	DRV	NONE	58	M	OR-Y OR>25	00		
N										02	NONE	0	STRTGHT	E -W	PRVTE	PSNGR CAR	01	DRV	INJC	53	M	OR-Y OR<25	015
02172	N	N	N	N	N	06/05/2015	16	DUBARKO RD	INTER	CROSS	N	CLR	ANGL-OTH	01	NONE	0	STRTGHT					02	
NONE		FR						EAGLE CRK-SANDY HY	CN	STOP SIGN	N	DRY	TURN		PRVTE	W	-E				00		
N		7A	45	23	22	.76	-122	15	017200100S00	04	0	PDO	PSNGR CAR	01	DRV	NONE	24	M	OR-Y OR<25	028			
N		6P	45	23	22	.76	-122	15	017200100S00			02	NONE	0	TURN - L	SW - W	01	DRV	INJC	77	M	OR-Y OR<25	00
03589	N	N	N	N	N	08/05/2016	16	DUBARKO RD	INTER	CROSS	N	CLR	ANGL-OTH	01	NONE	0	STRTGHT					02	
CITY		FR						EAGLE CRK-SANDY HY	CN	STOP SIGN	N	DRY	ANGL		PRVTE	E	-W				00		
N		7A	45	23	22	.76	-122	15	017200100S00	01	0	INJ	PSNGR CAR	01	DRV	INJC	77	M	OTH-Y N-RES	028			
N		6P	45	23	22	.76	-122	15	017200100S00			02	NONE	0	STRTGHT	N -S	01	DRV	NONE	29	M	OR-Y OR<25	00
03967	N	N	N	N	N	08/30/2016	16	DUBARKO RD	INTER	CROSS	N	CLR	ANGL-OTH	01	NONE	0	STRTGHT					02	
CITY		TU						EAGLE CRK-SANDY HY	CN	STOP SIGN	N	DRY	ANGL		PRVTE	W	-E				00		
N		12P	45	23	22	.76	-122	15	017200100S00	04	0	INJ	PSNGR CAR	01	DRV	INJC	61	F	OTH-Y N-RES	028			
N										01	NONE	0	STRTGHT	W -E	PRVTE	PSNGR CAR	02	FSNG	INJC	6	F	000	
										02	NONE	0	STRTGHT	S -N	PRVTE	PSNGR CAR	01	DRV	INJB	53	F	OR-Y OR<25	00
02427	N	N	N	N	N	05/31/2016	16	DUBARKO RD	INTER	CROSS	N	CLR	ANGL-OTH	01	NONE	9	STRTGHT					03 , 32	
CITY		TU						EAGLE CRK-SANDY HY	CN	STOP SIGN	N	UNK	ANGL	N/A		W	-E				00		
N		11A	45	23	22	.76	-122	15	017200100S00	03	0	PDO	PSNGR CAR	01	DRV	NONE	00	Unk	UNK	00			
N																				00			

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CITY OF SANDY, CLACKAMAS COUNTY

DUARKO RD at EAGLE CRK-SANDY HY, City of Sandy, Clackamas County, 01/01/2015 to 12/31/2019
13 - 17 of 27 Crash records shown.

S	D	M	CLASS	CITY STREET	RD CHAR	INT- TYPE (MEDIAN)	OFFRD	WTHR	CRASH	SPCL USE TRLR QTY	MOVE	A	S	PED	ACT	EVENT	CAUSE		
SER#	P	R J S W	DATE	FIRST STREET	DIRECT	INT-REL LEGS	RNDBT	SURF	COLL	OWNER	FROM	FRTC	INJ	G	E	LICNS	LOC	ERROR	
INVEST	E A U	I C O	DAY	SECOND STREET	CONTN	TRAF- (#LANES)	DRWY	LIGHT	SURVY	V# TYP	TO	P# TYPE	SURVY	E	X	RES	LOC		
RD DFT	B L G	N H R	TIME	LRS	LONG	CONTN	DRWY	LIGHT	SURVY	02 NONE	9	STRGHT	N -S	N/A	01 DRVR	None	00	00	
UNLOCK?	D C S	V L K	LAT							PSNGR CAR									
02031	N	N	N	05/06/2016	16	DUBARKO RD	INTER	CROSS	N	CLR	ANGL-OTH	01 NONE	9	STRGHT				02	
CITY		FR		EAGLE CRK-SANDY HY	CN		STOP SIGN	N	DRY	ANGL	N/A		N -S					00	
N	4P	45 23 22 .76	-122 15	017200100800	01	0		N	DAY	PDO	PSNGR CAR		01 DRVR	None	00	Unk UNK	000	00	
N	4P	45 23 22 .76	-122 15	017200100800	04	0		N	DAY	INJ	PSNGR CAR		02 NONE	9	STRGHT	E -W	01 DRVR	None	00
N	4P	45 23 22 .76	-122 15	017200100800	04	0		N	DAY	INJ	PSNGR CAR		N/A	PSNGR CAR		01 DRVR	None	00	
00805	N	N	N	03/01/2017	16	DUBARKO RD	INTER	CROSS	N	CLD	ANGL-OTH	01 NONE	0	STRGHT				082,013	
CITY		WE		EAGLE CRK-SANDY HY	CN		STOP SIGN	N	DRY	ANGL	PRVTE		W -E					02	
N	3P	45 23 22 .76	-122 15	017200100800	04	0		N	DAY	INJ	PSNGR CAR		01 DRVR	INJ/C	17	F	OR-Y OR<25	028	
N	3P	45 23 22 .76	-122 15	017200100800	04	0		N	DAY	INJ	PSNGR CAR		02 NONE	0	STRGHT	S -N	01 DRVR	INJ/C	43
N	3P	45 23 22 .76	-122 15	017200100800	04	0		N	DAY	INJ	PSNGR CAR		03 NONE	0	STOP	E -W	01 DRVR	INJ/B	27
N	3P	45 23 22 .76	-122 15	017200100800	04	0		N	DAY	INJ	PSNGR CAR		02 NONE	0	PRVTE	E -W	01 DRVR	INJ/B	27
00846	N	N	N	03/04/2017	16	DUBARKO RD	INTER	CROSS	N	RAIN	ANGL-OTH	01 NONE	0	STRGHT				02	
CITY		SA		EAGLE CRK-SANDY HY	CN		STOP SIGN	N	WET	ANGL	PRVTE		W -E					00	
N	6P	45 23 22 .76	-122 15	017200100800	04	0		N	DLIT	INJ	PSNGR CAR		01 DRVR	None	21	M	OR-Y OR<25	028	
N	6P	45 23 22 .76	-122 15	017200100800	04	0		N	DLIT	INJ	PSNGR CAR		02 NONE	0	STRGHT	N -S	01 DRVR	INJ/C	21
N	6P	45 23 22 .76	-122 15	017200100800	04	0		N	DLIT	INJ	PSNGR CAR		02 NONE	0	PRVTE	E -W	01 DRVR	INJ/C	21
02225	N	N	N	06/07/2017	16	DUBARKO RD	INTER	CROSS	N	CLR	ANGL-OTH	01 NONE	0	STRGHT				02	
CITY		WE		EAGLE CRK-SANDY HY	CN		STOP SIGN	N	DRY	ANGL	PRVTE		S -N					00	
N	4P	45 23 22 .76	-122 15	017200100800	04	0		N	DAY	INJ	PSNGR CAR		01 DRVR	INJ/B	40	M	OR-Y OR<25	000	
N	4P	45 23 22 .76	-122 15	017200100800	04	0		N	DAY	INJ	PSNGR CAR		02 NONE	0	STRGHT	W -E	01 DRVR	INJ/C	38
N	4P	45 23 22 .76	-122 15	017200100800	04	0		N	DAY	INJ	PSNGR CAR		02 NONE	0	PRVTE	E -W	01 DRVR	INJ/C	38

Disclaimer: The information contained in this report is compiled from individual driver and police crash reports submitted to the Oregon Department of Transportation as required in ORS 811.720. The Crash Analysis and Reporting Unit is committed to providing the highest quality crash data to customers. However, because submittal of crash report forms is the responsibility of the individual driver, the Crash Analysis and Reporting Unit can not guarantee that all qualifying crashes are made that all details pertaining to a single crash are accurate. Note: Legislative changes to DMV's vehicle crash reporting requirement, effective 01/01/2004, may result in fewer property damage only crashes being eligible for inclusion in the Statewide Crash Data File.

CITY OF SANDY, CLACKAMAS COUNTY

DUBARKO RD at EAGLE CRK-SANDY HY, City of Sandy, Clackamas County, 01/01/2015 to 12/31/2019
18 - 21 cf 27 Crash records shown.

SER#	D	M	CITY STREET	RD CHAR	OFFRD	WTHR	CRASH	MOVE	FRTC	INJ	A	S	PED	ACT	EVENT	CAUSE
INVEST	P	R J S W DATE	CLASS	RD DIST	INT-REL (MEDIAN)	TRAF- LEGS	RNDBT SURF	COLL	FROM	G	E	LICNS	LOC	ERROR		
RD DFT	B	L G N H R TIME	FROM	FIRST STREET	TRAF- (LEG#)	DRWY	LIGHT	SVRTY	TO	P#	TYPE	SVRTY	E	X	RES	
UNLOCK?	D	C S V L K LAT	LONG	SECOND STREET	(#LANES)	CONTNL		O-1 L-TURN	01 NONE	0	TURN-L					02
02958	N	N	N	07/21/2017	16	DUBARKO RD	INTER	CROSS	N	CLR	O-1 L-TURN	01 NONE	0	TURN-L		
CITY	FR	EAGLE CRK-SANDY HY	CN	STOP SIGN	N	DRY	TURN		S -W					000	000	00
N	8P	45 23 22 .76 -122 15	017200100S00	01	0	DAY	INJ	PSNGR CAR		01 DRVR	NONE	28 M	OR-Y OR<25	028	000	02
00647	N	N	N	02/18/2017	16	DUBARKO RD	INTER	CROSS	N	RAIN	ANGL-OTH	01 NONE	9	STRGHT		
CITY	SA	EAGLE CRK-SANDY HY	CN	STOP SIGN	N	WET	ANGL	N/A	W -E					000	000	03
N	7P	45 23 22 .76 -122 15	017200100S00	03	0	DAY	DLIT	PDO	PSNGR CAR	01 DRVR	NONE	00	Unk UNK UNK	000	000	00
03467	N	N	N	08/23/2017	16	DUBARKO RD	INTER	CROSS	N	CLR	ANGL-OTH	01 NONE	9	STRGHT		
CITY	WE	EAGLE CRK-SANDY HY	CN	STOP SIGN	N	DRY	ANGL	N/A	NE -SW					000	000	02
N	8A	45 23 22 .76 -122 15	017200100S00	01	0	DAY	PDO	PSNGR CAR		01 DRVR	NONE	00	Unk UNK UNK	000	000	00
03265	N	N	N	09/14/2018	16	DUBARKO RD	INTER	CROSS	N	FLASHBCN-R	DRY	TURN	PRVTE	W -N		
CITY	FR	EAGLE CRK-SANDY HY	CN	FLASHBCN-R	N	DARK	INJ	PSNGR CAR		01 DRVR	NONE	38 M	OR-Y OR<25	028	000	00
N	9P	45 23 22 .52 -122 15	017200100S00	03	0					01 NONE	0	TURN-L			000	000

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CITY OF SANDY, CLACKAMAS COUNTY

DUBARKO RD at EAGLE CRK-SANDY HY, City of Sandy, Clackamas County, 01/01/2015 to 12/31/2019
22 - 24 of 27 Crash records shown.

SER#	D	M	CITY STREET	INT- TYPE (MEDIAN)	OFFRD	WTHR	CRASH	MOVE	FRTC	INJ	A	S	
INVEST	P	R J S W DATE	CLASS	RD CHAR	RNDBT	SURF	COLL	FROM	G	E	LICNS	PED	
RD DFT	B	L G N H R TIME	DI ST	DIRECT	LEGS	DRWY	LIGHT	TO	P#	TYPE	SVRTY	LOC	
UNLOCK?	D	C S V L K LAT	FROM	LOCTN	(#LANES)	CONTL						CAUSE	
03281	N	N	N	09/23/2019	16	DUBARKO RD	INTER	CROSS	N	CLR	ANGL-OTH	0	STRGHT
CITY	MO		EAGLE CRK-SANDY HY	CN			STOP SIGN	N	DRY	PRVTE	NE-SW	000	00
N	7A	45 23 22.59 -122.15	017200100800	02	0			N	DAWN	PRVTE	PSNGR CAR	000	00
	48.49												
00075	N	N	N	01/08/2019	16	DUBARKO RD	INTER	CROSS	N	CLR	ANGL-OTH	0	STRGHT
CITY	TU		EAGLE CRK-SANDY HY	CN			STOP SIGN	N	DRY	PRVTE	N -S	000	00
N	4P	45 23 22.54 -122.15	017200100800	03	0			N	DLIT	PRVTE	PSNGR CAR	000	00
	48.5												
00908	N	N	N	03/14/2019	16	DUBARKO RD	INTER	CROSS	N	CLR	ANGL-OTH	0	STRGHT
NO RPT	TH		EAGLE CRK-SANDY HY	CN			STOP SIGN	N	DRY	PRVTE	S -N	000	00
N	2P	45 23 22.76 -122.15	017200100800	04	0			N	DAY	PRVTE	SEMI TOW	000	00
	48.39												
01291	N	N	N	04/22/2019	16	DUBARKO RD	INTER	CROSS	N	CLD	ANGL-OTH	0	STRGHT
CITY	MO		EAGLE CRK-SANDY HY	CN			STOP SIGN	N	DRY	PRVTE	S -N	000	00
N	5P	45 23 22.54 -122.15	017200100800	04	0			N	DAY	PRVTE	PSNGR CAR	000	00
	48.5												

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CITY OF SANDY, CLACKAMAS COUNTY

OREGON - DEPARTMENT OF TRANSPORTATION - TRANSPORTATION DEVELOPMENT DIVISION
TRANSPORTATION DATA SECTION - CRASH ANALYSIS AND REPORTING UNIT
URBAN NON-SYSTEM CRASH LISTING

DUBARKO RD at EAGLE CRK-SANDY HY, City of Sandy, Clackamas County, 01/01/2015 to 12/31/2019
25 - 27 of 27 Crash records shown.

S	D	M	P	R	J	S	W	DATE	CLASS	CITY STREET	RD CHAR	INT-TYPE (MEDIAN)	OFFRD	WTHR	CRASH	SPCL USE TRLR QTY	MOVE	FROM	FRTC	INJ	A	S	ACT	EVENT	CAUSE															
SER#	INVEST	RD	DEPT	B	L	G	N	H	R	TIME	FROM	SECOND STREET	TRAF- LEGS	RNDBT	SURF	COLL	OWNER	TO	P# TYPE	SYRTY	E	G	B	LICNS	PED	LOC	ERROR													
UNLOCK?	D	C	S	V	V	L	K	LAT	LONG	LOCTN	(#LANES)	CONTLN	DRWY	LIGHT	SRTY	02	NONE	0	STRGHT	W -E	PVTE	PSNGR	CAR	01	DRV	None	37	M	OR-Y	028	015	000	02							
03399	N	N	N	N	10/03/2019	16				DUBARKO RD	INTER	CROSS	N	RAIN	ANGL-OTH	01	NONE	STRGHT													0.2	0.2	0.0	0.0						
CITY	TH									EAGLE CRK-SANDY HY	CN	STOP SIGN	N	WET	ANGL	PRVTE		N -S																0.0	0.0	0.0	0.0			
N	N	7P	45	23	22	.78	-122	15	48.4	017200100800	03	2		N	DLIT	INJ	PSNGR	CAR	01	DRV	INJ	48	F	OR-Y	OR<25									0.0	0.0	0.0	0.0			
																0.2	NONE	STRGHT	W -E	PRVTE	PSNGR	CAR	01	DRV	None	19	M	OTH-Y	028	0.15	0.00	0.0	0.0							
04270	N	N	N	N	11/29/2019	16				DUBARKO RD	INTER	CROSS	N	CLR	ANGL-OTH	01	NONE	STRGHT													0.2	0.2	0.0	0.0						
CITY	FR									EAGLE CRK-SANDY HY	CN	STOP SIGN	N	DRY	ANGL	PRVTE		N -S																0.0	0.0	0.0	0.0			
N	N	5P	45	23	22	.55	-122	15	48.51	017200100800	01	0		N	DLIT	INJ	PSNGR	CAR	01	DRV	None	49	F	OR-Y	OR<25									0.0	0.0	0.0	0.0			
																0.2	NONE	STRGHT	E -W	PRVTE	PSNGR	CAR	01	DRV	INJ	59	F	OR-Y	OR<25								0.15	0.0	0.00	0.2

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CITY OF SANDY, CLACKAMAS COUNTY

LANGENSAND RD at DUBARRO RD, City of Sandy, Clackamas County, 01/01/2015 to 12/31/2019

1 - 1 of 1 Crash records shown.

S	D	M	P	R	J	S	W	DATE	CLASS	CITY STREET	RD CHAR	INT- TYPE (MEDIAN)	OFFRD	WTHR	CRASH	SPCL USE	A	S		
INVEST	E	A	U	I	C	O	DAY	DI ST	FIRST STREET	DIRECT	LEGS	INT-REL	RNDBT	SURF	TRLR QTY	MOVE	G	LICNS		
RD DFT	B	L	G	N	H	R	TIME	FROM	SECOND STREET	LOCTN	(#LANES)	TRAF-	DRWY	LIGHT	OWNER	FROM	FRTC	PED		
UNLOCK?	D	C	S	V	L	K	LAT	LRS	LONG	LONG	CONTL	DRWY	LIGHT	SURF	V# TYP	TO	P# TYPE	SVRTY		
03066	N	N	N	06/09/2015	16	DUBARKO RD	INTER	CROSS	N	CLR	ANGL-OTH	0	NONE	0	STRGHT	083	ACT	EVENT	CAUSE	
NONE	TU	0						CN	STOP SIGN	N	DRY	ANGL	PRVTE	N -S			000	00	00	
N	12P	45	23	23	.89	-122	14	03	0	DAY	PDO	PSNGR CAR	01	DRVR	NONE	23	M	OR-Y	000	000
N											02	NONE	0	STRGHT	W -E					015
											PRVTE	PSNGR CAR	01	DRVR	NONE	16	F	OR-Y	028	000 083
														OR<25					02	

Preliminary Traffic Signal Warrant Analysis



Project Name: Deer Meadows Development

Intersection: Highway 26 at Langensand Road

Scenario: 2023 Background Plus Site Trips

Number of Major Street Lanes: 2 PM Peak Hour Volume 2485 (sum of both approaches)
 Number of Minor Street Lanes 1 PM Peak Hour Volume 40 (highest-volume approach)^a
 Posted or 85th percentile speed > 40 mph: Yes
 Isolated Population Less than 10,000: No

Warrant 1, Eight-Hour Vehicular Volume

Condition A - Minimum Vehicular Volume

Number of lanes for moving traffic on each approach		Vehicles per hour on major street (total of both approaches)				Vehicles per hour on minor street (total of both approaches)			
Major Street	Minor Street	100%	80%	70%	56%	100%	80%	70%	56%
1	1	500	400	350	280	150	120	105	84
2 or more	1	600	480	420	336	150	120	105	84
2 or more	2 or more	600	480	420	336	200	160	140	112
1	2 or more	500	400	350	280	200	160	140	112

Condition B - Interruption of Continuous Traffic

Number of lanes for moving traffic on each approach		Vehicles per hour on major street (total of both approaches)				Vehicles per hour on minor street (total of both approaches)			
Major Street	Minor Street	100%	80%	70%	56%	100%	80%	70%	56%
1	1	750	600	525	420	75	60	53	42
2 or more	1	900	720	630	504	75	60	53	42
2 or more	2 or more	900	720	630	504	100	80	70	56
1	2 or more	750	600	525	420	100	80	70	56

Warrant Anaylsis Calculations

8th Highest Hour^b Minimum Volume Warrant Satisfied?

Condition A - Minimum Vehicular Volume

Major Street Volume	1404	420	
Minor Street Volume	23	105	No

Condition B - Interruption of Continuous Traffic

Major Street Volume	1404	630	
Minor Street Volume	23	53	No

Combination Warrant^c

Major Street Volume	1404	504	
Minor Street Volume	23	84	No

^a Minor-Street right turn volumes are reduced to account for the impact of right-turns on red.

^b Eighth-highest hour volumes are calculated as 5.65 percent of the expected daily traffic volume.

^c This warrant should be used only after adequate trial of other alternatives has failed to solve traffic problems.

Preliminary Traffic Signal Warrant Analysis



Project Name: Deer Meadows Development

Intersection: Highway 211 at Dubarko Road

Scenario: 2023 Background Plus Site Trips

Number of Major Street Lanes: 1 PM Peak Hour Volume 837 (sum of both approaches)
 Number of Minor Street Lanes 1 PM Peak Hour Volume 87 (highest-volume approach)^a
 Posted or 85th percentile speed > 40 mph: Yes
 Isolated Population Less than 10,000: No

Warrant 1, Eight-Hour Vehicular Volume

Condition A - Minimum Vehicular Volume

Number of lanes for moving traffic on each approach		Vehicles per hour on major street (total of both approaches)				Vehicles per hour on minor street (total of both approaches)			
Major Street	Minor Street	100%	80%	70%	56%	100%	80%	70%	56%
1	1	500	400	350	280	150	120	105	84
2 or more	1	600	480	420	336	150	120	105	84
2 or more	2 or more	600	480	420	336	200	160	140	112
1	2 or more	500	400	350	280	200	160	140	112

Condition B - Interruption of Continuous Traffic

Number of lanes for moving traffic on each approach		Vehicles per hour on major street (total of both approaches)				Vehicles per hour on minor street (total of both approaches)			
Major Street	Minor Street	100%	80%	70%	56%	100%	80%	70%	56%
1	1	750	600	525	420	75	60	53	42
2 or more	1	900	720	630	504	75	60	53	42
2 or more	2 or more	900	720	630	504	100	80	70	56
1	2 or more	750	600	525	420	100	80	70	56

Warrant Anaylsis Calculations

8th Highest Hour^b Minimum Volume Warrant Satisfied?

Condition A - Minimum Vehicular Volume

Major Street Volume	473	350	
Minor Street Volume	49	105	No

Condition B - Interruption of Continuous Traffic

Major Street Volume	473	525	
Minor Street Volume	49	53	No

Combination Warrant^c

Major Street Volume	473	420	
Minor Street Volume	49	84	No

^a Minor-Street right turn volumes are reduced to account for the impact of right-turns on red.

^b Eighth-highest hour volumes are calculated as 5.65 percent of the expected daily traffic volume.

^c This warrant should be used only after adequate trial of other alternatives has failed to solve traffic problems.

Preliminary Traffic Signal Warrant Analysis



Project Name: Deer Meadows Development

Intersection: Dubarko Road at Langensand Road

Scenario: 2023 Background Plus Site Trips

Number of Major Street Lanes: 1 PM Peak Hour Volume 159 (sum of both approaches)
 Number of Minor Street Lanes 1 PM Peak Hour Volume 68 (highest-volume approach)^a
 Posted or 85th percentile speed > 40 mph: No
 Isolated Population Less than 10,000: No

Warrant 1, Eight-Hour Vehicular Volume

Condition A - Minimum Vehicular Volume

Number of lanes for moving traffic on each approach		Vehicles per hour on major street (total of both approaches)				Vehicles per hour on minor street (total of both approaches)			
Major Street	Minor Street	100%	80%	70%	56%	100%	80%	70%	56%
1	1	500	400	350	280	150	120	105	84
2 or more	1	600	480	420	336	150	120	105	84
2 or more	2 or more	600	480	420	336	200	160	140	112
1	2 or more	500	400	350	280	200	160	140	112

Condition B - Interruption of Continuous Traffic

Number of lanes for moving traffic on each approach		Vehicles per hour on major street (total of both approaches)				Vehicles per hour on minor street (total of both approaches)			
Major Street	Minor Street	100%	80%	70%	56%	100%	80%	70%	56%
1	1	750	600	525	420	75	60	53	42
2 or more	1	900	720	630	504	75	60	53	42
2 or more	2 or more	900	720	630	504	100	80	70	56
1	2 or more	750	600	525	420	100	80	70	56

Warrant Anaylsis Calculations

8th Highest Hour^b Minimum Volume Warrant Satisfied?

Condition A - Minimum Vehicular Volume

Major Street Volume	90	500	
Minor Street Volume	38	150	No

Condition B - Interruption of Continuous Traffic

Major Street Volume	90	750	
Minor Street Volume	38	75	No

Combination Warrant^c

Major Street Volume	90	600	
Minor Street Volume	38	120	No

^a Minor-Street right turn volumes are reduced to account for the impact of right-turns on red.

^b Eighth-highest hour volumes are calculated as 5.65 percent of the expected daily traffic volume.

^c This warrant should be used only after adequate trial of other alternatives has failed to solve traffic problems.

Left-Turn Lane Warrant Analysis (ODOT Methodology)

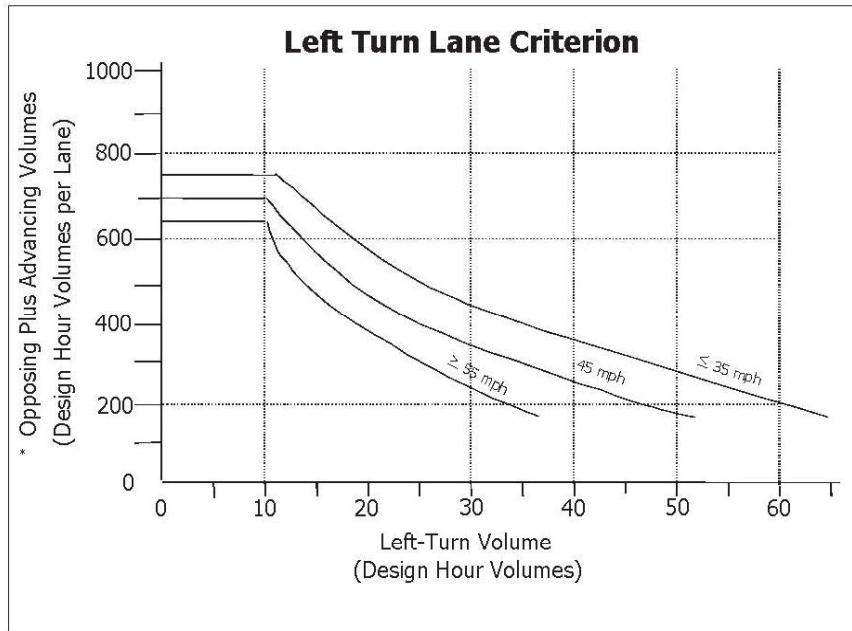


Project Name: Deer Meadows Development
Approach: Highway 211 NB at Dubarko Road
Scenario: 2021 Existing Conditions

Number of Advancing Lanes: 1
Number of Opposing Lanes: 1
Major-Street Design Speed: 45 mph

	AM Volume	PM Volume
Advancing Volume for Design Hour:	341	386
Opposing Volume for Design Hour:	271	337
Design Hour Volume Per Lane:	612	723
Number of Left Turns per Hour:	44	61
Left-turn lane warrants satisfied?	YES	YES

Exhibit 7-1 Left Turn Lane Criterion (TTI)



*(Advancing Volume/Number of Advancing Through Lanes) + (Opposing Volume/Number of Opposing Through Lanes)

Right-Turn Lane Warrant Analysis (ODOT Methodology)



Project Name: Deer Meadows Development

Approach: Highway 211 Northbound at Dubarko Road

Scenario: 2021 Existing Conditions

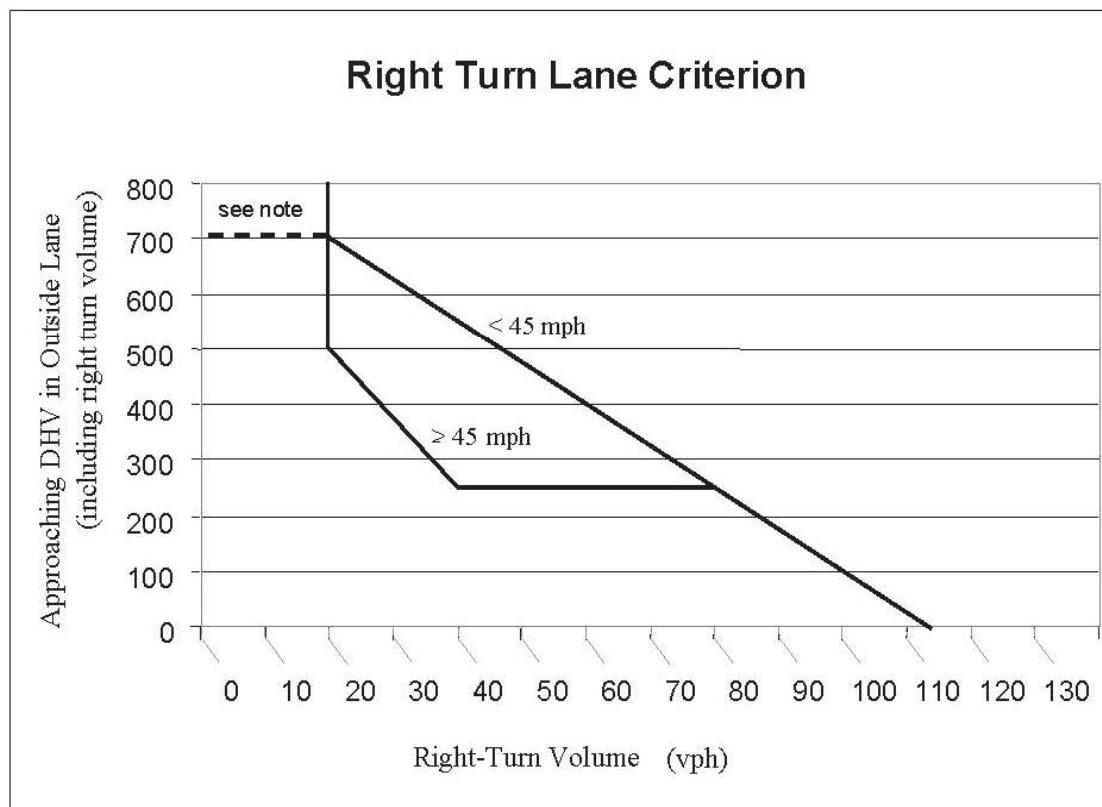
Major-Street Design Speed: 45 mph

	AM Volume	PM Volume
Number of Right Turns per Hour:	14	54
Approaching DHV in Outside Lane:	341	386
Calculated Turn Volume Threshold:	33	29
Right Turn Volume Exceeds Threshold?	NO	YES

Criterion 1: Vehicular Volume

The vehicular volume criterion is intended for application where the volume of intersecting traffic is the principal reason for considering installation of a right turn lane. The vehicular volume criteria are determined using the curve in Exhibit 7-2.

Exhibit 7-2 Right Turn Lane Criterion



Note: If there is no right turn lane, a shoulder needs to be provided. If this intersection is in a rural area and is a connection to a public street, a right turn lane is needed.

Left-Turn Lane Warrant Analysis (ODOT Methodology)



Project Name: Deer Meadows Development

Approach: Dubarko Road westbound at Langensand Road

Scenario: 2023 Background plus Site Trips

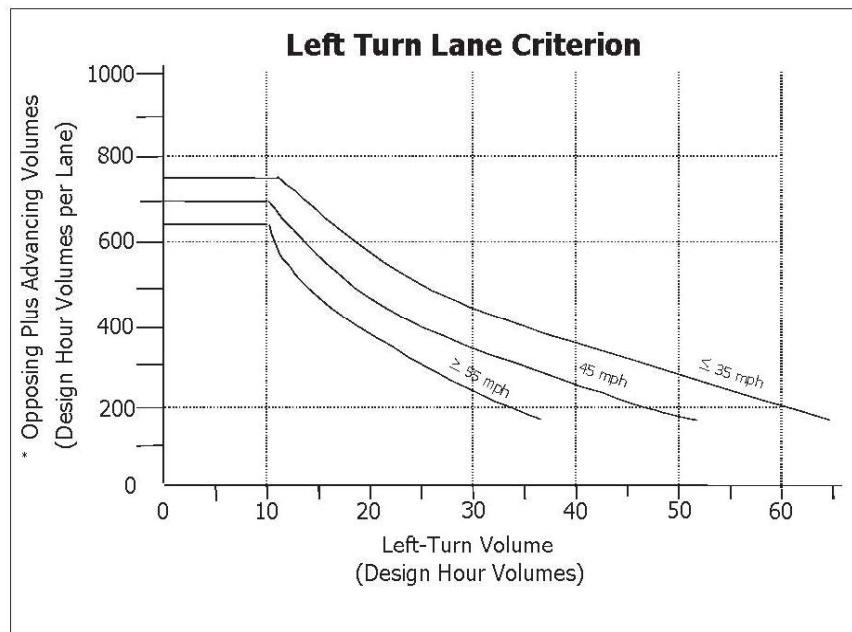
Number of Advancing Lanes: 1

Number of Opposing Lanes: 1

Major-Street Design Speed: 25 mph

	AM Volume	PM Volume
Advancing Volume for Design Hour:	101	54
Opposing Volume for Design Hour:	34	69
Design Hour Volume Per Lane:	135	123
Number of Left Turns per Hour:	1	2
Left-turn lane warrants satisfied?	NO	NO

Exhibit 7-1 Left Turn Lane Criterion (TTI)



*(Advancing Volume/Number of Advancing Through Lanes) + (Opposing Volume/Number of Opposing Through Lanes)

Right-Turn Lane Warrant Analysis (ODOT Methodology)



Project Name: Deer Meadows Development

Approach: Dubarko Road Westbound at Langensand Road

Scenario: 2023 Background Plus Site Trips

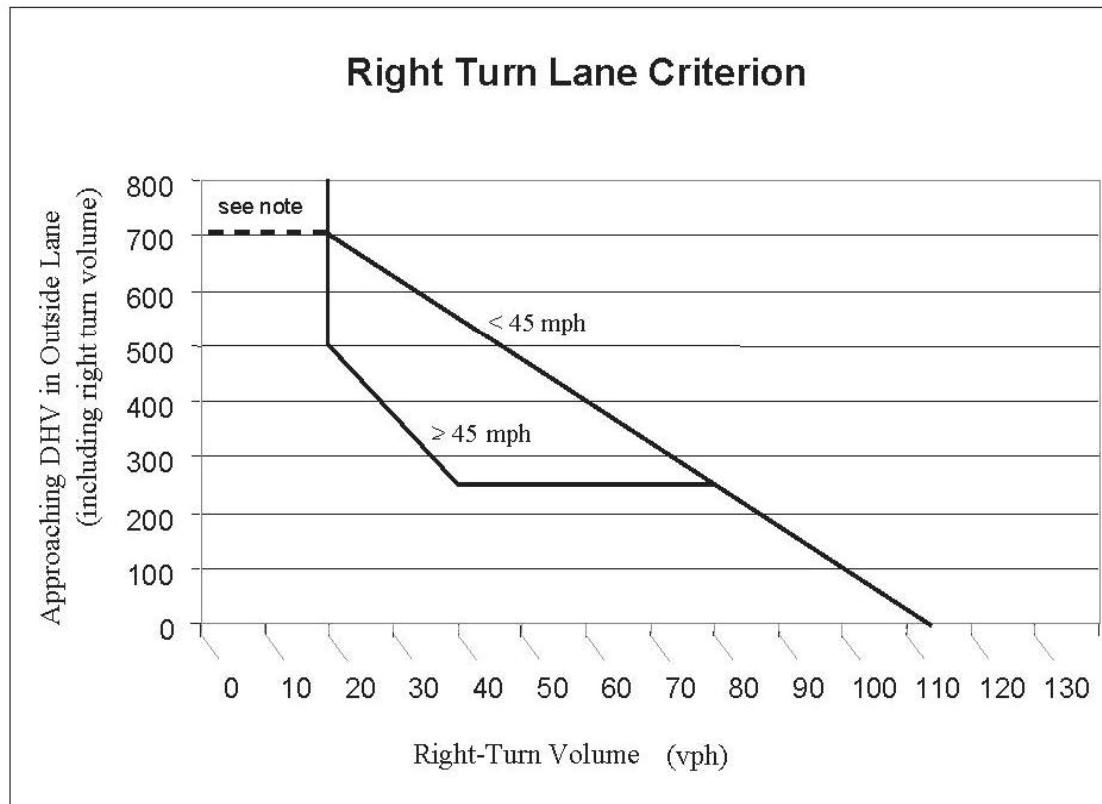
Major-Street Design Speed: 25 mph

	AM Volume	PM Volume
Number of Right Turns per Hour:	62	30
Approaching DVH in Outside Lane:	101	54
Calculated Turn Volume Threshold:	100	106
Right Turn Volume Exceeds Threshold?	NO	NO

Criterion 1: Vehicular Volume

The vehicular volume criterion is intended for application where the volume of intersecting traffic is the principal reason for considering installation of a right turn lane. The vehicular volume criteria are determined using the curve in Exhibit 7-2.

Exhibit 7-2 Right Turn Lane Criterion



Note: If there is no right turn lane, a shoulder needs to be provided. If this intersection is in a rural area and is a connection to a public street, a right turn lane is needed.